Hydrological response of a peri-urban catchment exploiting conventional and unconventional rainfall observations: the case study of Lambro catchment

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Abstract. Commercial Microwave Links (CMLs) can be used as opportunistic and unconventional rainfall sensors by converting the received signal level into path-averaged rainfall intensity. Since in meteorology and hydrology the reliable reconstruction of the ~~rainfall spatial distribution~~ spatial distribution of rainfall is still a challenging issue, there is a wide-spread interest in integrating the precipitation estimates gathered by the ubiquitous CMLs with the conventional rainfall sensors, i.e. rain gauges (RGs) and weather radars. Here we investigate the potential of a dense CML network ~~,~~ for the estimation of river discharges via a semi-distributed hydrological model. The analysis is conducted ~~on Lambro,~~ in a peri-urban catchment, Lambro, located in northern Italy and covered by 50 links. A two-level comparison is made between CML- and RG-based outcomes, relying on 12 storm/flood events. First, rainfall data are spatially interpolated and assessed in a set of significant points of the catchment area. Rainfall depth values obtained from CMLs are definitively comparable with direct RG measurements, except for the spells of persistent light rain, ~~due to~~ probably due to the limited sensitivity of CMLs caused by the coarse quantization step of raw power data. Moreover, it is ~~showed~~ shown that, when changing the type of rainfall input, a new calibration of model parameters is required. In fact, after the re-calibration of model parameters, CML-driven ~~outputs~~ model performances are comparable with RG-driven ones, confirming that the exploitation of a CML network may ~~lead to benefit in hydrological modelling~~ be a great support to hydrological modelling in those areas lacking of a well designed and dense traditional monitoring system.

1 Introduction

Precipitation is the main downward forcing of the water cycle (Kidd and Huffman, 2011) and consequently one of the most relevant inputs in hydrological models, which are key tools in early warning systems for flood risk forecasting and mitigation (EU Water Directors, 2003). However, precipitation exhibits a significant temporal and spatial variation over a catchment area or region (Dawdy and Bergmann, 1969; Bengtsson, 2011; Parkes et al., 2013) and this is a critical aspect leading to difficulties in reconstructing a reliable rainfall field. In the past, several studies investigated the effects of spatio-temporal variability of rainfall on the hydrological model outputs (e.g., Obled et al., 1994; Bárdossy and Das, 2008; Younger et al., 2009; Arnaud

et al., 2011) proving that precipitation inputs have a marked influence on the simulated outflow hydrographs. It is also known that the reconstruction of rainfall input is more accurate as the number of rainfall measurements increases over a study area (Chen et al., 2010; Xu et al., 2013)~~, even if there are some~~. However, because of economic or geographical ~~circumstances that~~
25 ~~prevent the presence of~~ factors, an adequate density of rainfall sensors is often not ensured.

Currently, the most common ground-based technology for rainfall measurement is the rain gauge (RG), which provides single-point measurements (New et al., 2001). In addition, high-precision ground sensors, namely ~~the~~ disdrometers, provide size and velocity of hydrometeors (Jaffrain et al., 2011; Cugerone and De Michele, 2015). One of the major problems encountered when dealing with single-point measurements is to transfer the information ~~in~~ to ungauged sites or to reconstruct the
30 rainfall field over the catchment of interest. Such estimates can be performed by the use of spatial interpolation techniques. Several methods are now available, with different degrees of complexity. They can be either deterministic (e.g., the inverse distance weighting (IDW) method (Shepard, 1968) and the Thiessen polygon method (Thiessen, 1911)) or stochastic (e.g., the Kriging technique (Delhomme, 1978) and co-Kriging (Myers, 1984)). However, the outcome of these techniques is proved to be highly sensitive to the gauge density (Xie et al., 1996)~~.~~, depending on the temporal resolution. Specifically, the shorter is the
35 aggregation time, the more critical is the rain gauge density. Alternatively, the rainfall field at ground level can be indirectly obtained by weather radars, when available. The radar retrieves the average rainfall intensity across a volume from measurements of reflectivity through power-law formulas as the one proposed by Marshall and Palmer (1948)~~, see also Raghavan (2013)~~ ~~for a review~~. A recent survey of reflectivity-rainfall intensity formulas is in Raghavan (2013). Ignaccolo and De Michele (2020) and Jameson and Kostinski (2002), have argued about the purely statistical nature of the reflectivity-rainfall intensity
40 formulas, with important consequences about ~~the use of these formulas~~ their use where calibration with local data is missing. There are in addition other drawbacks associated ~~to~~ with the use of radar reflectivity, including the problem of spurious echoes, as ground clutter (Alberoni et al., 2001; Rauber and Nesbitt, 2018), which restrict the use of radars to plain areas and the fact that the radar reflectivity provides only information about precipitable water. Recently, the dual-polarization upgrade on radars (Zhang et al., 2019; Chen et al., 2021) has added information about shape, composition, and phase of the hydrometeors.
45 Hence, the quantitative precipitation estimation (QPE) could greatly benefit from such advancements.

~~Indeed~~For all these reasons, measuring the spatial distribution of rainfall is still an open issue, which may be tackled through the integration of ~~the~~ conventional sensors, and/or the complement of new instruments. In this context, the use of opportunistic rainfall sensors~~is rising a great interest nowadays. Among these, there are the~~, such as Commercial Microwave Links (CMLs), ~~that~~ has raised considerable interest. CMLs are the point-to-point radio links connecting the base stations of a mobile network
50 to the core infrastructure. The use of ~~a mesh of~~ microwave links as opportunistic rainfall detectors was ~~first proposed by~~ ~~Giuli et al. (1991) through simulations; their~~ firstly proposed by Atlas and Ulbrich (1977). The method exploits the relationship between the rainfall intensity and the attenuation (i.e., the loss of signal power) experienced by the electromagnetic wave along the propagation path from the transmitter to the receiver. Later, Giuli et al. (1991) made use of a mesh of microwave links for the 2D reconstruction of the rainfall field, through simulation. A pioneering experimental campaign was carried out
55 during the Mantissa project (Rahimi et al., 2003). However, at that time, the need to install ad-hoc microwave links made the technique impractical. A few years later, the scenario changed following the ~~boom in~~ dramatic expansion of cellular telephony.

The use of the ubiquitous CMLs connecting ~~cellular network base stations~~ the base stations of cellular networks was first proposed by Messer et al. (2006). Their paper triggered many studies that were conducted worldwide to investigate the ~~CMLs' potential~~ potential of CMLs for meteorological and hydrological applications. From a hydrological point of view, CML-based rainfall products were firstly exploited by Fencl et al. (2013) to improve urban drainage modelling in a small scale (2.33 km^2) impervious catchment, in Prague (Czech Republic). Later, Brauer et al. (2016) investigated the effects of the use of CML data in discharge simulations, for a natural low land catchment in the Netherlands, at small scale (6.5 km^2). A further study by Smiatek et al. (2017) used microwave links derived precipitation estimates as rainfall input in a distributed hydrological model applied to the Ammer basin (Germany), ~~at a larger scale (~~with an area of 609 km^2~~). In this~~. In that work, authors employed the IDW method for interpolation of RG and CML rainfall data on a 100 × 100 m grid. Another case study was carried out on an agglomeration of cities (~~16km^2) in~~ 16 km^2) in the Czech Republic by Stransky et al. (2018), to check the potential of a dense CML network for urban drainage management. Pastorek et al. (2019) ~~also~~ assessed the impact of ~~both CML sensitivity to rainfall and CML position~~ CML quantitative precipitation estimates (QPEs) on urban drainage modelling. ~~Authors found out that the bias propagated thoughout simulationsis inversely proportional to CML length.~~ The authors found that the sensitivity of CMLs is the factor which mostly affects the QPEs and that the bias on QPEs propagates throughout rainfall-runoff simulations. Moreover, they showed that the position of CMLs over the drainage area impacts the reconstruction of the runoff dynamic. In Italy, Roversi et al. (2020) conducted a ~~meteorological analysis~~ validation of the CML rainfall estimates in the Po valley ~~in Emilia Romagna~~ (northern Italy) ~~to assess the accuracy of CML retrieved rainfall rates using data purchased from Vodafone~~ by comparing them with different data sources (RGs, the ERG5 meteorological data set, and radar products). However, still no one carried out ~~an~~ a hydrological application of CML-based rainfall estimates.

Here, the analysis aims at investigating and validating the ~~operational~~ potential of a CML network~~in a semi-distributed hydrological model in~~, located in Lombardia (northern Italy), exploited for hydrological purposes. Specifically, ~~we figured out relying on a semi-distributed hydrological model, we assessed~~ whether rainfall data ~~from~~ collected by a large CML network ~~made up~~ of 50 links may be ~~exploited~~ used to provide a reliable reconstruction of the hydrological process in a medium-sized basin and if it is comparable with those achieved with a well designed RG network. ~~To this aim, we~~ We investigated a set of summer and autumn precipitation events (both convective and stratiform), that occurred over the Lambro catchment during the years 2019 and 2020. The analysis of events taking place in different seasons allowed us to point out some ~~limits~~ limitations of CMLs in detecting specific types of precipitation. In this work we firstly focused on the spatial interpolation of rainfall observations comparing results from conventional (RGs) and unconventional (CMLs) instruments and their combined use. In fact, the issue of spatial interpolation is crucial when dealing with point (RGs) or linear ~~measurements~~ (CMLs) measurements used as input into a semi-distributed hydrological ~~models~~ model, especially when the study area is quite large (in the order of 100 km^2 or even larger). ~~Differently from Stransky et al. (2018), we modified~~ We relied on the traditional IDW method to ~~account for CML length in the spatial interpolation. Specifically, in addition to the inverse distance weight, we defined a further weight, which exponentially decreases with CML length.~~ spatially interpolate precipitation measurements/estimates from RGs/CMLs. Secondly, we implemented a semi-distributed rainfall-runoff model using three types of inputs: (1) RG

measurements, (2) CML estimates and (3) the combination of RG and CML data. ~~We~~ Given the different nature of rainfall inputs we also assessed three calibrations of model parameters. We then compared results in terms of river discharge.

The remainder of this paper is structured as follows. In Sec. 2 we present the case study, the ~~features of~~ experimental setup, and the features of the networks of conventional and unconventional sensors~~networks and the experimental setup~~. Section 3 includes a description of all methods implemented for the analysis and Sec. 4 reports results including a comparison of rainfall spatial interpolation carried out with the different data types and of stream flow simulations against hydrometric measurements. Discussion and conclusions are ~~given~~ in Sec. 5 and 6, respectively.
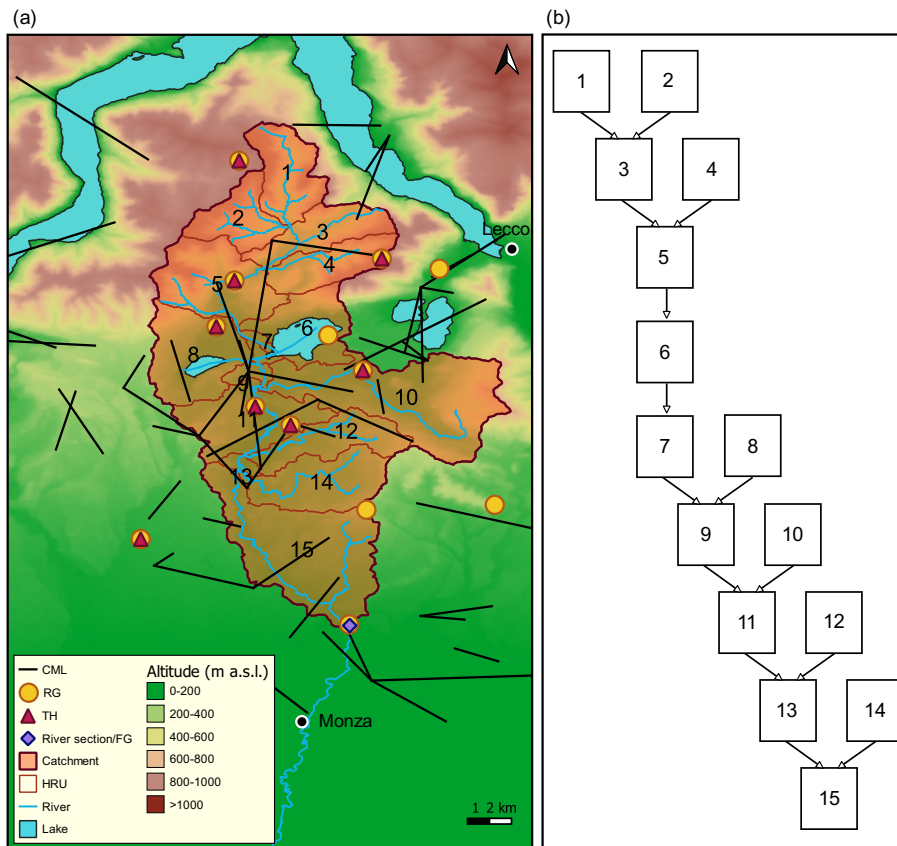
Figure 1. Case study area. Panel a shows the Lambro catchment, the partitioning in 15 sub-basins (HRUs), and the position of the sensors, while panel b reports the scheme of HRU interaction in the network. The digital terrain model (DTM) is freely available at https://www.geoportale.regione.lombardia.it.

2   Case study and experimental setup

The case study ~~is~~ was carried out in the Lambro catchment, a peri-urban catchment ~~,~~and a left side tributary of the Po river, shown in Fig. 1.a. It is located north of Milan metropolitan area and covers three different provinces: Como, Lecco, and Monza

~~and Brianza.~~ e Brianza. The Lambro river, at the Lesmo river section (in purple in Fig. 1.a), drains an area of $260\,\mathrm{km}^2$ which can be mainly divided in two zones, with different morphology and land use. The northern one is the Prealpine region, where the Lambro river rises, at $944\,\mathrm{m}$ a.s.l.. The southern one, between Pusiano lake and the outlet section, at $178\,\mathrm{m}$ a.s.l., is instead a flat area subjected to massive urbanization, which results in large impervious surfaces and, consequently, fast runoff processes with a lag time of few hours. The catchment includes the presence of two lakes: Pusiano lake (which is the biggest one) and Alserio lake, both located in the middle part of the catchment. According to Köppen (1925) climate classification, the inland northern portion of Italy belongs to the humid subtropical climate (Cfa). Heavy convective cells characterize the basin, while the highest monthly rainfall accumulation occurs in spring and autumn. The local meteorological drivers, added to urban sprawl, lead to the hydrological vulnerability of the region. In order to mitigate hydrological risk in Monza and Milan urban areas (downstream to our case study), structural works have been carried out along the Lambro river in past years. Moreover, great efforts have been put in the implementation and development of non-structural measures ~~providing support to early warning system~~ (e.g., Ravazzani et al., 2016; Masseroni et al., 2017; Lombardi et al., 2018), including a dense monitoring system managed by ARPA Lombardia (Regional Agency for Environmental Protection). From this, we exploited $10\,\mathrm{min}$ resolution rainfall depths and temperatures respectively from 13 tipping-bucket RGs and eight thermometers (~~TH~~THs), for years 2018, 2019, and for the first six months of year 2020. In addition, we used $10\,\mathrm{min}$ resolution water level measurements of a flow gauge (FG), located at the outlet section of the Lambro basin in the municipality of Lesmo. All these meteorological and hydrological data are available at https://www.arpalombardia.it. A rather dense CML network, owned by Vodafone Italia S.p.A., covers the central and southern catchment area and its surroundings. In contrast, the northernmost portion of the Lambro basin is covered by few and unevenly distributed CMLs, given that it is thinly populated and characterized by higher altitudes. The CMLs available over the area are 50. The key features of CMLs as rainfall sensors are the operation frequency and the path length. Regrouping the CMLs according to the frequency:

1. 5 links are in the frequency range [11.4,13.1] GHz, with length between 3.5 and $8\,\mathrm{km}$;

2. 37 links are in the frequency range [18.8,23.0] GHz, with length between 1 and $8.5\,\mathrm{km}$;

3. 8 links are in the frequency range [38.5,42.6] GHz, with length between 1.4 and $2.2\,\mathrm{km}$.

~~As showed~~ We investigated 12 storm events and the associated floods, in the period June 2019–June 2020. The RG- and CML-based precipitation data sets, aggregated at hourly time scale, cover a wide range of rainy events from summer thunderstorms to low-intensity autumn events. In Table 1 we reported initial and final date and time, accumulated precipitation averaged over 13 RG measurements, $1\,\mathrm{h}$ maximum rain rate and observed total flow volume, for the 12 selected events. We defined a storm event as the time lapse where at least one RG, available on the area, detected precipitation with possible dry intervals no longer than $5\,\mathrm{h}$. An hour is considered dry when the detected rainfall depth is lower than $1\,\mathrm{mm}$ and wet otherwise. The beginning of the flood event is conventionally set at the hour in which the flow rate experienced a sudden deviation from the average. The end is instead set when the flow rate reverts to the initial condition, at the end of the depletion curve. According to the maximum observed rain rates, we classified events in *low rain rate* and *high rain rate*, adapting the

classification reported in Met Office (2007) to our specific case study. The former group includes storm events 5, 6, 7, and 12, for which the maximum rain rate is lower than 15 mm h^{-1}, while the latter covers the remaining events with maximum rain rate higher than 15 mm h^{-1}.

As shown in Fig. 1.a, the catchment area is divided into 15 sub-basins, hereinafter referred to as hydrological response units (HRU HRUs), for the sake of implementation of the hydrological model (see Sec. 3.2). In particular, the semi-distributed model here adopted adopted here requires, as input data, rainfall depths estimated in the HRU centroids. The estimates were gathered through the IDW technique (see Sec. 3.3) and for each HRU a different number of sensors was exploited, according to a defined maximum distance of 10 km from the HRU centroid. Figure 2 shows some features of the rainfall sensors used for spatial interpolation in each HRU: the number of exploited RGs, CMLs, and their sum, the ratio between CMLs and RGs number, the mean distance between rainfall sensors and HRU centroids, and the mean length of CMLs. It is Please, note that the CML-HRU centroid distance is calculated considering the CML middle point. It is also worth to mention that the total number of available CMLs can be less than the nominal was less than 50 for some events due to maintenance or malfunctioning during some events. The numbers in Fig. 2 are hence averaged over all the events. Figure 2.a shows highlights a significant increase of the exploited CMLs from HRU 1 to 6. This could be a potential problem leading to more inaccurate estimates, at the stage of spatial interpolation, for the northern HRUs with respect to the southern ones. On the other hand, the number of RGs has undergoes minor variations from one HRU to another. In Fig. 2.b we can see that the lowest ratios between CMLs and RGs correspond to HRU from 1 to 5 and the HRU 10. Moreover, Fig. 2.c shows that, considering HRUs from 1 to 9, the mean distance between sensors and HRU centroids is always higher when CMLs are considered. The opposite trend, with a single exception for HRU 12, occurs for HRUs located further downstream. Lastly, the mean CML length, in Fig. 2.d, has a decreasing trend from upstream to downstream.

We investigated 12 storm events and the associated floods, in the period June 2019–June 2020. The RG- and CML-based precipitation datasets, aggregated at hourly time scale, cover a wide range of rainy events from summer thunderstorms to low-intensity autumn events. In Table 1 we reported initial and final date and time, accumulated precipitation averaged over 13 RG measurements, maximum detected rain rate and observed total flow volume, for the 12 selected events. We defined a storm event as the time lapse where at least one RG, available on the area, detected precipitation with possible dry intervals no longer than 5 . A hour is considered dry when the detected rainfall depth is lower than 1 and wet otherwise. The beginning of the flood event is conventionally set at the hour in which the flow rate experienced a sudden deviation from the average. The end is instead set when the flow rate reverts to the initial condition, at the end of the depletion curve. According to the maximum observed rain rates, we classified events in *Low rain rate* and *High rain rate*. The former group includes storm events 5, 6, 7, and 12, for which the maximum rain rate is lower than 15 , while the latter covers the remaining events with maximum rain rate higher than 20 . .

Figure 2. From panel a to d: number of rainfall sensors used for spatial interpolation in each HRU centroid, ratio between the number of RGs and CMLs, mean distance between rainfall sensors and HRU centroids, and CML mean lengths.

	Storm event			Flood event	
ID event	Date and Time (LT)	Cumulative precipitation (mm)	Max rain rate (mm h^{-1})	Date and Time (LT)	Flow volume (m^3 10^6)
1	22 Jun 2019, 06:00 22 Jun 2019, 15:00	40.5	37.6	22 Jun 2019, 08:00 22 Jun 2019, 23:00	0.5
2	14 Jul 2019, 22:00 16 Jul 2019, 03:00	63.6	49.0	22 Jun 2019, 08:00 16 Jul 2019, 23:00	3.0
3	05 Sep 2019, 01:00 09 Sep 2019, 10:00	68.7	36.6	05 Sep 2019, 21:00 11 Sep 2019, 23:00	2.2
4	18 Oct 2019, 17:00 22 Oct 2019, 12:00	108.4	35.2	19 Oct 2019, 00:00 24 Oct 2019, 23:00	8.7
5	14 Nov 2019, 19:00 16 Nov 2019, 17:00	34.5	12.6	15 Nov 2019, 04:00 17 Nov 2019, 04:00	2.9
6	17 Nov 2019, 01:00 17 Nov 2019, 19:00	25.6	6.4	17 Nov 2019, 10:00 19 Nov 2019, 03:00	3.3
7	18 Nov 2019, 23:00 20 Nov 2019, 00:00	27.9	4.0	19 Nov 2019, 04:00 21 Nov 2019, 23:00	5.4
8	14 May 2020, 20:00 16 May 2020, 07:00	64.2	31.0	14 May 2020, 22:00 17 May 2020, 02:00	4.6
9	03 Jun 2020, 16:00 05 Jun 2020, 04:00	79.9	24.8	03 Jun 2020, 18:00 05 Jun 2020, 19:00	4.1
10	07 Jun 2020, 08:00 08 Jun 2020, 02:00	42.1	29	07 Jun 2020, 15:00 08 Jun 2020, 19:00	3.5
11	08 Jun 2020, 17:00 09 Jun 2020, 19:00	32.2	22.8	08 Jun 2020, 20:00 10 Jun 2020, 01:00	3.8
12	10 Jun 2020, 11:00 11 Jun 2020, 07:00	16.5	7.4	11 Jun 2020, 02:00 11 Jun 2020, 15:00	1.0

3 Methods

In this section we firstly ~~present the processing of raw data . Concerning~~ describe the algorithms of data processing. While for conventional sensors (RGs, THs, and FG), the ~~data handling merely consists in (1) replacement of missing data or outliers, (2) resampling, and (3) conversion of hydrometric measurements into discharge values. Data reduction instead allows to retrieve rainfall intensity from CML-based raw data. Then,~~ procedure is simple, it is not straigthforward to extract quantitative rainfall information from CML raw data, which are generated for network monitoring purposes. Second, we discuss the semi-distributed hydrological model and its calibration/validation ~~procedure are exhibited~~procedures. Finally, we illustrate ~~methods exploited for~~ the methods of spatial interpolation of RG- and CML-based rainfall data.

3.1 Data processing

Conventional sensors (rain gauges, thermometers, and flow gauge)

Raw data from RGs, THs and the FG were firstly processed to correct invalid measurements (missing data and outliers), which account for less than 1% in the period January 2018–June 2020. The process is different depending on the type of measurement. Invalid RG data were replaced interpolating valid observations from the nearby sensors by the IDW algorithm. Invalid TH data, as well as invalid FG measurements, were instead replaced by ~~a~~ linear interpolation. After data correction, the 10 min raw data were resampled to hourly time scale. Lastly, water level observations were converted into river discharge measurements by using the rating curves, validated by the Hydrographic Office of ARPA Lombardia.

Commercial Microwave Links

~~The~~ CML raw data ~~, collected by a network monitoring tool~~ are minimum and maximum values of the transmitted and received power levels (TSL and RSL, respectively) every 15 min.

Microwave links of mobile networks are usually two-ways and provide dual-frequency operation~~, hence adding a certain degree of redundancy when it comes to rainfall estimates~~. The CML data set used here has two to four channels available for every link, which usually permits to deal with missing or invalid data appearing sometimes over a certain channel. Procedures for the conversion of RSL into rainfall rate have been detailed by several authors ~~(e.g., Schleiss and Berne, 2010; Fenicia et al., 2012; Overeem and are not discussed here extensively. The analysis of CML data in this work~~ (e.g., Schleiss and Berne, 2010; Fenicia et al., 2012; Overeem . As the format of the available CML data is the same as in Overeem et al. (2016), we built from the procedure outlined there. Specifically, data processing went through the following steps~~(?)~~: (1) identification and removal of outliers and artifacts (i.e., occasional spikes, which are not caused by rain); (2) classification of each 15 min time slot into dry or wet (i.e., rainy) by thresholding the difference between maximum and minimum RSL values; (3) estimation of the baseline, i.e., ~~the RSL in~~ RSL in the absence of rain; (4) calculation of total signal attenuation as the difference between the baseline and the actual RSL; (5) identification and subtraction of the components of total attenuation not due to rainfall (e.g. wet antenna attenuation); (6)

conversion of rain attenuation into rainfall intensity. Details of the major processing steps are discussed in the following.

Dry/wet classification at step (2) is required by subsequent steps (3) and (5). First, the RSL is thresholded by an hysteresis method (see Nebuloni et al., 2020b). Then, each CML is given a score equal to the product of the binary outcome (0/1) of thresholding by the inverse of its sensitivity to rainfall, the latter depending on CML frequency and length. Finally, a CML is flagged as wet if the aggregate score of the CML itself and of all its neighbors exceeds 0.5, otherwise it is dry. Two CMLs are neighbors if they fulfill any of the following conditions: (1) they have a terminal in common, (2) their paths intersect, and (3) their distance is less than a defined maximum value. The baseline on step (3) is obtained through a windowing algorithm. An N-sample window is centered around each sample of the RSL time series. If enough samples in the window are dry, the baseline value in the center of the window is the average of minimum and maximum RSL. Once the entire time series has been processed, the baseline missing points are obtained by linear interpolation. In step (5), it is assumed that wet antenna attenuation is the only relevant component of total path attenuation not due to rain. This contribution is subtracted from total attenuation using the model proposed by Schleiss and Berne (2010), which predicts an exponential increase of attenuation during the wetting transient, a constant value while raining and an exponential decrease during the drying transient. The input parameters of the model, that are the duration of the initial transient and the maximum value of wet antenna attenuation, are 900 s and 2 dB, respectively. They were determined analyzing a set of RSL and TSL time series sampled every 10 s, which were made available over a few CMLs. The relationship between rain attenuation per unit path length γ_R (dB km^{-1}) and rainfall intensity R (mm h^{-1}) is usually modelled by the following power-law function:

$$\gamma_R = \kappa R^\alpha. \tag{1}$$

The coefficients κ and α have been tabulated by the International Telecommunication Union as a function of signal frequency and polarization (ITU-R P.838-3, 2005). In principle, the $\gamma_R - R$ relationship is dependent on the microphysics of rain as well, hence κ and α should be calibrated, provided that the characteristics of precipitation are known in the

climatic area where CMLs are deployed. In this work, raindrop size distribution data gathered from disdrometers were used to calculate the optimum value of κ and α coefficients following the procedure outlined in Luini et al. (2020).

~~Third, in~~ In the available CML data format, only the two extreme values of TSL and RSL are saved in every 15 min window. Therefore, if the average rainfall rate ~~is~~ has to be estimated, for instance to calculate hourly accumulations, it is necessary to derive it from the extremes. To this aim, TSL and RSL time series sampled each 10 s were made available for a ~~set of sample CMLs during~~ subset of CMLs during some of the events considered here and processed as shown in ~~?.~~ Nebuloni et al. (2020a). Average, min and max rainfall rate within 15 min windows were calculated from the 10 s time series and the following unbiased estimator of the average rainfall rate was derived:

$$R_{\mathrm{MIN-MAX}} = \frac{1}{1.14} \frac{R_{\mathrm{MIN}} + R_{\mathrm{MAX}}}{2}. \tag{2}$$

~~Fourth, it is assumed that~~ Two aspects of the above procedure deserve more discussion. First, the available RSL sequence have a coarse 1 dB quantization step, which produces a zero-mean random error with rectangular distribution and limiting values equal to $\pm$ 0.5 dB ($\pm$ 12% when the power is measured on a linear scale). It turns out that it is impossible to distinguish between rain and quantization-induced noise below a certain rainfall intensity threshold. Figure 3 shows the minimum detectable rainfall intensity without ambiguity as a function of the CML path length with the CML frequency as parameter. The square markers correspond to the 50 CMLs in the study area divided into three groups according to their frequency band. Continuous lines are drawn at three reference frequencies as well. Moreover, quantization affects the accuracy of rainfall intensity estimates. The accuracy of instantaneous measurements (at the 95% confidence level) is within 20% if the rainfall intensity exceeds 3 mm h^{-1} for the link with the most favorable combination between length and frequency. However, in the worst case, the above accuracy is achieved only if the rainfall intensity is above 10 mm h^{-1}. The only way to mitigate quantization effects is to average in time. Second, it is assumed that the rain attenuation measured over a CML of length L, is L times the attenuation per unit path length in Eq. ~~-~~(1), ~~that is~~ which implies that rain is considered uniform along the path. The effect of the inhomogeneity of precipitation can be relevant as CML paths range from about 1 km to nearly 9 km (Fig. ~~-~~3). Some authors proposed to retrieve the spatial distribution of the rainfall field across the measurement area by processing all the CML data together, for instance through tomographic techniques. In this work, a simpler approach is used. Each CML is considered independently of the ~~other~~ others and the corresponding rainfall measurement is given a weighting coefficient dependent on ~~CML length~~ the distance between its midpoint and the point where rainfall has to be estimated, as discussed in Sec. ~~3.3.~~ 3.3.

Please note that, apart from the initial calibration of the $\gamma_R - R$ relationship, carried out through disdrometer data, CMLs are a fully independent network of rainfall sensors, as no external information is used. In order to validate the rainfall estimates provided by CMLs, we compared the accumulated rainfall during each of the events in Table 1 with RG direct measurements. To carry out a fully fair comparison, an ad-hoc array of RGs should be deployed along the CML path. However, this is seldom feasible. Here, CMLs and RGs are associated according to their mutual distance. Each RG is given a different weight depending on its position with respect to an associated CML, as follows: the CML path is divided into short segments, the distance between the RG and each CML segment (approximated by its midway point) is calculated, and all the above distances are averaged.

265 using the average CML–RG distance. The scatter plot between CML– and RG–based accumulated rainfall is plotted in Fig. 4 for the eight high-intensity (panel a) and the four low-intensity events (panel b), respectively. Only RGs within 5 km (average distance) from a CML are considered. During high-intensity events, there is a good match between CML and RG estimates, whereas CMLs exhibit an evident underestimation (more than 30% on the average) in the case of low-intensity events. This pattern can be explained by the lack of sensitivity of CMLs to low rainfall intensities due to signal quantization. In Appendix
270 A we present a further local comparison, between CML and nearby RGs on rainfall time series.

3.2 Hydrological model

We used a semi-distributed rainfall-runoff model, at an hourly time scale. The catchment area is divided into 15 HRUs, which are considered meteorologically, geologically, and hydrologically homogeneous. The model parameters are hence set at HRU scale.

275 The river discharge at time t, $Q(t)$, in HRU's outlets is calculated as the sum of two main components

$$Q(t) = Q_s(t) + Q_g(t), \tag{3}$$

where Q_s is the contribution given by the surface runoff R^*, i.e., the portion of rainfall ~~rejected by~~ not infiltrated into the soil, an Q_g is the groundwater discharge.
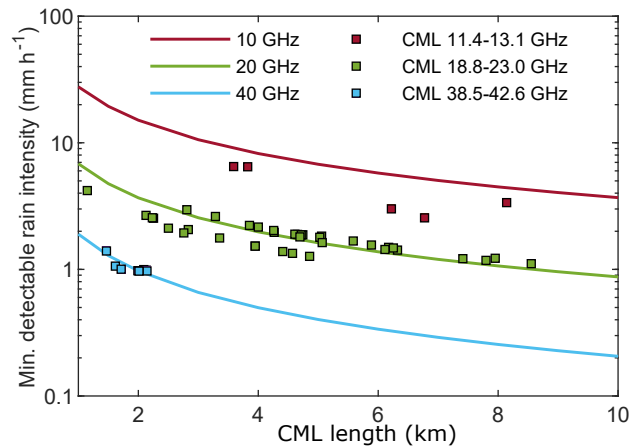
Figure 3. Minimum detectable rainfall intensity as a function of path length for link frequencies of 10, 20, and 40 GHz, assuming a 1 dB quantization step on RSL. Squares represent the frequency and length of the 50 available CMLs in the study area.
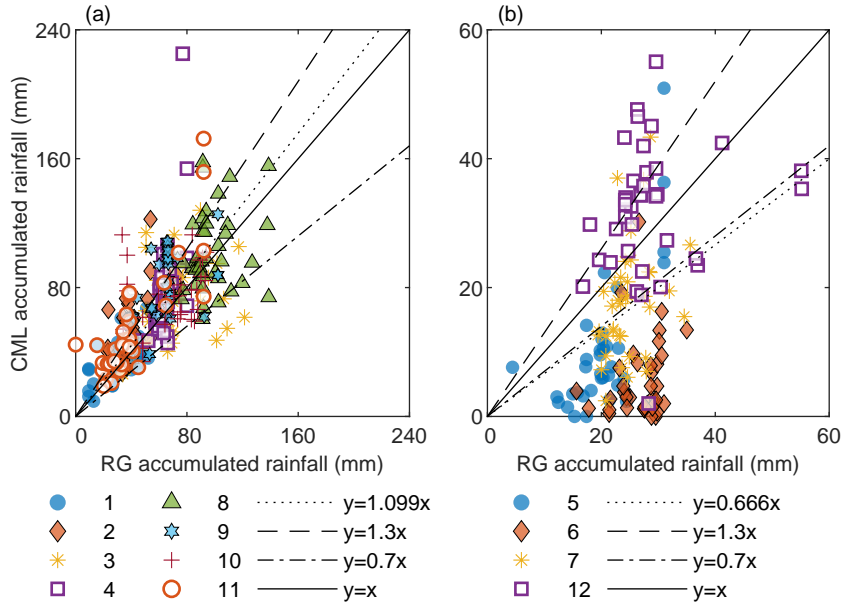
Figure 4. Accumulated rainfall during (a) *high rain rate* events and (b) *low rain rate* events: CMLs against nearby RGs. The best fit of data, the $\pm 30\%$ bounds, and the 45° line are shown as well.

The computation of R^* (in mm) relies on the SCS-CN method (US Department of Agriculture Soil Conservation Service, 1985):

$$R^* = \frac{(P - I_a)^2}{P - I_a + S}, \tag{4}$$

where $\cancel{P} \; P \geq I_a$ (in mm) is the ~~rain~~ rainfall depth, S (in mm) is the maximum soil potential retention, and I_a (in mm) is the initial abstraction (calculated as a percentage, 20%, of S). According to USDA-SCS guidelines, the soil moisture condition antecedent to a storm event is classified depending on the value of the five-day antecedent rainfall. Here, we account for the actual soil moisture in a dynamical way, as proposed in the AnnAGNPS model (Bingner and Theurer, 2005) and also implemented in Ravazzani et al. (2007). In particular, the value of $S(t)$ is updated as a continuous function of the degree of soil saturation $\epsilon(t)$ (-)

$$S(t) = S_I \cdot \left\{ 1 - \left[\frac{\epsilon(t)}{\epsilon(t) + \exp(W_I - W_{II} \cdot \epsilon(t))} \right] \right\}, \tag{5}$$

where the weights W_I (-) and W_{II} (-) are defined as follows

$$W_I = \ln \left[\frac{1}{1 - S_{III}/S_I} - 1 \right] + W_{II}; \tag{6}$$

$$W_{II} = 2 \cdot \left[\ln \left(\frac{0.05}{1 - S_{II}/S_I} - 0.5 \right) - \ln \left(\frac{1}{1 - S_{III}/S_I} - 1 \right) \right], \tag{7}$$

~~being~~ where S_I, S_{II}, S_{III} are the retention parameters associated with the curve numbers CN_I, CN_{II}, and CN_{III}, respectively. Finally, $\epsilon(t)$ is calculated as

$$\epsilon(t) = \frac{\theta(t) - \theta_{res}}{\theta_{sat} - \theta_{res}}, \tag{8}$$

where θ_{sat} (-) is the soil moisture at saturation conditions and θ_{res} (-) is the residual soil moisture.

To calculate Q_s at time t, the runoff is routed to the HRU's outlet, representing each HRU as a linear reservoir model (Dooge, 1973):

$$Q_s(t) = \int_0^t a \cdot r^*(\tau) \cdot A \cdot T_{lag}^{-1} \cdot \exp\left(-\frac{t-\tau}{T_{lag}}\right) d\tau, \tag{9}$$

where $a = 10^{-3}$ $\mathrm{m\,mm^{-1}}$ is a conversion factor, r^* is the surface runoff rate (in $\mathrm{mm\,s^{-1}}$), A (in $\mathrm{m^2}$) is the HRU area, and T_{lag} (in s) is the lag time calculated as 0.6 times the concentration time, T_{c}, for average natural watershed conditions and an approximately uniform distribution of runoff according to Mockus (1957) and de Simas (1996). The calculation of T_{c}, in each HRU, relies on the formula proposed by Ferro (2006).

The portion I (in mm) of total rainfall that infiltrates in the shallower layer of soil can either be lost by evapotranspiration, ET (in mm), or by percolation, D (in mm). Potential evapotranspiration (PET) is here calculated by the Hargreaves and Samani (1985) equation, which requires temperature data. The actual evapotranspiration (ET) is computed as a fraction of PET following Ravazzani et al. (2015). The water balance equation, referred to the shallower layer of soil with depth z (in mm) at time t, is formulated as

$$\theta(t) = \theta(t-1) + \frac{I(t-1) - D(t-1) - ET(t-1)}{z}, \tag{10}$$

where θ (-) is the actual soil moisture. $D(t)$ is the drainage flux calculated as

$$D(t) = c \cdot K_{sat} \cdot \epsilon(t)^{\frac{2+3B}{B}}, \tag{11}$$

where $c = 3.6 \cdot 10^6$ (in $\mathrm{mm\,s\,m^{-1}}$) is a conversion factor, K_{sat} (in $\mathrm{m\,s^{-1}}$) is the hydraulic conductivity at saturation, and B is the Brooks-Corey index (Brooks and Corey, 1964). Finally, $Q_g(t) = a \cdot \Delta T^{-1} \cdot D(t) \cdot A$, with $\Delta T = 3600$ s. The interaction among HRUs is represented by in series or in parallel reservoirs, according to the development of the river network, as exhibited in Fig. 1.b. CN values were taken from https://www.isprambiente.gov.it while θ_{res}, θ_{sat}, and B parameters were taken from Maidment (1993). K_{sat} and z are instead calibrated, as reported below.

Calibration and validation of the hydrological model

The hydrological model was calibrated using, as input, the hourly rainfall depths from the RG network in Fig. 1.a. The chosen period for calibration is 1 January 2019–31 December 2019. We ~~carried out a sensitivity analysis on model's parameters to identify the most important ones, to be adjusted with calibration, and we selected two of them, namely~~ tested different combinations of the two parameters, K_{sat} and z~~. The initial value of K_{sat}, for each HRU, was taken from Maidment (1993)~~

~~. Then, ,~~ and we selected the combination maximizing the Nash and Sutcliffe (1970) efficiency (NSE). Concerning K_{sat} ~~parameter was calibrated by applying a multiplicative coefficient. We tested , as multiplicative coefficients, several~~ we tested the values reported in Maidment (1993) multiplied by several different powers of 10, ~~$\{10^{-2}, 10^{-1}, 10^0, 10^1, 10^2\}$. The range of variation for~~ $\{10^{-2}, 10^{-1}, 10^0, 10^1, 10^2\}$. The values of K_{sat} taken from the literature are different depending on the type of soil

325 characterizing each HRU. With regard to z ~~was instead ,~~ we tested all the values inside the range [10 cm, 3 m] ~~. The parameters ,~~ with a 10 cm step. The parameter validation was carried out ~~for a total of one year time interval, subdivided into two periods: the first is~~ over two non-consecutive six-months periods: 1 July 2018–31 December 2018 and ~~the second is~~ 1 January 2020–30 June 2020. ~~As model performance metric we selected the Nash and Sutcliffe (1970) efficiency,~~ The NSE ~~. We identified the optimum combination of parameters through a trial and error calibration. The chosen parameters provided a NSE of~~ value is

330 0.69 for the calibration and ~~of~~ 0.56 for the validation, respectively. Please note that the calibrated parameters provide a NSE larger than 0.5 for the overall one-year validation period, which is the minimum value recommended by Moriasi et al. (2007) to consider a simulation reliable. ~~We recall that the discharge simulation performances are generally considered reliablewhen NSE > 0.5 (Moriasi et al., 2007).~~

It is also worth to notice that the calibration and validation steps were particularly troublesome due to the presence of the
335 Cavo Diotti dam, which artificially regulates the outflow of Pusiano lake during flood events.

3.3 Spatial interpolation of rainfall data

~~In the past, various~~ Several methodologies have been ~~tested~~ proposed and applied for the spatial interpolation of ~~CML retrieved rainfall measurements , considering different spatial scales~~ rainfall measurements retrieved from CMLs (e.g., Fencl et al., 2013; Overeem et al., 2013; D'Amico et al., 2016; Haese et al., 2017; Chwala and Kunstmann, 2019; Graf et al., 2020; Eshel et al.,
340 2021). Here, we exploited the simple and robust IDW method (Shepard, 1968) for both RG and CML measurements. ~~However, since CMLs measurements are path-averaged we considered CMLs as point sensors collapsed in their midway point but we used an extra weight only to dump the effect of rainfall inhomogeneity associated to long links.~~

~~Following the~~ According to the IDW method, given n measurements $\{u(\mathbf{x}_1), ..., u(\mathbf{x}_n)\}$ at given points $\mathbf{x}_i$, with $i = 1, ..., n$ the interpolated value u, in $\mathbf{x}$, is calculated as

$$u(\mathbf{x}) = \begin{cases} \frac{\sum_{i=1}^{n} w_i' \cdot u(\mathbf{x}_i)}{\sum_{i=1}^{n} w_i'} & \text{if } d(\mathbf{x}_i, \mathbf{x}) \neq 0 \text{ for all } i; \\ u(\mathbf{x}_i) & \text{if } d(\mathbf{x}_i, \mathbf{x}) = 0 \text{ for some } i, \end{cases} \tag{12}$$

345 with

$$w_i' = \frac{1}{d(\mathbf{x}_i, \mathbf{x})^{\gamma}}, \tag{13}$$

where $d(\mathbf{x}_i, \mathbf{x})$ is the distance between the measuring point $\mathbf{x}_i$ and the coordinates of HRU's centroid $\mathbf{x}$, and $\gamma > 0$. The number of contributing measurements n is the one within a distance d_{max} from the query point. We identified ~~appropriate values for the exponent~~ the appropriate value for γ and d_{max} by ~~an iterative procedure presented herein~~ leave-one-out cross validation. We
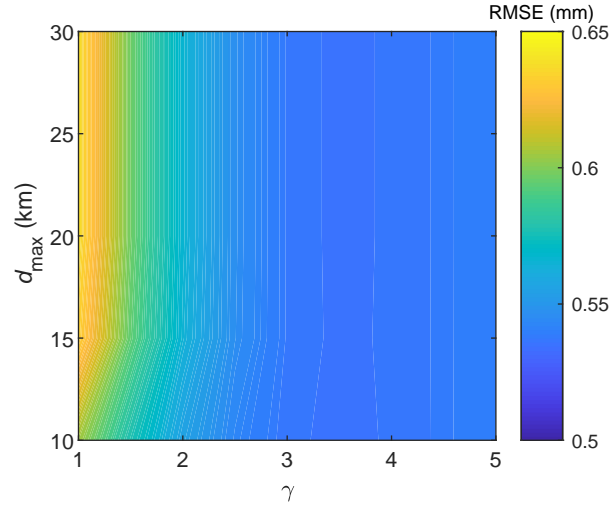
Figure 5. IDW calibration based on RGs: RMSE between observed and simulated rainfall depths, for different values of γ and d_{max}.

estimated the precipitation, at each RG point, from the remainder of RGs at a distance smaller than d_{max}, and calculated the Root Mean Square Error (RMSE) between observations and estimates. The process was repeated for several values of γ and d_{max}. We set a minimum ~~of 10 for~~ d_{max} value equal to 10 km, to have at least one neighbour available for every considered RG. To this aim, we exploited a larger set of 38 RGs (including the 13 RGs in Fig. 1.a) located on a wider area compared to ~~only the Lambro basin~~ the Lambro catchment and we used data from January 2018 to June 2020. The resulting RMSE distribution as a function of d_{max} and γ is reported in Fig. 5. We observe that the choice of d_{max} has a rather ~~limited~~ marginal effect on the estimates ~~, for~~ if $\gamma \geq 2$. The minimum RMSE is achieved when γ is slightly above 3. We therefore chose $\gamma = 3$. ~~Finally~~Moreover, we selected $d_{max} = 10$ km, which provides the best RMSE when $\gamma = 3$.

To spatially interpolate CMLs rain rates, we ~~took into account the CML length. Firstly, we handled CMLs~~ handled them as *virtual* RGs, ~~considering the rain estimate as collapsed in the middle point~~ assuming the rainfall measurement is collapsed into the midpoint of the CML ~~length~~path. Again we used IDW ~~, this time with a weight given by w' multiplied by a length-dependent term w'' being~~

$$w'' = e^{-\beta^{-1} L},$$

~~where L is the CML's length and β^{-1} the decreasing rate. In fact, the longer is the CML, the higher the probability that rainfall is unevenly distributed along its length. We chose $\beta = 5$, hence for $L = 8.6$ (maximum CML path length), ω'' is equal to 0.18, while for $L = 1.1$ (minimum path length), $\omega'' = 0.80$. Finally, as done in the case of RGs, to spatially interpolate CMLs data into each HRU centroid, we only considered CMLs whose middle points fall within $d_{max} = 10$ from the HRU centroid.~~

method and the same values of γ and d_{max} as above. In addition of considering only RGs, or only CMLs, we accounted for the integration of RGs and CMLs measurements. In the following, we will refer to this option as CML+RG.

4 Results

370 The results ~~of this paper~~ are presented in the following two subsections. Sec. 4.1 provides a comparison ~~on~~ of rainfall depths interpolated in the HRU centroids, by using data ~~both from conventional and~~ either from conventional or opportunistic sensors. Both accumulated rainfall values and hourly rainfall depths are considered at basin and sub-basin scale, for the 12 events. We therefore investigate whether there are some critical issues that might help to explain differences in the rainfall-runoff model outputs. Sec. 4.2 analyzes discharge performances, by comparing RG-, CML-, and RG+CML-driven simulations with

375 the observed flow rates.

4.1 Comparison between RG and CML rainfall data in each HRU

Figure 6 shows the scatter plot of the ~~areal-averaged rainfall depths~~ rainfall accumulated at the end of each ~~storm event~~ of the 12 storm events and averaged over the entire catchment area. Yellow markers are CML against RG rainfall depths, while in orange are CML+RG against RG rainfall depths. ~~The corresponding regression lines indicate a general good agreement between the~~

380 ~~two sets of sensors. For all the~~ On the one hand, for all the ~~Low~~ *low* rain rate events (squares), estimates from CMLs ~~(and~~ and from CMLs+RGs ~~)~~ are lower than ~~those~~ the ones from RGs. On the other ~~side, CMLs tend to return higher estimates than RGs during~~ hand, CML (and CML+RG)-estimates of ~~High~~ *high* rain rate events~~(circles)~~ ~~, even though the trend is not as evident,~~ are very nearly the same (with either lower and higher values) to the RG-based ones, with the only exception of event 3. From a more general perspective, the two regression lines indicate a good agreement between the two sets of sensors.

385 We further ~~investigated the above behaviour~~ assessed CML and RG rainfall estimates on the hourly time scale and on the sub-basin spatial scale by calculating the relative error ~~, ΔE,~~ of CML estimates ~~,~~ with respect to RG ones, assuming the latter as benchmark, for the hourly ~~rain~~ rainfall depths inferred in the 15 HRU centroids. ~~We considered only~~ The relative error (ΔE) is evaluated as:

$$\Delta E = \frac{R_{CML} - R_{RG}}{R_{RG}}, \tag{14}$$

390 where R_{CML} and R_{RG} are the 1 hour rainfall depths estimated in each HRU centroid, respectively from CMLs and RGs. For the calculation, we only considered wet hours (~~rain depth ≥ 1~~ $R_{RG} \geq 1$ mm ~~for RG estimates~~ in HRU centroids), ~~that are 2061. Figure 7 shows a binned scatter plot (orange circles) of ΔE against RG-based rain depth, taken as the benchmark. The small circles in cyan are the~~ relying on a data set of 2061 ~~measured values. The x-axis has been divided into 11 bins. The x coordinate of each orange circle is the mid point of each bin interval, whereas the y coordinate is~~ values. Hence, when the CML

395 estimate yields 0 and the RG estimate is greater than 0 (*false negative*), the ~~mean of the~~ relative error is -1. Figure 7 shows a 2D histogram representing the count of rain hours falling in a given range of RG-estimated rainfall depths and in a given range of relative errors. The increasing spread of ΔE values ~~calculated over all the cyan circles falling within the bin. Finally, vertical~~

bars represent the ±1 standard deviation bounds. For rain depths smaller than about 3 , mean ΔE values are negative (CMLs estimates lower than RG estimates), with respect to the decrease of the RG-based rainfall depths is due to the large numbers of zeroes in CML rain depths during wet hours (i.e. $\Delta E = -1$). Conversely, for rain rateshigher than 3 , mean ΔE values are slightly higher than 0. Indeed, if the rain greater uncertainty of CMLs in detecting low rain rates. If the RG-based rainfall depth is smaller than 3 mm, only 30% of ΔE values falls in the range [-0.4, 0.4], whereas if it is larger than 3 mm, the percentage increases up to nearly 70%.

Moreover, for the lowest rainfall depths there are fewer negative values of ΔE as we set to zero all the CML rain rate estimates lower than the sensitivity of the link itself. The high count related to $\Delta E = -1$ and RG-based rainfall depths < 5mm is due to the occurrence of *false negatives*.
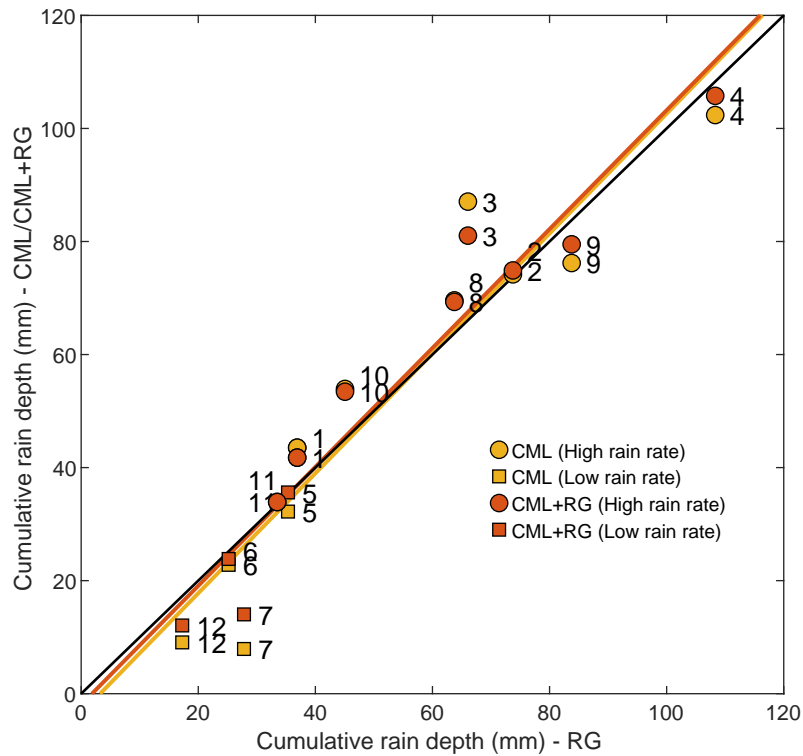
Figure 6. Rain Rainfall depths, averaged over the catchment area and accumulated at the end of each eventof the 12 events. The number next to the markers refer to the ID event while the two different markers, circles and squares, respectively stand for *High rain rate* *high rain rate* and *Low rain rate* *low rain rate* events. The black line represents the 1:1 line of perfect matching between rain rainfall depth estimates from RGs and from CMLs (yellow) or from the combination of RGs and CMLs (orange). Yellow and orange lines are the corresponding regression lines.

We therefore focused on the CML hourly wet-dry ~~(see Sec. 2)~~ classification, inferred in HRU centroids, again considering RG estimates as benchmark. ~~A~~ We recall that dry hours are those in which the detected rainfall depth is lower than 1 mm and vice versa for wet hours (see also Sec. 2). Figure 8 depicts box plots of the percentage of *false negatives* and *false positives* for *low rain rate* and *high rain rate* events. In contrast to a *false negative*, a *false positive* occurs when an hourly slot is classified as wet by CMLs ~~while it is dry according to RGs, and viceversa for a *false negative*. Figure 8 depicts a box plot of~~ and dry by RGs. The two box plots on the left were obtained from a population of 120 samples (8 ~~*false positive*~~ *high rain rate* ~~and~~ events x 15 HRUs), while the two on the right were computed from 60 samples (4 ~~*false negative*~~ *low rain rate* ~~percent occurrences. Events have been grouped according to their intensity.~~ events x 15 HRUs). For example, the maximum percentage of false negatives is 60%, which corresponds to HRU 2 during the *low rain rate* event 7 of Table 1. From a general point of view ~~*Low*~~ *low* rain rate events ~~clearly~~ exhibit a higher median and a larger dispersion of *false* ~~*negativepercentage than*~~ *negatives* than ~~*High*~~ *high* rain rate events, whereas the occurrence of a *false positive* is relatively rare in both cases. These results confirm the inability of CMLs in detecting low rain rates, which depends on the quantization error issue discussed in Sec. ~~-~~3.

Finally, in Fig. 9 we report box plots of ΔE values calculated for ~~rain depths accumulated at the end of the~~ rainfall accumulated by each HRU over each of the 12 ~~events, for each HRU . It does not emerge a general positive or negative trend and the median~~ event. Again, the events are grouped in two classes according to rainfall intensity. The contrasting behaviour between *low rain rate* and *high rain rate* events is evident. In the former case, ΔE ~~values are inside the range~~ is mostly much lower than 0 and it is much scattered. Once again, this result confirms that during *low rain rate* events, CMLs are not able to properly detect the lowest rainfall intensities. Regarding *high rain rate* events, there is not a clear trend. The median ΔE values
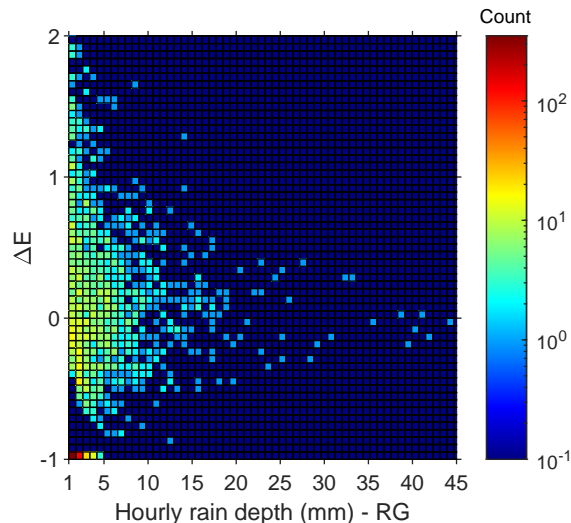
Figure 7. ~~Relative difference ΔE between CML and RG~~ 2D histogram of hourly ~~rain depths against RG rain depths~~ rainfall depth from RGs and ΔE. ~~X-axis~~ The colour of each equally spaced 2D bin represents its height, which is the count of data falling in the bin. The scale bar has a logarithmic scale and the dark blue bins correspond to 0 counts. Values of ΔE equal to -1 represent false negatives.
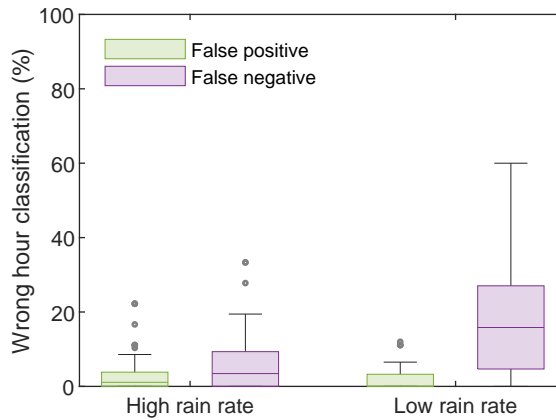
Figure 8. Percentage ~~variation~~ of hours subjected to wrong wet-dry classification (*false negative* or *false positive*), with respect to ~~*High rain rate*~~ *high rain rate* and ~~*Low rain rate*~~ *low rain rate* events. The box plots display the median, the 0.25 (lower) and 0.75 (upper) quantiles, outliers (computed using the interquartile range), and the minimum and maximum values excluding any outliers.

425 are always within the range [-~~0.2,0.2~~ -0.25,0.25] ~~for 12 out of 15 HRUs. Moreover, in all the cases medians are lower than 0.3.~~ ~~Negative values of ΔE are mostly observed in HRUs 1, 2, 5, and 11. Such underestimations take place since CMLs, which~~ ~~have a non-negligible weight in the rainfall spatial interpolation within these HRUs, are the least sensitive to low rain rates. In~~ ~~particular, the highest differences between RG and CML estimates are associated to HRU 11 where the two major contributing~~ ~~links are characterized by quite low frequencies ($\sim$ 18–19) and have lengths up to 6.3 . We observed that such two CMLs~~

430 ~~systematically provide lower estimates, if compared to the closest RGs (analysis not reported here),with greater evidence for~~ ~~low rain rates hours~~ and their dispersion is low as well.

4.2 Comparison between RG- and CML-driven discharge simulations

In ~~this section, we investigate the matching between RG and CML focusing on discharge simulations at Lesmo river section.~~ ~~Again, the comparison is carried out for the 12 flood events selected. The model was fed by interpolated rainfall data from~~ the

435 following, discharge simulations obtained from three different data inputs (RGs, CMLs ~~, and RGs+CMLs.~~ and CMLs+RGs) are assessed and compared with hydrographic measurements at Lesmo river section.

The output performances were evaluated with three indices: (1) the well-known Nash–Sutcliffe efficiency ~~,~~ (NSE), (2) the relative error on peak discharge ~~,~~ (REP~~,~~) and (3) the relative error on flow volume ~~,~~ (Dv~~. The formulations of these~~). The last two indices are ~~respectively~~ defined as follows:

440 $$\text{REP} = \frac{Q_{sim}^{max} - Q_{obs}^{max}}{Q_{obs}^{max}};$$ (15)

$$\mathrm{Dv} = \frac{V_{sim} - V_{obs}}{V_{obs}}, \tag{16}$$

where Q_{sim}^{max} is the simulated peak discharge, Q_{obs}^{max} is the observed peak discharge, V_{sim} is the simulated total flow volume, and V_{obs} is the observed total flow volume.

445   Performances of the 12 discharge simulations, grouped by rainfall data input, are summarized in Fig. 10, through box plots. The statistical dispersion (represented by the interquartile range, ~~IQR~~) of CML-based discharge simulations are larger than RG-based simulations. The use of CML ~~interpolated~~ data into the rainfall-runoff model seems to produce higher uncertainty, with respect to RG data. The combined use of RGs and CMLs instead decreases the statistical dispersion of results and leads to performances closer to those achieved through RG. Generally, CMLs exhibit worse performance than RGs in terms of NSE

450   and Dv. As for REP, the two ~~set~~ sets of sensors produce comparable errors of opposite sign, hence their combined use leads to an optimum value of the median error (0.06).

~~Figure 11 shows a~~

To gain a deeper understanding of the model performances on flow peaks, we produced a scatter plot of observed against simulated flow peaks ~~. Firstly, it shows~~ in Fig. 11. The scatter plot firstly reveals that the best match between observations and

455   simulations is not always achieved by RG-based simulations. In fact, for events 1, 3, 5, and 11 the optimum matching is given from CML- or CML+RG-based simulations. Moreover, the ~~Low~~ low rain rate events typically result in underestimated peak flow simulations with respect to the observations, considering either conventional or unconventional sensors, with an exception for event 5.
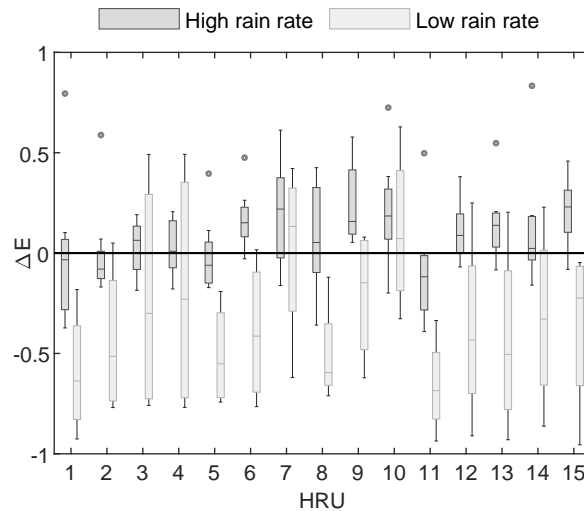
Figure 9. Box plots of ΔE in each HRU for the 12 storm events ~~grouped by HRU~~(divided in two groups according to rainfall intensity). The ranges of the box plots are those reported in Fig. 8.

Figure 12 reports model inputs and outputs for ~~event~~ events 5 and 2. We selected these two examples as they are characterized by a different meteorological configuration and lead to contrasting model performances. The first event is an autumn stratiform event, characterized by low rain intensity. In Fig. 12.a we can see that CML-based estimates in HRUs 1, 2, 5, 8, and 11 are quite low, with respect to RGs, due to the difficulties of ~~relevant~~ CMLs in detecting light rain~~, as also highlighted in Fig. 9.~~. In contrast, in the southern HRUs 10, 12, 13, 14, and 15, which have much more influence in the generation of river discharge, CML estimates are higher than RG ones. ~~Finally,~~ Fig. 12.b shows ~~that~~ an example where the CML-driven simulation better represents the observed outflow hydrograph, with respect to the RG-driven simulation. In particular, the best performance is gained when both the two types of rainfall sensors are used, and it provides an excellent Dv, equal to 0.03. The ~~most problematic CMLs are mostly located in~~ highest discrepancies between CML and RG estimates mostly involve the northern portion of the basin and have less impact in generating discharge. Event 2 is instead a typical intense convective summer event, characterized by a single rainfall peak. As rain rates are high all over the basin, contrary to event 5, we ~~do not observe lower CML estimates with respect to RG estimates, for HRU 1, 2, and 5. However, lower values are still present in HRU 11. In general,~~ observe in Fig. 12.c ~~reveals a good agreement between RG and CML~~ a better agreement between CML and RG estimates. River discharge simulations, reported in Fig. 12.d, are satisfactory, considering all the 3 input data. NSE values obtained respectively from RGs, CMLs, and RGs+CMLs data are 0.86, 0.77, and 0.80.

As the hydrological model has been calibrated with RG-detected rainfall data, it can be ~~easily guessed~~ assumed that the best model performances are mostly achieved with RG data as input, as well. ~~In fact, the major drawback of the present work is~~
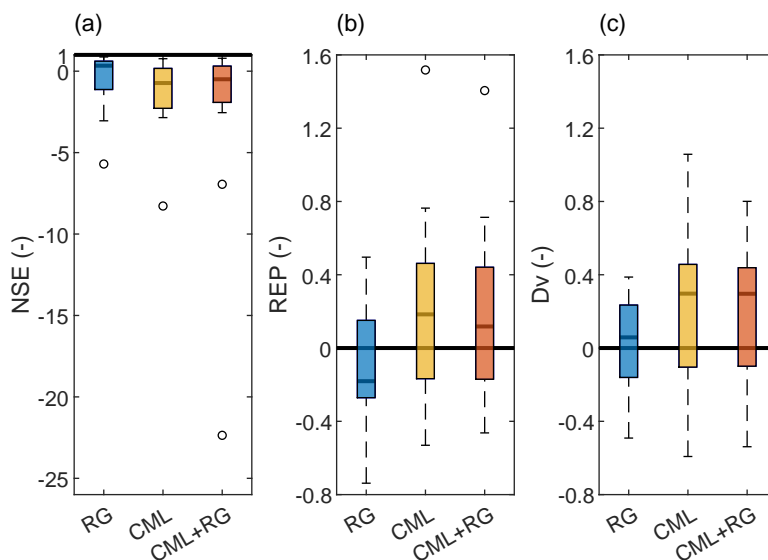
Figure 10. From panel ~~(a)~~ to ~~(c)~~, box plots of performance metrics, namely NSE, REP, and Dv, for the 12 selected flood events. The optimum values correspond to the bold black lines. The ranges of the box plots are those reported in Fig. 8.

~~definitely that~~ Unfortunately, we did not ~~rely on a large and real-time~~ have at our disposal a database of CML events large enough to carry out a CML-based ~~dataset and a calibration similar to that implemented with RG data cannot be carried out calibration~~. Nevertheless, we tried to overcome this problem, by recalibrating model parameters, with CML and CML+RG rainfall estimates as input, relying on the 12 available flood events. The same event-based calibration, was hence conducted using RG data as rainfall inputs. In such a way the comparison on discharge simulations may be led in a fair manner. We considered as optimum ~~parameter values~~ parameters those providing the highest median NSE values. Performance indices, subdivided by type of rainfall data input and type of calibration, are summarized in Fig. 13 by box plots. Please note that CML- and CML+RG-based ~~calibration improves~~ calibrations improve the performance of the model when fed by unconventional input data. In particular, NSE values are comparable with ~~those~~ the ones achieved by the use of RG data with a RG-based calibration. In fact, median NSE values for RG inputs with RG-based calibration, CML inputs with CML-based calibration, and CML+RG inputs with CML+RG-based calibration are ~~0.34~~0.37, 0.35, and 0.38, respectively. For REP values we generally observe underestimations of ~~the~~ observed peak flow, considering CML- and CML+RG calibration but ~~smaller IQR~~ a smaller interquartile range when compared with RG-based calibration. Concerning D_v values, performances for the CML- and CML+RG-based calibration are quite satisfactory, despite the combination providing the best performance is still RG inputs with RG-based calibration.
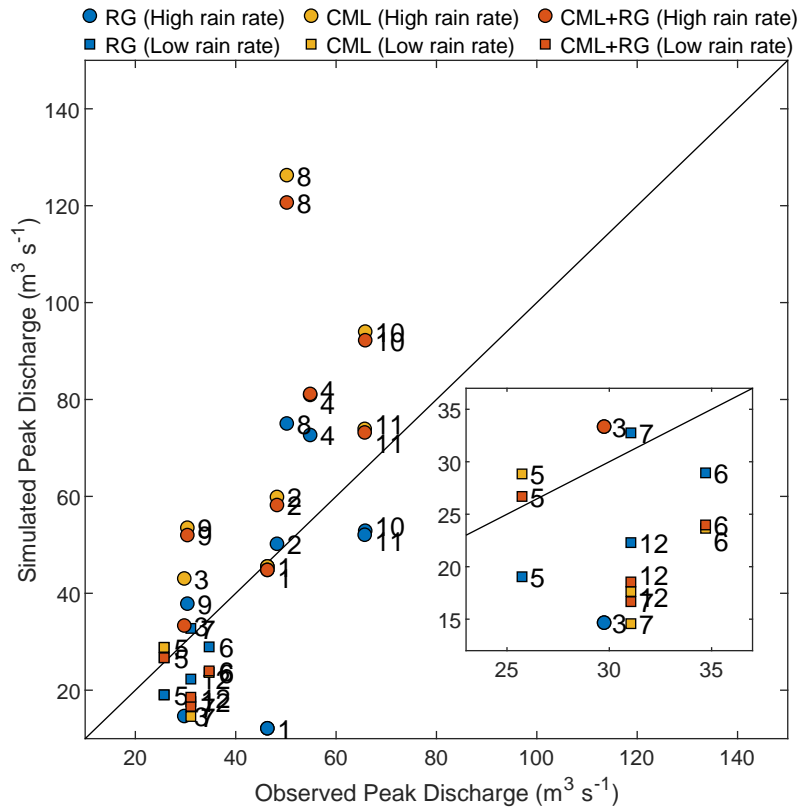
Figure 11. Observed peak flow against simulated one from RG (blue), CML (yellow), and CML+RG (orange). The two different markers, circles and squares, respectively stand for *high rain rate* and *low rain rate* events. Inset figure reports a zoom for events with a low peak discharge.
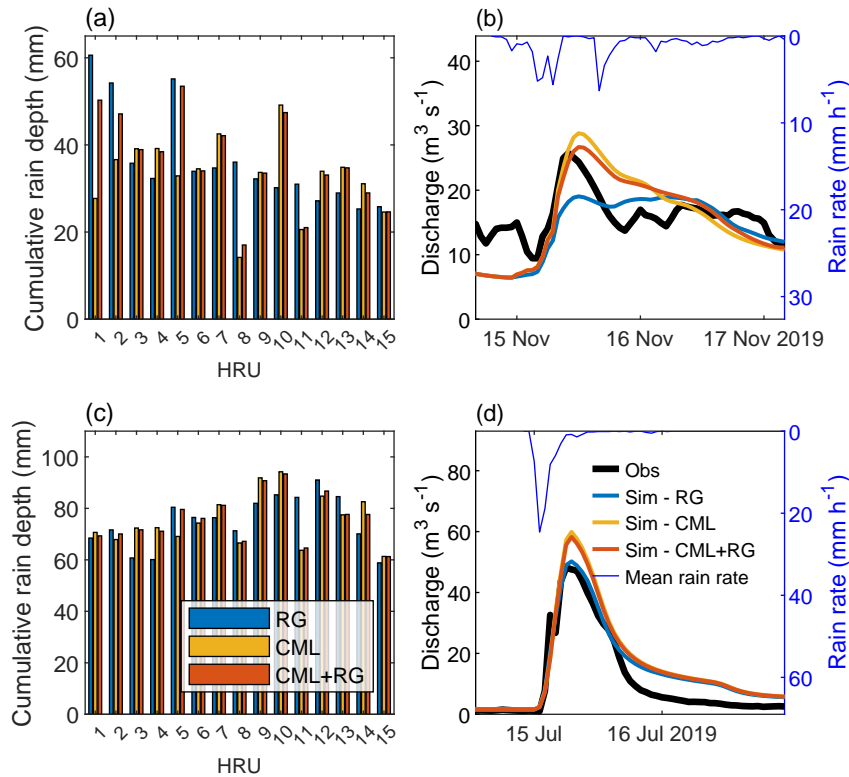
Figure 12. ~~Observed peak discharge against simulated one~~ In panel a and c, cumulative rainfall depth estimates from RG~~(blue)~~, CML~~(yellow)~~, and CML+RG ~~(orange). The two different markers, circles~~ for storm/flood events 5 and ~~squares~~2, respectively~~stand for *High rain rate* and *Low rain rate* events. Inset figure reports a zoom~~ In panel b and d discharge observations and simulations gathered by using the three different input data, for the two same events~~with a low peak discharge~~.

~~In panel (a) and (c), cumulative rain depth estimates from RG, CML, and CML+RG for storm/flood events 5 and 2, respectively. In panel (b) and (d) discharge observations and simulations gathered by using the three different input data, for the two same events.~~
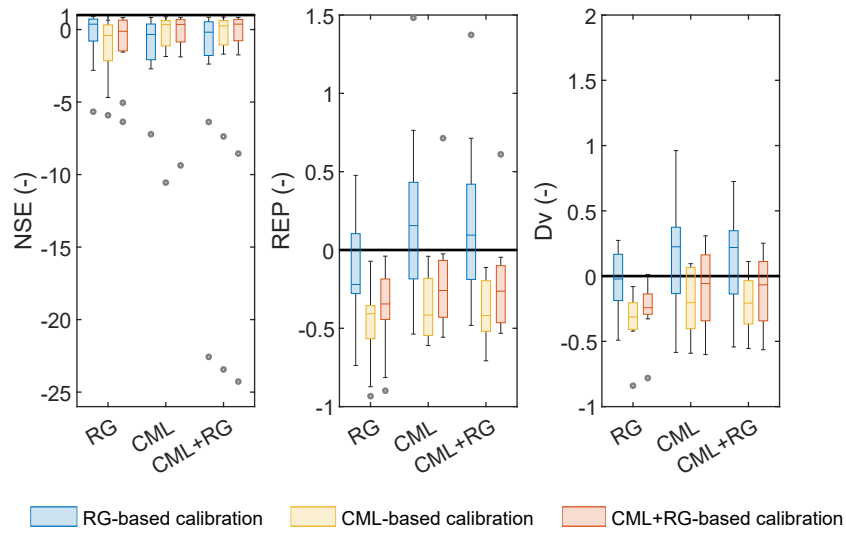
Figure 13. Box plots of the performance indices for the 12 flood events obtained with three different calibration sets (drawn in as many colours) and with three different input types (on the x-axis). The ranges of the box plots are those reported in Fig. 8.

5 Discussion

The analysis ~~on~~ of interpolated rainfall data carried out in Sec. 4.1 reveals that CML and RG estimates of accumulated areal-averaged rainfall depths are comparable. However, two issues emerged.

First, CMLs exhibit a different behaviour depending on event ~~'s~~ intensity, since their sensitivity varies with length and frequency. Specifically, they return lower values of rainfall depth and rainfall accumulation in correspondence of ~~Low~~ low *rain rate* events~~, both at the basin and~~. This aspect becomes evident either at different spatial scales (sub-basin ~~scale~~and basin scales) and at different temporal scales (hourly and event-based time scales). In fact, due to a coarse quantization of the raw data, CMLs are not sensitive to low rain intensity, hence, when the rainfall depth value over a certain lapse of time is required (in our case 1 h), this limitation may lead to large errors, especially when light rain goes on for a long time. ~~We instead~~ Despite some discrepancies in the behaviour of single CMLs with respect to their nearby RGs, as highlighted in Appendix A, we observed a good agreement between CML- and RG-based estimates in HRU centroids for high rain rates ~~. The problem of CML underestimation mainly involves northern HRUs (specifically HRUs 1, 2, 5, and 11), since are covered by less sensitive CMLs. However, we observed that these underestimations do not strongly impact the river discharge simulations since the northernmost HRUs influence less the runoff generation in the outlet section.~~ due to a mitigation effect.

The second issue is the different CML density over the HRUs. It is well known that spatial interpolation methods are sensitive to ~~sensors'~~ sensor density (Xu et al., 2013), and consequently the relatively large distance of the available CMLs from the HRU centroids in the most scarcely populated areas (northern HRUs) may lead to loss of reliability of estimated rainfall depths. However, such an aspect appears to be less relevant when compared with the first one, at least when dealing with quite large HRUs (dozen of km^2). In fact, a careful comparison of Fig. ~~-~~2 and Fig. ~~-~~9 does not show an evident correlation between the mean CMLs-centroids distance and ΔE values.

Last but not least, model performances are influenced by the calibration process of model parameters. Similar model performances, in terms of NSE index, can be achieved with all the three types of input data (RGs, CMLs, and CMLs+RGs) if the calibration is carried out with the respective data inputs. This means that, after a proper calibration, opportunistic sensors could be exploited in semi-distributed hydrological models, as well as RGs. In particular we found that, after calibration, the set RGs+CMLs is the one providing the highest median NSE. However, it is worth ~~to highlight~~ highlighting that we calibrated the parameters on the basis of only 12 flood events. In order to assess a robust calibration and the associated validation, a larger ~~dataset~~ data set of CML-based rainfall events should be processed.

Limitations and future improvements

One of the major difficulties encountered during analyses was the small amount of CML data, as we relied on only 458 hours of CML raw data grouped in 12 events. On the other hand, the database from RG observations was much more wider and we disposed of real-time data. An extension of the CML-based data set of events, or better yet, to have access to real-time CML raw data would definitely bring great benefits to the present work. Firstly, it would allow the development of a more

robust statistical analysis on storm/flood events. Secondly, it would enable a proper calibration, and a validation as well, of the hydrological model based on CML data as rainfall input.

525   To enhance this work, it would also be useful to resort to the implementation of a CML-driven distributed model, which is expected to provide a more accurate description of the spatial variability of the precipitation field with respect to a semi-distributed one. In such a case, the CML measurements would be better exploited by the use of advanced methods for spatial reconstruction of the rainfall field. For instance, techniques such as the tomographic reconstruction algorithm (D'Amico et al., 2016) or the stochastic reconstruction based on copulas (Haese et al., 2017; Salvadori et al., 2007), take advantage of the path integrated

530   nature of CML measurements.

It is also worth to notice that, although we showed that CML rainfall data can be successfully assimilated into hydrological models, their integration into real-time operational platforms (e.g. early warning systems) remains challenging. A number of aspects should be still considered including:

– generation of CML raw data formats suitable for rainfall estimation;

535   – real-time collection of raw data, which should be transparent to network operation;

– data transfer to a control center;

– non trivial data reduction process, especially if large sets of CMLs are managed.

The above mentioned issues suggest a systematic cooperation with mobile operators, who are the owners of CML network infrastructure.

540   Up to now, we mainly investigated the exploitation of CML-based rainfall estimates with the purpose to test their impact on the hydrological simulations of river discharge, with respect to the use of RG data. However, still important hydrological issues could be addressed by dealing with CML data. One of this is definitely the modelling of the Areal Reduction Factor (ARF), the factor which transforms a point rainfall for a given duration and return period into the areal average value for the same duration and return period (Natural Environmental Research Council (NERC), 1975). In last decades, great efforts have been put for the

545   modelling of the ARF (De Michele et al., 2001), useful in the design of hydraulic and hydrological infrastructures, for flood risk evaluations, and rainfall threshold estimations in early warning systems (e.g., Kim et al., 2019; Biondi et al., 2021). As we dealt with a semi-distributed hydrological model we needed to transform point (from RG) and linear (from CML) precipitation measurements into areal values, over the HRU areas. Therefore, from a different perspective, this work could be also seen as a first step in order to test the modelling of ARF by using a combination of conventional and unconventional sensors.

550   **6 Conclusions**

In this work, we assessed the use of ~~CMLs~~ commercial microwave links (CMLs) as opportunistic rainfall sensors within the hydrological modelling. We focused on Lambro, a peri-urban catchment, 260 km^2 in area, located north of Milan (Italy) and covered by 50 CMLs that are part of the network owned by a major mobile operator. Lambro's area is covered by 13 ~~RGs~~

rain gauges (RGs) as well, which we used both as an independent rainfall ~~dataset~~ data set and in combination with CMLs.
We implemented a semi-distributed hydrological model and carried out two types of comparison between CML and RG data. First, we considered rainfall data (hourly rainfall depths, the input of the hydrological model, and total accumulations at the storm end, for a sample of 12 storm events) interpolated at the HRU centroids. Then, we compared river discharge simulations (model output) from RGs, CMLs and RGs+CMLs against flow measurements.

Concerning the comparison on rainfall data, we found out that high intensity events detected by CMLs are in accordance with RG measurements. On the other hand we came across a critical aspect, which is the inability of CMLs to detect low rain rates, due to the coarse 1 dB quantization step of raw data (i.e. received power levels). The minimum detectable rainfall intensity depends on the operation frequency of CMLs as well as on their length, and, for the available set of CMLs, it ranges from ~~3~~ 1 mm h^{-1} to 10 mm h^{-1}. Such a limitation results in the underestimation of ~~rain~~ rainfall depths interpolated in the HRU centroids for low intensity storm events, when compared ~~from RG based~~ to RG-based rainfall data.

The hydrographs simulated by the hydrological model highlight better performances in terms of Nash-Sutcliffe efficiency, NSE~~and~~, and the relative error on flow volume, Dv~~metrics~~, in the case of RGs rather than of CMLs. This result is not surprising as the model was calibrated using RG data throughout one year data. Nevertheless, satisfactory ~~REP values~~ values of relative error on peak discharge, REP, are achieved through the use of CML and CML+RG data as inputs into the RG-based calibrated model. ~~We notably observed that the underestimating behaviour of CMLs during low intensity events does not impact much discharge simulations, in fact peak discharges simulated from CMLs are not systematically lower than RG-driven simulations. This happening since the least sensitive CMLs are located over the norther HRUs.~~

By calibrating the model with CML data and by using the same as input, it is possible to improve the model performance, which becomes comparable with the case of RG-calibration and RG-input. Even a slightly better performance can be gathered with a CML+RG-based calibration and CML+RG data as input.

~~An extension of the CML-based dataset of events would bring great benefits to the present work. Firstly, it would allow the development of a more robust statistical analysis on storm/flood events.Secondly, itwould enable a proper calibration and validation of the hydrological model based on CML dataas rainfall input.~~

~~To enhance this work, it would also be useful to implement a CML-driven distributed model, which is expected to provide a more accurate description of the spatial variability of the precipitation field with respect to a semi-distributed one. In such a case, the CML measurements would be better exploited by the use of advanced methods for spatial reconstruction of the rainfall field. For instance, techniques as the tomographic reconstruction algorithm (D'Amico et al., 2016) or the stochastic reconstruction based on copulas (Haese et al., 2017; Salvadori et al., 2007), take advantage of the path integrated nature of CML measurements.~~

~~Finally, it is worth to notice that , although we showed that CML rainfall data can be successfully assimilated into hydrological models, their integration into real-time operational platforms (e. g. early warning systems)remains challenging. A number of aspects should be considered including:~~

~~– generation of CML raw data formats suitable for rainfall estimation;~~

Author contributions. C.D.M., R.N., M.D.A., A.G. made the conceptualization; R.N. handled CML data; G.C. handled gauge data, G.C., C.D., C.D.M. developed the hydrological model; G.C. made the model calibrations and validations; G.C. prepared Figs. 1-2, 5-13, and A1;
595 R.N. prepared Figs.3 and 4; G.C. wrote the first draft of the manuscript; all the authors reviewed the manuscript.

Competing interests. All authors declare that they have no conflicts of interest.

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Appendix A: Local comparison between CML and RG rainfall time series

600 Rainfall amounts collected from CMLs and RGs are here compared through an analysis of the corresponding time series. To this aim, we selected four CMLs having at least one RG within 5 km, as done for the scatter plots of Fig. 4 and we plotted the CMLs and RGs time series of rainfall intensity and cumulative rainfall depth during the storm events 7, 8, 9 and 10 of Table 1. Please note that rainfall intensity is obtained from slightly different resolution times, i.e. 15 min for CMLs and 10 min for RGs, respectively. Fig. A1 shows the results. The differences between individual CMLs and nearby RGs are not surprising,
605 due to three main factors: (1) the different nature of the sensors, (2) the CMLs were not calibrated using other rainfall sensors as weather radars, and (3) the relative position between CML and RG. What is mostly evident are the differences between the low (event 7) and the high intensity events (events 8, 9, and 10). In the former case, the CMLs miss most of rainfall occurrences, causing a large underestimation of the rainfall accumulated at the end of the event. During event 7, RGs detected rain intensities from 1 up to 6 mmh^{-1}. The last value is approximately the minimum detectable rain intensity for the CML
610 with lowest frequencies (in Fig. A1.a). However, it is worth to underline that such an underestimating behaviour is observable not only for short and low-frequency CMLs but also for the most sensitive ones. In fact, large underestimations are reported in panels k, l, and q which are related to links with minimum detectable rain intensity equal to 1.6, 1.4, and 1 mmh^{-1}, respectively. This systematic underestimating behaviour also impact estimates of rainfall depths at the basin and sub-basin scale, as shown by results in Sec. 4.1. The three *high rain rate* events highlight different behaviours depending on the considered CML and its

615 relative location respect to the RGs. The short and low frequency CML, given in panel a, shows quite large discrepancy with its nearby RG, regarding to either the peaks timing or the total observed rainfall depth, and reveals both underestimating and overestimating behaviours (see panels c-e of Fig. A1). The performances of the two medium-length and medium-frequency CMLs (second row panels h-j and third row panels m-o of Fig. A1) are in mutual agreement and are definitely better with respect to the case shown in the first row (see panels c-e of Fig. A1). Specifically, it can be noticed that in most of the cases

620 these two CMLs well reproduce the highest peaks observed by the closest RGs, which are also those located right next to their middle point. However, they show some discrepancies (lower values with respect to RGs) as the rain rate decreases. This behaviour is particularly evident in panels i and n, for event 9. Finally, the highest-frequency CML (fourth row in Fig. A1) exhibits different performances during the three *high rain rate* events (panels r-t). For example, at odds with the previous cases, in event 10, the CML tends to overestimate the lowest rain rates, leading to a large overestimation (up to 60%) of the

625 cumulated rainfall depth. In this case, the differences between CML and RGs could be also due to the not-optimal relative location between CML and RGs. Results reported in Sec. 4.1 show that interpolating several CML data at HRU centroids mitigates the inaccuracy of individual CMLs and leads to acceptable estimates of the flow except in the case of low intensity events due to their limited sensitivity.
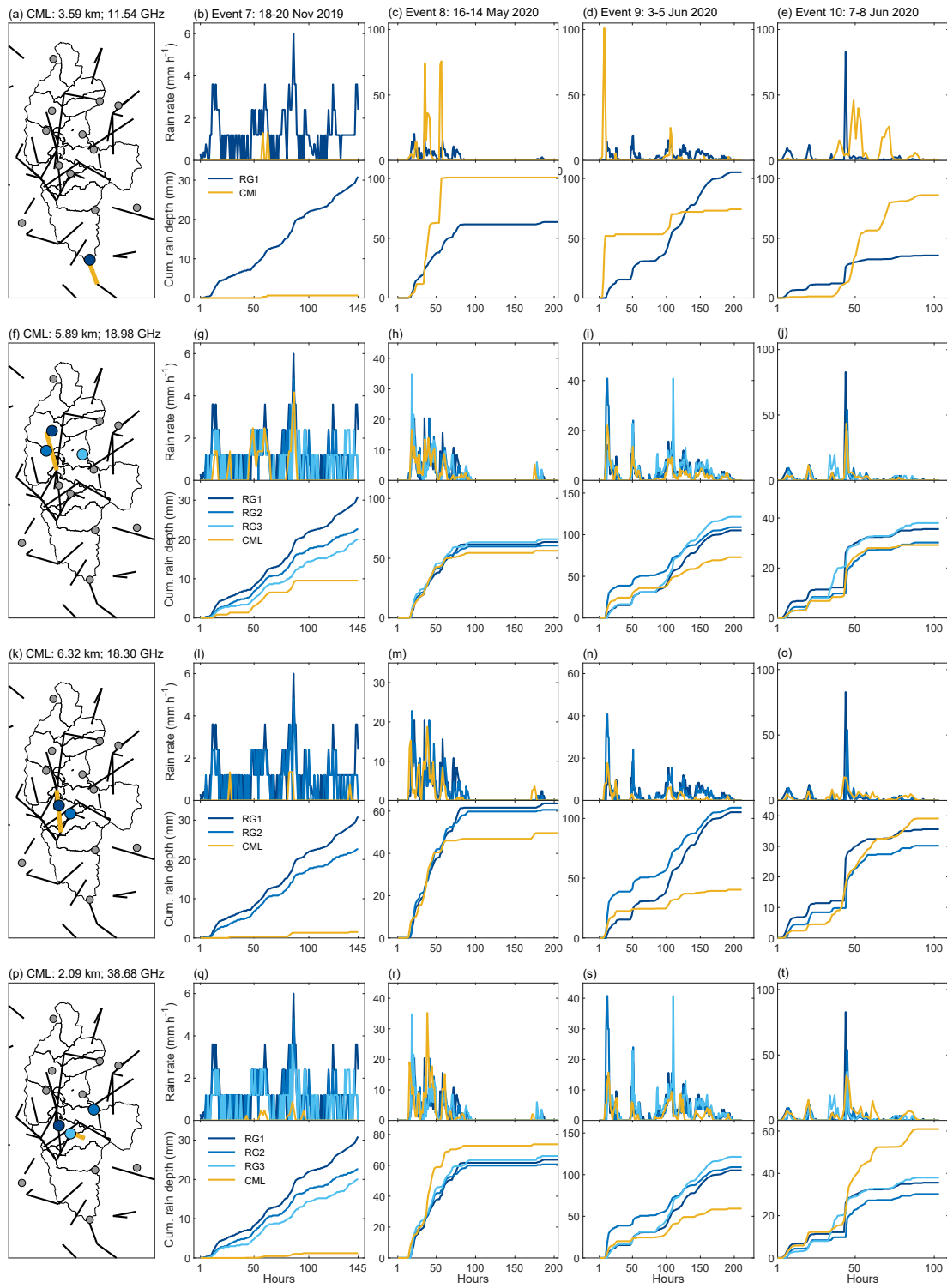
Figure A1. Comparison between single CML and nearby RGs. Each row shows (1) the location over the Lambro basin of the selected CML and its nearby RGs, (2) the CML-RGs comparison on rain rate time series, and (3) the CML-RGs comparison on cumulated rainfall depths.

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