Influence of bank slope on sinuosity-driven hyporheic exchange flow and residence time distribution during a dynamic flood event

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Abstract. This study uses a reduced-order two-dimensional (2-D) horizontal model to investigate the influence of riverbank slope on the ~~bank storage and~~ sinuosity-driven hyporheic exchange ~~flux (HEF)~~process along sloping alluvial riverbanks during a transient flood event. The Deformed Geometry Method (DGM) is applied to quantify the displacement of the sediment-water interface (SWI) along the sloping riverbank during river stage fluctuation. This new modeling approach serves as the initial step ~~to~~focusing on the impact of bank slope on the hyporheic exchange flux (HEF) and the residence time distribution (RTD) of pore water in the fluvial aquifer for a sinuosity-driven river corridor ~~consider complicated floodplain morphologies in physics-based models for better predictions of HEF~~. Several controlling factors, including sinuosity, alluvial valley slope, ~~and~~ river flow advective forcing and duration of flow are incorporated into the model to investigate the effects of bank slope o~~i~~n aquifers of variable hydraulic transmissivity. Compared to simulations of a vertical riverbank, sloping riverbanks were found to increase the HEF. For sloping riverbanks, the hyporheic zone (HZ) encompasses~~d~~ a larger area and penetrated deeper into the alluvial aquifer, especially in aquifers with smaller transmissivity (i.e., due to ~~larger aquifer~~increased hydraulic conductivity or ~~smaller~~ reduced specific yield). Furthermore, consideration of sloping banks as compared to a vertical river bank can lead to both underestimation or overestimation of the pore water ~~residence time~~travel time. The impact of bank slope on residence time was more pronounced during a flood event for high transmissivity aquifer conditions, while it had a long-lasting influence after the flood event in lower transmissivity aquifers. Consequently, this decreases the ~~residence~~ travel time of ~~HEF~~ water discharging~~e~~ into the river relative to ~~the~~ base flow conditions. These findings highlight the need for (re)consideration of the importance of more complex riverbank morphology as control of hyporheic exchange in ~~alluvial aquifers~~floodplains. The results have potential implications for river management and restoration and the management of river and groundwater pollution.

Key words: hyporheic exchange, sloping riverbank, ~~transient river stage~~deformed geometry, ~~peak flow event~~numerical simulation, residence time distribution

Nomenclature

ΔL	Nodal spacing [m]
$\bigtriangledown$	Laplace operator
α_L	Longitudinal dispersivity [L]
α_T	Transverse dispersivity [L]
$\mathbf{D}$	Dispersion-diffusion tensor [L^2T^{-1}]
D_L	Water diffusivity [L^2T^{-1}]
J_x	Base groundwater gradient [-]
K	Hydraulic conductivity [LT^{-1}]
n	Scaling number [-]
n_0	Intensity of flood event [-]
n_d	Skewness of flood event [-]
S_y	Specific yield [-]
t_d	Duration of flood event [T]
t_p	Time to peak river stage [T]
α	Amplitude of the river boundary [L]
Γ_d	Dimensionless aquifer transmissivity [-]
δ	Bank slope angle [°]
δ_{ij}	Kronecker delta function [-]
ϵ	Tortuosity [-]
η	Degree of flood event asymmetry[T^{-1}]
θ	Effective porosity [-]
λ	River boundary wave length [L]
σ	River boundary sinuosity [-]
τ	Residence time [T]
ω	Flood event frequency[T^{-1}]
$h(\mathbf{x}, t)$	Transient groundwater head [L]
Δh^*	Dimensionless parameter of ambient groundwater flow [-]

$A^{**}(t)$	Dimensionless variation of HZ area relative to base flow conditions [-]
$C(\mathbf{x}, t)$	Solute concentration~~s~~ in the aquifer [ML^{-3}]
$C_0(\mathbf{x})$	Solute concentration~~s~~ as~~in~~ initial condition [ML^{-3}]
$C_S(\mathbf{x}, t)$	Solute concentration~~s~~ in the river [ML^{-3}]
$d^{**}(t)$	Dimensionless variation of HZ penetration distance relative to base flow conditions [-]
$H(\mathbf{x}, t)$	Thickness of the saturated aquifer [L]
$H_0(\mathbf{x})$	Initial river stage [L]
H_p	Peak river stage during the flood event [L]
$H_r(t)$	River stage at the downstream end [L]
$h_r(x, t)$	Transient river stage [L]
$M(t)$	Displacement of the sediment-water interface [L]
P_e	Péclet number [-]
$\mathbf{q}$	Specific discharge or Darcy flux [LT^{-1}]
$\mathbf{Q}$	Aquifer-integrated discharge [L^{2}T^{-1}]
$Q^*_{in,\,HZ}\,(t)$	Dimensionless net flux along the river boundary [-]
$Q^*_{in,\,HZ}\,(t)$	Dimensionless exchange flux from the aquifer to the river [-]
$Q^*_{out,\,HZ}\,(t)$	Dimensionless exchange flux from the river to the aquifer [-]
$Y(x, t)$	Location of the sediment-water interface boundary [L]
$z_b(\mathbf{x})$	Elevation of the underlying impermeable layer [L]
Γ_d	Dimensionless parameter of aquifer transmissivity [-]
$\mu(\mathbf{x}, 0)$	Mean (first order of) residence time distribution [T]
$\mu^*_{out}(x, t)$	Flux-weighted ratio of mean RT to mean RT under baseflow conditions [-]
$\mu_n(\mathbf{x}, t)$	n-th moment of residence time distribution [T^n]
$\mu_r^*(\mathbf{x}, t)$	Residence time distribution ratio between slope and vertical river bank model [-]
$\mu_{\tau 0\text{-max}}$	Maximum RT in the domain [T]

$\mu_{\tau\text{-}S}(\mathbf{x}, t)$	Residence time distribution of slope river bank model [T]
$\mu_{\tau\text{-}V}(\mathbf{x}, 0)$	Residence time distribution of vertical river bank model [T]
$\rho(\mathbf{x}, t, \tau)$	Residence time distribution [T]
Abbreviations	
HZ	Hyporheic zone
HEF	Hyporheic exchange flux
DGM	Deformed Geometry Method
SWI	Sediment-water interface
RTD	Residence time distribution
RT	Residence time
ALE	Arbitrary Lagrangian–Eulerian
2-D	Two-dimensional
BTS	Biogeochemical timescale

1. Introduction

The hyporheic zone (HZ) can be described as the region that connects the river channel and adjacent aquifer, and includes riverbed and riverbanks. Mixing and transporting of different water types (groundwater, surface water) and ages in the HZ driven by hydrodynamic and hydrostatic factors cause~~s~~ spatially and temporally varying exchange of water and~~,~~ biogeochemical species~~, and energy~~ between river channel, riverbed and aquifer (Cardenas, 2009b; Hester and Gooseff, 2010; Krause et al., 2011, 2017, 2022; McClain et al., 2013; Boano et al., 2014). The hyporheic exchange flux (HEF) represents the interaction flux between surface water and groundwater~~Hyporheic exchange flow~~ in vertical (e.g., bedform-driven) and horizontal (e.g., meander-driven) ~~domains~~ directions, which can add to general regional groundwater ex-filtration and infiltration~~upwelling or downwelling~~.~~, with HEF representing those surface flow components that penetrate and transport through the hyporheic sediment and back into the stream.~~ The distribution of hyporheic flow paths strongly determines the spatial and temporal distribution of hydro~~bio~~geochemical characteristics of water within the riverbed and the wider river corridor as well as the formation of so-called hot zones and hot moments (Krause et al., 2013, 2017; Cardenas, 2015; Pinay et al., 2015).

Hyporheic exchange flux (HEF) is controlled by parameters such as stream discharge dynamics, recharge, riverbed and aquifer hydraulic properties, local ~~pressure~~ hydraulic head fluctuations, ~~and~~ as well as river geometry and morphology including sinuosity and riverbank slope (Larkin and Sharp, 1992; Gomez-Velez et al., 2012; 2017; Schmadel et al., 2016). For example, Cardenas et al. (2004) demonstrated how riverbed characteristics and especially the heterogeneity of hydraulic conductivity could increase ~~the hyporheic exchange intensity~~HEF by 17% to 32%. As such, to be able to better estimate the relative importance of HEF on catchment water fluxes and bio~~geochemistry~~geochemical processes ~~requires~~require a good understanding of ~~the~~ its

~~interactions of its~~ different drivers and controls. This is imperative as the spatiotemporal evolution of HEF ~~paths~~, the resulting change in HZ ~~extent~~ (area) and thus also the ~~mean~~ residence or travel time (RT) of the exchanged water in the HZ have significant impact on flow dynamics and transient storage along the river continuum and in turn control the ~~attenuation~~ capacity for contaminant attenuation (Weatherill et al., 2018) and biogeochemical functions of river corridors (Bertrand et al., 2012; Boulton et al., 2010; Brunke and Gonser, 1997).

Both lateral exchange between river and its flood~~-~~plain, as well as bedform-induced vertical exchange at the streambed interface have been found to be crucial with regards to HEF and the biogeochemical transformation potential along the river corridor (Boano et al., 2010, 2014; Gomez-Velez and Harvey, 2014; Gomez-Velez et al., 2015, 2017; Kiel and Cardenas, 2014; Stonedahl et al., 2013). ~~Considerable progress of~~Through using numerical simulations,~~ numerical simulation~~ considerable progress has been made with regards to~~in~~ our understanding of how river planform geometry (Boano et al., 2006, 2010; Cardenas 2006; 2008; 2009a, 2009b; Stonedahl 2013), dynamic flood events (Gomez-Velez et al., 2012; 2017) and evapotranspiration (Kruegler et al., 2020) control HEF. Focusing on lateral exchange flow processes, Cardenas (2008; 2009a, 2009b) ~~developed~~ utilized numerical models to investigate HEF and residence time distribution (RTD) for various river channel morphologies and regional groundwater flow conditions. Their simulations indicate that channel morphology, represented by sinuosity, is a dominant factor controlling HEF, the total HZ area, and RTD. In addition, Boano et al. (2010) used a similar modeling framework to study the relationship between RTD and biogeochemical transformation by introducing surface water as a major source of dissolved organic matter that triggers a sequence of redox reactions within the HZ. Reactive transport simulations showed a good relationship between RTD and denitrification reaction potential. Based on these studies, Gomez-Velez et al. (2012) conducted numerical simulations to investigate the impact of aquifer parameters (water table gradient, hydraulic conductivity, dispersivity) and channel sinuosity on HEF and RTD. By comparing RTD with the timescale of ~~nitrate forming~~nitrification/denitrification reactions~~ or reducing reactions~~, a meander can be classified as a source or sink of nitrate~~ for (de)nitrification activities~~. More recent modeling studies ~~have~~ focused predominantly on the effects of dynamic river/groundwater stage fluctuations on lateral (e.g., Schmadel et al., 2016; Gomez-Velez et al., 2017) and vertical (e.g., Singh et al., 2019, 2020; Wu et al., 2018, 2020, 2021) hyporheic exchange and RTD. For example, Gomez-Velez et al. (2017) explored the HZ response to a dynamic river stage ~~under~~ due to ~~different parameter values for~~variable hydraulic conductivity, ~~river stage during flood events,~~ groundwater flow gradient and river sinuosity conditions. Their results indicate that the dynamic forcing greatly influences net HEF, the area of HZ and RTD across different scenarios, whereby higher aquifer transmissivity will likely result in a stronger but shorter response of HEF and RTD to a flood event.

Although there is a considerable body of numerical research on the lateral hyporheic response to the various geometrical (e.g., geometry of river channel, river slope, etc) and dynamic drivers (e.g., fluctuation of river/groundwater, gaining and losing conditions of groundwater, etc), many HZ studies do not specifically consider floodplain-driven processes or they apply vertical riverbanks with straight river planimetry in an attempt to reduce model complexity in line with the analytical or numerical solutions used (Cooper and Rorabaugh, 1963; Hunt, 1990; Schmadel et al., 2016; Gomez-Velez et al., 2017;). However, riverbanks are usually ~~tilted~~ sloping (inclined) rather than vertical (Liang et al., 2018) as they undergo erosion (Osma and Thorne, 1988). Previous research has proven that bank erosion and bank collapse are globally spreading processes controlled by various factors, such as initial bank slope angle (Zingg, 1940; Lindow et al., 2009), surface flow forces (Hagerty et al., 1995; Fox and Wilson, 2010), vegetation cover (Mayor et al., 2008; Gao et al., 2009; Puttock et al., 2013) and sediment properties (Millar and Quich, 1993). Neglecting bank slope in analytical and numerical model solutions may therefore have a significant influence on the prediction accuracy of HEF (Doble et al. 2012a, 2012b) and RTD (Derx et al., 2014; Siergieiev et al., 2015) in an unconfined floodplain aquifer. Thus, a detailed analysis of the floodplain drivers of HEF should require a more detailed consideration of the floodplain geometry including riverbank slope in bank storage conceptual models (Sharp, 1977).

A ~~F~~few previous studies have used numerical modeling where the model is bounded by a sloping riverbank to assess the influence of bank slope on HEF for a vertical section of an alluvial aquifer. In such cases, the aquifer was considered variably saturated, homogenous, and isotropic, while flow in the unsaturated zone was calculated using the Richards equation (Li et al., 2008; McCallum et al., 2010; Doble 2012a; b). These studies have confirmed that neglecting bank slope can lead to an underestimation of the bank storage volume as well as the temporal HEF in vertical cross-sectional profiles, especially under relatively small bank angles.

In turn, river sinuosity and ambient groundwater gradient (along the river channel) have not been studied as potential drivers of sinuosity-driven lateral HEF and RTD and their biogeochemical implications ~~under complex riverbank morphological conditions~~when a sloping river bank exists and it needs to be determined whether considering both drivers can lead to significantly different findings as compared to previous cross-sectional profile models (Doble et al., 2012; Siergieiev et al., 2015; Derx et al., 2014). In this study, we therefore quantify the effect of bank slope on the ~~simulated~~ spatial extent (area) of the HZ in sinuosity-driven river meanders and how it impacts the evolution of HEF and RTD under varying aquifer transmissivity conditions to better understand lateral HEF through the alluvial plain. We build on the ~~conventional~~ numerical modeling approach introduced by Gomez-Velez et al. (2017) and consider lateral bank slope by ~~using a deformed geometry method (DGM) approach. For this, we~~ coupling~~e~~ the deformed geometry method (DGM) ~~with the Boussinesq equationin~~to the flow (Liang et al. 2020), the ~~vertically integrated~~ solute transport ~~equation~~ and the residence time distribution equation~~ to study HEF~~. Our results reveal how and when bank slope plays an important role~~s~~ in ~~predicting HEF will help to reveal the importance of bank slope for the prediction of HEF and RTD in~~ sinuosity-driven meandering rivers with respect to HEF and RTD~~.~~, which in turn will lead to an improved understanding of the river channel-aquifer-floodplain system and provide guidance on the placement of monitoring locations in river management studies. ~~for the conceptualization hypothesis of numerical model and the monitoring location selection of field study in the future.~~

2. Methodology

2.1 Model setup ~~with~~ using deformed geometry method

~~Our modeling approach builds on the work of~~ The ~~conventional model~~modeling approach and dimensionless parameterization metrics used ~~in~~by Gomez-Velez et al. (2017)~~, who developed a comprehensive simulation tool in dimensionless form that~~ can represent most riverbank-aquifer situations and dynamic flood conditions. In our study, we use their conceptual model to set up a baseline case ~~as a baseline~~ with the same model frame, equations and parameterization metrics. Additional information regarding the implementation of this baseline ~~model~~ case can be found in the SI ~~as S1 to S3 and Gomez-Velez et al. (2017)~~. However, where their previous research assumed a vertical river bank for sinuosity-driven HEF models~~where Gomez-Velez et al. (2017) assume a vertical riverbank~~, we consider a sloping riverbank and use the DGM approach to capture the dynamic evolution of the SWI along the river course. A constant sloping angle (δ [°]) along the alluvial riverbank of a sinusoidal river was implemented in our model (see blue lines of conceptual model in Figure S1 and the corresponding mathematical model in Figure S2a) while the surface water interface (SWI) was assumed to be always vertical (vertical solid red and green lines in Figure S2c). As such, the contraction or expansion of the simulated domain, i.e., displacement of the SWI can be characterized by the sloping angle (there is no movement of the SWI for the vertical riverbank case) and river stage. As the river stage changes, so does the location of the SWI.

When the river stage changes in our model, the sinusoidal boundary will migrate towards or away from the floodplain meaning that the submerged part of the riverbank is considered contracted and our model only considers the alluvial aquifer that is not submerged. The evolution of the SWI during a flood event can be calculated by considering river stage and bank slope via:

$$Y(x, t) = Y_0(x) + M(t) \tag{1}$$

where $Y(x, t)$ [L] is the location of the SWI boundary while~~;~~ $Y_0(x)$ [L] is the initial location of the SWI. In contrast to Gomez-Velez et al. (2017), the displacement of the SWI caused by the deformation of the model domain ($M(t) = [h(t) - h(0)]/\tan(\delta)$, where $h(t)$ [L] is transient hydraulic head) is added in Eq. (1), which represents the displacement of the river boundary in y-direction due to river stage fluctuation and bank slope angle (see the horizontal distance between the vertical red and green solid line in Figure S2c).

To simulate the model domain deformation and mesh displacement, we use the DGM interface in COMSOL. In this interface, the deforming feature of a specified domain can be defined as a boundary condition with a given moving velocity or displacement. DGM is based on the arbitrary Lagrangian–Eulerian (ALE) method, which is a hybrid method that allows both the model domain and mesh to move or deform simultaneously in a predefined manner. More details on ALE can be found in Donea et al. (2014). While it has previously been used for simulating general free-surface problems (e.g., Duarte et al., 2004; Maury, 1996; Pohjoranta and Tenno, 2011), to our knowledge, DGM has not yet been implemented to solve moving boundary problems in hyporheic exchange studies. Here we used Eq. (1) as an input to the DGM interface to simulate the displacement of the SWI (water flow) during a dynamic flood event. Infiltration and seepage face before and after the peak time of the flood event, respectively, were neglected (Boano et al., 2006; Cardenas. 2009a, b; Kruegler et al., 2020). Fig. 1 illustrates the river stage hydrograph of this study (Fig. 1a, calculated by Eq. (S2)) and the diagram of the displacement of the SWI (Fig. 1b) during the flood event after coupling D~~MG~~GM into the model. The colored river boundaries in Fig. 1b are corresponding to the times of colored dots in Fig. 1a. Additionally, solute transport and RTD were simulated based on the extent of the flow field according to Gomez-Velez et al. (2017), as shown in the SI ~~as~~ (S2 and S3, respectively).

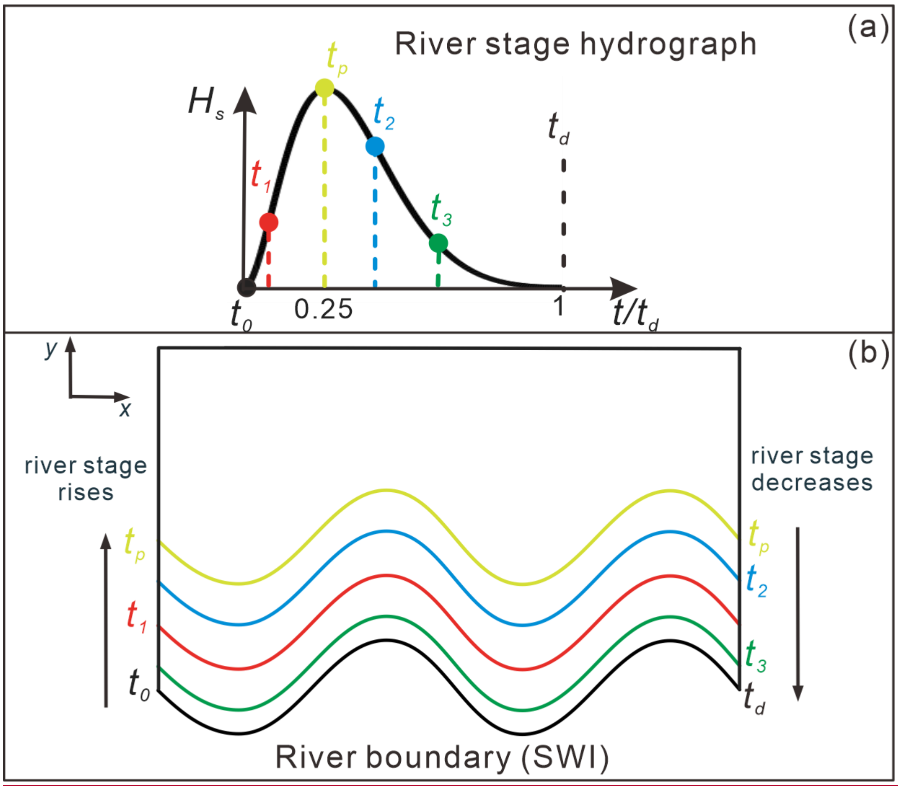


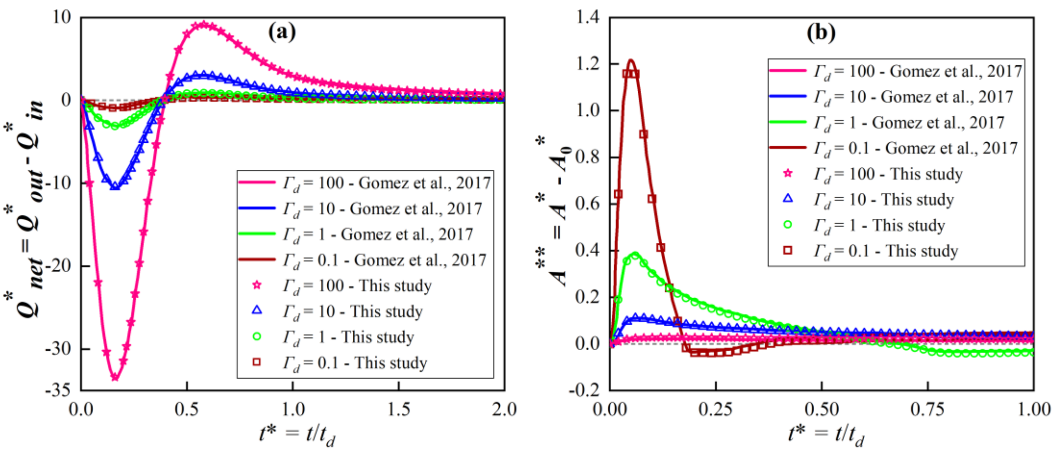
Figure 1. (a) River stage hydrograph during the flood event; (b) diagram showing~~of~~ displacement of SWI~~ (Fig. 1b)~~ during the flood event. The colored SWIs in ~~Fig. 1b~~(b) ~~are~~ correspond~~onding~~ to the times of colored dots in ~~Fig. 1a~~(a). ~~The~~When the river stage increases, the river boundary migrates~~migrates~~ into the aquifer ~~during the raising of river stage,~~ and recovers to ~~the~~its initial location as river stage decreases. The upward and downward arrow in Fig. 1b indicates the raising and decreasing of river stage, respectively.

~~Additionally, solute transport and RTD were simulated based on the extent of the flow field according to Gomez-Velez et al. (2017), as shown in the SI.~~

2.2 Model parameterization, testing and scenarios

~~Model h~~Hydraulic conditions used in our numerical modeling study are based on values from Gomez-Velez et al. (2017), who conducted a Monte Carlo analysis. They found that the dynamic variations of HEF and RTD are mainly determined by ambient groundwater flow ~~w (referred to as dimensionless parameter $\Delta h^* = \frac{J_x\lambda^2}{0.5(1+n_0)H_0}$, see Table 1)~~ and the ratio of aquifer hydraulic conductivity to the duration of the flood event (referred to as dimensionless constant $\Gamma_d = \frac{S_y\lambda^2}{0.5K(1+n_0)H_0t_d}$, see Table 1 and Fig. S2, where S_y is specific yield [-]; λ is wave length of sinuous river; K is hydraulic conductivity [LT^{-1}]; n_0 is intensity of flood event [-] H_0 is base river stage [L]; t_d is duration of flood event [T]).

After setting up the ~~original model of Gomez-Velez et al. (2017) as a~~ baseline model case with a vertical riverbank (δ = 90°), we compared our model results for that case with those obtained by Gomez-Velez et al. (2017) for (a) net HEF represented by $Q^*_{net,\,HZ}(t)$; (b) area of HZ, $A^{**}(t)$; (c) penetration of the HZ, $d^*(t)$ for~~in~~ Γ_d = 0.1, 1, 10 and 100, and found that our model simulated those cases with high accuracy (Fig. 2~~1~~). Parameters $A^{**}(t)$ and $d^*(t)$ are based on modeling the transport of a conservative solute while $Q^*_{net,\,HZ}(t)$ is based on modeling water flow. Slight differences between our model and that of Gomez-Velez et al. (2017) might be due to the use of a much more refined mesh in this study as well as~~nd~~ different length scales.



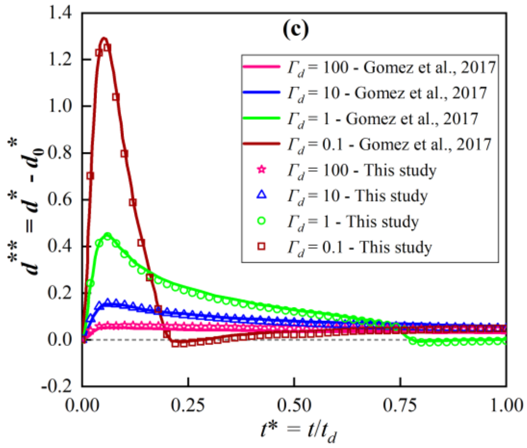


Figure 2~~1~~. Comparison of results obtained in this study with those of Gomez et al. (2017) for the baseline case with a vertical river bank and variable Γ_d: (a) net hyporheic exchange flux represented by $Q^*_{net,HZ}(t)$; (b) extent of the hyporheic zone $A^{**}(t)$ and (c) penetration distance $d^*(t)$ of the hyporheic zone into the alluvial valley. A more refined mesh and different length scales used in this study~~,~~ can explain~~ occasional~~ slight ~~differences ~~variations between our model and that of Gomez et al. (2017)~~ might occur~~. Information regarding model fits can be found in the SI.

To test, whether our assumption

~~Furthermore, the appropriateness of the assumption of~~of considering a vertical SWI and ~~the implementation of~~using ~~DM~~the DGM to characterize the migration of the SWI was ~~validated~~appropriate, we ~~ by ~~compar~~ing~~ed the vertical 2-D model ~~and the~~with a 1-D model coupled with ~~DMG~~the DGM. Detailed information ~~for the implementation~~on this comparison as well as~~ of validation models and the~~ validation results are listed in the SI in section~~as~~ S4. The results show that ~~the~~our ~~assumptions vertical SWI and using of DMG~~approach is– reasonable ~~of this study are appropriate for~~when simulating HEF in a sloping river-bank aquifer.

We then considered a series of riverbank scenarios where the bank slope angle ranged from δ = 90° (vertical riverbank) to 10° (nearly horizontal case) and Γ_d values ranged from 0.1 to 100, ~~(~~corresponding to aquifer hydraulic conductivity ranging from 480 to 0.048 m/d, indicating high to low transmissivity. Table 1 presents the parameters used in our numerical modeling study. The finite-element models proposed in this study were ~~developed~~ set up using the COMSOL Multiphysics (COMSOL) software. Eq. (S1), Eq. (S3) and Eq. (S6) were implemented by using customized~~ing~~ a Partial Differential Equation (PDE) interface to include the Boussinesq equation, vertical integrated solute transport equation and ~~RTD~~ equation for calculating residence (travel) time distributions (RTD), respectively. The model domain was discretized into about 0.5 million variably-sized triangular elements, with refinement imposed near the river boundary. Mesh-independent numerical solutions are achieved by limiting grid size (ΔL) to less than 0.2 m. Thus, the transverse and longitudinal Peclet numbers (calculated by $P_e = \Delta L/\alpha_L$ and $P_e = \Delta L/\alpha_T$, respectively) in both advection and diffusion dominated zones are less than 1, which is smaller than the upper limit of $P_e = 4$ to effectively avoid numerical oscillations and instabilities.

Table 1. Parameters and values used in our numerical model simulations. ~~(adopted from Gomez-Velez et al. (2017)).~~

Parameters	Value	Description
Constant model parameters		
S_y	0.3	Specific yield [-]
λ	40	River boundary wave length [L]
α	5	River boundary amplitude [L]
θ	0.3	Efficient porosity [-]
J_x	0.0025	Base groundwater gradient [-]
σ	1.14	River boundary sinuosity [-]
t_d	10	Duration of flood event [T]
n_d	0.25	Skewness of flood event [-]
t_p	$n_d t_d$	Time to peak river stage [T]
H_0	1	Base river stage [L]
n_0	1	Intensity of flood event [-]
α_L	2	Longitudinal dispersivity [L]

α_T	$0.1\alpha_L$	Transverse dispersivity [L]
Variable~~ed~~ model parameters		
Γ_d	0.1 1 10 100	Dimensionless aquifer transmissivity [-]
δ	90 70 50 20 10	Bank slope angle [°]

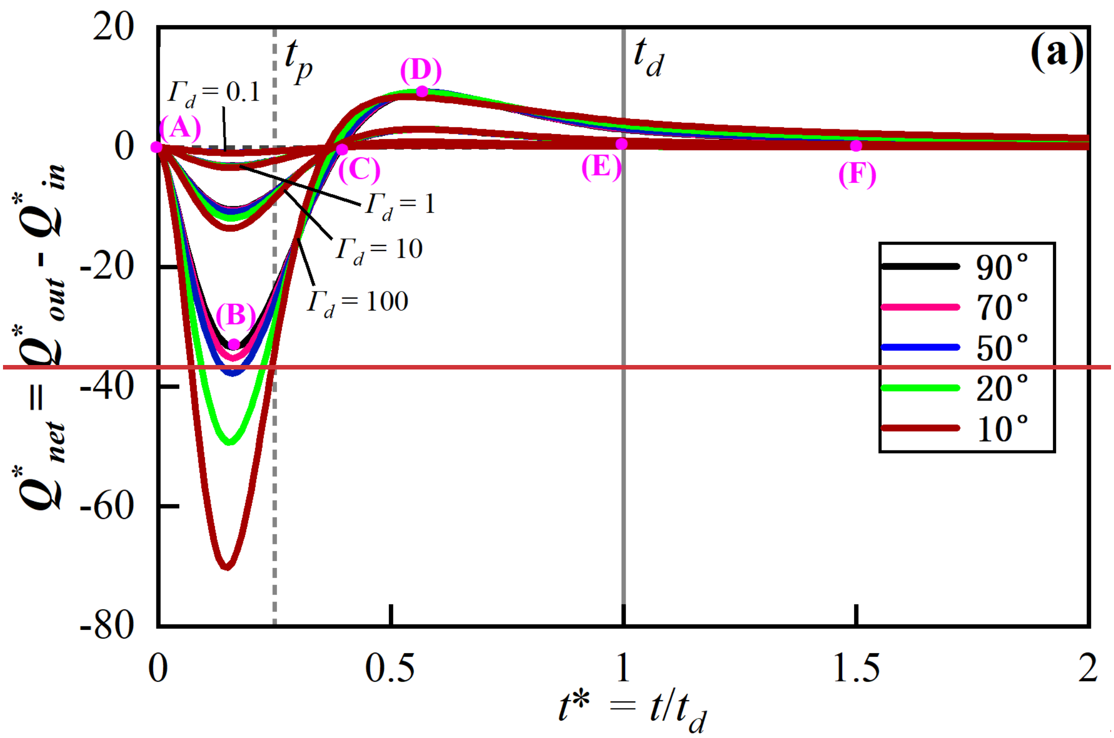
Similar to Gomez-Velez et al. (2017), we evaluate the impact of bank slope by comparing the net hyporheic exchange flux ($Q^*_{net,\ HZ}(t)$), area of HZ ($A^{**}(t)$), penetration distance of the HZ ($d^{**}(t)$) and RTD ($\mu_r^*(\mathbf{x}, t)$) between vertical and sloping river bank models. A ~~D~~detailed definition of these ~~comparison variables~~variables ~~are listed~~is provided in the SI (section ~~as~~ S5).

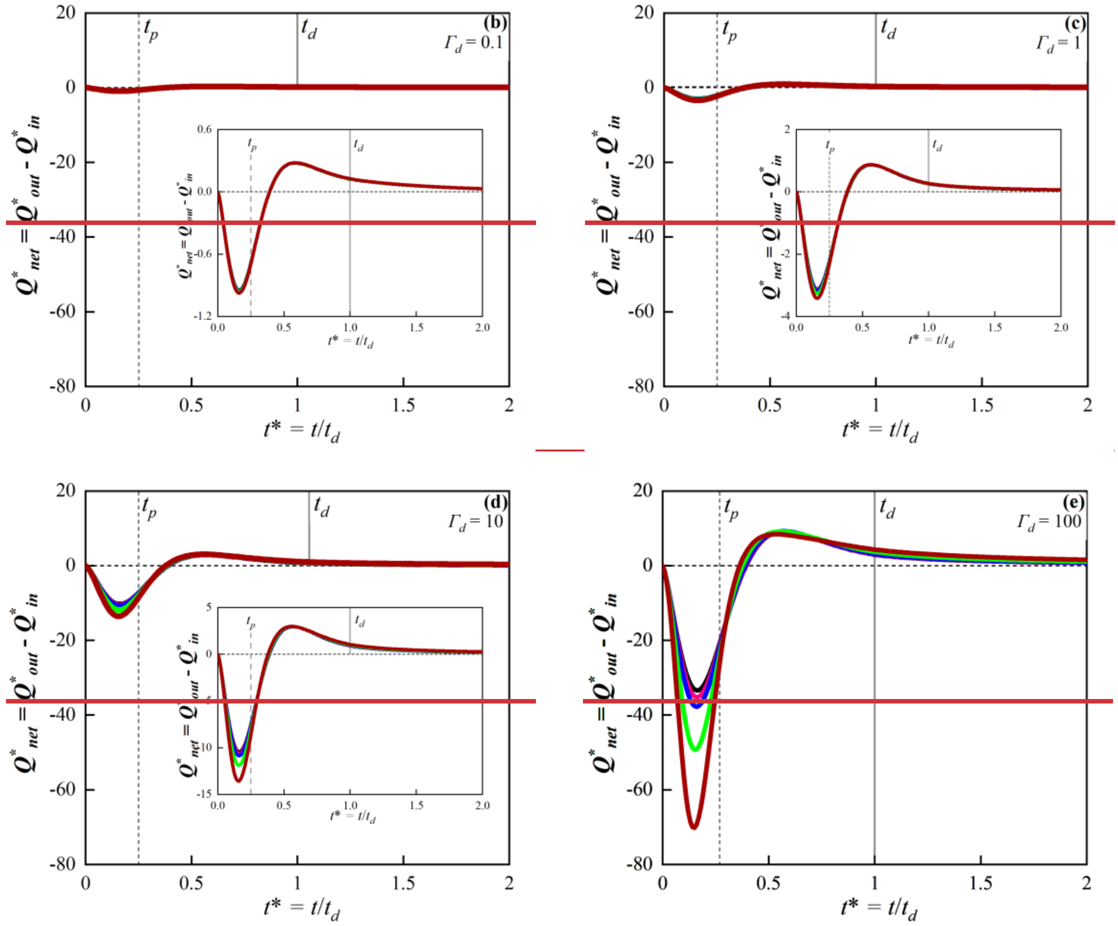
3. Results

3.1 Effect of bank slope on hyporheic exchange flow and HZ ~~patterns~~extent

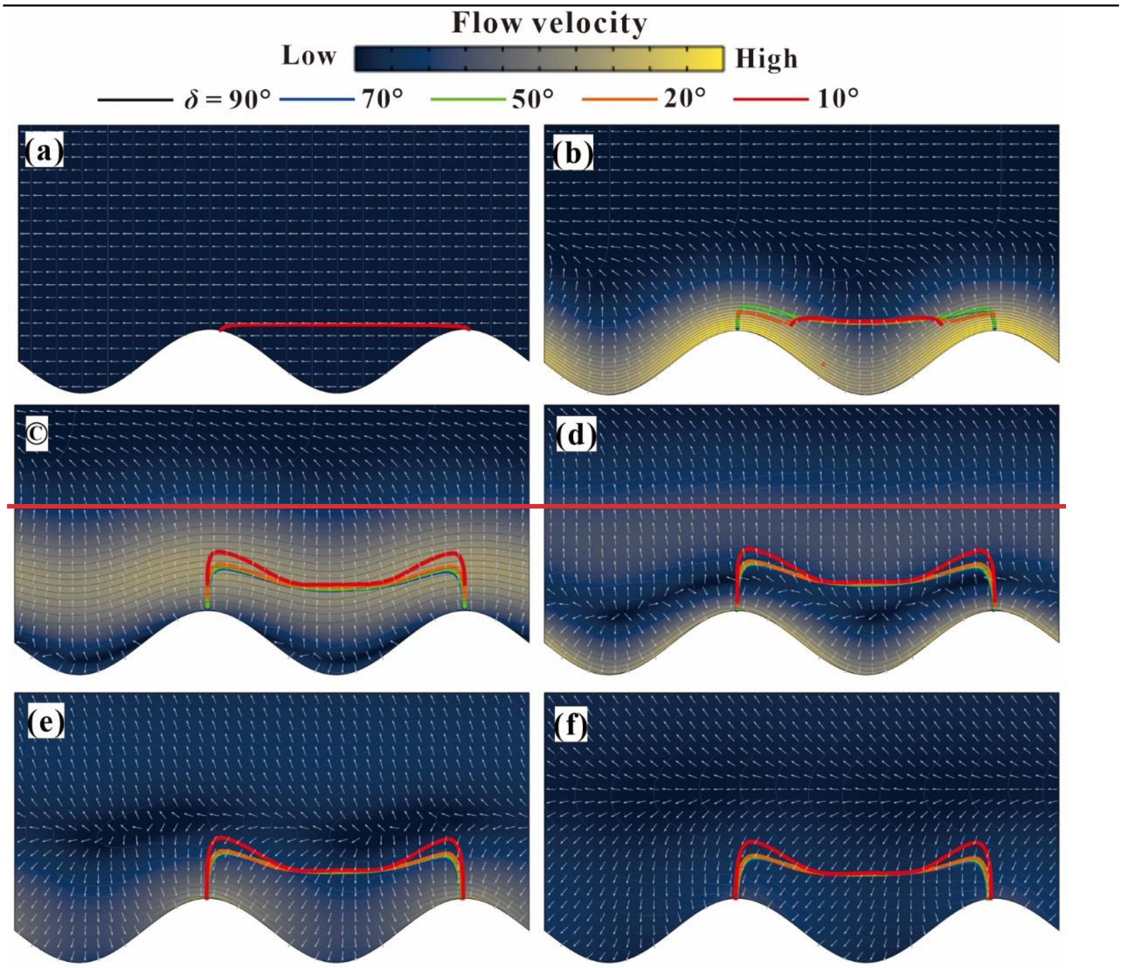
~~3.1.1 Hyporheic exchange flow~~

~~The flow field (velocity magnitude and direction) and net HEF ($Q^*_{net,\ HZ}$ (t)) changed dynamically during and after the simulated flood event. Fig. 2a shows a comparison of $Q^*_{net,\ HZ}$ (t) values for different values of δ and Γ_d. In order to illustrate the influence of δ on $Q^*_{net,\ HZ}$ (t) under different Γ_d conditions more clearly, Fig. 2b – 2e highlight the $Q^*_{net,\ HZ}$ (t) evolution for a given Γ_d at smaller scale. Snapshots of the flow field and the boundary of the HZ area (isolines of $C(\mathbf{x}, t) = 0.5$ as concentration of a conservative solute) for different δ conditions at different times (pink dots in Fig. 2a) for $\Gamma_d = 1$ are shown in Fig. 3a – 3f.~~





~~**Figure 2.** (a) Temporal evolution of dimensionless net flux for alternative values of Γ_d and δ (colored lines). The results for each Γ_d condition from 0.1 to 100 and different slopes are shown again in Fig. 2b – 2e separately, to represent smaller-scales. In each figure, time-to-peak (t_p) and flood duration (t_d) are marked by vertical dashed lines. Pink dots in (a) marked by (A) – (F) correspond to the snapshots of the flow field shown in Fig. 3. A negative flux value here represents water flow from river to aquifer.~~

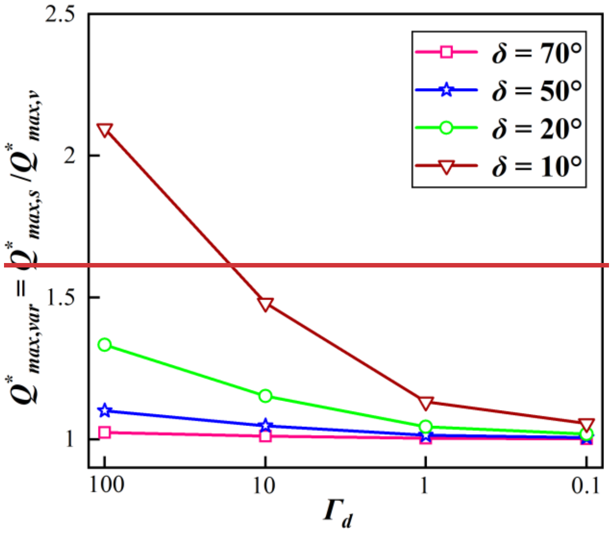


~~**Figure 3.** Temporal evolution of the alluvial flow field and spatial extent of the HZ. Snapshots of the flow field at different time steps during the simulated event (pink dots in Fig. 2a). Colored surfaces represent the magnitude of the Darcy flux vector (blue is low and yellow is high) and white isolines the dimensionless hydraulic head. Bold colored lines correspond to the HZ extent for different bank slope conditions.~~

~~Before the flood event ($t = 0$), steady state base flow conditions are assumed, as shown in Fig. 3a. The inflow and outflow (along the upstream and downstream meander bend, respectively) are in balance. The HZ boundaries for different δ conditions in Fig. 3a are the same before the flood event because the bank slope has no influence on the flow field and HZ extent. The onset of the flood event is indicated by the rising river stage and forces the river to infiltrate into the aquifer along the SWI (negative values of $Q^*_{net,\,HZ}(t)$ in Fig. 2), resulting in the expanded HZ as shown in Fig. 3b. The influx~~

~~of river water into the HZ ($-Q^*_{net, HZ}(t)$) reaches its maximum before the time-to-peak river stage ($t = 0.25t_d$) because the pressure wave propagates into the aquifer and decreases the head gradient between the river and the connected aquifer. An aquifer with larger Γ_d limits the propagation of the pressure wave due to the low transmissivity, which leads to a larger head gradient near the SWI. This, consequently, leads to larger dimensionless net fluxes under increasing Γ_d conditions.~~

~~The maximum dimensionless flux ratios $Q^*_{max, var} = Q^*_{max, s} / Q^*_{max, v}$ of sloping ($\delta < 90°$, $Q^*_{max, s}$) and vertical ($\delta = 90°$, $Q^*_{max, v}$) riverbank cases are shown in Fig. 4. The bank slope is found to increase the infiltration flux by up to 120% ($Q^*_{max, var} \approx 2.2$) for $\Gamma_d = 100$ with $\delta = 10°$ while for larger slope angles or smaller Γ_d the dimensionless infiltration flux gradually decreases. This is because aquifers with smaller Γ_d (higher hydraulic transmissivity) are more sensitive to river stage variation and have a strong ability to transmit the pressure wave into the aquifer. In such cases, the influence of δ on the net flux becomes less important. On the other hand, a smaller δ induces a longer displacement of the SWI ($M(t)$) away from the river, where the groundwater head adjacent to the SWI is always relatively low (i.e., the head in base flow condition).~~



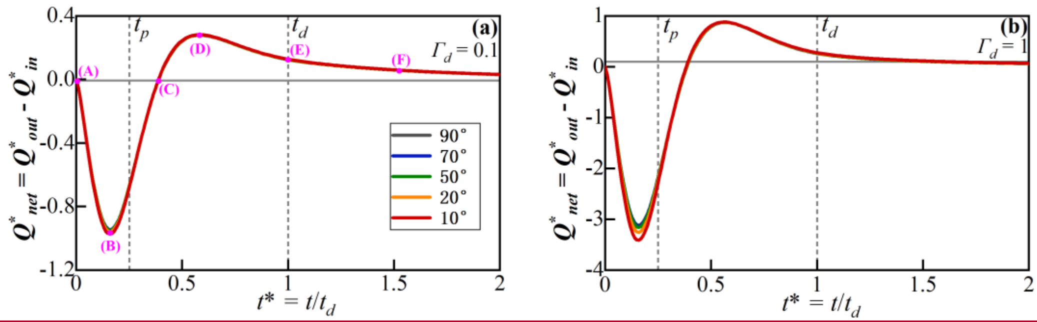
~~**Figure 4.** Ratio of maximum negative net flux of slope to no-slope (vertical river bank)~~

~~conditions $Q^*_{max,var} = Q^*_{max,s}/Q^*_{max,}$ and aquifer transmissivities. The ratios of alternative slope condition are marked by different symbols and colors.~~

~~As the river stage decreases after t_p, the head gradient near the SWI gradually reverses and the net outflux starts increasing (the river is gaining water). This is associated with the river stage declining below the groundwater level (see Fig. 3c – 3f). Fig. 2 shows that the bank slope has little impact on the net outflux. Where $\Gamma_d = 100$, bank slope can slightly extend the time required for the system to recover to initial condition after t_p but in general, the response of the net outflux to bank slope is negligible when compared to that of the influx. Eventually, the net flux converges to zero, which indicates the flow field within the aquifer recovers to the initial conditions.~~

3.1.1 Hyporheic exchange flow

The flow field (velocity magnitude and direction) and net HEF ($Q^*_{net,\ HZ}(t)$) changed dynamically during and after the simulated flood event. Fig. 3a – 3d shows the evolution of net HEF for different aquifer transmissivity (Γ_d) and bank slope angle (δ) condition. Snapshots of the flow field and the boundary of the HZ area (isolines of $C(\mathbf{x}, t) = 0.5$ as concentration of a conservative solute) for different δ conditions at different times (pink dots in Fig. 3a) for $\Gamma_d = 1$ are shown in Fig. 4a - 4f.



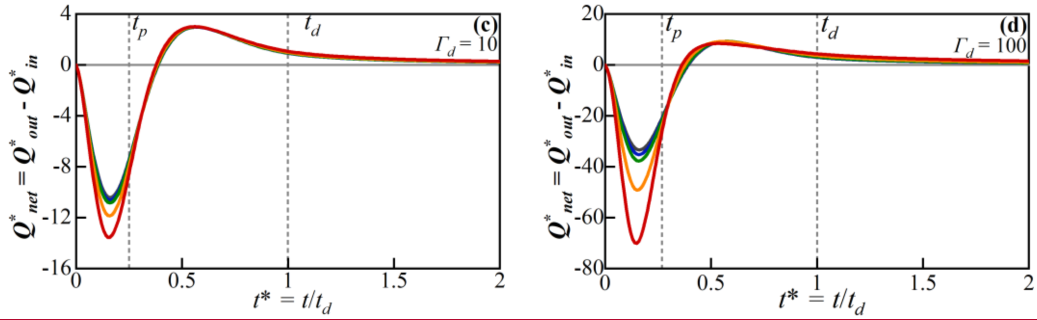


Figure 3. Temporal evolution of dimensionless net ~~flux~~HEF ($Q^*_{net,\ HZ}$ (t)) for four different aquifer transmissivity values (represented by Γ_d) and bank slopes ~~condition~~angles (δ, from 10-90 degrees). Time-to-peak flood (t_p) and flood duration (t_d) are marked by vertical dashed lines. Pink dots in (a) marked by (A) - (F) correspond to the snapshots of the flow field shown in Fig. 4. A negative flux value here represents water flow from river to aquifer. Note that Γ_d negatively correla~~tion~~tes with the transmissivity of aquifer.

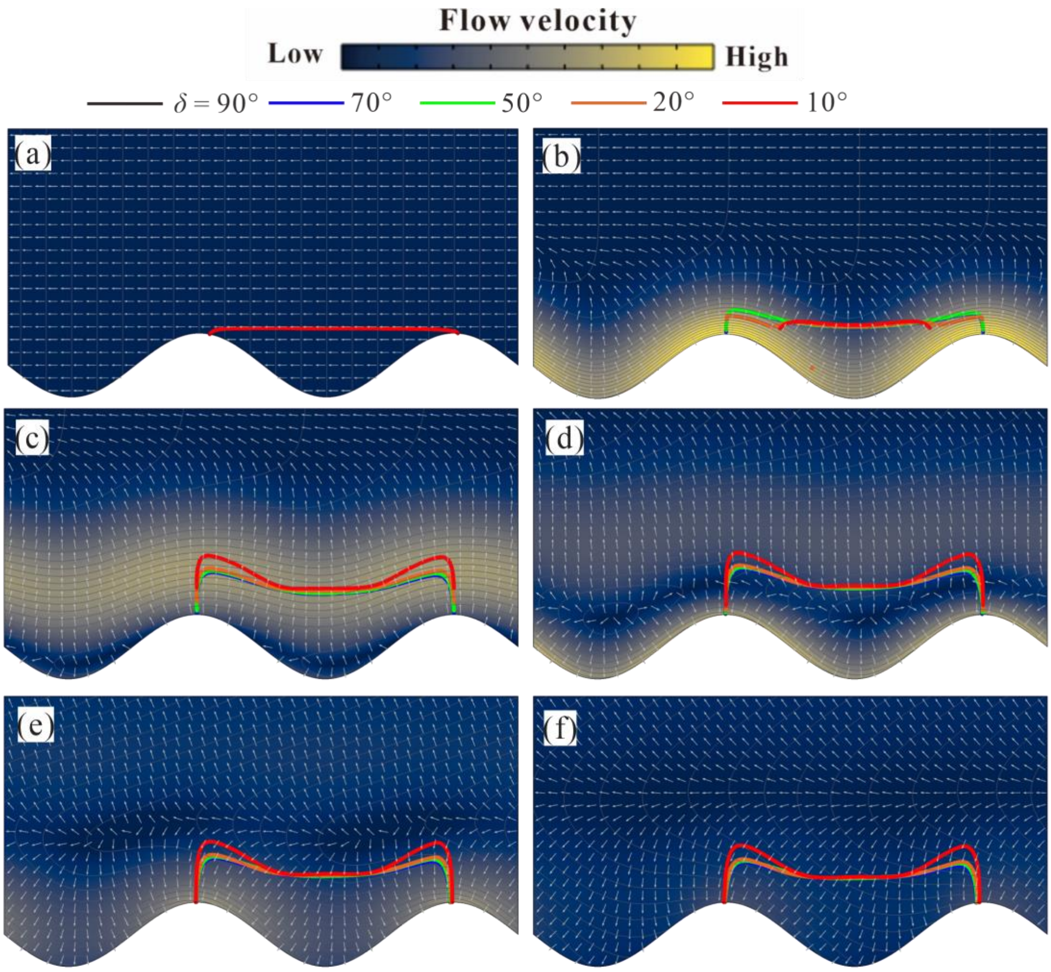


Figure 4. Pla~~i~~n view of the river channel and aquifer showing~~of~~ the temporal evolution of the alluvial flow field and spatial extent of the HZ. (a)-(e) are ~~S~~snapshots of the flow field at different time steps during the simulated event (pink dots in Fig. 3a). Colored surfaces represent the magnitude of the Darcy flux vector (blue is low and yellow is high) and white isolines the dimensionless hydraulic head. Bold colored lines correspond to the HZ extent for different bank slope conditions. ~~The blue lines for δ = 70° were covered due to the small difference compared with other δ conditions.~~

Before the flood event (t = 0), steady-state base flow conditions are assumed, as shown in Fig. 4a. The inflow and outflow (along the upstream and downstream meander bend, respectively) are in balance. The bank slope has no effect on the HZ boundaries before the flood event.

Before ~~the~~ peak river stage of the flood event is reached ($0 < t < 0.25t_d$), the onset of the flood event is indicated by the rising river stage and forces the river to infiltrate into the aquifer along the SWI (negative values of $Q^*_{net,\,HZ}\,(t)$ in Fig. 3), resulting in the ~~expand~~expansion of the~~ed~~ HZ as shown in Fig. 4b. The influx of river water into the HZ ($-Q^*_{net,\,HZ}\,(t)$) reaches its maximum before the time-to-peak river stage ($t = 0.25t_d$) because the pressure wave propagates into the aquifer and decreases the head gradient between the river and the connected aquifer. For higher transmissivity aquifers (Lower Γ_d values ~~conditions~~ in Fig. 3), bank slope ~~plays~~has a ~~minor~~reduced impact on ~~the calculation of~~ net out~~flow~~ flux as the fast propagation of the pressure wave results in the hydraulic ~~, mainly because the fast propagation of pressure wave results in the hydraulic~~ head near the SWI to be very~~were~~ similar. Among different aquifer transmissivity conditions. As ~~transmissivity of aquifer decreased~~aquifer transmissivity decreases, the ability of the aquifer to transmit the pressure wave ~~was~~becomes limited, ~~while~~and the interaction flux ~~was~~is dominated by the location (displacement) ~~of~~of the SWI and the river stage. On the other hand, a smaller slope angle induces a longer displacement of the SWI ($M(t)$) away from the river, where the groundwater head adjacent to the SWI is always relatively high (i.e., the head in base flow condition). This, consequently, leads to a larger head gradient near the SWI as well as larger dimensionless net fluxes under increasing Γ_d conditions as shown in Fig. 3.

The maximum dimensionless flux ratios $Q^*_{max,\,var} = Q^*_{max,\,s} / Q^*_{max,\,v}$ of sloping ($\delta < 90°$, $Q^*_{max,\,s}$) ~~and~~vs vertical ($\delta = 90°$, $Q^*_{max,\,v}$) riverbank cases are shown in Fig. 5, which indicates the deviation in predicting peak net flux when neglecting the slope of the river~~-~~bank. The bank slope is found to increase ~~the infiltration flux~~infiltration by up to 120% ($Q^*_{max,\,var} \approx 2.2$) for $\Gamma_d = 100$ with $\delta = 10°$ while for larger slope angles or higher hydraulic transmissivities~~y~~ the dimensionless ~~infiltration flux~~infiltration gradually decreases.

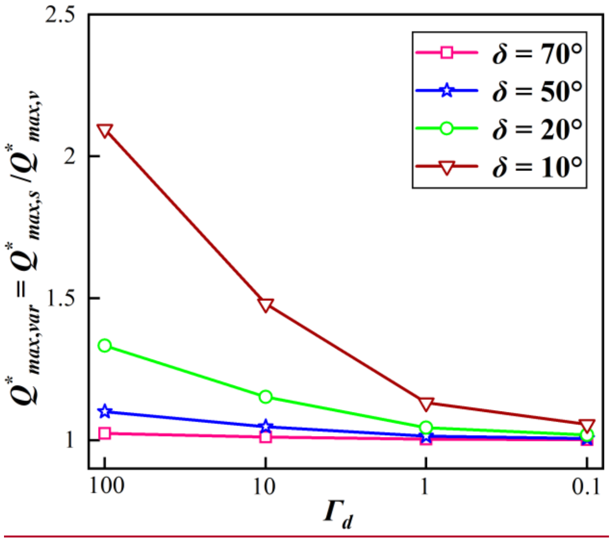
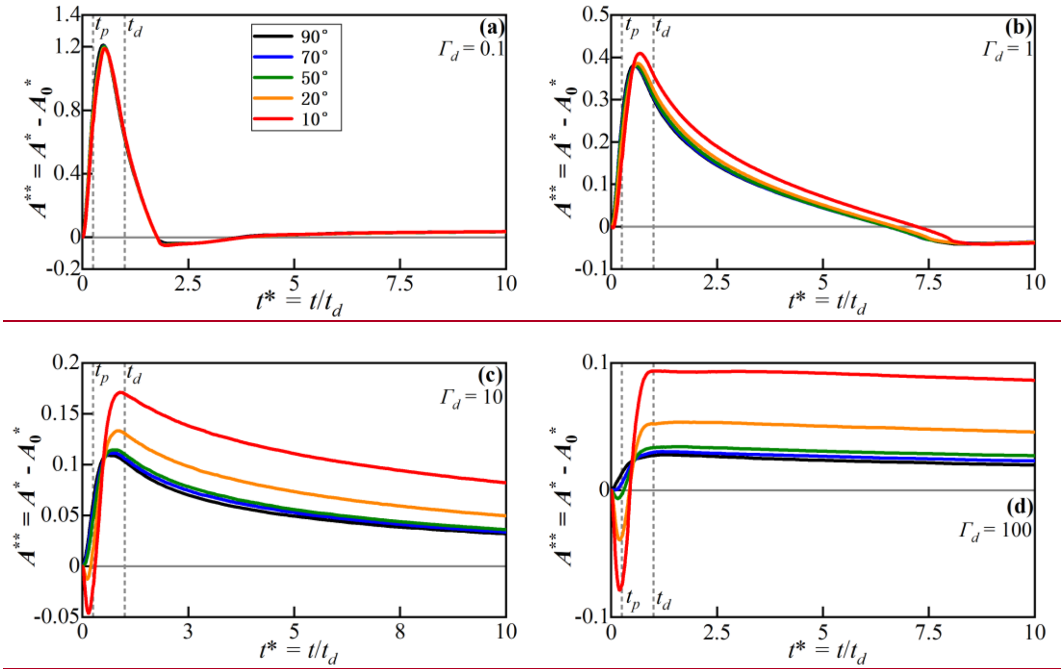


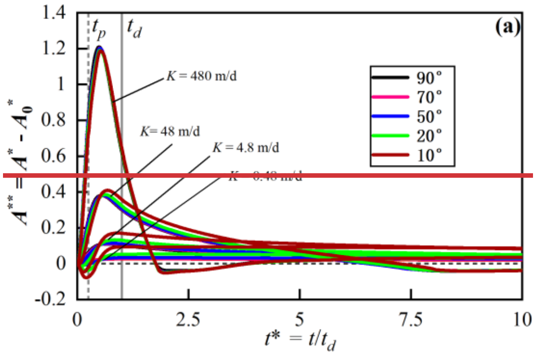
Figure 5. Ratio of maximum~~ negative~~ net flux ~~of~~for slope to no-slope (vertical river bank) conditions $Q^*_{max,var} = Q^*_{max,s}/Q^*_{max,v}$ for ~~various~~four aquifer transmissivities and slope angles. ~~The ratios of alternative slope condition are marked by different symbols and colors.~~ Note that Γ_d negatively correlat~~ion~~es with ~~the~~aquifer ~~transmissivit~~transmissivity.~~y of aquifer.~~

As the river stage decreases after t_p, the head gradient near the SWI gradually reverses and the net outflux starts increasing (the river is gaining water) as shown in Fig. 3. This is associated with the river stage declining below the groundwater level (see Fig. 4c - 4f). ~~The groundwater stage near SWI were similar among different bank slope angle condition after the peak time of flood event, thus, Fig. 3 shows that the bank slope has little impact on the net outflux.~~For the ~~Where for the~~ lowest hydraulic transmissivity condition (Γ_d = 100), bank slope can slightly extend the time required for the system to recover to initial conditions after t_p but in general, the response of the net outflux to bank slope is negligible when compared to that of the influx. Eventually, the net flux converges to zero, which indicates the flow field within the aquifer recovers to the initial conditions. The bank slope has no impact on the HEF after the duration of flood event.

3.1.2 Patterns of hyporheic area and penetration distance

Fig. 6~~5a~~ and Fig. 7~~6a~~ show the temporal evolution of the dimensionless HZ area ($A^{**}(t)$) and penetration distance ($d^{**}(t)$) into the alluvial valley relative to the initial condition for varying aquifer transmissivity (Γ_d) ~~Γ_d~~ and slope angles~~, while Fig. 5b – 5e and Fig. 6b – 6e illustrate the impact of δ on $A^{**}(t)$ and $d^{**}(t)$ for different values of Γ_d in a close up.~~. ~~The vertical dash lines in the subfigures of Fig. 6 and Fig. 7 are the time time-to-peak (t_p) and flood duration (t_d)~~





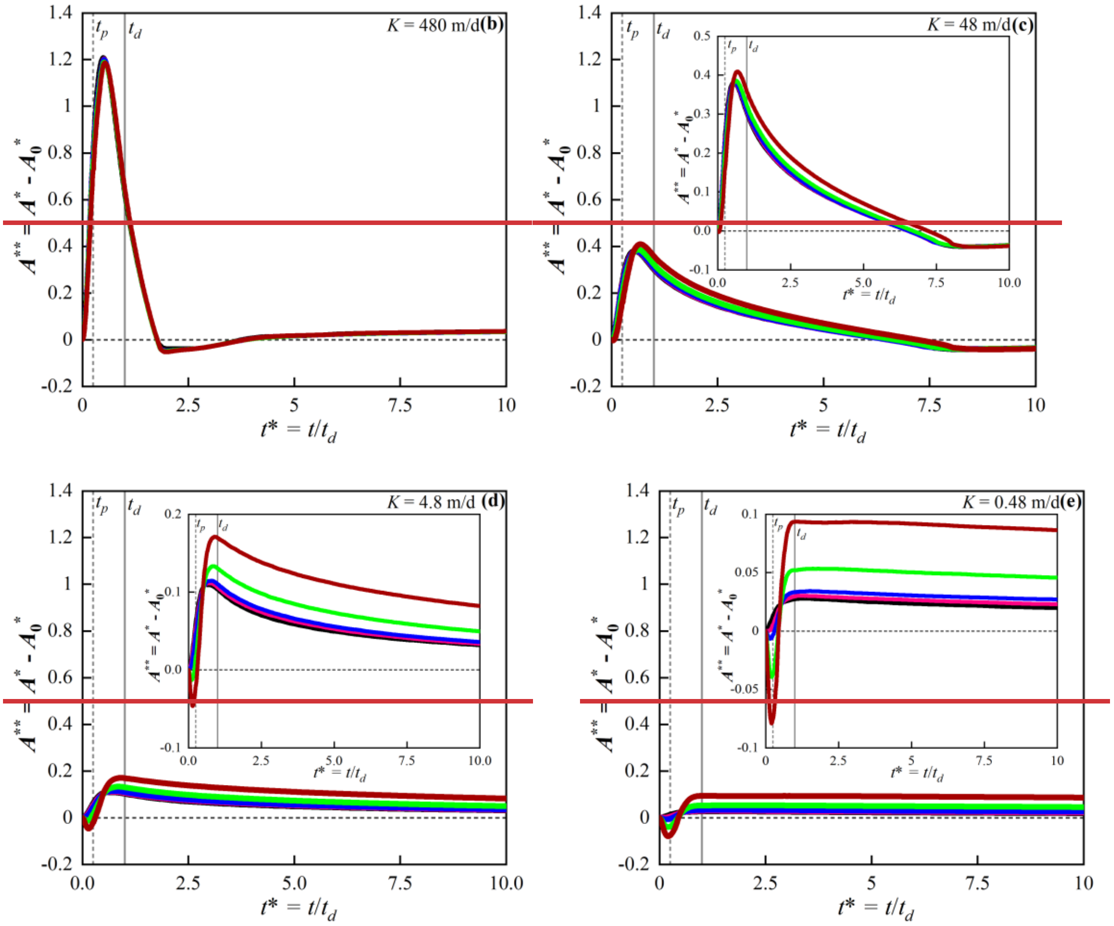
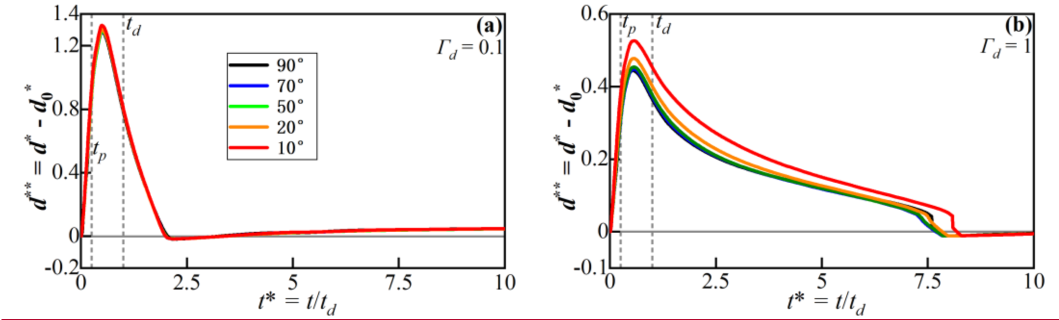
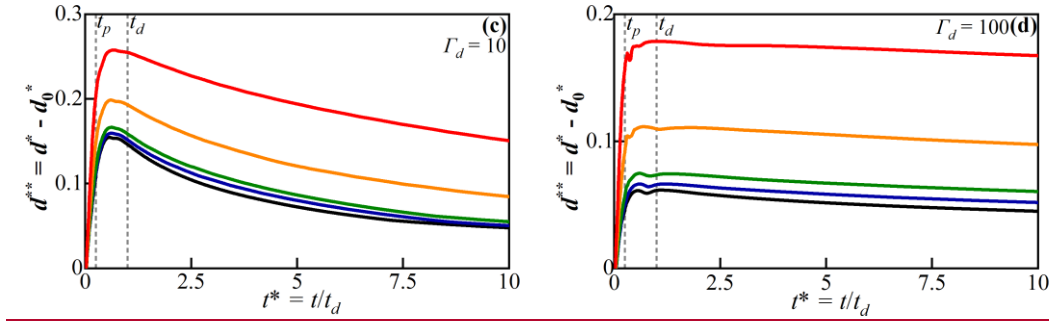
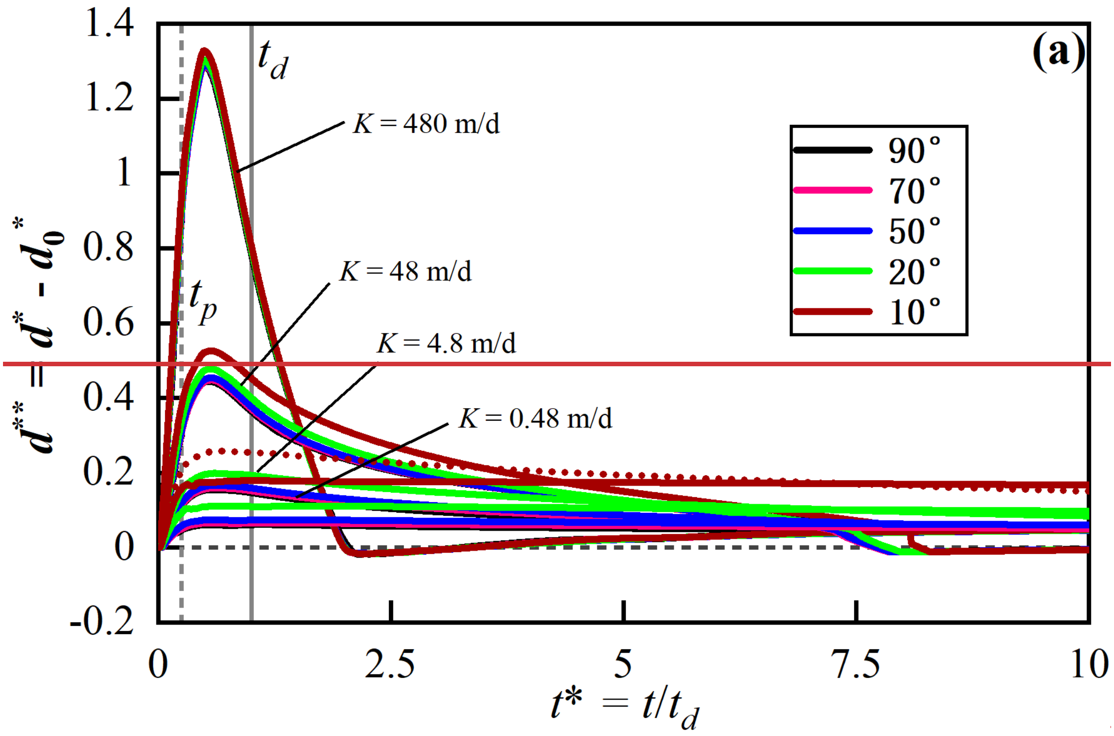


Figure ~~5~~6. ~~(a)~~ Temporal evolution of dimensionless HZ area for different values of Γ_d and δ (colored lines). Time-to-peak (t_p) and flood duration (t_d) are marked by vertical dashed lines.~~For clarity, the results for each Γ_d condition from 0.1 to 100 are shown again separately in (b) to (e), with inserts representing smaller scales.~~

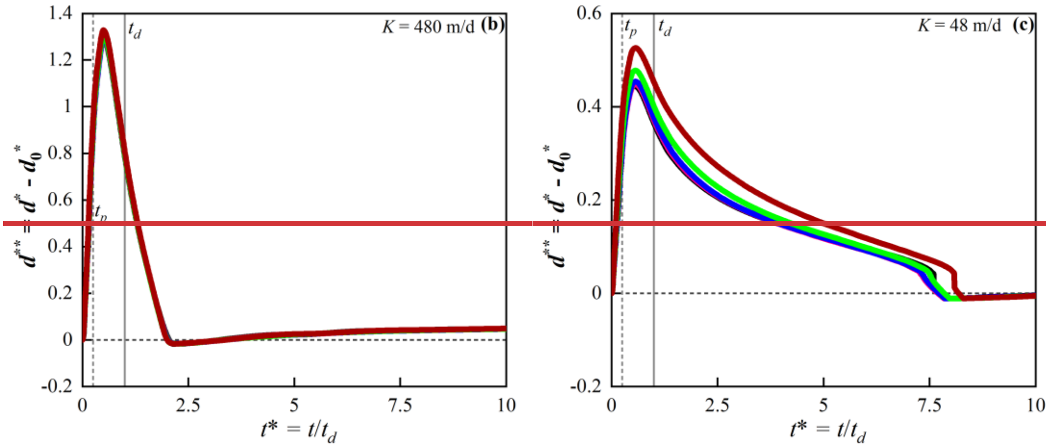




0.3
0.2
0.1
0
t_p
t_d
(c)
$\Gamma_d = 10$
$d^{**} = d^* - d_0^*$
0
2.5
5
7.5
10
$t^* = t/t_d$
0.2
0.1
0
t_p
t_d
$\Gamma_d = 100$ (d)
$d^{**} = d^* - d_0^*$
0
2.5
5
7.5
10
$t^* = t/t_d$



(a)
t_d
t_p
$K = 480$ m/d
$K = 48$ m/d
$K = 4.8$ m/d
$K = 0.48$ m/d
90°
70°
50°
20°
10°
1.4
1.2
1
0.8
0.6
0.4
0.2
0
-0.2
$d^{**} = d^* - d_0^*$
0
2.5
5
7.5
10
$t^* = t/t_d$



$K = 480$ m/d (b)
t_p
t_d
1.4
1.2
1
0.8
0.6
0.4
0.2
0
-0.2
$d^{**} = d^* - d_0^*$
0
2.5
5
7.5
10
$t^* = t/t_d$
$K = 48$ m/d (c)
t_p
t_d
0.6
0.4
0.2
0
-0.2
$d^{**} = d^* - d_0^*$
0
2.5
5
7.5
10
$t^* = t/t_d$

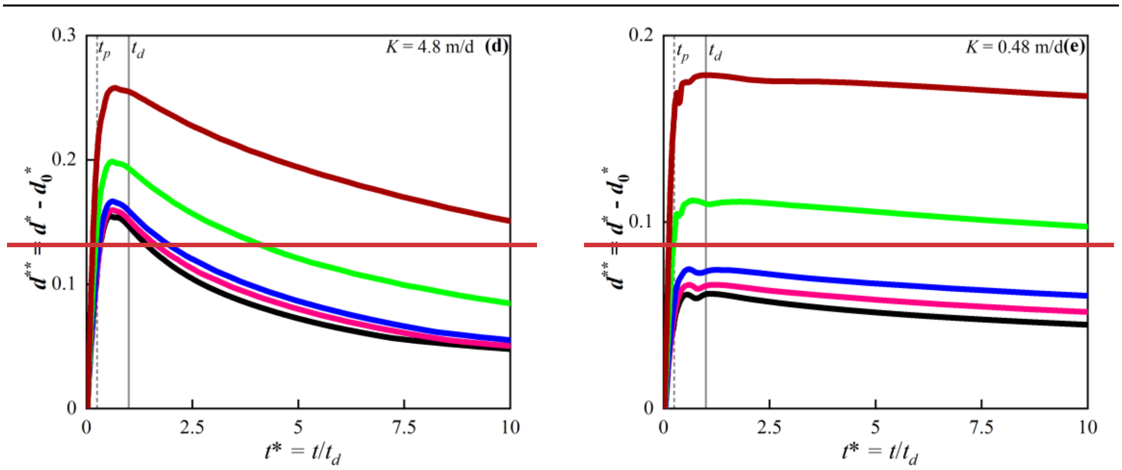


Figure 7~~6~~. ~~(a)~~ Temporal evolution of dimensionless HZ penetration distance into the alluvial valley (d^{**}) for ~~alternative~~ different values of Γ_d and δ (color lines). Time-to-peak (t_p) and flood duration (t_d) are marked by vertical dashed lines.~~. For clarity, the results for each Γ_d condition from 0.1 to 100 are shown again in (b) to (e), with inserts representing smaller scales.~~

For vertical banks (δ = 90°, ~~grey~~ black lines in Fig. 6~~5~~), the ~~A^{**}(t)~~HZ area increases synchronously with the river stage ($t < t_p$). After the peak time of the flood event ($t > t_p$), the HZ area ~~A^{**}(t)~~ continues to ~~rise~~ extend as river~~due to the~~ water ~~in the river~~ still ~~discharging~~ recharges~~into~~ the aquifer. ~~Furthermore, the groundwater mound (raised water table) continues to expand, migrating into the aquifer (see the more penetrated groundwater mound from in Fig. 34b vs Fig. 34c).~~ After the flood event ($t > t_d$), the river water that was stored in the aquifer ($C(\mathbf{x}, t) > 0$) slowly discharges back into the river channel. Thus, the HZ area and penetration distance gradually rebound to initial conditions.

Under sloping riverbank conditions, the riverbank will at times be submerged by the rising river stage. Fig. ~~5~~6a~~b~~ and ~~6~~7a~~b~~ show that the effects of bank slope on HZ area ($A^{**}(t)$ in Fig. 6) and penetration distance ($d^{**}(t)$ in Fig. 7) are almost counteracted by the high transmissivity of the aquifer and the influence of bank slope ~~on HZ area and penetration distance~~ ~~is~~ was negligible. At the beginning of the flood event, Fig. ~~5~~6b~~e~~ ~~-~~ – ~~5~~6d~~e~~ show that for conditions with smaller ~~δ~~sloping angle, ~~A^{**}(t~~HZ area~~)~~ can be less than zero (HZ at these times are smaller than the initial condition). This is due to the fact that the movement of the SWI during a rising river stage towards the alluvial valley will submerge parts that were previously unsaturated as the aquifer with low transmissivity will propagate water more slowly. As ~~F_d~~ aquifer transmissivity de~~in~~creases from Fig. ~~5~~6b~~d~~ ~~–~~ – ~~5~~6d~~e~~, ~~smaller values of A^{**} were observed that~~the relative HZ area ~~stay~~ remains negative for a longer time for smaller bank slopes ~~δ~~. This indicates that ~~the~~ bank slope has a more ~~significant~~ pronounced effect on HZ ~~area~~ extent in cases where ~~F_d~~aquifer transmissivity is large as a low-transmissivity aquifer takes more time to propagate infiltrating river water.

After about half ~~of~~ the flood duration ($t > 0.5t_d$), ~~all of $A^{**}(t)$relative HZ area~~the HZ area (A^{**}) becomes positive in all scenarios as t~~due to the re-emergence of t~~he model domain previously submerged during the flood event re-emerges. As ~~F_d~~aquifer transmissivity de~~in~~creases (~~from~~ Fig. ~~5~~6a~~b~~ ~~–~~ – ~~5~~6d~~e~~ and ~~from~~ Fig. 7~~6~~a~~b~~ ~~–~~ – 7~~6~~d)~~e~~, the impact of bank slope~~δ~~ gradually ~~emerges~~ increases especially in low aquifer transmissivity ~~larger F_d~~ conditions, whereb~~by~~ smaller bank slope~~δ~~ can increase the peak values of ~~$A^{**}(t)$ and $d^{**}(t)$~~area and penetration distance ~~of HZ~~, and delay the arrival time-to-peak value of the ~~of the maximum value of $A^{**}(t)$~~relative HZ area. After the flood event ($t > t_d$), the effect of bank slope is counteracted by the higher aquifer transmissivity and only ~~for large~~ lower transmissivities ~~has~~have a significant impact on the HZ resulting in larger $A^{**}(t)$ and $d^{**}(t)$ as shown in Fig. ~~5~~6b~~e~~ ~~–~~ – ~~5~~6d~~e~~ and Fig. ~~6~~7b~~e~~ ~~–~~ – ~~6~~7d~~e~~. For low transmissivity scenarios, the bank slope can increase the peak area and penetration of HZ by almost 200%.~~, and the lasting time of that impact positively related to the aquifer transmissivity.~~

3.2 Spatiotemporal evolution of mean residence time distribution

The evolution of spatiotemporal patterns of mean RTD (i.e., travel time of river water in aquifer) is a useful evaluation method for identifying the dynamic variation of aging and rejuvenation of hyporheic water. Here we use the mean RT ratio between a sloping model and a vertical model $\mu_r^*(\mathbf{x}, t) = \log_{10}(\mu_{\tau\text{-}S}(\mathbf{x}, t)/\mu_{\tau\text{-}V}(\mathbf{x}, 0))$ to evaluate the influence of bank slope on the prediction of~~ed~~ RTD for a given location and time ~~(overestimates or underestimates)~~. Fig. ~~7~~8 presents RTDs for the initial condition, where $\mu_{\tau0\text{-max}}$ is the maximum RT in the domain. It can be seen that the isolines representing the RT are almost horizontal in the area extending from the river but RT near the upstream river bend is smaller than downstream because the initial flow direction is towards the negative direction of the x axis. Notably, $\mu(\mathbf{x}, 0)$ grows almost exponentially as y increases, and a positive correlation to Γ_d at a given location is observed.

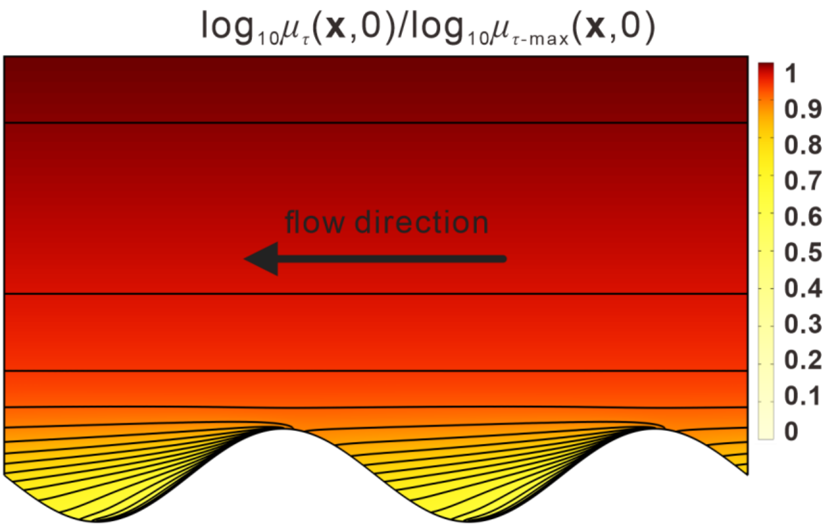
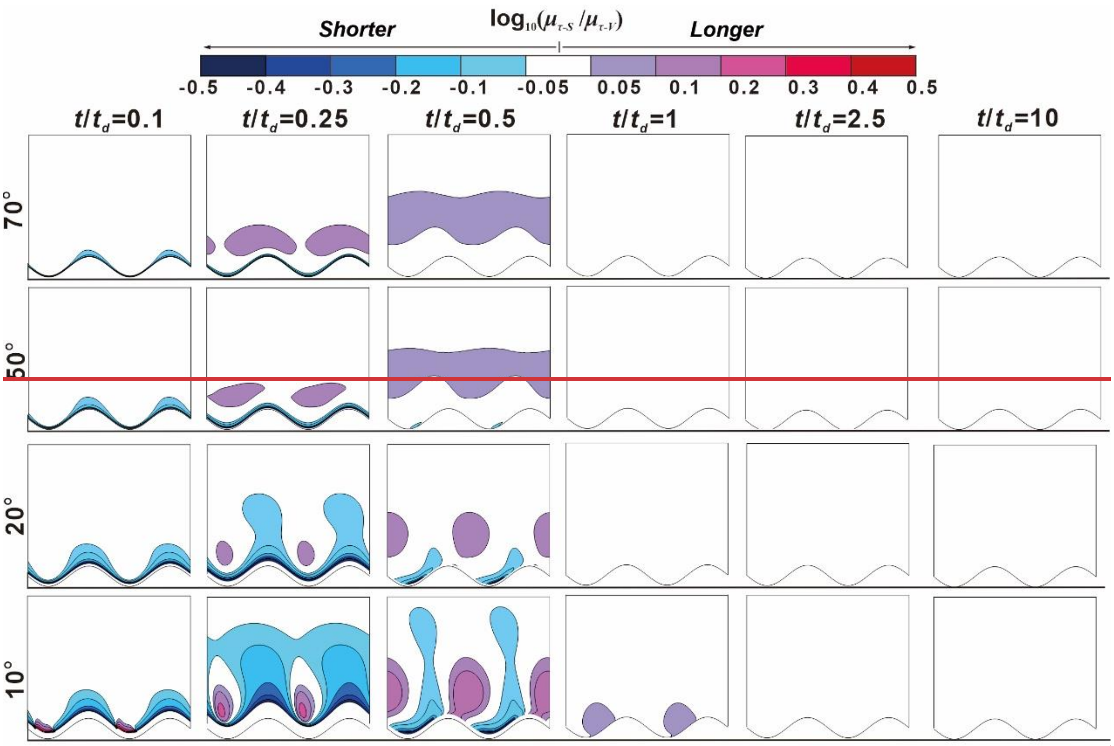


Figure ~~7~~8. Relative mean residence time distributions [-] for baseline flow conditions (no bank slope), which are represented by $\log_{10}\mu_\tau(\mathbf{x}, 0)/\log_{10}\mu_{\tau\text{-max}}(\mathbf{x}, 0)$ to show the distribution pattern. The value of the contour lines grows exponentially with the distance from the river meander.

Fig. 9~~8~~ - 12~~1~~ present five snapshots of μ_r^* for different bank slope angles~~s~~ ~~for~~ and different aquifer transmissivity aquifers (Γ_d = 0.1, 1, 10 and 100, respectively)~~,~~. The five snapshots represent ~~at~~ the rising limb of the flood event (t/t_d = 0.1), the peak of the flood event (t/t_d = 0.25), the falling limb of the flood event (t/t_d = 0.5) and a time after the flood event (t/t_d = 1, 2.5 and 10). The RT differences between sloping and vertical riverbank models are within 12.2% in the white-colored areas (-0.05 < μ_r^* < 0.05) of Fig. ~~8~~9 - 1~~1~~2, which indicates a minor effect of bank slope on RTD. The colored areas in Fig. 9 – ~~21~~12 indicate model results where~~re~~ neglecting bank slope ~~in models will result~~ will lead to~~in~~ overestimated (μ_r^* < -0.05) or underestimated (μ_r^* > 0.05) ~~RT prediction.~~of residence (travel) time.



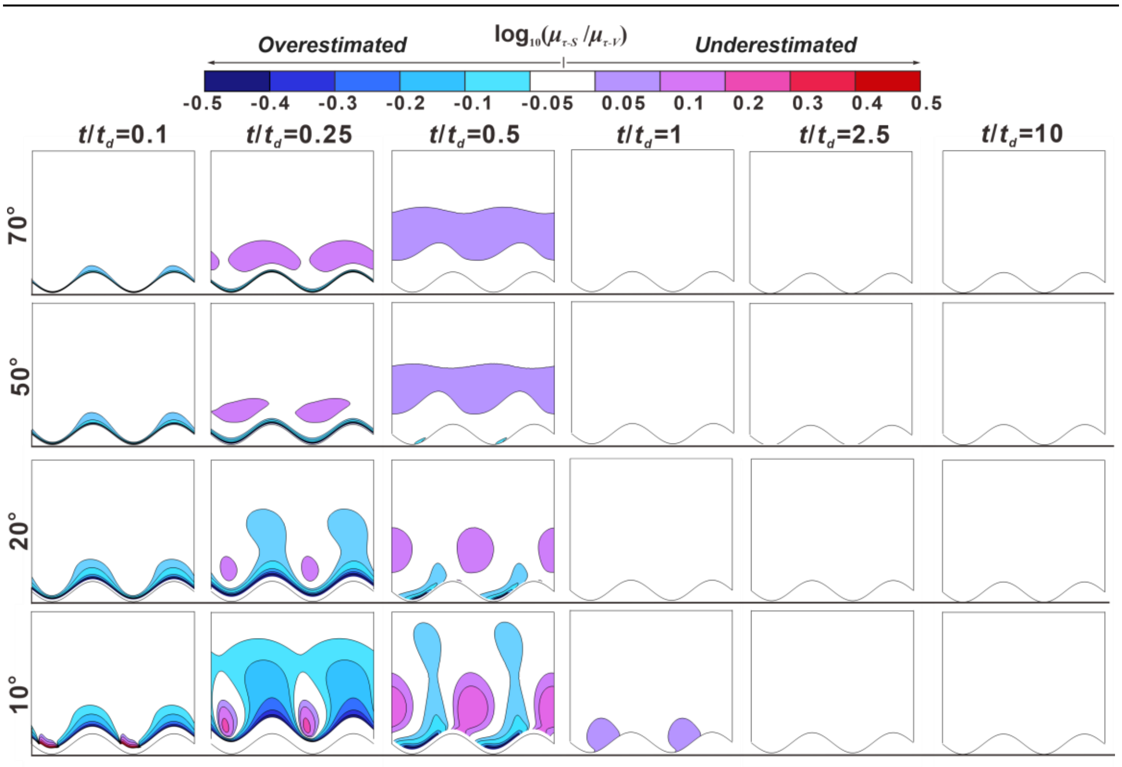
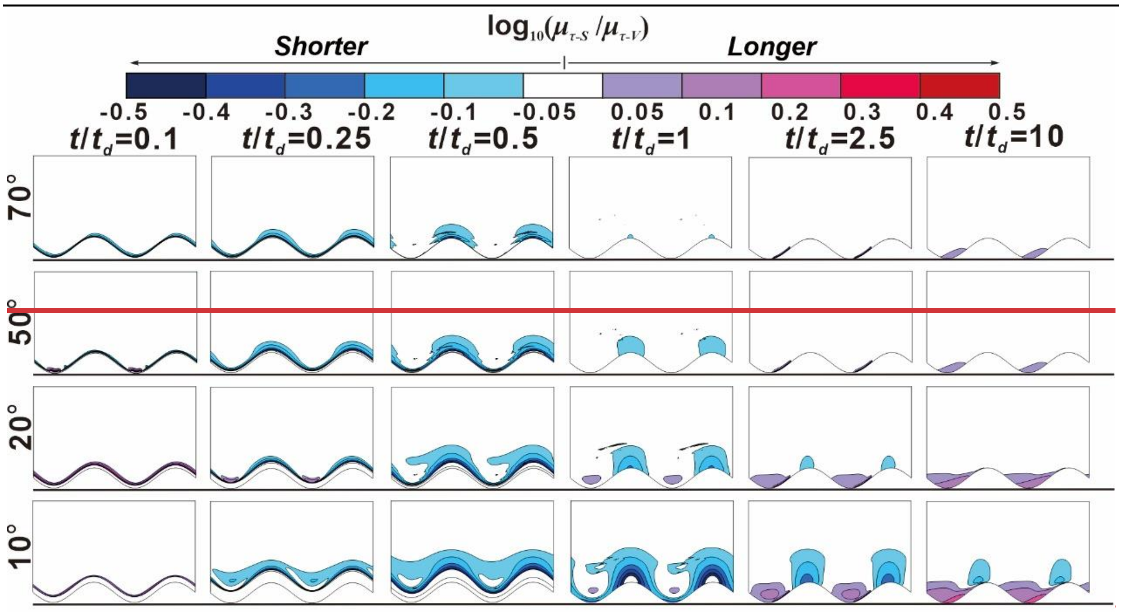


Figure ~~8~~9. Five ~~S~~snapshots for the RTD ratio ($\mu_r^*(\mathbf{x}, t) = \mu_{r\text{-}S}^*(\mathbf{x}, t)/ \mu_{r\text{-}V}^*(\mathbf{x}, t)$) ~~$\mu_r^*(\mathbf{x}, t)$~~ between sloping ($\mu_{r\text{-}S}^*(\mathbf{x}, t)$) and vertical riverbank conditions ($\mu_{r\text{-}V}^*(\mathbf{x}, t)$) at different times t/t_d as a function of δ for $\Gamma_d = 0.1$. ~~Warm and cold colors in Fig. 9 indicates neglecting bank slope would underestimate and overestimate in the prediction of RT, respectively.~~ The horizontal lines beneath each figure are the reference lines to show the initial location of the peak point of the ~~SWI~~point bar. The lower sinuous lines at the reference lines are the initial SWIs. The colored areas indicate where the bank slopes have significant impact ~~o~~on~~f~~ RT (difference in RT between sloping and vertical model~~s~~ larger than 12.2%) and residence (travel) times of river water in the aquifer would be overestimated or underestimated.~~.~~



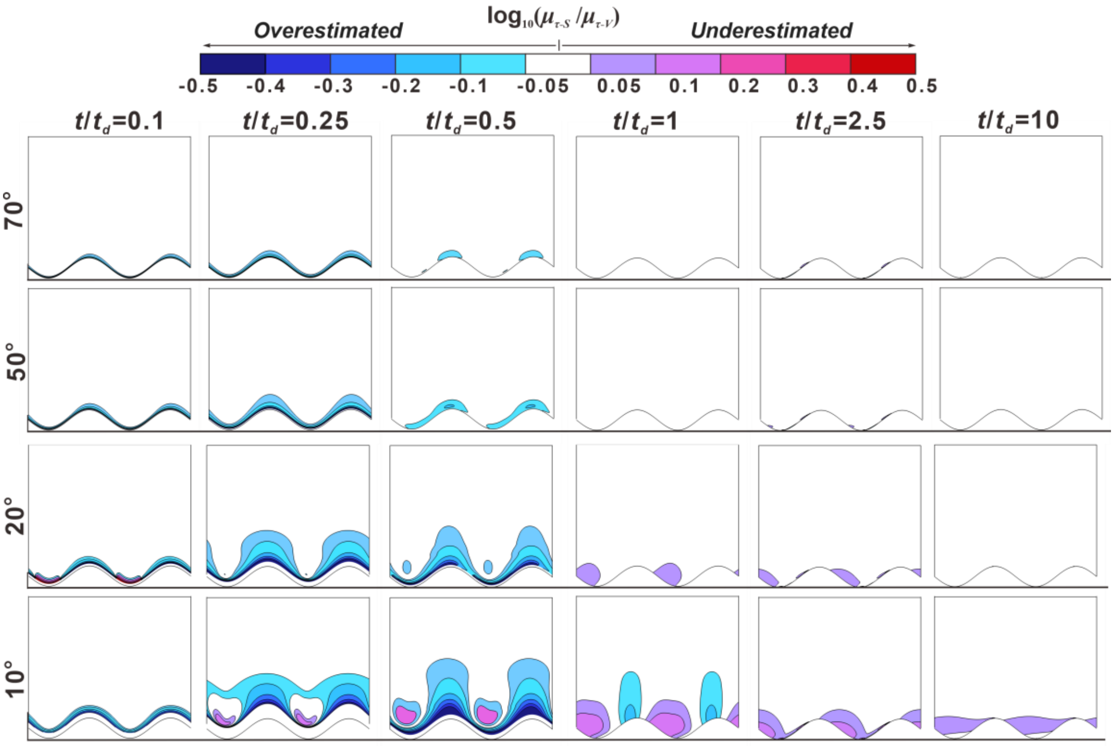
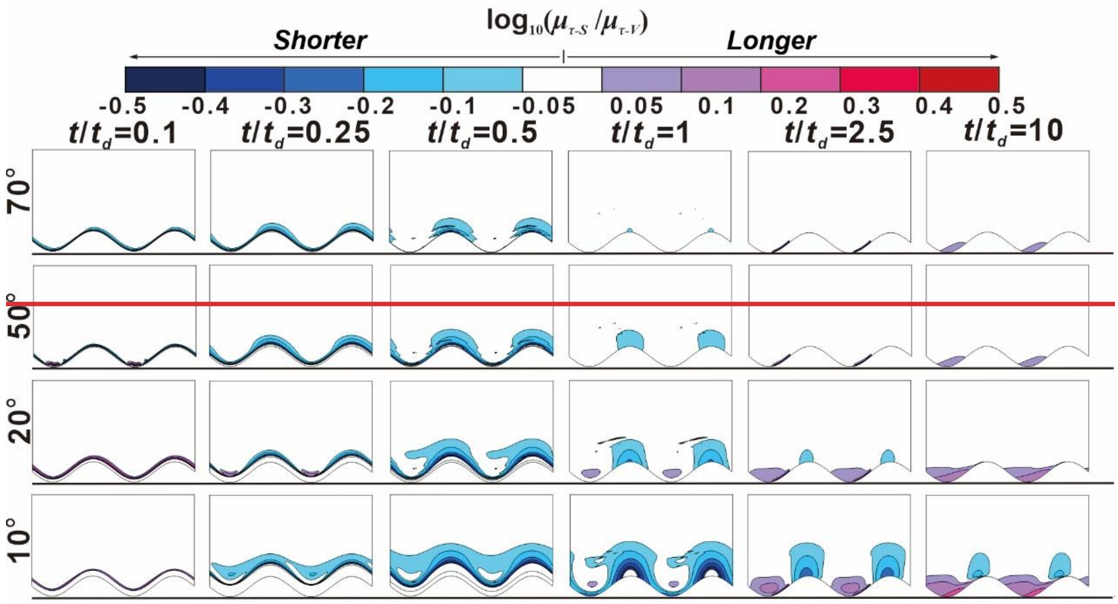


Figure ~~9~~10. Five snapshots for the RTD ratio ($\mu_r^*(\mathbf{x}, t) = \mu_{r\text{-}S}^*(\mathbf{x}, t)/\ \mu_{r\text{-}V}^*(\mathbf{x}, t)$) between sloping ($\mu_{r\text{-}S}^*(\mathbf{x}, t)$) and vertical riverbank conditions ($\mu_{r\text{-}V}^*(\mathbf{x}, t)$) at different times t/t_d as a function of δ for $\Gamma_d = 1$.The horizontal lines beneath each figure are the reference lines to show the initial location of the peak point of the point bar. The lower sinuous lines at the reference lines are the initial SWIs. The colored areas indicate where the bank slopes have significant impact on RT (difference in RT between sloping and vertical model larger than 12.2%) and residence (travel) times of river water in the aquifer would be overestimated or underestimated~~Snapshots for the RTD ratio $\mu_r^*(\mathbf{x}, t)$ between sloping and vertical riverbank conditions at different times t/t_d as a function of δ bank slope angle for $F_d = 1$. The horizontal lines beneath each figure are the reference lines to show the initial location of peak point of point barSWI. The lower sinuous lines at the reference lines are the initial SWIs (difference in RT between sloping and vertical models larger than 12.2%).~~



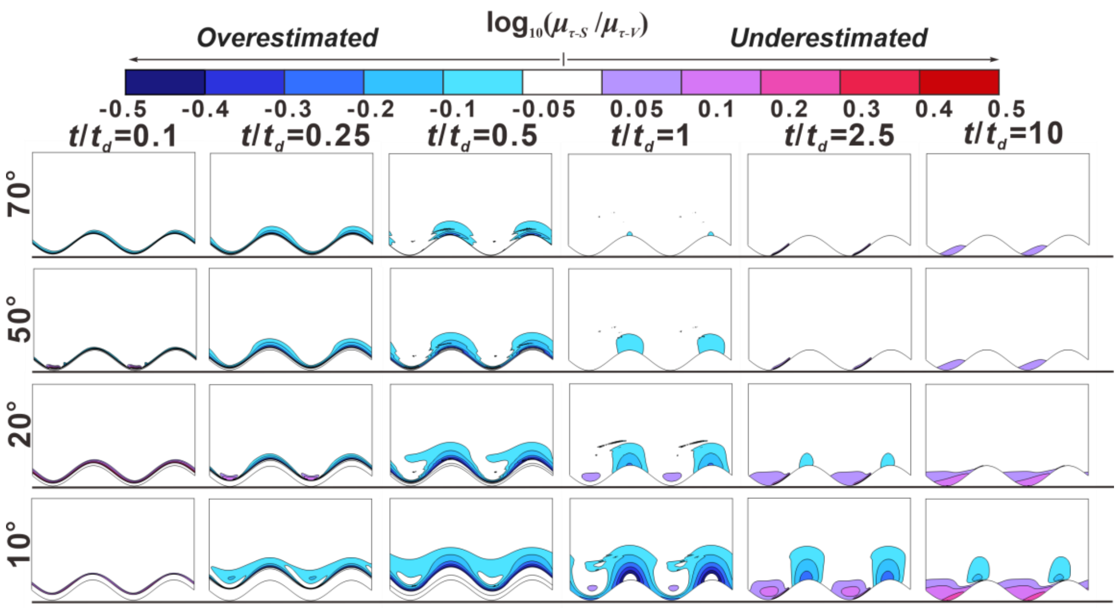


Figure 11~~0~~. Five snapshots for the RTD ratio ($\mu_r^*(\mathbf{x}, t) = \mu_{r\text{-}S}^*(\mathbf{x}, t)/\mu_{r\text{-}V}^*(\mathbf{x}, t)$) between sloping ($\mu_{r\text{-}S}^*(\mathbf{x}, t)$) and vertical riverbank conditions ($\mu_{r\text{-}V}^*(\mathbf{x}, t)$) at different times t/t_d

as a function of δ for $\Gamma_d = 10$.The horizontal lines beneath each figure are the reference lines to show the initial location of the peak point of the point bar. The lower sinuous lines at the reference lines are the initial SWIs. The colored areas indicate where the bank slopes have significant impact on RT (difference in RT between sloping and vertical model larger than 12.2%) and residence (travel) times of river water in the aquifer would be overestimated or underestimated~~Snapshots for the RTD ratio $\mu_r^*(\mathbf{x}, t)$ between sloping and vertical riverbank conditions at different times t/t_d as a function of bank slope angle δ for $\Gamma_d = 10$. The horizontal lines beneath each figure are the reference lines to show the initial location of SWIpeak point of point bar. The lower sinuous lines at the reference lines are the initial SWIs .(difference in RT between sloping and vertical models larger than 12.2%).~~

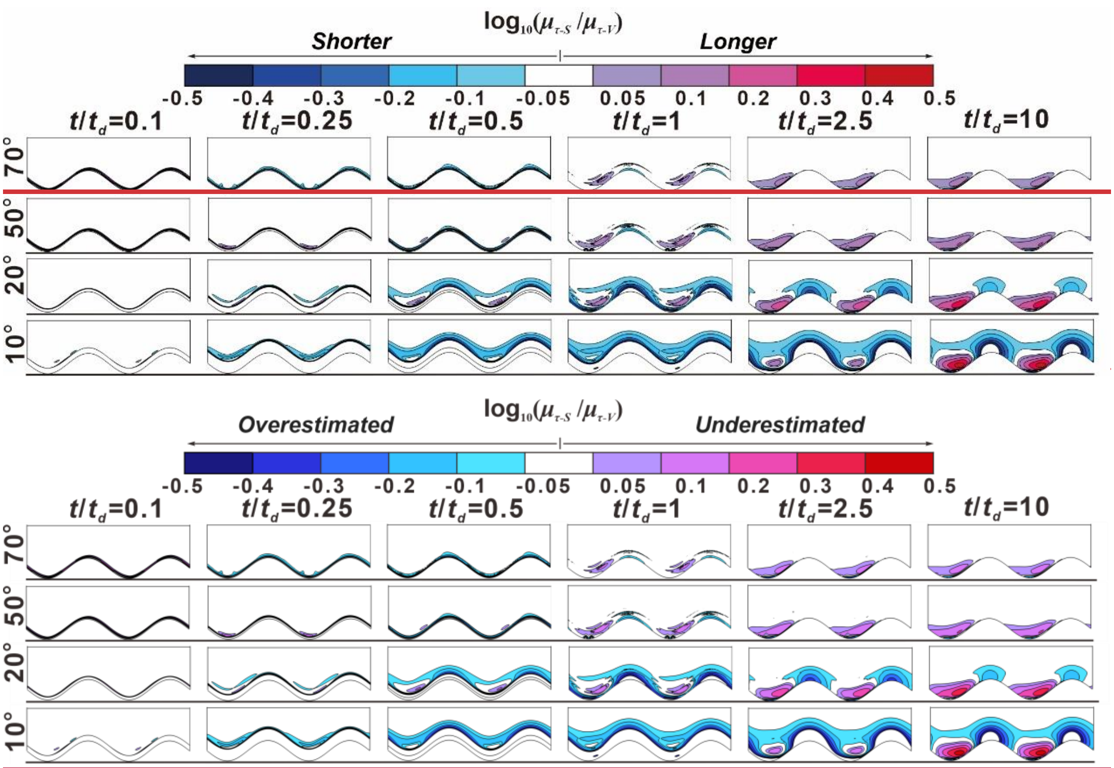


Figure 12~~1~~. Five snapshots for the RTD ratio ($\mu_r^*(\mathbf{x}, t) = \mu_{r\text{-}S}^*(\mathbf{x}, t)/ \mu_{r\text{-}V}^*(\mathbf{x}, t)$) between sloping ($\mu_{r\text{-}S}^*(\mathbf{x}, t)$) and vertical riverbank conditions ($\mu_{r\text{-}V}^*(\mathbf{x}, t)$) at different times t/t_d as a function of δ for $\Gamma_d = 100$.The horizontal lines beneath each figure are the reference lines to show the initial location of the peak point of the point bar. The lower sinuous lines at the reference lines are the initial SWIs. The colored areas indicate where the bank slopes have significant impact on RT (difference in RT between sloping and vertical model larger than 12.2%) and residence (travel) times of river water in the aquifer would be overestimated or underestimated~~Snapshots for the RTD ratio $\mu_r^*(\mathbf{x}, t)$ between sloping and vertical riverbank conditions at different times t/t_d as a function of δ~~ bank slope angle ~~for $\Gamma_d = 100$. The horizontal lines beneath each figure are the reference lines to show the initial location of~~ peak point of point bar~~SWI. The lower sinuous lines at the reference lines are the initial SWIs. (difference in RT between sloping and vertical models larger than 12.2%).~~

At $t/t_d = 0.1$, a smaller bank slope can lead to shorter ~~RT~~ travel time of river water in the aquifer (negative values of μ_r^*) near the SWI compared to the vertical riverbank ~~condition~~scenario. The area of shorter travel time ~~RT~~ caused by bank slope was positively related to aquifer transmissivity. The effect of bank slope~~δ~~ is small for Γ_d = 10 and 100 because the groundwater mound (the raised groundwater stage) piles up around the river boundary, but that small area extended deeper into the alluvial valley for smaller ~~δ~~slope angles. Due to the scattered and nested flow paths near the ~~inner bend (~~cut bank~~)~~ and ~~outer bend (~~point bar~~)~~, respectively, the ~~penetration distance of the~~area of negative value of μ_r^* ~~area~~ at the cut bank of SWI is larger than that at the point bar. The change of flow direction near the point bar leads to a prolonged flow path for the water in the river as well as to forced groundwater mixing with the slightly older water (as shown in Fig.8 that the water was more aged in y direction compared to $-x$ direction in the point bar). This effect was amplified with decreasing bank slope angle, but it is only statistically significant ($\mu_r^* < -0.05$ or $\mu_r^* > 0.05$) when $\delta = 10°$ at t/t_d = 0.1.

At the time of peak flood (t/t_d = 0.25), the river still infiltrates into the aquifer. For Γ_d = 0.1, ~~R~~results of μ_r^* in Fig. 9 show~~s~~ that bank slope can lead to both overestimated and underestimated RT area~~younger and older water, i.e., water undergoing shorter and longer RT~~. Both magnitude of relative RT (μ_r^*) and associated ~~RT~~ area increase with decreasing slope due to the longer ~~penetration~~ travel distance of river water into the aquifer. As the ~~δ~~ slope angle decreases, the underestimated travel time area~~positive values of μ_r^* are~~ w~~ere~~as located closer to the peak of the ~~downstream point bar~~cut-bank. The impact of bank slope on RTD for Γ_d = 1 ~~is~~ was rather similar in its pattern compared to Γ_d = 0.1, but ~~μ_r^* was significant only for δ = 10°~~the degree of that impact was reduced. For Γ_d = 10 and 100, only overestimated travel time area can be seen near the river bank with a smaller area of impact ~~area~~ compared to smaller Γ_d conditions, because the groundwater ~~mounds have~~has not sufficiently ~~not~~ propagated into the aquifer ~~in these low~~due to lower transmissivity ~~aquifers~~.

~~the effect of bank slope can lead to larger and deeper penetration of the river water~~

~~into the alluvial valley (Fig. 8 - 11) but this effect is smaller than when looking at smaller Γ_d because of the lower hydraulic transmissivity.~~

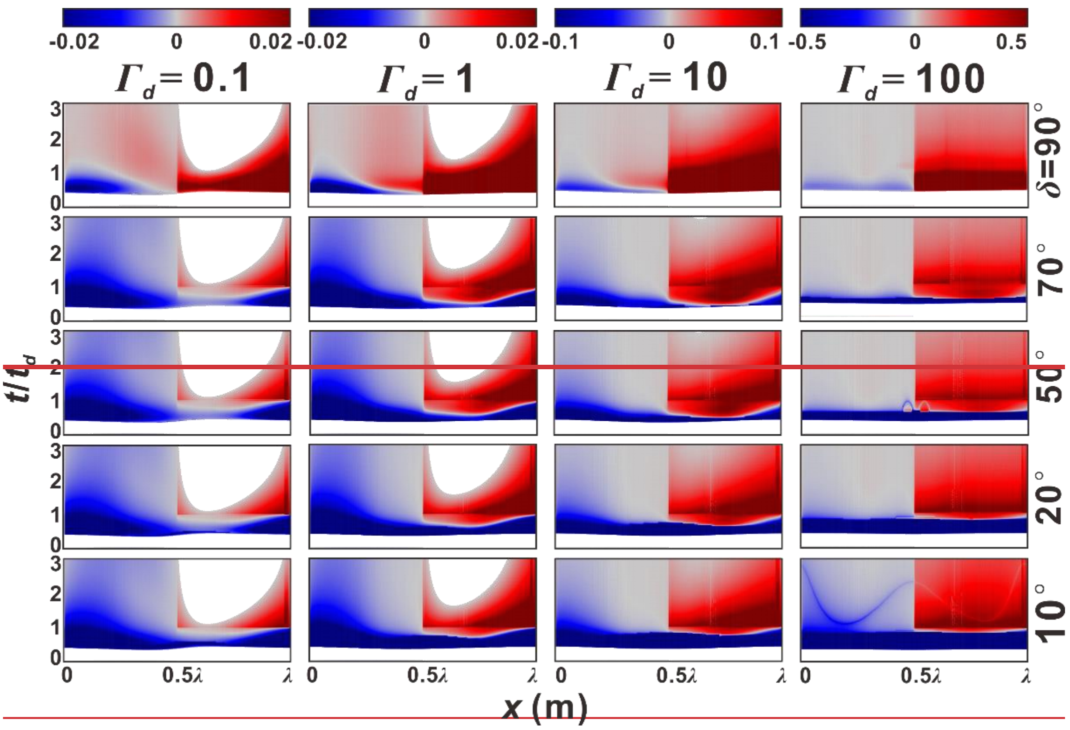
At t/t_d = 0.5, part of the ~~submerged~~ aquifer that was submerged at t/t_d = 0.25 reemerges due to the decline in river stage. In most cases, smaller bank slopes can lead to wider reemergence of the aquifer, ~~and~~ which therefore results in overestimated travel time area~~smaller μ_r^*~~ near the river boundary; however, this ~~is~~ was not the case for Γ_d = 0.1 where bank slope can both lead to overestimated and underestimated travel time area~~increase and decrease the RT of pore water~~. Furthermore, compared to when t/t_d = 0.25, the impact of bank slope becomes weaker for Γ_d = 0.1, but more relevant for the larger Γ_d values.

After the flood event (t/t_d > 1), the influence of bank slope on ~~RT~~ travel time is nearly eliminated for Γ_d = 0.1 and 1 due to the high aquifer transmissivity. However, for aquifers with lower transmissivity (Γ_d = 10 and 100), bank slope still has a significant effect on RT at t/t_d = 10 and leads to underestimated and overestimated RT area ~~older water~~ near the point bar and the cut bank, respectively~~, which indicates the bank slope has a more lasting influence on aquifer RT, as more time is required to recover to initial condition.~~

Overall, Fig. 9~~- to Fig.~~ 12 indicate that the time when bank slope was relevant in predicting RT (travel time of groundwater in aquifer) was determined by the transmissivity of aquifer. For higher transmissivity aquifer, the impact of bank slope on the prediction of groundwater travel time cannot be neglected during the flood event ($0 < t < t_d$), but that impact will be eliminated after flood event due to the quickly recovery of aquifer to the base condition. For lower transmissivity aquifer, bank slope plays an important role on groundwater travel time after the half time of flood event ($t > 0.5*t_d$) and has a more lasting influence on aquifer RT, as more time is required to recover to initial condition for lower transmissivity aquifer.

3.3 Relative flux-weighted residence time

Fig. 13~~2~~ shows the evolution of the flux-weighted relative RT $\mu^*_{out}(x, t) = \mathbf{n}\cdot Q^*_{out}(x, t)\log_{10}(\mu_\tau(x, t)/\mu_\tau(x, 0))$ for different slopes and aquifer transmissivities. $M^*_{out}(x, t)$ represents the difference in flux-weighted RT of the water discharged into the river compared to the initial condition. At the start of the flood event, there is no μ^*_{out} as river water infiltrates the aquifer. Following the decline in river stage, the aquifer begins to discharge the mixed water with different RT back into the river (see Fig. 4~~3~~c).



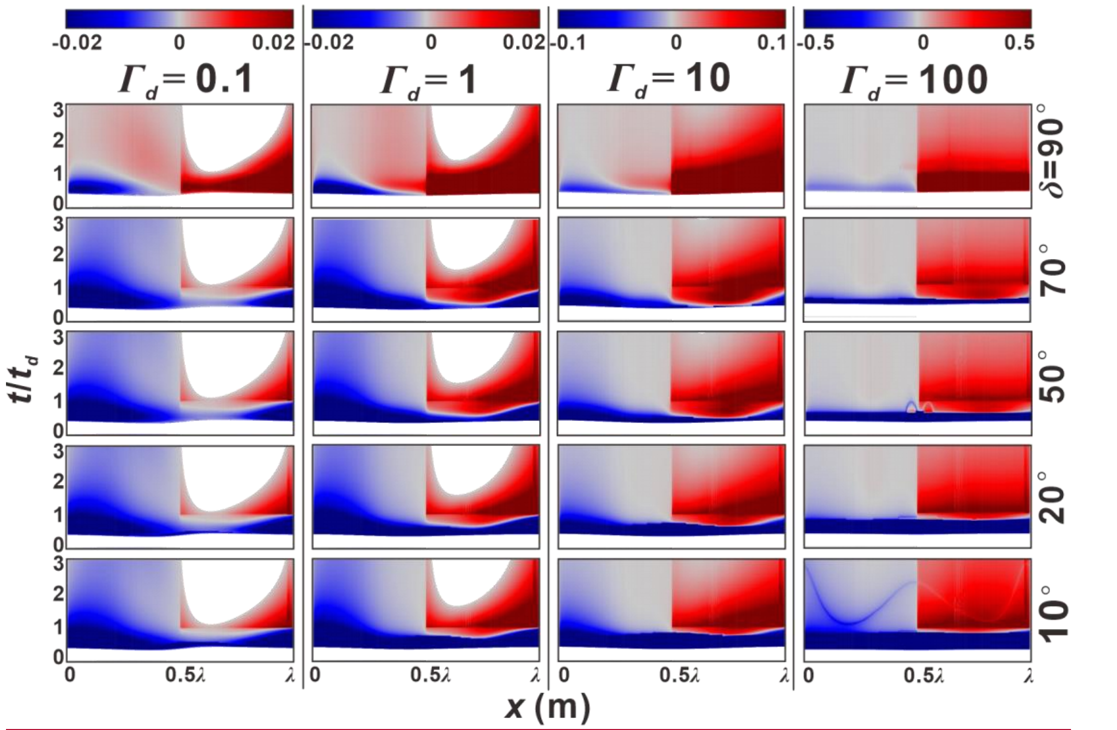


Figure 13~~2~~. Temporal evolution of flux-weighted ratios of RT to the RT for ~~base~~ ~~flow~~baseline conditions ($\mu^*_{out}(x, t) = \mathbf{n}\cdot Q^*_{out}(x, t)\log_{10}(\mu_\tau(x, t)/\mu_\tau(x, 0))$) along the river meander as a function of δ and Γ_d. $\mu^*_{out}(x, t)$ indicates the difference of flux weighted water RT (travel time) that the aquifer discharges into river compared to the initial condition.

For vertical riverbank conditions ($\delta = 90°$, top row in Fig. 13~~2~~), upstream ($0.5\lambda < x < \lambda$) and downstream ($0 < x < 0.5\lambda$) boundaries of the meander bend discharge older and younger water, respectively. The rejuvenated or aged waters ~~with relatively younger or older RT~~ that represent shorter and longer travel times compared to the baseline~~base~~ condition, respectively, ~~are~~ were mostly discharged before the flood event ($t/t_d < 1$) due to the greater outflux as shown in Fig. 3~~2~~a. It also can be seen that water ~~is~~ was ~~older~~ aged along the upstream bend compared to the more rejuvenated water along the downstream bend. After the flood event, μ^*_{out} gradually disappears along the upstream meander (blank areas) for $\Gamma_d = 0.1$ and 1, because the flow fields ~~are~~ were recovering to baseline ~~flow~~ conditions. Therefore, the upstream meander gradually becomes the inflow boundary.

For cases with lower values of Γ_d (left columns in Fig. 13~~2~~), μ^*_{out} reaches equilibrium earlier compared to cases with higher Γ_d . As δ ~~decreases from the top row to the bottom row in Fig. 132,~~ the increased impact of bank slope causes μ^*_{out} to gradually decrease the ~~RT~~ travel time of the outflowing~~ux~~ water during the flood event. For larger Γ_d, μ^*_{out} ~~is~~ was totally dominated by rejuvenated ~~younger~~ water during the flood event. Furthermore, the stronger impact of smaller bank slope angles can both extend the time over which and increase the magnitude with which younger water ~~is~~ was discharging along the downstream meander.

4. Discussion

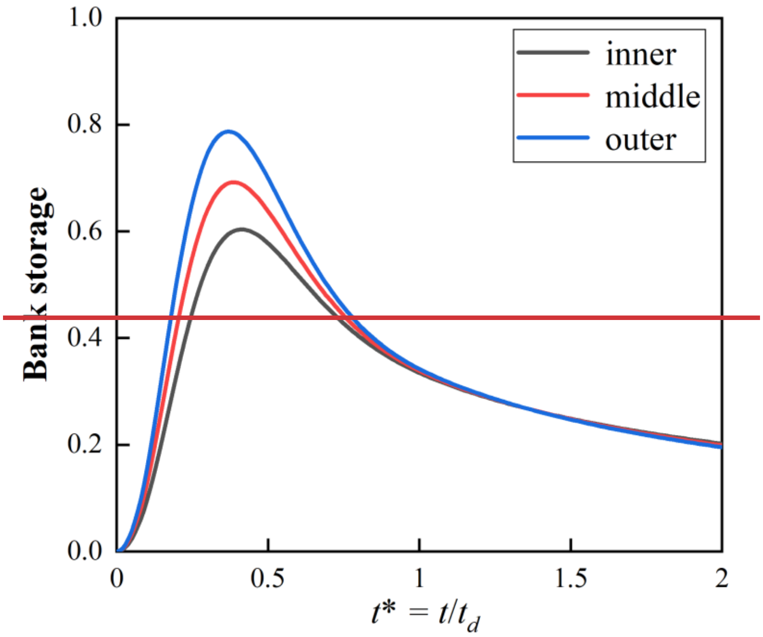
4.1~~.~~ Why we should account for bank slope

Tilted riverbanks are common in nature and caused by erosion and bank collapse, as has been observed at multiple scales (Zingg, 1940). Previous studies have shown that bank erosion is stronger where the river planimetry is more sinuous, river stage varies more frequently, or where the riverbank has larger sloping angles, ultimately leading to a flatter bank (Zingg. 1940; Hagorty et al., 1995; Mayor et al., 2008; Puttock et al., 2013). ~~Hence, recent studies have recognized the need for a comprehensive analysis of how riverbank topography affects lateral hyporheic exchange along meandering streams (Boano et al., 2014) and the specific importance of bank slope on hyporheic exchange has been highlighted by Doble et al. (2012) and Liang et al. (2018).~~ Yet, ~~in most previous studies,~~ the impact of riverbank geometry and in particular bank slope on sinuosity-driven lateral hyporheic exchange was ignored in most previous studies~~.~~ ~~Flow was usually only considered perpendicular to the river axis, i.e., HEF in river flow direction caused by the alluvial valley slope and river sinuosity was not considered~~. ~~However, as river planimetry can vary significantly along river corridors (Hooke, 2013; Seminara, 2006), and the alluvial valley slope has a potentially non-negligible impact~~

~~on hyporheic exchange (Gomez-Velez et al., 2017), we considered it important to close this knowledge gap by specifically focusing on the impact of bank slope and the ambient groundwater gradient for various groundwater flow conditions (as manifested through aquifer transmissivity) on HEF.~~ Our results clearly indicate that HZ characteristics (~~flow field~~HEF, area and penetration distance of HZ into alluvial valley) can ~~significantly~~ be underestimated ~~vary~~ along a meandering river depending on bank slope conditions.

We show that ~~N~~not accounting for bank slope and river sinuosity can lead to an underestimation of the infiltration rate of water from the river to the alluvial aquifer (~~with maximum quantity of 120%~~by up to 120%), as well as the area and penetration distance ~~of HZ~~. This effect is more pronounced for smaller bank slope angles (Fig 5), ~~and losing conditions can be significantly underestimated~~ which can be more likely found in lowland streams (Laubel et al., 2003), especially in areas with extensive cattle grazing streamside (Trimble, 1994).

~~.~~ Doble et al. (2012), Siergieiev et al. (2015) and Liang et al. (2018), assessed the influence of bank slope on HEF using a vertical cross-sectional profile. Siergieiev et al. (2015) found that the impact of bank slope on HEF was proportional to the hydraulic conductivity of the aquifer. However, we argue here that bank slope is more relevant in rivers connected to aquifers with low hydraulic transmissivity (high hydraulic conductivity or low specific yield). Furthermore, we show (Fig. 14 as example~~3~~) that using only one cross-sectional river profile perpendicular to the river axis does not capture the effect of river sinuosity on HEF as bank storage decreases from point bar to cut bank. ~~That means~~This indicates ~~the previous vertical cross-sectional profile models~~ that the accuracy of bank storage estimates can be improved ~~could not calculate~~~~will reduce the accuracy in the calculation of the bank storage evolution accurately when~~by including ~~neglecting the sinuosity of river~~ river sinuosity, which has often been omitted in the past. In a meandering river with variable bank slope, river geometry thus has a sizable effect on bank storage evolution and HEF, and should be included in any scenarios ~~into the future analytical/ numerical models~~.



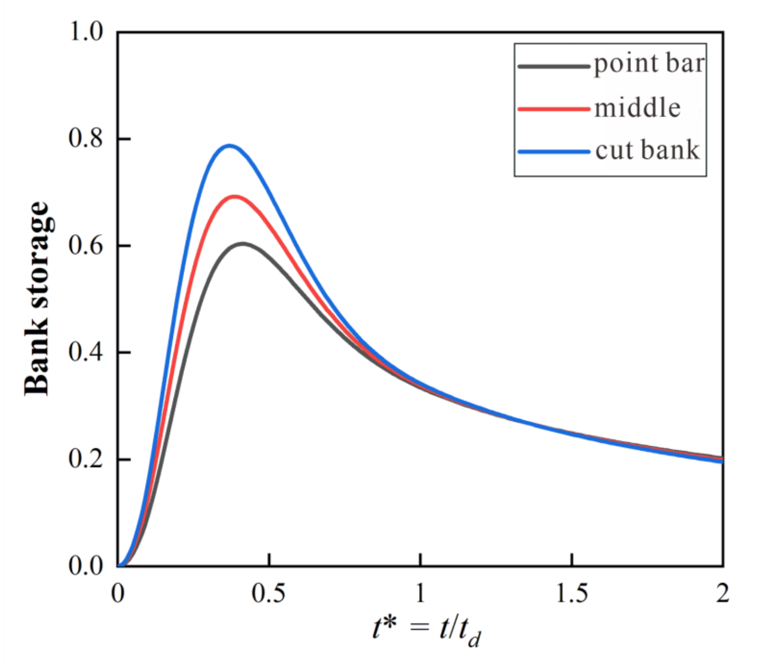


Figure 14~~3~~. Bank storage versus time for $\Gamma_d = 1$ and $\delta = 90°$ condition at: ~~inner bend~~the peak of point bar ($x = 0$); middle ~~bend~~ ($x = 0.25\lambda$); ~~outer bend~~peak of cut bank ($x = 0.25\lambda$). Dimensionless bank storage was calculated by $\frac{\int_{Y(x,\, t)}^{Y(x,\, t)+4\lambda}[h\text{-}z_b\text{-}H_0]\mathrm{d}y}{\lambda H_p}$.

~~**The impact of bank slope on RT is basically controlled by aquifer transmissivity. When aquifer transmissivity increases, the impact of bank slope appears to be more pronounced when river stage rises during a flood event. For decreasing aquifer transmissivity, bank slope seems more relevant for RTD after the flood event and its impact is more long-lasting. Bank slope could result in longer (near the point bar) or shorter (near the cut bank) pore water RT at various times of a flood event. This means that point bars with bank slopes are more conducive for river restoration (e.g., removal of dissolved organic carbon) while cut banks with bank slope may have adverse effects on the groundwater quality near rivers. This is important to keep in mind when assessing the influence of bank slope on biogeochemical efficiency. For example, previous research indicates that the residence time of river water in the HZ can control, and is often proportional to nutrient cycling (McCallum and Shanafield, 2016; Wondzell and Swanson, 1999; Zarnetske et al., 2011, 2012). As such, an analysis of RTD can provide valuable information on whether and where riverbank slope can induce biogeochemical hotspots and hot moments and help guide choices to be made in biogeochemical field surveys regarding location and sampling time under dynamic river stage conditions, especially when the connected aquifers have low hydraulic transmissivity.**~~

4.2 Implications of bank slope on biogeochemical reactions

~~The impact of bank slope on RT is basically controlled by aquifer transmissivity. When aquifer transmissivity increases, the impact of bank slope appears to be more pronounced when river stage rises during a flood event. For decreasing aquifer transmissivity, bank slope seems more relevant for RTD after the flood event and its impact is more long-lasting. Bank slope could result in longer (near the point bar) or shorter (near the cut bank) pore water RT at various times of a flood event. This means~~

~~that point bars with bank slopes are more conducive for river restoration (e.g., removal of dissolved organic carbon) while cut banks with bank slope may have adverse effects on the groundwater quality near rivers. This is important to keep in mind when assessing the influence of bank slope on biogeochemical efficiency. For example, previous research indicates that the residence time of river water in the HZ can control, and is often proportional to nutrient cycling (McCallum and Shanafield, 2016; Wondzell and Swanson, 1999; Zarnetske et al., 2011, 2012). As such, an analysis of RTD can provide valuable information on whether and where riverbank slope can induce biogeochemical hotspots and hot moments and help guide choices to be made in biogeochemical field surveys regarding location and sampling time under dynamic river stage conditions, especially when the connected aquifers have low hydraulic transmissivity.~~

~~The RTD~~Residence time distributions of river water in the alluvial aquifer ~~were widely~~have been used to evaluate the potential of biogeochemical reactions by comparing the RT with biogeochemical timescales (BTS~~s~~) for given solutes (Boano et al., 2010b; Gomez-Velez et al., 2012). L~~The l~~ocations where the ratio of RT ~~and~~to BTS is small indicate a high reaction potential for that chemical species. It has been documented that the BTS for dissolved organic matters (DOC) ~~are~~is site~~-~~-dependent and can vary over ten~~9~~ orders of magnitude (10^{-1} – 10^{9} d) (Hunter et al., 1998), while~~and~~ BTS~~s~~ for oxygen and nitrite have been found to vary over eight~~9~~ orders of magnitude (10^{-2} – 10^{6} d) (Gomez-Velez et al., 2012). Here~~Thus~~, we compare the RTD within these two BTS ranges ~~(10^{-1} – 10^{6} d) between~~for vertical and sloping~~e~~ riverbank condition (δ = 10°) at the peak time of the flood event (t/t_p = 0.25) for different aquifer transmissivity conditions, and show~~s~~ the zonation of residence times~~RT relative to the BTSs for DOC, O_2 and NO_3^-~~ by using a BTS range of~~as~~ 10^{-1} – 10^{6} d (~~, as shown in~~ Fig. 15).

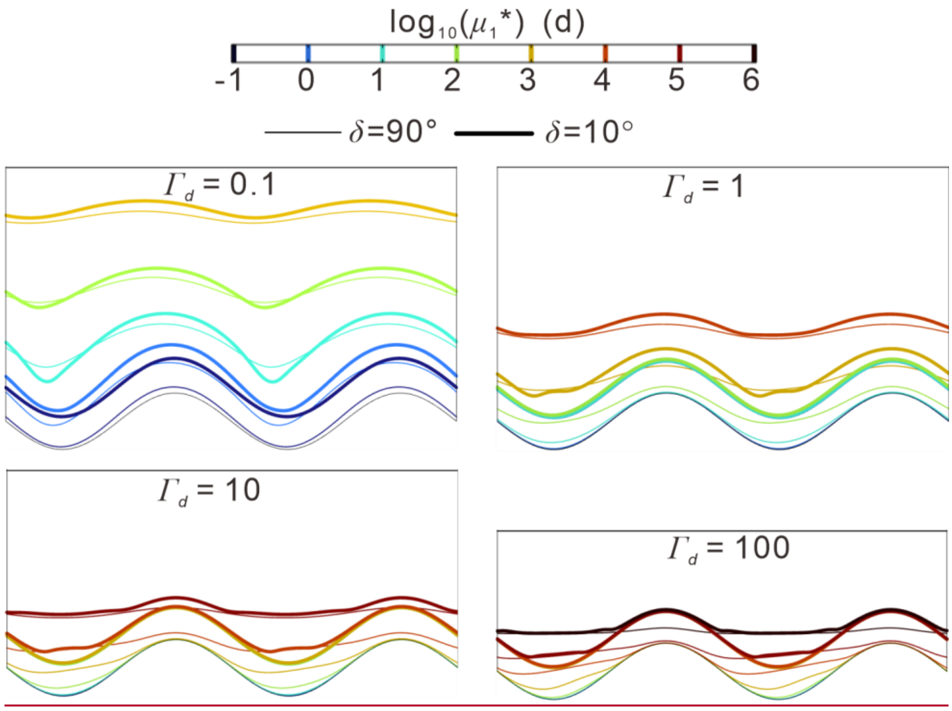


Figure 15. Zonation of biogeochemical timescales (BTS, range of 10^{-1} – 10^{6}) ~~BTSs~~ for common HZ constituents such as~~reactions of~~ DOC, oxygen or~~and~~ nitrate ~~by using BTS range as 10^{-1} – 10^{6} d~~ for different aquifer transmissivities~~y conditions~~ at t/t_p = 0.25. ~~Thin and~~ thick and thin ~~colored~~ lines indicate~~s~~ the ~~results~~comparison of vertical vs~~and~~ sloping~~e~~ riverbank (δ = 10°) conditions, ~~respectively.~~while the different colors indicate the different exponents.

Fig. 15 indicates that neglecting bank slope will impact the prediction of reaction potentials ~~of DOC, oxygen and nitrite~~ during the hyporheic exchange process, especially for ~~the site with short BTS~~locations with short time scales. For sloping bank conditions, ~~T~~the reaction hot ~~areas~~spots (areas) expanded into the aquifer~~r for sloping bank condition~~, ~~which are~~ identical ~~with~~to the overestimated areas in Fig. 9 to Fig. 12. Note ~~Recall~~ that we did not aim to include specific reaction models ~~for calculation~~in our study ~~,~~but used~~ing~~ RTD as an indicator for various biogeochemical reactions in the aquifer.

The ~~i~~Impact of bank slope on RT is basically controlled by aquifer transmissivity. When aquifer transmissivity increases, the impact of bank slope appears to be more pronounced when the river stage rises during a flood event. For decreasing aquifer transmissivity, bank slope seems more relevant for RTD after the flood event and its impact is more long-lasting. Bank slope could result in longer (near the point bar) or shorter (near the cut bank) pore water travel times ~~at various times of a~~throughout the flood event. This means that point bars with bank slopes are more ~~conducive~~ favorable for ~~river restoration (e.g.,~~ removing~~al of~~ dissolved organic carbon and for~~) and other oxidation reactions (e.g.,~~ nitrification~~)~~ while cut banks with bank slope may have adverse effects on the groundwater quality near rivers. As such, an analysis of ~~RTD~~residence time distributions can provide valuable information on whether and where riverbank slope can induce biogeochemical hotspots and hot moments and help guide choices to be made in biogeochemical field surveys regarding location and sampling time under dynamic river stage conditions, especially when the connected aquifers have low hydraulic transmissivity.

4.3~~2.~~ Advantages and limitations of using a reduced 2-D model

In this study, we propose a parsimonious reduced-order, idealized horizontal 2-D model that simplifies the variation of the river-aquifer interface by using the moving boundary method to depict the displacement of the SWI along a sloping riverbank. An advantage of this approach is reduced model complexity as compared to a three-dimensional model, which greatly reduces time and data requirements during model building and computational demand during the simulation of HEF and especially residence time distributions. Thus, our reduced-order model acts as a first step to gain insight into the patterns of hyporheic exchange, riverbank storage and RTD in settings with more complex riverbank morphology and dynamic forcing. Future efforts should be focused on optimizing the computational method applied here and on including more detailed morphology and hydrodynamic characteristics.

In ~~It is important to note that in oO~~our simulations we assume a constant ~~angle of~~

~~bank slope~~bank slope angle along the entire meandering river while natural riverbanks ~~often~~often change their slope angle from reach to reach as well as with time. This variability ~~have non-uniform slopes which~~ could lead to ~~a different behavior~~more complex SWI travel distances and residence time distributions and~~. Thus,~~ new conceptualizations that account for the contribution of bank slope on ~~time-varying~~time varying RTD and HZ extent are needed. ~~can be applied to gain better understanding of a hyporheic zone, especially in cases where bank slope is small, or where the system is relatively insensitive to changes during peak flow.~~

In our simulations we tested the model using a range of aquifer hydraulic conductivities. Although hydraulic conductivity (or transmissivity) is a critical parameter in the quantification of exchange fluxes and RTD between the two systems under varying slope conditions, other parameters such as valley water head fluctuation, water ~~drinking water~~ abstraction e.g. for agriculture or drinking water supply, peak flood event characteristics or larger scale groundwater head fluctuation, e.g., due to changing groundwater recharge ~~patterns~~in the context of changing rainfall patterns have not been considered here but might also impact HZ extent, RTD and river-aquifer exchange flux. For example, ~~the~~ valley water head fluctuation and ~~drinking~~ water abstraction in the aquifer will lead to a lower groundwater table, increasing the hydraulic gradient between river and aquifer. This will lead to the formation of ~~which makes the riparian aquifer to gain water from river more easily, and form~~ a larger ~~area of hyporheic zone~~HZ area as well as ~~a~~ longer travel distances and times of river water in the aquifer. Thus, reducing the slope of the ~~managers should consider reducing the slope of~~ river bank could~~to~~ reduce ~~prevent~~ the infiltration ~~of river pollutions into aquifer.~~of polluted river water into the riparian aquifer.

The current study assumes a perennial stream and ~~only focused on the~~ un-confined ~~aquifer~~ (phreatic ~~aquifer~~) conditions in the connected aquifer as well as changing hydraulic gradients leading to gaining and loosing conditions in the river. Where there is no hydraulic gradient between river and aquifer, no large-scale infiltration of river water into the riverbanks will occur, while local turbulent flow (e.g., due to obstacles in the river channel) might lead to localized infiltration over short distances and short time scales (Sawyer et. al., 2011; Stonedahl et al., 2013; Käser et al., 2013). Where the unconfined layer is small (e.g., in mountainous headwater streams with a rather small sediment layer overlying a hard-rock aquifer with relatively low hydraulic conductivity), the HZ is limited in its maximum extent, and travel times and distances are considerably shorter. However, in mountainous settings, slope angles are often much steeper due to erosion (here rivers incising into the bedrock) and further simulations are required to better understand the feedback between banks lope angle, hydraulic gradient and maximum extent of the unconfined layer allowing for reasonable river water infiltration. These simulations will also help us better understand the impact of bank slope on water supply and water quality to abstraction wells, e.g., used for the production of drinking water.

While the using the Boussinesq equation neglects the influence of the vadose zone, this approach as well as the assumption of vertically integrated distribution of hydraulic head have been widely used in the literature and proven adequate when simulating sinuosity-driven HEF patterns (Boano et al., 2006; 2010., Cardenas. 2008; 2009a, b; Gomez-Velez et al., 2012; 2017, Kruegler et al., 2020). While we found differences in HEF patterns when comparing simple models using the Boussinesq with those using Richard’s equation (S4 in SI) these differences exist independent of using the DGM. However, we recommend in future studies to more systematically consider these two different approaches with respect to their advantages and limitations, e.g., in terms of computability or efficiency in predicting HEF under various conditions. While in an ideal scenario a 3-D modeling approach includes vadose zone and riverbank slope angle (both variable in time and space), for the moment the implementation of such detailed models in practice suffers from limited computing capabilities.~~and did not address the confined condition. Because the elastic specific yield coefficient of groundwater in~~

~~confined aquifer is much smaller than the gravitational one in phreatic aquifer, the confined aquifer is expected to be more conductive for the propagation of hydraulic pressure than phreatic aquifer. However, the SWI will be constant both in location and length for confined aquifer, thus, we can expect that the bank slope will play non effect on the HEF as well as RTD in confined alluvial aquifer condition.~~

5. Conclusions

The deformed geometry method was applied to characterize the expansion and contraction of hyporheic zones along sloping riverbanks, and to evaluate the impact of bank slope on hyporheic exchange flux, evolution of the HZ area and residence (travel) time distributions of the infiltrating water~~RTD~~. To achieve this, several ~~various~~ unconfined alluvial aquifers with varying slope angles and aquifer transmissivity values were simulated. Our results show that bank slope in a sinuosity-driven river ~~can have significant impact~~was non-negligible when the aims of numerical/analytical models are the prediction of ~~on~~ the evolution of the hyporheic zone during and after a flood event (transient flood forcing).

The overall findings of our work underline the need for including ~~the assumptions of~~ more realistic riverbank ~~morphology~~morphological conditions into simulations when ~~focus~~studying ~~on the~~ ~~a detailed analysis of~~ lateral hyporheic exchange flow responses to dynamic forcings ~~(including the assumption of more realistic riverbank morphology conditions)~~. Furthermore, our results show that more detailed information on bank slope (e.g., through more measurements) can lead to a better understanding of hyporheic flow patterns and potentially result in improved bio~~geo~~chemical process understanding for real-world conditions ~~in~~ for more complex ~~morphology~~ morphological and depositional environments. Several conclusions can be drawn from our study:

1. Sloping riverbanks can considerably increase HEF during ~~the~~a flood event, especially when the river is connected to an ~~low transmissivity~~ alluvial aquifer with rather high hydraulic conductivity and small bank slope angles ~~are small~~as water can more easily infiltrate the connected aquifer.~~ due to the lower ability to propagates the pressure wave and longer displacement of SWI~~. Smaller bank slope angles can lead to an extended hyporheic zone with river water infiltrating deeper (penetration distance) into the aquifer. –However, bank slope has only a minor impact on the hyporheic outflow flux (water re-entering the stream).

~~2. During a flood event, the bank slope can increase the area and penetration distance of the HZ into the alluvial aquifer. This effect increases for smaller bank slope angle and is is more pronounced and long-lasting for low-transmissivity aquifers, as it need more time to eliminates the impact of bank slope.~~

~~3.~~2. During a flood event, the impact of bank slope on residence time distributions (RTD) is more pronounced for high transmissivity aquifers, due to the ~~as~~ larger area and deeper penetration distance of the HZ for~~in~~ these conditions. On the contrary, the impact of bank slope on RTD for lower transmissivity aquifers is minor during the flood event, but bank slope can have a significant and long-lasting effect ~~under~~ for post-flood conditions.

~~4.~~3. River sinuosity should be considered when assessing the impact of bank slope on RTD. Variable bank slope can lead to both longer and shorter ~~RT~~ residence times when compared to vertical riverbank conditions.

~~5.~~4. Bank slope has a greater impact on the residence time of hyporheic water in lower-transmissivity aquifers, thereby delaying the time of younger water discharge downstream of a meander bend, which also delays the outflow of older water upstream of that bend.

Code and data availability

Additional information regarding methodology and results is provided in the supporting information (SI).

Author contributions

YL: Conceptualization, Formal analysis, Methodology, Investigation, Writing
US: Conceptualization, Methodology, Writing
ZW: Funding acquisition, Software, Supervision
SK: Validation, Writing, Supervision
HL: Project administration, Supervision

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Competing interests

The authors declare that they have no conflict of interest.

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