



1 **Joint optimal operation of the South-to-North Water Diversion Project considering the evenness**
2 **of water deficit**

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7 **Abstract.** Inter-basin water transfer project is the main measure to address the water deficit crisis
8 caused by uneven distribution of water resources. The current water transfer operation mainly tends to
9 be in areas with small water transfer costs and is prone to encounter the problem spatial and temporal
10 imbalances in water allocation. To address the aforementioned issues, this paper defines a Water
11 Deficit Evenness Index (WDEI) aimed at minimizing the regional differences in water scarcity and
12 sharing the pressure of water scarcity as a social demand objective. This index is incorporated into a
13 joint optimization model for the South-to-North Water Diversion project (J-SNWDP) in Jiangsu, which
14 comprises both the ecological objective of the total water deficit (TWD) and the economic objective of
15 the pumping water (PW). Further, the NSGA-III algorithm and multi-attribute decision-making were
16 applied to solve the model and obtain an optimal operation strategy. The results showed that: 1) The
17 WDEI defined in this paper can mitigate the synchronized water scarcity in certain water users. In
18 typical normal years (wet year and dry year), the Water Deficit Evenness Index shows a reduction of
19 94.2% (81.8%, 76.7%) compared to the historical operation strategy; 2) The optimized operation
20 strategy can significantly reduce TWD and PW by 82.06% (37.69%, 52.36%) and 45.13% (3.25%,
21 21.51%) compared with the historical values, respectively, which can improve the water supply
22 satisfaction and reduce the project cost. At the same time, the lake storage capacity of the optimal
23 operation strategy performs well, and the water transfer efficiency of the river is significantly improved.
24 3) In this paper, targeted optimal operation strategies and potential ways to secure the project tasks are
25 proposed for different natural flow. Overall, it is of great significance to study the water supply equity
26 in the Jiangsu section of the South-to-North Water Diversion Project to alleviate the concentrated water
27 deficit in Jiangsu Province and other similar regions.



28 **Key Words.** South to North Water Diversion Project, China; Water deficit Evenness index; multi-
29 objective optimization; NSGA-III; Multi-attribute decision-making.

30 **1 Introduction**

31 Water demand has been increasing rapidly in recent years with economic development and
32 population growth (Dolan et al., 2021; Liu and Yang, 2012). As the demand for water increases, the
33 availability of water resources for human use continues to decline, resulting in water scarcity, increased
34 risk of flood and drought disasters, and exacerbation of the conflict between water supply and demand.
35 These social issues have become one of the key factors constraining sustainable development and
36 environmental protection worldwide. (Denaro et al., 2017; Florke et al., 2018; Jiang, 2009; Li et al.,
37 2020; Ma et al., 2020; Wang et al., 2017; Zhao et al., 2016). Inter-basin Water transfer projects have
38 been widely constructed worldwide as an effective way to address water scarcity issues caused by
39 uneven distribution of water resources and improve their utilization efficiency (Sun et al., 2021). The
40 California State Water Project, the Colorado River Aqueduct (Lopez, 2018), the Senqu-Vaal transfer in
41 South Africa and Lesotho (Gupta and van der Zaag, 2008), the Snowy Mountains Scheme in
42 southeastern Australia (Pigram, 2000), the IBWD project of the Agrestic region of Pernambuco (Cirilo
43 et al., 2021; Neto et al., 2014), and other inter-basin water transfer projects have all effectively
44 alleviated water scarcity issues in various regions. China is home to approximately 18% of the global
45 population. However, the country's water resources account for only around 6% of the world's total.
46 This imbalance between population and water resources presents China with significant water resource
47 challenges. As a result, inter-basin water transfer projects have been more extensively constructed in
48 China, such as the well-known South-to North Water Diversion Project (Guo and Li, 2012), Yunnan
49 Central Water Diversion Project (Xiang et al., 2022), and so on. At least 10 % of the cities worldwide
50 receive water from IBWD projects (McDonald et al., 2014). Specifically, with an estimated investment
51 of around 78 billion USD, the South-to North Water Diversion Project (SNWDP) is regarded as the
52 largest inter-basin water transfer project in the world. The project runs along numerous water users, and
53 the water resources it provides have already benefited hundreds of millions of people, with even more
54 expected to be served in the future (Pohlner, 2016).

55 With the ongoing emergence of issues such as environmental pollution and degradation, global



56 climate change, and population growth, the problem of water scarcity has become increasingly
57 prevalent worldwide. Hence, effectively operating inter-basin water transfer projects and enhancing the
58 dispatching benefits is a challenging task. Currently, most IBWD projects primarily follow various
59 laws, regulations, policy guidelines, and historical experience in dispatching strategies set by the
60 government. However, there is a lack of detailed operating rules for different natural scenarios. Leading
61 to an imbalance in water supply across regions and putting some water users at high risk of water
62 scarcity. Addressing the aforementioned issues, there are considerable studies on the water resources
63 operating strategy of the supply-oriented IBWD projects in terms of social, economic, ecological, and
64 environmental (Gan et al., 2011; Liu and Zheng, 2002; Xu et al., 2013; Zhu et al., 2014). In general,
65 meeting the water demand of various users is the main task of the IBWD project, with the
66 consideration of minimizing water deficit in previous studies (Guo et al., 2020; Wang et al., 2008).
67 Rather than the total amount of water deficit, the crux of the problem may actually be the concentration
68 of water deficit in a certain period of time or region, which has not yet received sufficient attention and
69 remains a major challenge. Therefore, both the total and spatial-temporal distribution of water deficit
70 should be considered in the optimization process. (Xu et al., 2013). In addition, users' demands and
71 decision makers' benefits should be considered as priorities (Zhang et al., 2012), so minimizing
72 pumped water (PW) is a direct way to reduce costs. At the same time, the proportion of the amount of
73 abandoned water and the water withdrawn from the river in the process of water diversion should also
74 be considered as secondary considerations. In order to solve the above problems and define reasonable
75 objectives, existing studies are mainly carried out from water supply index and cost index. Liu and
76 Zheng define the ratio of regional water consumption to water availability as the water pressure to
77 reflect water supply reliability (Liu and Zheng, 2002). Guo et al., consider social benefits, maximum
78 power generation and environmental flow satisfaction, into account (Guo et al., 2018). In addition,
79 Ouyang and Iop set the minimum water power loss as a target to support reservoir operation in terms of
80 energy conservation (Ouyang and Iop, 2018). However, due to the data on natural water and user water
81 demand as the determining factors of the operation strategy, and the obvious regional differences, most
82 of the objectives determined by the existing studies can only solve small-scale projects, otherwise it
83 would lead to failure. Xi et al., used the rainfall forecast information from the Global Forecast System
84 (GFS) and calculated user water demand by ration, and found that the resulting operation strategy
85 couldn't be effectively compared with the historical operation strategy, because it is impractical to



86 apply these objectives to guide operation (Xi et al., 2010).

87 The China's South-to-North Water Diversion Project (SNWD), as the world's largest inter-basin
88 water transfer project, has provided $30.6 \cdot 10^8 \text{m}^3$ of water to the Hai River Basin since its official
89 operation in 2013. This has significantly alleviated the water supply deficit of large- and medium-sized
90 cities along the Beijing-Tianjin-Hebei-Henan route, accounting for 70 % and 90 % of the domestic
91 water in Beijing and Tianjin, respectively, demonstrating remarkable benefits (Liu et al., 2013). In
92 recent years, experts and scholars have extensively discussed the impact of the SNWDP on ecological
93 environment (Hu et al., 2022), changes in groundwater storage in the North China Plain (Zhang et al.,
94 2021), project benefits (Yang et al., 2021), and water quality (Wang et al., 2016), among other issues.
95 However, as the project continues to operate, it is necessary to shift the focus to dispatch management
96 in order to enhance the sustainability of the project. The SNWDP connects China's four major river
97 basins: the Yangtze River, Yellow River, Huai River, and Hai River, involving multiple provinces such
98 as Shandong, Jiangsu, and Anhui, and presenting a highly complex and dynamic water situation,
99 especially in the Jiangsu section (Vogel et al., 2015). The project utilizes regulating reservoirs, sluice
100 stations, and pump stations to connect numerous water users along the route, supplying water to
101 various water-consuming sectors from both sides of the canal. However, due to differences in the
102 location and timing of natural inflows and water users, an imbalance in water supply has arisen. The
103 actual operation of the water supply plan is usually implemented under the guidance of the government
104 authorities, with reference to historical dispatch strategies. This approach may lack objectivity and
105 accuracy, potentially leading to inefficiencies in water resource utilization and the ineffectiveness of
106 operation strategies (Chen et al., 2019; Peng et al., 2015; Sheng et al., 2020). At present, there have
107 been some studies attempting to address this issue, but they tend to focus on meeting the total water
108 demand and improving the overall benefits (Li et al., 2017; Zhuan et al., 2016), neglecting the fairness
109 of water supply among different regions. As a result, water supply may become concentrated on a
110 specific user or time period. Therefore, it is of great theoretical significance and practical application
111 value to establish a scientific and systematic optimal operation model, to quantitatively analyze the
112 water resources allocation in users, to reflect the sophisticated water diversion process to guarantee
113 water supply, and to give full play to the comprehensive benefits of the IBWD project (Nazemi and
114 Wheeler, 2015).

115 To address the above problem, this paper studies the Jiangsu section of South-to-North Water



116 Diversion project (J-SNWDP). The three main contributions of this paper are as follows: 1) defines the
117 Water Deficit Evenness Index (WDEI), and incorporates it into the optimization model together with
118 the Total Water Deficit (TWD) and the Pumped Water (PW), to meet the requirements of both decision-
119 makers and users; 2) incorporates the amount of abandoned water and the water withdrawn from the
120 Yangtze River into the decision indicator set, and uses the multi-attribute decision making method to
121 filter the Pareto front strategies of NSGA-III and finds the optimal operation strategy that balances the
122 economic and ecological benefits; 3) the paper compares the optimal operation strategy selected in
123 three typical years (wet year, normal year, and dry year) with the historical operation strategy under the
124 same natural conditions to verify the superiority of the optimization results, and puts forward
125 reasonable optimization suggestions for the SNWD project.

126 The paper is structured as follows: Section 2 presents the study area; Section 3 presents materials
127 and methods; Section 4 presents the results and discussion; Section 5 draws conclusions.

128 **2 Materials and methods**

129 **2.1 Study area**

130 **2.1.1 Regional Overview**

131 The Jiangsu section of the South-to-North Water Diversion Project (J-SNWDP) crosses the
132 Yangtze River and Huai River basins. The simplified map of the J-SNWDP is shown in Fig. 1. The
133 Beijing-Hangzhou Grand Canal runs through the north and south of Jiangsu Province, connecting the
134 Yangtze River basin, the Huai River basin and the Yishusi River basin. The total length of the Jiangsu
135 section is 404 km, along which six cities are involved, namely Yangzhou, Huaian, Yancheng, Suqian,
136 Lianyungang and Xuzhou. The J-SNWDP consists of 3 impounded lakes (Hongze Lake, Luoma Lake,
137 Nansi Lake), 6 sluices (e.g., Erhe sluice, Gaoliangjian sluice, etc.), and 13 pumping stations (Huaian
138 Station, Jiangu Station, etc.), forming a double-route water diversion system including the West Canal
139 and the East Canal Route. The water supply scope of the whole J-SNWDP covers three provinces of
140 Jiangsu, Anhui, and Shandong, supplementing the agricultural, industrial and domestic water supply as
141 well as navigation and ecological water supply in the areas along the water transfer route. The scale
142 parameters of the pumping stations and sluices in J-SNWDP are listed in Table 1. The characteristics of
143 the lakes in the J-SNWDP are shown in Table 2.



144 **Table 1 Pumping stations and sluices of the Jiangsu section of the South-to-North Water Diversion Project**

	Number	name	Scale (m ³ /s)	Number	name	Scale (m ³ /s)
Pumping station	P1	Jiangdu	100	P8	Sihong	120
	P2	Baoying	400	P9	Suining	110
	P3	Jinhu	400	P10	Zaohe	175
	P4	Huaian	300	P11	Pizhou	110
	P5	Hongze	150	P12	Liushan	125
	P6	Huaiyin	300	P13	Taierzhuang	125
	P7	Siyang	230			
Sluice	S1	Nanyunxi	400	S4	Yangzhuang	500
	S2	Gaoliangjian	500	S5	Yanhe	500
	S3	Erhe	500	S6	Huaiyin	500

145 **Table 2 Lakes of the Jiangsu section of the South-to-North Water Diversion Project**

Lake name		Hongze	Luoma	Nansi
Dead lake level (m)		11.30	21.00	31.30
Normal lake level (m)	Flood season	12.50	22.50	32.30
	Non-Flood season	13.50	23.00	32.80
Regulation storage (10 ⁸ m ³)	Flood season	15.30	4.30	4.94
	Non-Flood season	31.35	5.90	8.00
Monthly range lake level for water diversion (m)	Jul - Aug	12.00	22.22-22.10	31.80
	Sep - Oct	12.00-11.90	22.10-22.20	31.50-31.90
	Nov - Mar	12.00-12.50	22.10-23.00	31.90-32.80
	Apr - Jun	12.50-12.00	23.00-22.50	32.30-31.80

146 When the J-SNWDP is supplying water along the water transfer route, users closer to the water
 147 source (such as JBHD User, S-S User, LM User, etc.) are generally prioritized. On the other hand, users
 148 like the Feihuanghe User, Lianyungang User, and Siyang-Zaohe User, which are located farther from
 149 the water source and receive less water from Luoma Lake, often require water to be pumped from the
 150 Yangtze River and Hongze Lake for replenishment. Combined with the uneven spatial and temporal
 151 distribution of precipitation, these recipient zones are more susceptible to water shortages.

152 In Fig. 2, the complex relationship of the J-SNWDP is generalized into a schematic diagram
 153 according to the geographical location. In order to represent the main components of the system and the
 154 connection between the backbone rivers, we regard lakes, pumping stations, sluices as the nodes and

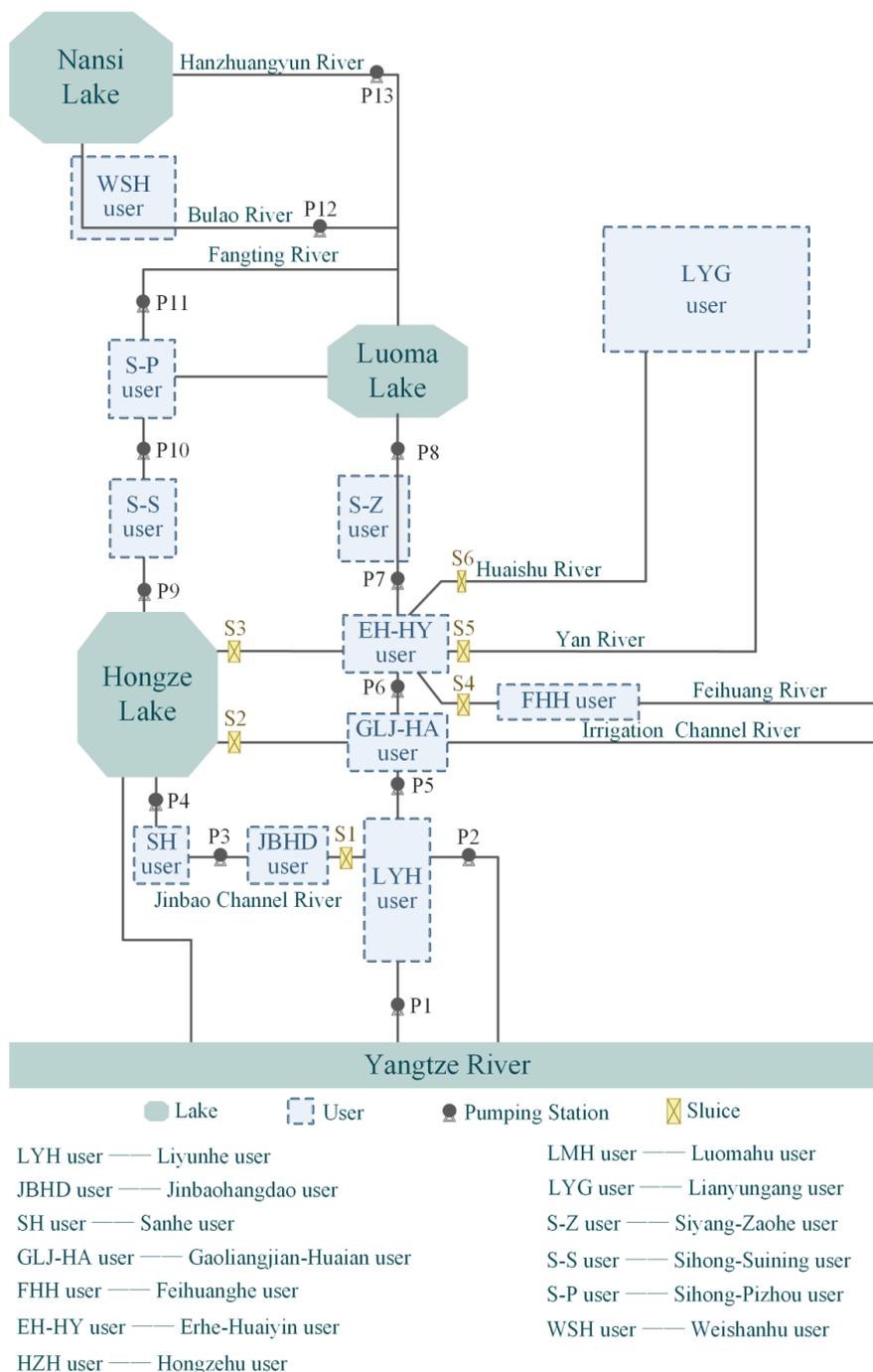


155 the backbone river as the connecting line, while the water users are reasonably distributed among the
156 nodes of the water transmission route.

157 The joint optimal operation process of the J-SNWDP is described as follows. In addition to natural
158 precipitation and lake inflow, the Yangtze River is the main water source of water, which is pumped
159 from the river to the West Route and the Canal Route by P1 pumping station (see Fig. 2). The West
160 Route is pumped step by step from P2, P3, and P4 to Hongze Lake through Sanyang River, and Jinbao
161 Channel, and pumped from P9, P10, and P11 to the north through Xuhong and Fangting Rivers. The
162 Canal Route along the Beijing-Hangzhou Grand Canal route is pumped north from P5, P6, P7, and P8.
163 The two water transmission routes supplement the water demand of users along the routes while
164 pumping to the north, and merge into one line at the intersection of Fangting River and Zhongyun River.
165 It is then pumped further north until P12 and P13 are transported to Nansi Lake via Bulao and
166 Hanzhuangyun Rivers, respectively, thus delivering water outside the Jiangsu province. When the
167 natural inflow is high, the Hongze Lake can be discharged via P1 on the West Route, or through S2, S3,
168 S4, S5, and S6 to the relevant rivers and users. Luoma and Nansi Lakes are discharged through the
169 original water transmission routes.



170
171 **Fig. 1. The Jiangsu section of the South-to-North Water Diversion Project. The orange, green and purple**
172 **lines represent the Canal Route, the West Route, and intersection of the two routes to transport water**
173 **outside the province, respectively.**



174

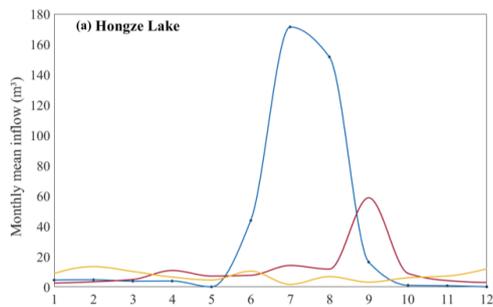
175 **Fig. 2. Schematic diagram of the Jiangsu section of the South-to-North Water Diversion Project.**



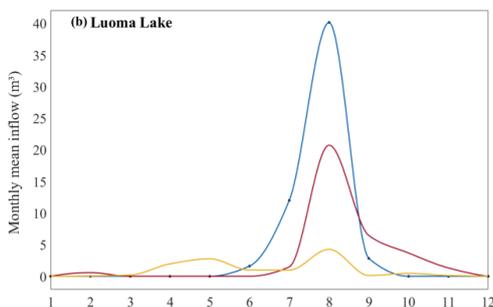
176 **2.1.2 Data**

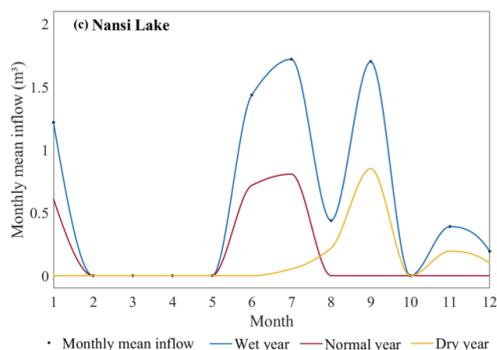
177 The joint optimal operating model of the J-SNWDP uses a monthly time step. According to the
178 annual natural inflow data of Hongze (HZ), Luoma (LM) and Nansi (NS) Lake since 2013 from
179 Jiangsu Water Resources Bulletin, three hydrological years were selected to represent wet (2017.10-
180 2018.09, annual mean inflow: $465.12 \times 10^8 \text{ m}^3$), normal (2019.10-2020.09, annual mean inflow: 172.80
181 $\times 10^8 \text{ m}^3$), and dry (2013.10-2014.09, annual mean inflow: $103.06 \times 10^8 \text{ m}^3$) years, respectively. The
182 typical annual runoff curves of HZ, LM and NS Lakes are shown in Fig. 3. The water supply area of
183 the J-SNWDP is divided into 13 receiving areas according to geographical location, and water demand
184 data is provided by the Jiangsu Provincial Water Resources Department: wet years (2017.10-2018.09,
185 water demand: $171.29 \times 10^8 \text{ m}^3$), normal years (2019.10-2020.09, water demand: $173.51 \times 10^8 \text{ m}^3$),
186 normal years (2013.10-2014.09, water demand: $181.75 \times 10^8 \text{ m}^3$).

187



188





189
 190 **Fig. 3. Monthly mean inflow curves of the (a) Hongze, (b) Luoma, (c) Nansi lakes in typical wet, normal, and**
 191 **dry years.**

192 **2.2 Defining water deficit evenness index**

193 For IBWT projects, issues of equitable water supply often arise, especially for projects like the J-
 194 SNWDP, which has 13 water users along its route. The issue is particularly severe here. Different users
 195 have significant spatial variations in location, as well as large differences in water supply costs. Luoma
 196 Lake, which almost receives no inflow during dry periods yet bears the primary responsibility for out-
 197 of-province water supply task, requires multilevel pumping stations to pump water upward (water
 198 needs to be pumped from Luoma Lake through two pumping stations, from Hongze Lake through six
 199 pumping stations, and from the Yangtze River through nine pumping stations), creating a substantial
 200 burden on water supply. In past water supply dispatching of the J-SNWDP, the pursuit of maximum
 201 benefit while neglecting this issue would obviously cause certain users to bear severe water shortage
 202 risks, resulting in significant damage to the water supply system. Therefore, considering the fairness of
 203 water supply is very important for the practical operation of the project. Hence, the WDEI (Water
 204 Deficit Evenness Index) is defined as an indicator representing the degree of water deficit
 205 concentration, and the variance can reflect the difference in water shortages among various recipient
 206 zones. By incorporating the WDEI index into the optimization objective and minimizing it, the
 207 difference in water shortage can be reduced as much as possible. So the WDEI is below:

208

$$WDEI = \min \frac{\sum_{i=1}^n (QR(i,t) - \frac{\sum_{i=1}^J QS(i,t)}{J})^2}{P}, \quad (1)$$



209 2.3 The joint optimal operation model of J-SNWDP

210 Herein, the joint optimal operating model of the J-SNWDP was constructed with 228 decision
211 variables (13 pumping stations and 6 sluices). The objective function and associated constraints are
212 formulated as follows.

213 2.3.1 Objectives

214 (1) Minimizing the total water deficit (TWD)

215 TWD is a measure of how well the operation strategy completion is being implemented. This
216 objective aims to minimize the total amount of water deficit at the end of a given operation period,
217 potentially improving the satisfaction of water demand for users and increasing the operation strategy
218 completion.

$$219 \quad TWD = \min \sum_{t=1}^T \sum_{i=1}^n QR(i, t), \quad (2)$$

220 where $QR(i, t)$ is the water deficit of the i th user at time step t , $i = 1, 2, \dots, n$, with n being the total
221 number of users, $t = 1, 2, \dots, T$, with T being the whole operating period.

222 (2) Minimizing water deficit evenness index (WDEI)

223 WDEI indicates the degree of concentration of the water deficit and can be used as an indicator of
224 the uniformity of water diversion. The lower the WDEI, the better the strategy is.

$$225 \quad WDEI = \min \frac{\sum_{t=1}^n (QR(i, t) - \frac{\sum_{i=1}^n QR(i, t)}{n})^2}{J} \quad (3)$$

226 where P is the total number of pumping stations ($P = 13$ in this study).

227 (3) Minimizing pumped water (PW)

228 PW reflects the economy of operation strategy. The lower the PW is, the less the operating costs.

$$229 \quad PW = \min \sum_{t=1}^T \sum_{p=1}^P QS(p, t), \quad (4)$$

230 where $QS(j, t)$ is the water pumped by the p th pumping station at time step t , $p = 1, 2, \dots, P$.

231 2.3.2 Constraints

232 Systems operation should obey operating rules and physical constraints, such as water balance,
233 pumping capacity, and lake storage constraints. The mathematical expressions of the constraints are
234 shown as below.



235 (1) Water balance constraint

236 The water balance constraint should be satisfied in the water diversion process.
237

$$V(i, t+1) = V(i, t) + Q(i, t) + DI(i, t) + PC(i+1, t) - DO(i, t) - W_l(i, t) - PR(i, t), \quad (5)$$

238 At time step t , where $Q(i, t)$ is the inflow of the i th lake; $W_l(i, t)$ is the water demand of the i th lake
239 (water to be supplemented by SNWD project after deducting the locally available water); $DO(i, t)$ is
240 water diversion to the north from the i th lake; $DI(i, t)$ is the water pumped into the i th lake; $PC(i, t)$ is
241 the water discharged into the i th lake; $PR(i, t)$ is the water discharged from the i th lake.

242 (2) Pumping capacity constraint

$$\begin{aligned} 243 \quad & 0 \leq DO(i, t) \leq DO_{\max}(i, t) \\ & 0 \leq DI(i, t) \leq DI_{\max}(i, t) \end{aligned}, \quad (6)$$

244 At time step t , where $DO_{\max}(i, t)$ is the maximum pumping capacity that is pumped into the i th lake;
245 $DI_{\max}(i, t)$ is the maximum pumping capacity that is diverted north from the i th lake.

246 (3) Sluice capacity constraint

$$247 \quad 0 \leq PR(i, t) \leq PR_{\max}(i, t), \quad (7)$$

248 where $PR_{\max}(i, t)$ is the maximum sluice capacity at time step t .

249 (4) Lake storage constraint

$$250 \quad V_{\min}(i, t) \leq V(i, t) \leq V_{\max}(i, t), \quad (8)$$

251 where $V_{\min}(i, t)$ and $V_{\max}(i, t)$ are the minimum and maximum water storage capacities at time step t ,
252 respectively. When $V(i, t) < V_{\min}(i, t)$ is water deficit, $QR(i, t) = V_{\min}(i, t) - V(i, t)$, ensure that
253 the lake level is above the limit level; when $V(i, t) > V_{\max}(i, t)$ is abandoned water, ensure that the
254 water storage of the lakes is within a reasonable range.

255 (5) Minimum lake levels for water diversion

$$256 \quad Z_{\min}(i, t) \leq Z(i, t) \leq Z_{\max}(i, t), \quad (9)$$

257 where $Z_{\min}(i, t)$ and $Z_{\max}(i, t)$ are the minimum and maximum level of the i th lake at time step t ,
258 respectively.

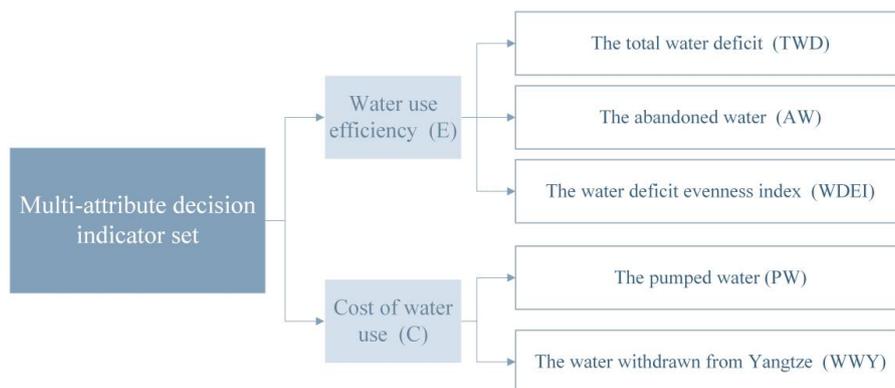
259 2.4 Model solving and solution selection

260 Considering the complex objectives and various physical constraints, the NSGA-III algorithm



261 (Deb and Jain, 2014) is used in this paper to solve the model in this paper. NSGA-III has been widely
262 used to solve various water-resource optimal diversion problems and to obtain optimal operation
263 strategies with the advantages of fast execution speed and high efficiency (Ni et al., 2019; Tang et al.,
264 2021; Zhou et al., 2020). The NSGA-III is used to solve the joint optimal operation model of the J-
265 SNWDP under three typical conditions: wet year, normal year and dry year. The model takes the actual
266 value of the pumped water of each pumping station as the decision variable. The population size,
267 generation, crossover rate and mutation rate are set to 200, 20000, 0.9 and 0.1, respectively.

268 After obtaining the set of operation strategies using NSGA-III, this paper further applies multi-
269 attribute decision-making methods to screen and determine the optimal operation strategy. The
270 abandoned water reflects the regulation and storage capacity of the lakes, and the water withdrawn
271 from the Yangtze River reflects the impact of the water transferred outside the system on the operation
272 strategy. A multi-attribute decision indicator set is constructed, which includes the above two indicators
273 and the three optimization objectives of the model. Indicators are divided into water use efficiency and
274 cost of water use indicators as shown in Fig. 4. In this paper, we adopt the Analytic Hierarchy Process
275 (AHP) method to determine the subjective weights and the entropy weighting method to determine the
276 objective weights, further the combination weights are obtained by linear weighted average.



277
278 **Fig. 4. Determining the set of indicators for selecting optimal operation strategy in typical wet, normal, and**
279 **dry years.**

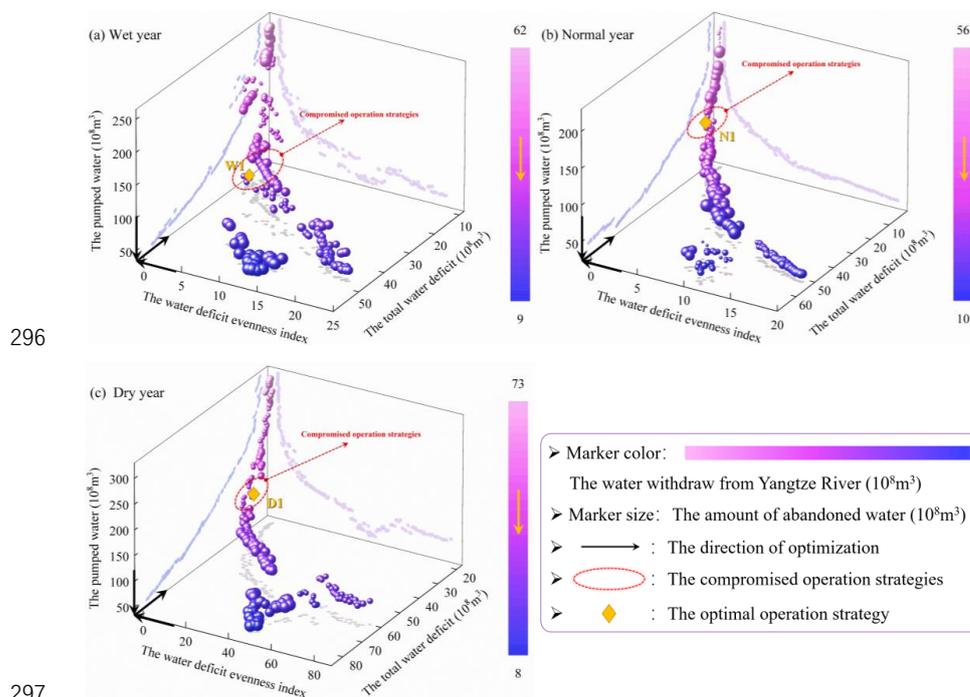
280 3 Results

281 3.1 Pareto front strategies of NSGA-III under different years

282 The historical operation strategy of the J-SNWDP follows the principle of 'replenishing the lake



283 during water shortage, and discharging during water surplus', with the primary goal of ensuring the
 284 completion of water supply tasks. Firstly, the simulation of a single lake is carried out, and then the
 285 simulation of each storage lake is carried out according to the top-down order, so as to complete the
 286 conventional operation of the engineering system. Such a method is simply "robbing Peter to pay Paul".
 287 The external water transfer is not fully deployed, but increases the operating costs. In contrast, the joint
 288 optimal operation exhibits several advantages, such as controlling the minimum WDEI of each user,
 289 ensuring that even if the total water shortage and the historical operating strategy are the same, the
 290 water shortage pressure is evenly distributed to avoid undesirable water shortage concentration. In
 291 addition, the optimal operation process pays more attention to the water storage function of the lake. In
 292 the non-flood season, the water level of the reservoir is kept at the normal water storage level as much
 293 as possible, and the water storage (discharge) is appropriate according to different natural inflow, so
 294 that the whole system can realize the lake storage in the flood season and non-flood season at the same
 295 time, and improve the utilization rate of water resources while fulfilling the water supply task.



297
 298 **Fig. 5. Illustration of Pareto front strategies for (a) wet, (b) normal, and (c) dry years. (W1, N1 and D1**
 299 **represent the location of the optimal operation strategy in different typical years.)**

300 Fig. 5 provides visualizations of the optimal operation strategy based on Pareto sets in different



301 years. Obviously, the compromised operation strategies are well distributed in three typical years, and
 302 the compromised operation strategies and the optimal operation strategy are located in the middle of
 303 the curve. The relative relationship between the optimization objectives has been analyzed, the total
 304 water deficit has a positive correlation with the water deficit evenness index. In contrast, the total water
 305 deficit and water deficit evenness index have a negative correlation with the pumped water, indicating
 306 that the water deficit gradually decreases with the increase of PW. The distribution pattern of the
 307 marker is formed under the interaction of three objectives (see Fig. 5).

308 Fig. 5 (a) shows the competitive relationships between TWD, WDEI and PW in wet years that PW
 309 decreases sharply and WDEI increases slowly at the beginning when TWD increases by $20 \times 10^8 \text{m}^3$,
 310 WDEI increases by about $3 \times 10^8 \text{m}^3$, and PW decreases by about $164 \times 10^8 \text{m}^3$, but when TWD exceeds
 311 $31.33 \times 10^8 \text{m}^3$, WDEI increases sharply and PW decreases slowly with TWD increasing by $20 \times 10^8 \text{m}^3$,
 312 WDEI increasing by about $9 \times 10^8 \text{m}^3$, and PW decreasing by about $51 \times 10^8 \text{m}^3$. The above relationships
 313 obey the law of diminishing marginal utility. As a part of PW, the water withdrawn from the Yangtze
 314 River represented by the marker color needs to be appropriately pumped according to the natural flow
 315 and water demand of users. Generally, the proportion of water withdrawn from the Yangtze in PW
 316 cannot be too high, affecting the lake storage capacity and pumping (operating) cost. Therefore, the
 317 middle part or the lighter marker color tend to represent better results in the figure. Meanwhile, to
 318 reduce the waste of water resources, the amount of abandoned water represented by the marker size
 319 should be reduced as much as possible. Therefore, the yellow marker point is obtained as the optimal
 320 operation strategy to compare with the historical operation strategy by multi-attribute decision-making.
 321 The weights of decision indicators and the results of the comparison are shown in Table 3 and Table 4.
 322 Similarly, the Pareto front strategies in Fig. 5 (b) and (c) also follow the above rules, but due to the
 323 difference in natural inflow, the specific data are also quite different (see Table 4).

324 **Table 3 Determining the weights of indicators for selecting optimal operation strategy in typical wet, normal,**
 325 **and dry years.**

Evaluation indicators	Wet year	Normal year	Dry year
The total water deficit	0.353	0.366	0.402
The abandoned water	0.194	0.203	0.264
The water deficit evenness index	0.207	0.248	0.215
The pumped water	0.134	0.101	0.074



The water withdrawn from Yangtze 0.111 0.080 0.045

3.2 Comparison with historical operation strategy

In order to verify the rationality of the strategies obtained by the joint optimal operation model, this section compares the optimal operation strategy selected in different typical years with the historical operation strategy.

Table 4 Comparison of the main operation performance indicators of the historical and optimal operation strategy in typical years. (units: 108 m³)

Typical year	Scenario	The total water deficit	The water deficit evenness index	The pumped water	The amount of abandoned water	The water withdrawn from Yangtze
Wet	Historical	65.77	22.87	119.27	374.56	25.74
	Optimal	31.33	4.16	93.62	310.91	23.65
	Decrement	52.36%	81.82%	21.51%	16.99%	8.12%
Normal	Historical	75.19	26.18	184.06	56.53	42.07
	Optimal	13.49	1.51	100.99	25.17	30.08
	Decrement	82.06%	94.24%	45.13%	55.47%	28.50%
Dry	Historical	63.22	21.54	159.88	13.60	26.62
	Optimal	39.39	5.01	154.68	0.05	37.34
	Decrement	37.69%	76.72%	3.25%	99.63%	-40.27%

Table 4 shows the optimal historical operation strategy in typical years. Compared with the optimal operation strategy, the TWD and WDEI of the historical operation strategy are very high, which means that the water deficit concentration in the historical operation strategy leads to the inability to reduce the TWD. Meanwhile, the PW and abandoned water are slightly large, indicating that the historical operation strategy has more disadvantages on economic and ecological benefits. WDEI can directly reflect the difference of water deficit among users. WDEI has sound optimization effects, of which the reduction in the optimal operation strategy was 94.2% (81.8 %, 76.7%) of the historical values in the typical normal year (wet year and dry year). The optimized operation strategy can significantly reduce TWD and PW by 82.06% (37.69%, 52.36%) and 45.13% (3.25%, 21.51%) compared with the historical values, respectively, while maintaining a very low WDEI. It has been demonstrated to be an excellent strategy for inter-basin water diversion, which can make multiple users share the risk of water deficit and alleviate the problem of water deficit concentration. The reduction of



344 the abandoned water and the water withdrawn from the Yangtze River represents an increase in the lake
 345 storage capacity. The amount of abandoned water of the operation strategy is greatly reduced, and the
 346 amount of abandoned water is only $0.051 \times 10^8 \text{m}^3$ in the dry year. This indicates that the optimal
 347 operation can maximize the utilization efficiency of the limited water resources. However, the of
 348 amount of water withdrawn from the Yangtze River increased by $10.72 \times 10^8 \text{m}^3$ after optimization in dry
 349 years, indicating that the proportion of the water withdrawn from Yangtze River in PW was raised in
 350 the optimal operation strategy to ensure uniform distribution of water and higher economic benefits.

351 The comparison of water deficit in water users between the optimal operation strategy and the
 352 historical operation strategy in three typical years is illustrated in Table 5, which more intuitively
 353 reflects the optimization effect of WDEI. The ‘Variance’ and ‘Decrement’ represent the difference
 354 between the maximum and minimum values and the percentage reduction, respectively. The study
 355 found that after optimization, the variance of the users was reduced by 62.93 % (78.07 %, 54.09 %) in
 356 different typical years (i.e., bold font in Table 5). FHH user, S-Z user and HZH user, which have large
 357 water deficit in the historical operation strategy, decrease by $14.63 (14.41, 14.69) \times 10^8 \text{m}^3$, $5.61 (9.33,$
 358 $1.77) \times 10^8 \text{m}^3$ and $0.98 (7.98, 5.26) \times 10^8 \text{m}^3$ of the optimal operation strategy, respectively (see Table 5).
 359 The possible reason for the large water deficit of users is that they are far away from the main lakes and
 360 rivers, or there are many users on the water transmission route that need to be supplied. Users jointly
 361 share the water deficit risk in the optimal operation strategy in the case of little natural flow. The
 362 difference between the maximum and minimum water deficit is controlled within $8 \times 10^8 \text{m}^3$, where the
 363 difference in the normal year is only $3.13 \times 10^8 \text{m}^3$, and the problem of water deficit concentrated in a
 364 particular user is alleviated. For example, in the dry year, the water deficit of users under optimal
 365 operation strategy ranges from 0 to $7.64 \times 10^8 \text{m}^3$. Although the water deficit of GLH-HA user, LMH
 366 user, and WSH user is slightly higher after optimization, the total water deficit is much lower than the
 367 historical value and isn't concentrated in FHH user, HZH user, and S-Z user, compared with the results
 368 of historical operation. Overall, the optimal operation strategy is more reasonable, and may accord with
 369 the aspirations of both the government and the general public.

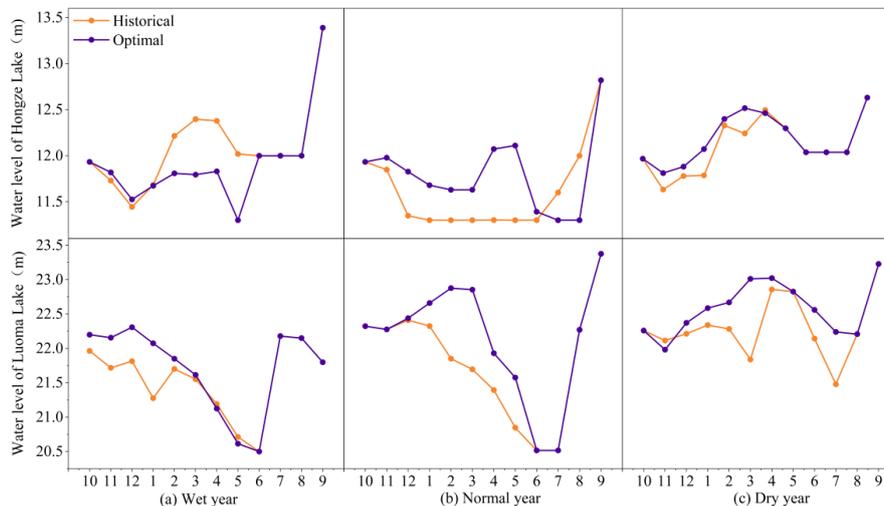
370 **Table 5 Comparison of water deficit in water users for (a) wet, (b) normal, and (c) dry years.**

Typical years	Wet year		Normal year		Dry year	
Users	Historical	Optimal	Historical	Optimal	Historical	Optimal



LYH user	5.04	0.00	10.94	0.00	2.78	2.68
JBHD user	0.52	0.00	0.00	0.00	0.00	0.00
SH user	1.97	0.92	0.00	0.00	0.52	0.52
GLJ-HA user	3.98	1.78	11.28	1.91	2.69	3.40
HZH user	7.07	6.19	8.35	0.38	8.66	6.70
FHH user	17.22	2.59	14.41	0.00	16.64	1.95
LYG user	7.44	2.15	1.68	0.00	6.10	3.82
EH-HY user	3.11	3.09	1.73	0.50	6.06	4.55
S-Z user	11.28	5.67	12.46	3.13	9.41	7.64
S-S user	1.02	0.32	1.39	0.49	1.34	0.74
S-P user	3.62	1.35	4.15	1.55	5.11	3.08
LMH user	0.58	0.12	2.69	2.37	1.35	2.53
WSH user	2.91	3.21	6.10	3.16	2.55	3.64
Variance	22.87	4.16	26.18	1.51	21.54	5.01
Decrement	81.82%		94.24%		76.72%	

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Fig. 6. The operation water level of the Hongze and Luoma lake for (a) wet, (b) normal, and (c) dry years under historical (orange line) and optimal (purple line) operation.

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Fig. 6 shows the water level variations of HZ Lake and LM Lake (two major storage lakes in Jiangsu Province). The water level of HZ Lake and LM Lake is slightly higher than the historical value after optimization for most of the time. Both lakes show a regular trend of change: the lake remains at a high level during the dry season, releases water before the flood season, then remains at a low level throughout the flood season for storing flood, and returns to a high level at the end of the flood season. Therefore, the water transmission system can simultaneously implement lake water storage in the flood



381 and non-flood seasons to ensure the fulfilment of water supply tasks and improve the utilization
 382 efficiency of water resources.

383 The optimized water level of HZ Lake was significantly lower than the historical value from
 384 February to June in Fig. 6 (a). This is due to the large amount of natural precipitation with uneven
 385 spatial-temporal distribution in the wet year, and the discharge time and discharge volume of HZ Lake
 386 are unreasonable in historical operating strategy, resulting in a large amount of water deficit and
 387 abandonment. Therefore, in the optimized operation strategy, a certain amount of lake water is
 388 discharged during the impoundment period for water supply. Similarly, LM Lake level in April-June
 389 below the historical level. In the Fig. 6 (b) and (c), the optimized HZ Lake level is significantly higher
 390 than the historical value before the flood season, while the flood season is decreased dramatically,
 391 indicating that the lake stores a certain amount of water while supplying water during the impoundment
 392 period, and discharges water before the flood season to ensure that the flood season is at a safe level.
 393 Similarly, for LM Lake, the water level increased after optimization in typical years, and tended to be
 394 consistent after the flood season.

395 Compared with the historical operation strategy, the water level of HZ Lake and LM Lake is
 396 slightly higher than the historical value after optimization for most of the time. Combined with the
 397 results of Table 4 and Table 5, the coordinated water diversion between LM Lake and HZ Lake
 398 increased obviously while reducing TWD and PW.

399 **Table 6 The water distribution of different routes between two adjacent lakes.**

Inflow/ outflow Lake	Inflow/ outflow Route	Multi-year average	
		Historical	Optimal
Outflow LM Lake	The ratio of inflow Hanzhuangyun and Bulao River	19: 8	1: 1
Inflow LM lake	The ratio of outflow Zhongyun and XuhongRiver	7: 9	7: 1
Outflow HZ Lake	The ratio of inflow Xuhong and Zhongyun River	1: 6	1: 14
Inflow HZ Lake	The ratio of outflow Liyun River and Jinbao channel	19: 1	25: 1
Water withdrawn from Yangtze River	The ratio of inflow Jinbao channel and Liyun River	8: 9	2: 9

400 This paper considers that the water distribution of different routes between two adjacent lakes is



401 an important factor affecting the water deficit in the users. The water transmission capacity of the route
402 is mainly affected by the water demand of users and the pumping capacity of the pumping station along
403 the way. Table 6 shows the distribution of different water transmission routes between neighboring
404 lakes in typical years. The ratio of water pumped from LM Lake into Hanzhuangyun River and Bulao
405 River is decreased from 19: 8 to 1: 1, which is convenient for simultaneous double-route water supply
406 outside the province and reduces the water supply pressure of Hanzhuangyun River. The ratio of water
407 from the Zhongyun River and the Xuhong River to the LM Lake increased from 7: 9 to about 7: 1,
408 gradually shifting the focus of the water transmission route to the Zhongyun River. The reason is that
409 Hongze Lake transports water to the Zhongyun River through the Erhe and Gaoliangjian Sluice without
410 pumping, which can reduce the pumped water and save project cost. In addition, there are many users
411 in Zhongyun River, the water demand is higher. When the water supply is sufficient, the drainage
412 through Huaiyin, Yanhe and Yangzhuang Sluice significantly reducing the water deficit of FHH and
413 LYG users (see Table 5). Similarly, the ratio of water from HZ Lake to Zhongyun River and Xuhong
414 River increased from 6:1 to 14:1. The ratio of pumping water from Liyun River and Jinbao Waterway
415 to HZ Lake also increased from 19:1 to 25:1, indicating that the water transport efficiency of Zhongyun
416 River is higher, and the operation strategy should be based on water transfer from the central canal,
417 with the western route as the auxiliary support route. Similarly, the ratio of pumping water from the
418 Yangtze River to the Liyun River and the Jinbao Channel has also increased (see Table 6).

419 Evidence from this research suggests that the water extracted from Hongze Lake is much greater
420 than that from other lakes, indicating that Hongze Lake is the main source of water to support water
421 supply and flood control within the system. Therefore, it is important to understand the water storage
422 period. It is important to ensure that the water level of each storage lake reaches the water level at the
423 end of the flood season to complete the water allocation in the non-flood season. If the reservoir water
424 storage is insufficient, the Yangtze River will be pumped in time to ensure the normal operation of the
425 entire water diversion project. The Yangtze River is the main source of water outside the J-SNWDP
426 system. The proportion of pumping water from the Yangtze River in the pumping water volume of the
427 system plays an important regulatory role. The actual operation should follow the following rules:
428 When the natural inflow is less, mainly through pumping the Yangtze River to complete the task of
429 water supply; when the natural inflow is large, the water withdrawn from Yangtze River is reduced, and
430 the focus of water allocation is shifted to the mutual replenishment between lakes.



431 **4 Discussion**

432 The joint optimal operating model is operated based on social demand (water deficit) and
433 economic (pumped water) objectives, focusing on the issue of water deficit concentration. Herein, the
434 limited available water is used to minimize the total water deficit of the system and water deficit
435 differences between users and applies to inter-basin water transfer projects with complex systems and a
436 large number of water users. The multi-attribute decision implemented in this study incorporates
437 ecological (the abandoned water) and the water withdrawn from Yangtze River into the multi-attribute
438 decision indicator set, which can provide optimal operation strategy with preferred weights for decision
439 makers who have different preferences.

440 From the historical operation strategy, it can be seen that the water deficit doesn't only occur under
441 less natural inflow conditions, but also there are still serious problems of water deficit and
442 abandonment in wet years. The main reasons for the coexistence of water deficit and water
443 abandonment are lakes' limited water storage capacity and the uneven spatial-temporal distribution of
444 natural inflow. Water deficit and the pumped water are greatly affected by natural inflow; we should
445 not expect to find a general operating strategy optimal in all natural conditions. This paper performs
446 well in water resources allocation and utilization on a monthly time step of three typical years (wet year,
447 normal year and water shortage year), which is representative and universal, and provides a useful
448 guide for IBWD under future uncertainty. Therefore, in addition to implementing the optimal operation
449 strategy, the flood control limit water level should be appropriately increased according to the natural
450 inflow to improve the lake's storage capacity in the flood season. This is a potential way to effectively
451 protect the water diversion function of the project.

452 **5 Conclusions**

453 As the largest inter-basin water transfer project in the world, the South-to-North Water Transfer
454 Project, scientifically operating decisions are important for improving the water allocation balance and
455 reducing the stress of concentrated water deficits.

456 1) From the perspectives of social demand, economy and ecology, this paper establishes a joint optimal
457 operation model for the Jiangsu section of the South-to-North Water Diversion Project (J-
458 SNWDP), and further uses a combination of NSGA-III algorithm and multi-attribute decision-



459 making for strategy preference, which has a certain persuasion. This method has a good
460 performance in solving the complex water transfer problems with multiple objectives and
461 engineering units, and is currently less applied.

462 2) After incorporating the Water Deficit Evenness Index into the joint optimal operation model, the
463 concentration of water deficit is reduced by 94.2% (81.8 %, 76.7%) in typical wet years (normal
464 year and dry year) compared with the historical strategy, which greatly ameliorated the
465 engineering problem of user water deficit concentration. The other two indicators of the model,
466 total water deficit (TWD) and pumping water (PW), were reduced by 82.1% (37.7%, 52.4%) and
467 45.1% (3.2%, 21.5%), respectively, with excellent performance.

468 3) After optimization, the storage capacity of the lakes is enhanced, and the water allocation between
469 different water transmission routes is more balanced, which improves water utilization and water
470 supply efficiency. It puts forward the potential ways to effectively guarantee the water diversion
471 task, providing the scientific basis and operating suggestions for the J-SNWDP.

472



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476 **Data availability**

477 Data will be made available on request.

478 **Author contribution**

479 Bingyi Zhou: Writing- Original Draft, Validation, Formal analysis, Software, Methodology,
480 Conceptualization, Visualization, Term Definition

481 Guohua Fang: Writing- Review and Editing, Funding acquisition, Resources, Supervision, Project
482 administration

483 Xin Li: Resources, Writing- Review and Editing, Visualization, Investigation.

484 Jian Zhou: Writing- Review and Editing, Supervision, Formal analysis.

485 Huayu Zhong: Resources, Data Curation, Visualization, Investigation.

486 **Competing interests**

487 The authors declare that they have no known competing financial interests or personal
488 relationships that could have appeared to influence the work reported in this paper.

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