In-situ estimation of subsurface hydro-geomechanical properties using the groundwater response to semi-diurnal Earth and atmospheric tides

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Abstract. Subsurface hydro-geomechanical properties crucially underpin the management of Earth's resources, yet they are predominantly measured on core-samples in the laboratory while little is known about the representativeness of in-situ conditions. The impact of Earth and atmospheric tides on borehole water levels are ubiquitous and can be used to characterise the subsurface. We illustrate that disentangling the groundwater response to Earth ($M2$) and atmospheric tidal ($S2$) forces in conjunction with hydraulic and linear poroelastic theories leads to a complete determination of the whole hydro-geomechanical parameter space for unconsolidated systems. Further, the characterisation of consolidated systems is possible when using literature estimates of the grain compressibility. While previous field investigations have assumed a Poisson's ratio from literature values, our new approach allows for its estimation under in-situ field conditions. We apply this method to water level and barometric pressure records from four field sites with contrasting hydrogeology. Estimated hydro-geomechanical properties (e.g. specific storage, hydraulic conductivity, porosity, shear-, Young's- and bulk- moduli, Skempton's and Biot-Willis coefficients and undrained/drained Poisson's ratios) are comparable to values reported in the literature, except for consistently negative drained Poisson's ratios which are surprising. Our results reveal an anisotropic response to strain, which is expected for a heterogeneous (layered) lithological profile. Closer analysis reveals that negative Poisson's ratios can be explained by differing in-situ conditions to those from typical laboratory core tests and the small strains generated by Earth and atmospheric tides. Our new approach can be used to passively, and therefore cost-effectively, estimate subsurface hydro-geomechanical properties representative of in-situ conditions. Our method can be used to improve our understanding of the relationship between geological heterogeneity and geomechanical behaviour.

1   Introduction

20   A perpetual challenge for subsurface water, mineral resource or geotechnical projects is a proper characterisation of the physical properties that may have bearings on the rate of resource extraction, operation, safety and environmental impact of the project. The main reason for this challenge is the subsurface's heterogeneous nature and that the sampling density necessary to describe it may be prohibitively expensive (e.g. by drilling and testing of core). This issue is further exacerbated by the difficulty in approximating in-situ environments in laboratory testing in regards to both scale and subsurface pressures (Hoek and

25   Diederichs, 2006; Cundall et al., 2008; Bouzalakos et al., 2016). These difficulties may be overcome by in-situ characterisation of hydro-geomechanical properties of the subsurface (Villeneuve et al., 2018). Here, the in-situ pressure, stress conditions, and the scaling and inclusion of heterogeneities can achieve a more representative estimate than possible from selective laboratory testing.

Detailed time-series analysis of water table fluctuations in boreholes due to Earth and atmospheric tides (EAT) has been

30   shown to be capable of deriving hydro-geomechanical properties (Hsieh et al., 1987; Rojstaczer and Agnew, 1989; Zhang et al., 2019). Further, with the assumption of key variables, previous authors have also been able to extend the use of EAT to estimate additional subsurface hydro-geomechanical properties (Bredehoeft, 1967; Beavan et al., 1991; Cutillo and Bredehoeft, 2011). However, methods which use EAT, referred to as tidal subsurface analysis (TSA) techniques, remains underutilised.

EAT are natural phenomena, causes by celestial bodies (e.g., Sun or Moon), that occur throughout the Earth's crust, which

35   have been measured and analysed in the subsurface since the mid-20th century (McMillan et al., 2019). Traditionally these techniques have been focused on either Earth tides (ET) (Bredehoeft, 1967; Hsieh et al., 1987; Cutillo and Bredehoeft, 2011; Zhang et al., 2019; Burbey, 2010), barometric pressure (Clark, 1967; Cutillo and Bredehoeft, 2011) or atmospheric tide (AT) loading (Acworth et al., 2016; McMillan et al., 2019; Rau et al., 2020) of the confined subsurface. Bredehoeft (1967) first proposed that once specific storage is calculated from the groundwater response to Earth tides, an aquifer porosity and com-

40   pressibility can be determined from the formation pressure response to a uniformly distributed surface load such as caused by barometric pressure changes (Narasimhan et al., 1984; Rojstaczer, 1988; Rojstaczer and Riley, 1990; Ritzi et al., 1991; Burbey et al., 2012). This concept has been reiterated in the literature but, to the best of our knowledge, never solved without the use of either an assumed Poisson's ratio or bulk modulus (Cutillo and Bredehoeft, 2011) due to difficulties in attributing the superimposed EAT effects to their appropriate drivers (e.g. celestial body gravitational or atmospheric loading forces). Recent

45   work ~~estimating~~ compares methods that estimate amplitudes and phases ~~using~~ of from monitoring datasets and concludes that *harmonic least squares* (HALS) ~~and synthetically predicted ETs has demonstrated that separating tidal components of very similar frequencies is now possible (Rau et al., 2020). This has opened opportunities to revisit existing methods to create a new integrated approach that remove the need to assume moduli and therefore instead derive the variables from the data.~~ is superior compared to *Fast Fourier Transforms* (FFT) (Rau et al., 2020).

50   In this paper, the theory of the groundwater response to Earth and atmospheric tides is combined, thereby providing a new methodology for the estimation of the primary subsurface hydrogeomechanical properties (storage, hydraulic conductivity, poroelastic properties). This new method improves upon the work of Cutillo and Bredehoeft (2011), as it quantitatively dis-

Tidal component (Darwinian name)	Frequency (cpd)	Tidal potential ($m^2\,s^{-2}$)	Tidal gravity variation ($m\,s^{-2}$)	Tidal dilatation / areal strain (-)	Description	Attribution
M_2	1.932274	42.060943	$6.477 \cdot 10^{-5}$	$2.625 \cdot 10^{-7}$	Principal lunar semi-diurnal	Earth
S_2	2.000000	19.309855	$2.973 \cdot 10^{-5}$	$1.205 \cdot 10^{-7}$	Principal solar semi-diurnal	Atmosphere/Earth

Table 1. Table of M_2 and S_2 tidal components, tidal potential, gravity and dilatation using tidal predictions (this does not include local variations). Extracted from Agnew (2010) and McMillan et al. (2019).

entangles the groundwater response to Earth and atmospheric tides within the frequency domain, removing non-harmonic signals (e.g. precipitation and episodic recharge events) and allowing the separate and objective estimation of properties from each driver before combining the strain responses. Here, the hydraulic and linear poroelastic works of Hsieh et al. (1987), Rojstaczer and Agnew (1989), Beavan et al. (1991) and Rau et al. (2020) are integrated and combined, leading to a complete determination of the parameter space for unconsolidated systems. Further, the characterisation of consolidated systems is possible when using literature estimates of the grain compressibility (van der Kamp and Gale, 1983; Green and Wang, 1990). Finally, the new methodology is applied to groundwater and atmospheric pressure records in five boreholes from four sites to estimate hydrogeological and geomechanical properties of various consolidated and unconsolidated stratigraphies.

2 Theoretical background

2.1 Extracting tidal components

Atmospheric heating and the gravitational pull of celestial bodies exert a loading of the Earth's crust (Agnew, 2010). The gravity variations and loading exerted by the movement of these celestial bodies (i.e., the Moon and Sun), as shown in Table 1, cause stress and strain responses in the Earth's crust. This causes a subsurface strain signal that is composed of numerous superimposed signals of various frequencies and amplitudes. For undrained conditions (~~pressurized~~pressurised) of either confined or semi-confined aquifers, this strain manifests as a groundwater pore pressure fluctuation (McMillan et al., 2019). A conceptual illustration of these processes is shown in Figure 1.

Three variables are required to calculate subsurface properties using specific harmonic components (McMillan et al., 2019): (1) a computed dilatation strain due to Earth tides (denoted by the superscript ET); (2) measured barometric pressure (denoted by the superscript AT in later equations); and (3) measured groundwater heads (denoted by the superscript GW). First, a moving average spanning across a time period of 3 days is applied. The tidally induced frequency components are then extracted by using *harmonic least-squares* (HALS) to estimate the harmonic components caused by tides whose frequencies are well known (Hsieh et al., 1987; Xue et al., 2016; Rau et al., 2020; Schweizer et al., 2021).The moving average acts like a high-pass
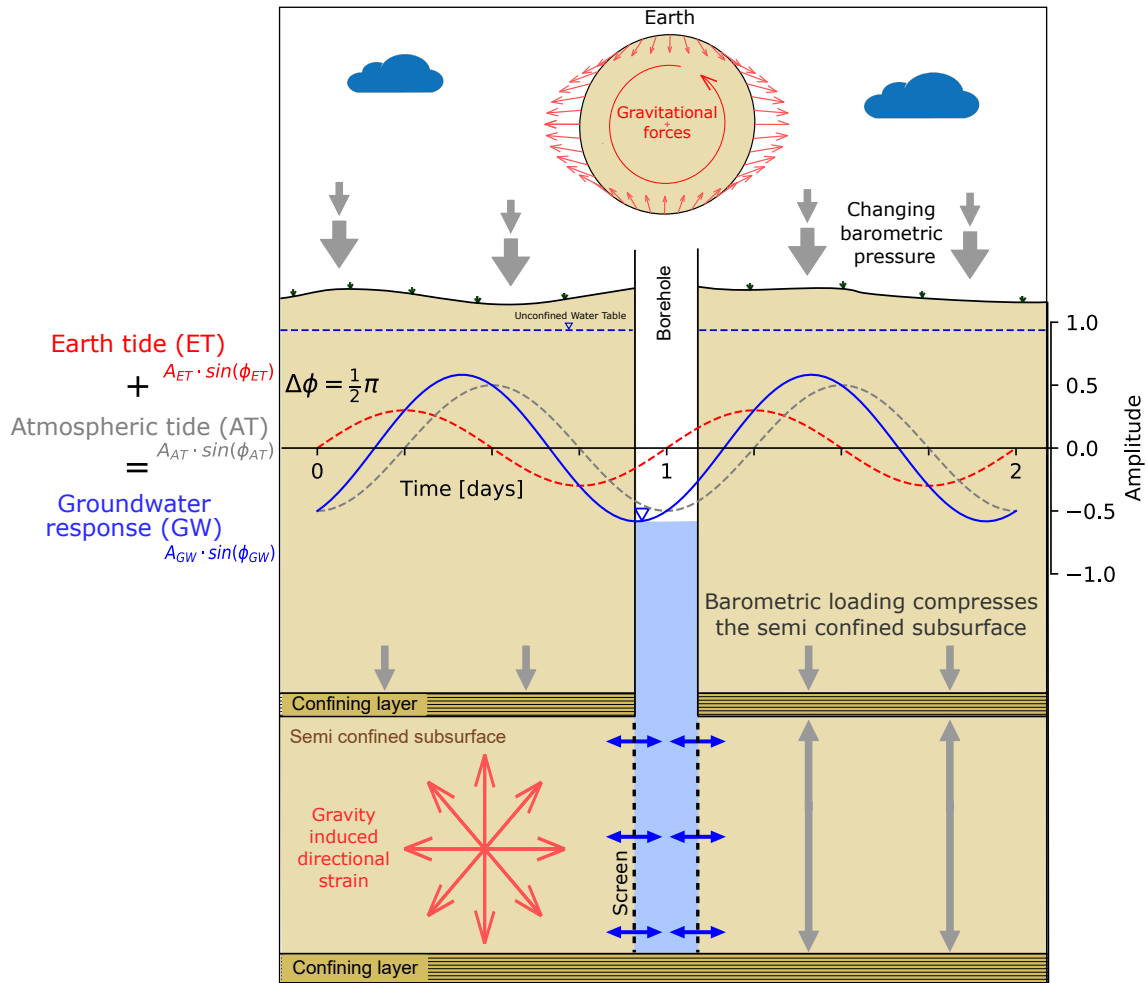
Figure 1. Representation of groundwater pressure head measured in a well penetrating a confined aquifer with a relatively rigid matrix subjected to ET (red) and AT (grey), adapted from (McMillan et al., 2019). The result of these two effects can be expressed as a function of harmonic addition within the groundwater level. Here, the gravity-induced directional strain and vertical barometric loading/unloading combine to force water into and out of the well.

75 filter and the extraction of the tidal components at specific frequencies means that ~~longer~~ higher frequency and episodic events (atmospheric pressure changes related to weather systems passing across a site, rainfall and recharge events, and recession from pumping or droughts) present in the groundwater level/pressure data is discarded and therefore of no consequence for the analysis. Further, since our analysis is exclusively valid for the semi-confined and confined subsurface, water movement in the overlying shallow vadose zone and unconfined saturated zone is irrelevant. The extracted frequency components are

80 complex numbers at discrete frequencies ($\hat{z}_{(f)}$, e.g. $\hat{z}_{M_2}$) for which amplitudes and phases can be calculated using the real and imaginary parts. The approach we use in our work (e.g. HALS) was comprehensively tested showing that it leads to accurate estimates of harmonics in the presence of noise levels not exceeding the measurement resolution (Schweizer et al., 2021). High quality pressure transducers generally fulfil this criteria (Rau et al., 2019).

2.2 Earth tide influences on well water levels

85 ### 2.2.1 Subsurface strain response to gravity changes

Rojstaczer and Agnew (1989) argued that for Earth tides horizontal areal strain is a sufficient approximation for depths of up to thousands of kilometres. This approximation is sufficient for application to groundwater resources as they are generally much shallower. The strain is often referred to as dilatation which is the total increase in volume of the material due to forcing by the Earth tides (in this case the tidal pull). In porous media, assuming incompressible grains, this dilatation is manifesting as

90 an opening of the total pore space, decreasing the water pressure within the material (Agnew, 2010). In this paper the term 'dilatation' is used broadly for both the dilation and compression due to the cyclical forcing of the tides, coherent with its previous literature use (Xue et al., 2016; Allègre et al., 2016). The distortions by dilatation can be estimated through the planar strain concept known as tidal dilatation (Schulze et al., 2000; Fuentes-Arreazola et al., 2018). Tidal dilatation can be defined as

95
$$e^t = \frac{V}{g} \cdot \frac{e^v - 3e^h}{R} \tag{1}$$

where e^t is the tidal dilatation strain (-), in this instance at the M_2 frequency, g is acceleration due to gravity ($\approx 9.81 m/s^2$), e^v is vertical displacement strain (-), e^h is horizontal displacement (-) (Agnew, 2010), R the average radius of the Earth (m) adjusted for any significant elevation and V is the tidal potential ($m^2 s^{-2}$) as defined in Table 1. The term ($e^v - 3e^h$) may also be approximated by Love-Shida numbers where e^v can be replaced by $^L_S h$ with an assumed value of 0.6032 and e^h

100 may be replaced with $^L_S l$ with an assumed value of 0.0839 (Agnew, 2010; Cutillo and Bredehoeft, 2011). Previous work has demonstrated the use of calculated strain for analysing the groundwater response to Earth tide forces Roeloffs (1996); Xue et al. (2016); Allègre et al. (2016); McMillan et al. (2019). As such, the terms e^v and e^h can be directly calculated from software that generates theoretical Earth tides or tidal dilatation strains based on geo-location, for example *ETERNA* (Wenzel, 1996), *TSoft* (Van Camp and Vauterin, 2005), or, as was done for this paper, using *PyGTide* (Rau, 2018) (based on ETERNA).

105 We note that these programs do not account for ocean tide loading, i.e., the deformation of the subsurface due to the weight of the ocean tides. Ocean tide loading causes harmonic subsurface strain that is added to that imposed by Earth tides. The actual

subsurface strain amplitude variation depends on the phase of both signals and is, in the worst case, either added or subtracted from the Earth tide. To understand the impact magnitude of this problem we used the ocean tide loading provider by Chalmers University of Technology (http://holt.oso.chalmers.se/loading/index.html) to estimate the M_2 aerial strain ($A_{M_2}^{OTe}$) for our five locations with the state-of-the-art finite element model FES2014b (Penna et al., 2008; Matviichuk et al., 2021). However, we further note that ocean tide loading is a complicated phenomenon (Jentzsch, 1997) and its detailed assessment is beyond the scope of this manuscript.

The first approach using ET to estimate specific storage, used the potential for water movement from the tides to the corresponding water movement in a monitoring well in a confined aquifer for undrained conditions. Here, Bredehoeft (1967) defined specific storage (S_s) for a medium that is presumed to be incompressible as

$$S_s = -\left[\left(\frac{1-2v}{1-v} \right) \left(\frac{2_S^L h - 6_S^L l}{R \cdot g} \right) \right] \frac{\Delta A_{M_2}^{ETp} \Delta A_{M_2}^{ET_{pot}}}{\Delta h \quad \Delta h}, \tag{2}$$

where $\Delta A_{M_2}^{ETp} \Delta A_{M_2}^{ET_{pot}}$ is the change in the tidal potential to the corresponding change in hydraulic head Δh and v is typically an assumed Poisson's ratio. Here, the tidal dilatation (Equation 1), has been incorporated into Equation 2. Equation 2 was then used by Cutillo and Bredehoeft (2011) for is advantages over methods such as provided by Hsieh et al. (1987), as it does not require the separation of individual tidal components, or the knowledge of a well's dimensions. Progressive improvements in the precision and duration of gravity measurement methods have since allowed for more accurate decomposition and cataloguing of the various tidal components (Agnew, 2010). These established catalogues of precise frequencies provide the basis for component separation using harmonic filtering techniques. The full separation of ET and AT at one frequency allows their individual and combined use towards better in-situ hydrogeomechanical characterisation (Rau et al., 2020).

2.2.2 Well water level response to harmonically forced pore pressure

In this paper, we focus on the ET component at the frequency of 1.932274 (cpd) cycles per day (denoted by a subscript of its Darwin name M_2) and the combined ET and AT component at the frequency of 2 cpd (denoted by a subscript of its Darwin name S_2), described in Table 1. While other frequency components can also be used (Hsieh et al., 1988; Merritt, 2004; Cutillo and Bredehoeft, 2011), M_2 and S_2 generally have the strongest tidal impact and their forces remains constant over time (Acworth et al., 2016; McMillan et al., 2019).

The relative amplitude response $(A_{M_2}^e)$ of the groundwater, as measured in a borehole in relation to the tidal dilatation strain can be expressed as (Hsieh et al., 1987; Xue et al., 2016; Allègre et al., 2016)

$$A_{M_2}^e = \left| \frac{\hat{z}_{M_2}^{GW}}{\hat{z}_{M_2}^{ETe}} \right| = \frac{A_{M_2}^{GW}}{A_{M_2}^{ETe}}, \tag{3}$$

where $\hat{z}_{M_2}^{GW}$ and $\hat{z}_{M_2}^{ETe}$ are the complex frequency component of the groundwater pressure head and tidal dilatation strain, respectively; $A_{M_2}^{GW}$ is the amplitude of the groundwater pressure head fluctuation and $A_{M_2}^{ETe} A_{M_2}^e$ is the amplitude of the tidal dilatation strain fluctuation, all at the frequency of the M_2 tidal component. Note that $A_{M_2}^e A_{M_2}^{ETe}$ is also referred to as areal strain sensitivity (Hsieh et al., 1987).

It is important to note the difference presented in Equation 3 from Xue et al. (2016) with the original dimensionless amplitude response calculated by Hsieh et al. (1987) as

$$A_{M_2} = \left| \frac{\hat{z}_{M_2}^{GW}}{\hat{z}_{M_2}^{p}} \right| = A^{eETe} M_2 S_s, \tag{4}$$

where $\hat{z}_{M_2}^{p}$ is the complex aquifer pore pressure response (superscript p reflects pore). In this equation, the denominator term has changed from the complex amplitude of the pressure fluctuation with the tidal dilatation, effectively incorporating Equation 2. This key difference allows for the addition of the term S_s within the amplitude response equations due to the sensitivity of storage to the amplitude response for ~~post- and pre-strain~~ confined and leaky responses described in Sections ~~?? and ??~~2.2.3 and 2.2.4. A_{M_2} from Equation 4 is dimensionless, with values $0 \leq A_{M_2} \leq 1$.

The phase shift (or difference) is defined as the strain response observed as the complex groundwater pressure head (water level) fluctuation, minus the phase of the complex tidal ~~dilation~~ dilatation (tidal forcing) stress, defined as

$$\Delta\phi_{M_2} = \arg\left(\frac{\hat{z}_{M_2}^{GW}}{\hat{z}_{M_2}^{ETe}} \right) = \phi_{M_2}^{GW} - \phi_{M_2}^{ETe}, \tag{5}$$

where $\phi_{M_2}^{GW}$ is the phase angle expressed in the groundwater response and $\phi_{M_2}^{ETe}$ is the phase angle of the theoretical Earth tide component, in this case at the frequency of the M_2. A negative phase shift is expressed where the groundwater response lags behind the induced strain (water level response lags behind the pressure head disturbance (Hsieh et al., 1987)), whereas a positive phase shift indicates the groundwater response is leading the strain response.

It should be noted that in this method development, a homogeneous, isotropic aquifer of infinite lateral extent is assumed for all derivations (Hsieh et al., 1987). All derived hydro-geomechanical variables are treated as bulk properties ~~(~~averaged over a distinct but unknown volume~~)~~, representative of the EAT area of influence around the monitoring wells screened interval, including effects from geological heterogeneities and the well construction, such as the inclusion of a gravel pack. The exact nature and dimensions of the volume of influence~~(~~, i.e., the volume of sub-surface around the well being 'sampled' ~~)~~ is currently unresolved. It is commonly assumed that the ET amplitude response is negligibly influenced by fluid flow when confined (Xue et al., 2016)~~; instead~~. Instead, it is predominantly controlled by the storage response. This is used as a justification to modify the first hydraulic diffusivity term in the amplitude response equations to $1/Ss$ when including the Earth tide strain estimation (Equations 6 and 13), i.e. the tidal dilatation (Hsieh et al., 1987; Wang, 2000; Xue et al., 2016).

2.2.3 ~~Post-strain~~ Confined water level response

Positive and negative phase shifts are either leading (~~pre-strain~~leaky) or lagging (~~post-strain~~confined), respectively, in relation to the strain response expressed by the water level in a well to formation tidal forcing. Hsieh et al. (1987) provided an analytical solution for the confined groundwater flow equation with harmonic forcing to describe the relationship between aquifer pore pressure and well water level. Their model assumes horizontal flow only and is formulated in terms of amplitude ratio and phase shift, thereby allowing for the solution of two properties, transmissivity and storativity from the amplitude and phase response. ~~This model works by exploiting the lack of sensitivity to storage within the phase shift equation and iterates to fit~~

for both transmissivity and storage (See Figure 2) (Rau et al., 2020). The post-strain The confined (negative phase) model is defined by Hsieh et al. (1988) as

$$A^{eETe}_{M_2} = \frac{1}{S_s}(E^2 + F^2)^{-\frac{1}{2}} \tag{6}$$

and

$$\Delta\phi_{M_2} = -\tan^{-1}\left(\frac{F}{E}\right) \tag{7}$$

where

$$E = 1 - \frac{\omega r_c^2}{2T}[\Psi Ker(\alpha_w) + \psi Kei(\alpha_w)] \tag{8}$$

and

$$F = \frac{\omega r_c^2}{2T}[\psi Ker(\alpha_w) - \Psi Kei(\alpha_w)] \tag{9}$$

and

$$\Psi = \frac{-[Ker_1(\alpha_w) - Kei_1(\alpha_w)]}{2^{\frac{1}{2}}\alpha_w[Ker_1^2(\alpha_w) + Kei_1^2(\alpha_w)} \tag{10}$$

and

$$\psi = \frac{-[Ker_1(\alpha_w) + Kei_1(\alpha_w)]}{2^{\frac{1}{2}}\alpha_w[Ker_1^2(\alpha_w) + Kei_1^2(\alpha_w)]} \tag{11}$$

where

$$\alpha_w = r_w\sqrt{\frac{\omega S}{T}} = r_w\sqrt{\frac{\omega}{D_h}}. \tag{12}$$

The storativity S and transmissivity T can be related to specific storage as $S = S_s b$ and hydraulic conductivity as $T = k^f b$, respectively; b is the aquifer thickness, here is related to the vertical screen length when the aquifer thickness is unknown; r_w is the internal radius of the well screen (accounts for well storage); r_c is the radius of the casing. Ker and Kei are Kelvin functions of zero order, and Ker_1 and Kei_1 are Kelvin functions of the first order. Figure 2a,b show the amplitude and phase solution space when considering the strain response as well as separation of hydraulic properties, respectively.

2.2.4 Pre-strain Leaky water level response

The pre-strain

The leaky water level model is based on the description of a periodic load on a half-space, as described by Wang (2000), and is used for Earth tides where a vertical head gradient exist (Xue et al., 2016; Allègre et al., 2016) pressure propagation and flow exist (Xue et al., 2016; Allègre et al., 2016). The Equations 13 and 14 were derived from the force equilibrium equations (refer to Wang (2000))

$$A^{eETe}_{M_2} = \frac{1}{S_s}\sqrt{1 - 2\exp\left(-\frac{z}{\delta}\right)\cos\left(\frac{z}{\delta}\right) + \exp\left(-2\frac{z}{\delta}\right)}, \tag{13}$$
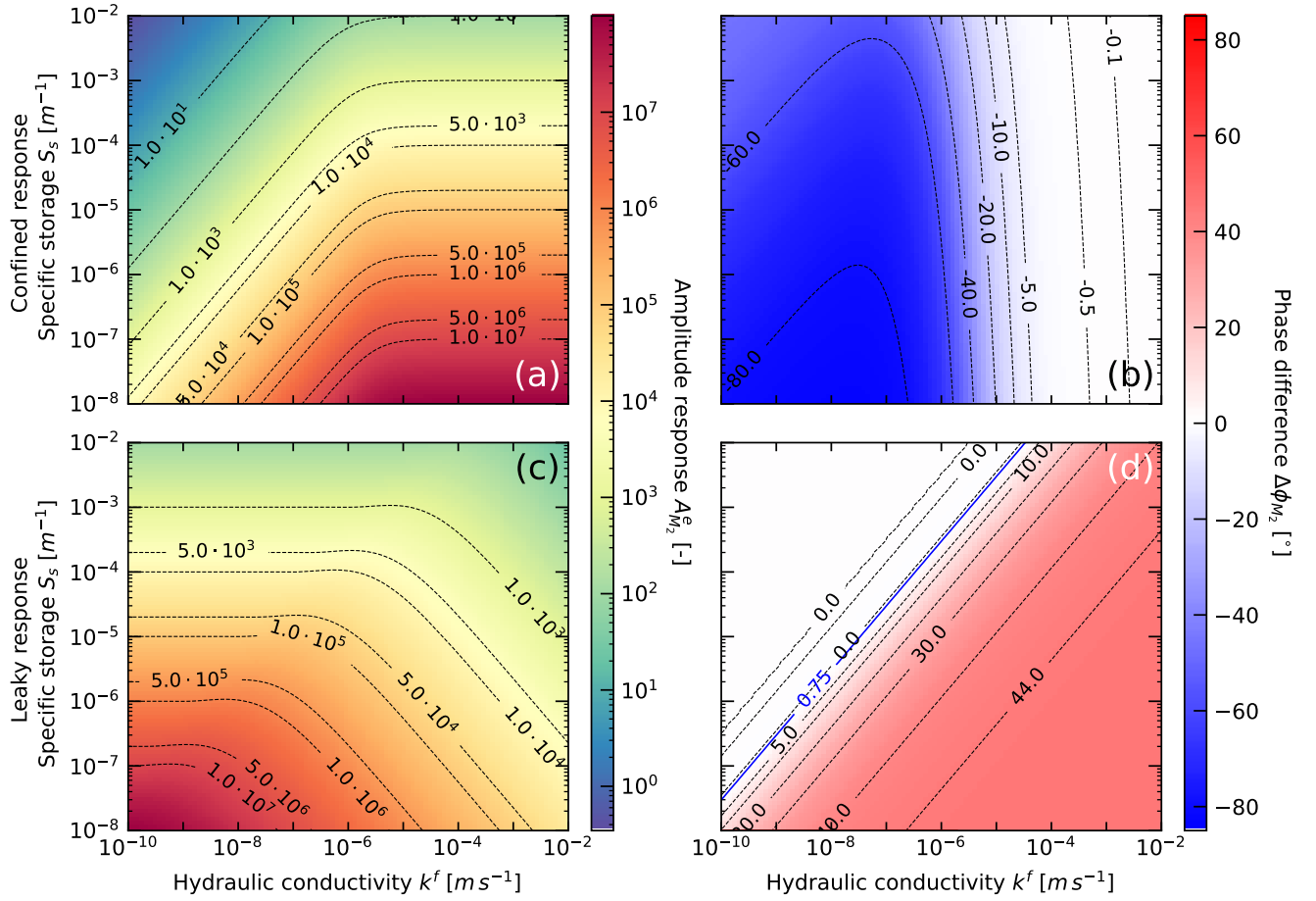
Figure 2. Pressure head amplitude and phase response to the Earth tide M_2 component as a function of ranges in hydraulic conductivity and specific storage: (a) amplitude (Equation 6) and (b) phase (Equation 7) response for confined conditions (here; the radius of borehole and screen are 0.1 m and the screen length is 2 m). (c) amplitude (Equation 13) and (d) phase (Equation 14) response for semi-confined conditions where vertical flow may exist (depth of screen is 20 m).

and

$$\Delta\phi_{M_2} = \tan^{-1}\left[\frac{\exp\left(-\frac{z}{\delta}\right)\sin\left(\frac{z}{\delta}\right)}{1-\exp\left(-\frac{z}{\delta}\right)\cos\left(\frac{z}{\delta}\right)}\right], \tag{14}$$

where z is depth of the midpoint open screen interval, ω is the angular frequency of the tidal component (M_2),

$$\delta = \sqrt{\frac{2D_h}{\omega}}, \tag{15}$$

200 and D_h is then the hydraulic diffusivity, defined as

$$D_h = \frac{T}{S} = \frac{k^f}{S_s} = \frac{k}{\mu S} = \frac{\rho_w g k^f}{\mu S_s^p} \tag{16}$$

where T is subsurface transmissivity ($T = k^f b$), k is permeability, k^f is hydraulic conductivity, ρ_w is the density of water (0.9982 kg/L at 20°C) and μ is the dynamic viscosity of water (8.90 · 10^{-4} Pa s), S is storativity and S_s^p is specific storage (1/Pa). Equations 13 and 14 require iterative solving for D_h and S_s.

205 ~~Both Equation~~ Figure 3 shows the solution space for Equations 13 and 14~~were developed for harmonic loading (i. e. ocean or barometric loading) where strain is produced at the surface of the Earth's crust and propagated down (Wang and Davis, 1996). ET (tidal dilatation) on the other hand, manifests within the subsurface where the stress is depth independent. Close attention is therefore required for the effect of depth when analysing combined ET and AT forcing effects (rather than just aloading), ensuring that the sensitivity to depth has adequately attenuated (e.g. deeper than 10 m), as shown in Figure 3~~. It is noteworthy

210 that the amplitude can be attenuated significantly for high hydraulic diffusivities and small depths. Overall, confined conditions should be given for low hydraulic diffusivities and larger depths where the amplitude ratio is approx. 1. More semi-confined conditions are indicated by positive phase shifts. Certain conditions can amplify amplitudes by a factor of approx. 1.07 (Figure 3a) for which phase shifts can become slightly negative (Figure 3b). Figure 2c,d show the solution space when considering the strain response as well as separation of hydraulic properties.
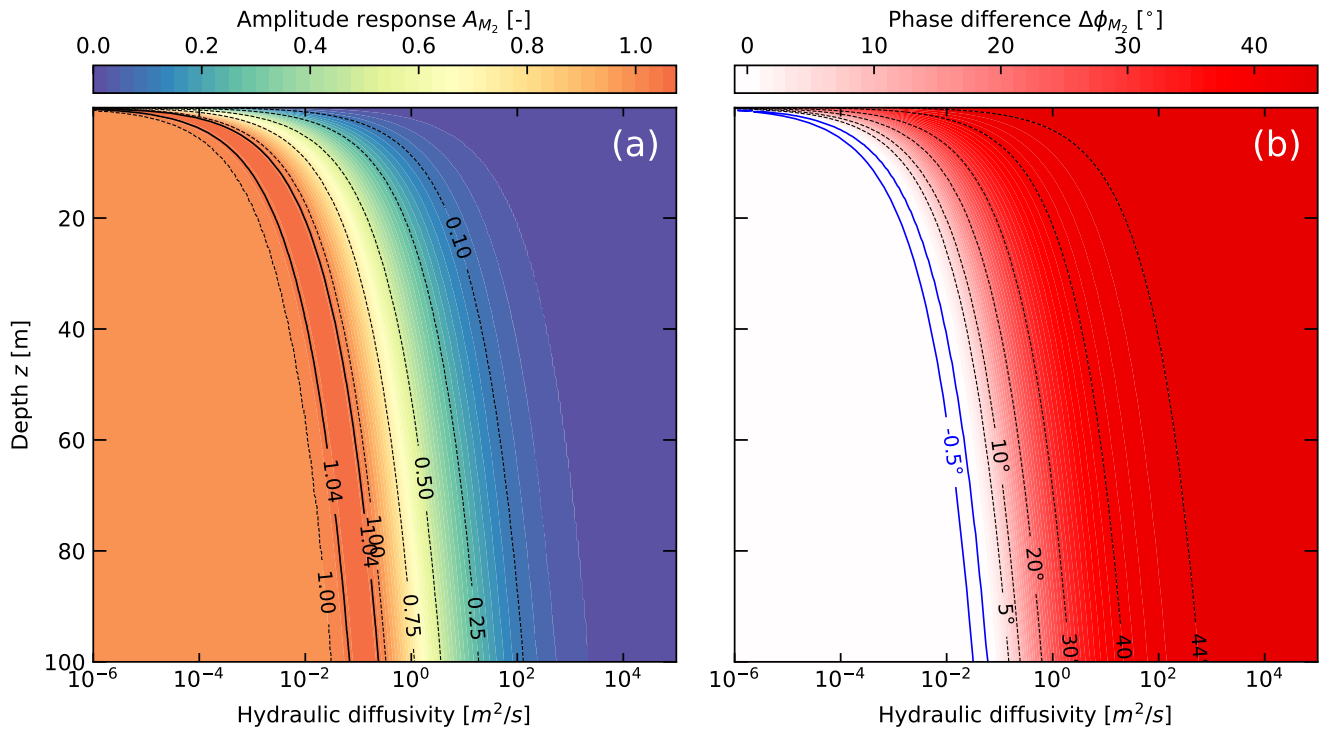
Figure 3. Periodic loading of a half space (applied to ET) as modeled by Equations 13 and 14. (a) Normalized relative amplitude response and hydraulic diffusivity as a function of depth (Equation 13). (b) Phase response and hydraulic diffusivity as a function of depth (Equation 14) (Wang, 2000).

2.2.5 Distinguishing between ~~pre-~~ leaky and ~~post-strain~~ confined conditions

The sets of Equations 6 and 7 (Hsieh et al., 1987) describe lateral water movement between the well-bore and surrounding subsurface, whereas Equations 13 and 14 (Wang, 2000) explain the positive phase shift by allowing vertical flow within the groundwater system. Both sets of equations have been used to estimate hydraulic conductivity and specific storage. This is achieved by decomposing the hydraulic diffusivity using the assumptions outlined at the end of Section 2.2.2.

The phase shift determines which of these sets of analytical solutions are appropriate. For a phase ~~between $0°$ to $-45°$ the post-strain~~ shifts $-45 \lesssim \Delta\Phi_{M_2} < 0$ the confined response model is used, and for a phase ~~between $-1°$ and $90°$ the pre-strain~~ shifts between $0 \lesssim \Delta\Phi_{M_2} \lesssim 90°$ the leaky response model is applied (both are visualised in Figure 2). Note that the ~~pre-strain~~ leaky model results in a slight negative phase shift for certain parameter ranges. Consequently, there is a range of ambiguity between phase shift values between $-1°$ to $0°$ in which both sets of solutions should be used, and the most physically plausible results should be selected (Xue et al., 2016). Note, the unit input as either pressure or hydraulic head will also be carried through the equations resulting in a unit difference where S_s^p is specific storage as pressure ($1/Pa$) whereas S_s is specific storage as a reciprocal meter length ($1/m$), as demonstrated in ~~equation~~ Equation 16.

~~Pressure head amplitude and phase response to the Earth tide M_2 component as a function of ranges in hydraulic conductivity and specific storage: (a) amplitude (Equation 6) and (b) phase (Equation 7) response for confined conditions (here; the radius of borehole and screen are 0.1 m and the screen length is 2 m). (c) amplitude (Equation 13) and (d) phase (Equation 14) response for semi-confined conditions where vertical flow may exist (depth of screen is 20 m).~~

2.3 Atmospheric tide influences on well water levels

Methods that quantify the barometric efficiency of subsurface systems are based on quantifying the groundwater response magnitude to atmospheric pressure changes (Clark, 1967; Rasmussen and Crawford, 1997; Barr et al., 2000; Gonthier, 2003) or atmospheric tides (Acworth et al., 2016). Turnadge et al. (2019) reviewed these methods and concluded that the method by Acworth et al. (2016) was the most robust and reliable. However, their approach was limited by the assumption of an instantaneous and undamped response. Rau et al. (2020) developed a new method that completely disentangles the influences of Earth and atmospheric tides at the same frequency, e.g. S_2. This further considers the damping of the ~~subsurface-well system amplitude~~ that can be caused by low hydraulic conductivity materials under confined conditions (Section 2.2.3) or attenuation of the amplitude under semi-confined conditions (Section 2.2.4). Their new approach is (Rau et al., 2020)

$$BE_{S_2} = \frac{1}{A_{M_2}} \cdot \left| \frac{\hat{z}_{S_2}^{GW.AT}}{\hat{z}_{S_2}^{AT}} \right|, \tag{17}$$

where

$$\hat{z}_{S_2}^{GW.AT} = \hat{z}_{S_2}^{GW} - \hat{z}_{S_2}^{GW.ET} = \hat{z}_{S_2}^{GW} - \frac{\hat{z}_{M_2}^{GW}}{\hat{z}_{M_2}^{ET}} \hat{z}_{S_2}^{ET}. \tag{18}$$

Here, A_{M_2} corrects for the damping of the subsurface-well system and can be inferred from Earth tides (Equation 4) depending on the response, e.g. for low hydraulic conductivity ~~, and can be inferred from Earth tides as calculated earlier~~

(Equation 4); (Equation 6), or damping under semi-confined conditions (Equation 13). $\hat{z}_{S_2}^{GW.AT}$ is the S_2 component of the groundwater response to atmospheric tides, and $\hat{z}_{S_2}^{AT}$ is the S_2 frequency component (atmospheric tide) embedded in atmospheric pressure measurements. BE forms a stress balance, described as (Jacob, 1940)

$$BE + \gamma = 1, \tag{19}$$

250 where γ is the loading efficiency.

2.4 Combining Earth and atmospheric tide responses

2.4.1 General relationships

Within the following derivations it is assumed that Earth tides only induce horizontal areal strain ($\epsilon_a = \epsilon_{11} + \epsilon_{22}$) whereas atmospheric tides only induce vertical strain ($\epsilon_{33} = -p_{AT}$) (Rojstaczer and Agnew, 1989; Cutillo and Bredehoeft, 2011)(Rojstaczer and Agnew
255 , all of which are assumed to act instantaneously on the subsurface as is consistent with previous literature (Wang, 2000; Rau et al., 2018). Under such conditions, van der Kamp and Gale (1983) has shown that the rigidity modulus (also known as the shear modulus, G) can be estimated, with the previous outlined assumptions, from combined Earth and atmospheric influences as

$$G = A_{M_2}^e \frac{\rho g}{2\gamma} = A_{M_2}^e \frac{\rho g}{2(1 - BE)}, \tag{20}$$

260 where, $A_{M_2}^e$ originates from Earth tides (Equation 3), whereas BE or γ is derived from atmospheric tides (Equation 19).

The disentanglement of Earth and atmospheric tides from the groundwater level response in a well, and the use of these separate frequency components to quantify hydrogeomechanical properties allows further geomechanical derivations to be made. Two methods are presented below which solve for the assumption of either incompressible (suitable for unconsolidated material) or compressible grains (suitable for consolidated material). The choice between which method to use is established
265 by examining an estimated *Biot-Willis* coefficient defined as (Wang, 2000)

$$\alpha = 1 - \frac{K}{K_s} = 1 - \frac{\beta_s}{\beta}. \tag{21}$$

Where K is the Bulk modulus, K_s is the Bulk modulus of the solid grain; respectively, β is the compressibility, and β_s the compressibility of the solid grain. For unconsolidated conditions, where the Bulk modulus is much smaller than the Bulk modulus of the grains ($K \ll K_s$) it is possible to assume that the grains do not contribute to the overall compressibility, thus
270 the grains are incompressible. The *Biot-Willis* coefficient $\alpha \to 1$ shows that the contribution of the grains to the compressibility of the bulk material is insignificant (Rau et al., 2018). By contrast, in consolidated cases K becomes larger, leading to a coefficient that deviates appreciably from one ($\alpha < 1$). In such cases, the grain compressibility is a significant proportion of the total material compressibility and must be accounted for.

2.4.2 Incompressible grainsUnconsolidated systems

275 ~~For incompressible grains (~~In unconsolidated systems, the compressibility of the grains is much smaller than that of the bulk leading to $\alpha = 1$~~) the~~. This allows simplification of the theory. In the following, undrained parameters are depicted by the superscript u. The uniaxial loading efficiency is related to the uniaxial bulk properties as (van der Kamp and Gale, 1983)

$$\gamma = \frac{\beta_v^u}{\theta \beta_f + \beta_v^u},$$
(22)

where β_f is the compressibility of the fluid (4.59×10^{-10} Pa^{-1} at $20°C$), β_v^u is the vertical undrained bulk compressibility

280 and θ is the total porosity of the formation. The uniaxial specific storage (assuming incompressible grains) is defined by Jacob (1940) as

$$S_s = \rho_w g(\beta_v^u + \theta \beta_f).$$
(23)

~~This equation was used by Acworth et al. (2016), with an S_s estimate from Equation 24, to constrain Equation 23 allowing β_v to be resolved.~~ Acworth et al. (2016) used Equation 22 to simplify Equation 23 as follows

285 $$S_s = \rho_w g \beta_f \frac{\theta}{BE} = 4.5 \times 10^{-6} \frac{\theta}{BE}.$$
(24)

However, this requires a prior estimate of the porosity θ which is often difficult to determine due to the lack of available ~~field measurements. Note also~~ information.

Note that the above equations assume that barometric loading is uniaxial, and as such use vertical compressibility ($\beta_v \beta_v^u$) rather than ~~that of~~ the volumetric (bulk) compressibility ($\beta \beta^u$). Here, were instead propose using the S_s derived from the ~~pre-

290 or post-strain~~ response to ET (Section 2.2) to instead ~~constrain Equation 24 to~~ estimate the subsurface porosity by rearranging Equation 24 (similar to Jacob (1940)) as

$$\theta = \frac{S_s BE}{\rho g \beta_f} \frac{S_s BE}{\rho_w g \beta_f} = \frac{S_s}{\rho g \beta_f} \frac{S_s}{\rho_w g \beta_f}(1 - \gamma).$$
(25)

To achieve a similar outcome as Acworth et al. (2016) this porosity, in addition to the calculated S_s, can ~~now be used in Equation 26, rearranged from Acworth et al. (2016),~~ also be used to provide a uniaxial (vertical) bulk compressibility (inverse

295 vertical undrained bulk modulus (K_v^u)) of the subsurface defined as (Acworth et al., 2016)

$$\beta_v^u = \frac{\gamma \theta \beta_f}{1 - \theta} = \frac{1}{K_v^u} = \theta \beta_f \frac{\gamma}{1 - \gamma}.$$
(26)

This approach is similar to ~~the one used by~~ Cutillo and Bredehoeft (2011) but uses the objective BE method developed by Rau et al. (2020) instead of the subjective correlation by Gonthier (2003). Within this subsection it has been shown that it is possible to derive an estimate of porosity from a loading strain if the specific storage is known. This assumes incompressible

300 grains ($\alpha \approx 1$) and is therefore suitable for ~~unconsolidated material~~ application in unconsolidated materials (Rau et al., 2019).

The assumption of incompressible grains allows for the removal of the grain compressibility and ~~provide~~ provides a simplification of the poroelastic ~~parameter~~ space. This step ~~,~~ combined with the new derivation of the shear modulus enables a linear

analytical solution of the remaining elastic variables in unconsolidated material ($\alpha \approx 1$). The first step can be taken by deriving the undrained bulk modulus (K_u) with the K_v^u from Acworth et al. (2016) as (Wang, 2000)

$$K^u = K_v^u - \frac{4}{3}G,$$

(27)

which allows ~~for the solving of~~ solving the Skempton coefficient defined as (Rau et al., 2018)

$$B = \gamma \frac{K_v^u}{K^u} = \gamma \frac{\beta^u}{\beta_v^u}.$$

(28)

Determination of the Skempton coefficient along with the loading efficiency unlocks the undrained Poisson's ratio using (Wang, 2000)

$$\nu^u = \frac{3\gamma - B}{3\gamma + B}$$

(29)

and drained Poisson's ratio as (Wang, 2000)

$$\nu = \frac{3\nu^u - B(1+\nu^u)}{3 - 2B(1+\nu^u)}.$$

(30)

Determination of the drained Poisson ratio further unlocks all remaining poroelastic properties such as Young's Modulus (E), defined as (Wang, 2000)

$$E = \frac{9KG}{3K + G}.$$

(31)

Equations 22-31 define the complete parameter space for unconsolidated materials.

2.4.3 ~~Compressible grains~~Consolidated systems

To determine the poroelastic properties of consolidated materials, the grain compressibility must be considered (~~leading to $\alpha < 1$~~). Further, the following two assumptions must be made: (1) Although pore fluids technically respond to cubic strains, the areal strain can be used to estimate the subsurface strain from ET; (2) The system is homogeneous and laterally extensive, thus ignoring topographic effects and considering the barometric loading to be uniform. The equations that define the remaining elastic properties for such conditions are (Beavan et al., 1991)

$$B = \frac{3\gamma(1-\nu)}{2\gamma\alpha(1-2\nu) + (1+\nu)},$$

(32)

and

$$\theta = \left(\frac{1}{B} - 1\right)\left(\frac{1}{K} - \frac{1}{K_s}\right)\left(\frac{1}{k^f} - \frac{1}{K_s}\right)^{-1},$$

(33)

and

$$\alpha = 1 - \frac{K}{K_s} = 1 - \frac{2G(1+\nu)}{3K_s(1-2\nu)}$$

(34)

and

$$S_s = \frac{\rho g}{\gamma(1-\nu)} \left(\frac{1-2\nu}{2G} - \frac{1+\nu}{3K_s} \right).$$ (35)

330 Equations 32-35 form a non-linear system which must be solved by iteration.

If the petrology of the lithology is known, appropriate literature compressibility values of the dominant grain mineralogy (K_s) could be used. Quartz is the most common naturally occurring mineral and is also one of the least compressible (it is also applicable for most of our case sites), and it will therefore be used to define the upper bounds of K_s here. Richardson et al. (2002) summarised literature values of poly-crystalline quartz for K_s to range between 36-40 GPa, and reported K_s values for

335 the quartz Ottawa Sandstone to be in a range of 30-50 GPa. The average of these ranges has been summarised as 42 GPa (Rau et al., 2018) and will be used in this work.

With the established inputs of $\gamma(BE)$, A_{M_2}, G (Equation 20), S_s and an estimate of K_s, it is possible to simultaneously solve Equations 32-35 for Skempton's coefficient (B), porosity (θ), *Biot-Willis* coefficient (α) and specific storage (S_s) (Beavan et al., 1991). This allows a complete calculation of all remaining poroelastic properties ~~whose inter-dependency is summarised~~

340 ~~in Table ??~~using the relationships that can be found in (Wang, 2000).

3 ~~Method application under different hydrogeological settings~~Example method applications

3.1 Field sites, ~~geological context and~~ monitoring, datasets and interpretation

To demonstrate the new method, groundwater and barometric pressure records from four field sites and five monitoring bores

345 were used. These sites were selected based on three main criteria: (1) Data availability and quality; (2) a ~~strong~~ significant groundwater response to Earth tides (M_2); (3) providing a variety of hydrogeological settings with existing studies for parameter comparisons. ~~The Cattle Lane site has unconsolidated materials and was processed using the approach for unconsolidated systems with the assumption of incompressible grains. All other sites were evaluated by assuming compressible grains.~~ Fur details about each site is described in separate sections below. Specific bore geometries and measurements used in the analysis

350 of these sites such as depths and bore construction are summarised in Table 2.

Groundwater pressure head and barometric pressure time-series were recorded at sub-hourly intervals at all sites (e.g. two sites shown in Figure 4) for at least three months which is longer than the ~28 days being suggested as the minimum requirement of the selected HALS method (Schweizer et al., 2021). Geo-position of the boreholes, theoretical Earth tide strain for the same duration and sampling frequency of each site as well as aerial strain amplitudes from ocean tide loading for M_2

355 are summarised in Table 2.

Additional information required for the analysis, such as casing and screen radius's, screen depth and length, were also noted for each bore and were presented in Table 2. All time-series were detrended by fitting a linear function through a moving 3-day window using the *SciPy* detrend function, and the main tidal components were extracted using *HALS* (Section 2.1). Tables 3 and 4 contain the estimated hydro-geomechanical properties for all the field sites and are discussed in the sections below.

Location	~~K~~ Borehole	~~K_v~~ Geolocation			~~ν~~	Inputs		| ~~E-G~~ Description
		Latitude	Longitude	Height		r_c	r_w	
K,G Cattle Lane	~~-~~BH30061	~~$K + \frac{4G}{3}$~~ -31.518340	~~$\frac{3K-2G}{2(3K+G)}$~~ 150.468332	~~$\frac{9KG}{3K+G}$~~ 313		~~G,E~~ 0.125	~~$\frac{EG}{3(3G-E)}$~~ 0.12	~~$G\frac{4G-\ldots}{3G-\ldots}$~~
E,K Thirlmere	~~-~~GW075409 1.2	~~$3K\frac{3K+E}{9K-E}$~~ -34.230666	~~$\frac{3K-E}{6K}$~~ 150.543996	~~-~~314		~~$\frac{3KE}{9K-E}$~~ 0.156	0.14	1...
G,ν Thirlmere	$G\frac{2(1+\nu)}{3(1-2\nu)}$ Thirlmere 2	~~$G\frac{2(1+\nu)}{3(1-2\nu)}$~~ -34.220836	~~-~~150.53646	~~$2G(1+\nu)$~~ 323		~~-~~0.114	0.108	4...
K,ν Dodowa	~~-~~BH11	~~$3K\frac{1-\nu}{1+\nu}$~~ 5.881675	~~-~~0.097244	~~$3K(1-2\nu)$~~ 88		~~$3K\frac{1-2\nu}{2+2\nu}$~~ 0.058	0.048	2...
E,ν Death Valley	$\frac{E}{3(1-2\nu)}$ BLM-1	~~$\frac{E(1-\nu)}{(1+\nu)(1-2\nu)}$~~ 36.408130	~~-~~-116.471360	~~-~~688		~~$\frac{E}{2+2\nu}$~~ 0.127	0.127	10...

~~Elastic constant relationships~~

Table 2. Inputs parameters for ~~isotropic stress and undrained conditions (Birch, 1996; Wang, 2000; Sheriff, 2002). Note that Young's modulus may also be used to provide~~ case sites where: A_{M_2} is the ~~uniaxial compression strength~~ dimensionless amplitude response, A^e is the amplitude response, ϕ_{M_2} is the phase shift of the M_2 component, r_c is the outer diameter (~~UCS~~m) ~~using~~ of the ~~linear relationship established by Colwell and Frith (2006)~~ bore casing, r_w the internal diameter (m) of the bore casing, b is the Aquifer thickness (m) or open interval of the screen, z is the depth (m) to the centre of the screen or open interval, and K_s is the assumed grain bulk modulus. Italicised values were not used in the applied leaky or confined models, but are provided for context.

Borehole	Hydraulic results							
	$A_{M_2}^{ETe}$	~~$A_{M_2}^e$~~ $A_{M_2}^{OTe}$	$A_{M_2}^e$	A_{M_2}	~~$\Delta\phi_{M_2}$~~ $\Delta\Phi_{M_2}$	~~r_c~~ $<K>$	~~r_w~~ S_s	~~b z~~ K_s
~~Cattle Lane~~ BH30061	20.62	2.48	$4.59 \cdot 10^4$	0.765	24.89	~~0.125~~ $5.4 \cdot 10^{-6}$	~~0.12~~ $1.7 \cdot 10^{-5}$	~~1 55~~ San
~~Thirlmere~~ GW075409.1.2	19.36	3.81	$3.24 \cdot 10^5$	1.039	10.05	~~0.156~~ $4.8 \cdot 10^{-7}$	~~0.14~~ $3.2 \cdot 10^{-6}$	~~12 78~~ 42 Quar
Thirlmere ~~Thirlmere~~ 2	19.07	3.81	$3.52 \cdot 10^5$	0.988	-4.74	~~0.114~~ $1.9 \cdot 10^{-5}$	~~0.108~~ $2.8 \cdot 10^{-6}$	~~4 72~~ 42 Quartz
~~Dodowa~~ BH11	27.57	4.10	$1.31 \cdot 10^6$	0.948	~~-13.8~~ -13.80	~~0.058~~ $2.6 \cdot 10^{-6}$	~~0.048~~ $7.2 \cdot 10^{-7}$	~~2 45~~
~~Death Valley~~ BLM-1	17.55	2.52	$1.49 \cdot 10^6$	0.998	-1.07	~~0.127~~ $4.3 \cdot 10^{-6}$	~~0.127~~ $6.7 \cdot 10^{-7}$	~~106 830~~

Table 3. ~~Inputs parameters for case sites~~ Hydraulic results where ~~;~~ A_{M_2} $A_{M_2}^{ETe}$ is the ~~dimensionless~~ Earth tide strain amplitude ~~response~~(nstr), ~~A^e~~ $A_{M_2}^{OTe}$ is the ocean tide strain amplitude ~~response~~(nstr), ~~ϕ_{M_2}~~ $A_{M_2}^e$ is the ~~phase shift of the M_2 component~~strain response amplitude (m/nstr), ~~r_c~~ A_{M_2} is the ~~outer diameter~~ amplitude ratio (~~m~~)~~of the bore casing~~, ~~r_w~~ $\Delta\Phi_{M_2}$ is the ~~internal diameter~~ phase shift (~~m~~degrees)~~of the bore casing~~, ~~b~~ $<K>$ is the ~~Aquifer thickness~~ hydraulic conductivity (m/s)~~or open interval of the screen~~, ~~z~~ S_s is the ~~depth~~ specific storage (1/m)~~to the centre of the screen or open interval~~, and ~~K_s~~ BE is the ~~assumed grain bulk modulus~~barometric efficiency.~~Italicised values were not used in the applied pre- or post-strain models, but are provided for context.~~

360   In this paper, all of the methodology and equations were implemented in the *Python* programming language, and joint iterative solving was completed using *SciPy*'s least-squares functionality. The following realistic parameter bounds were considered during root finding: $0 \leq B \leq 1$, $-1 \leq \nu \leq 0.5$, $0.005 \leq G \leq 40$ GPa, $0 \leq \theta \leq 0.5$. We note that (1) the parameter units were scaled to avoid bias towards parameters with large values, (2) the solver was set to 64-bit machine precision (epsilon $1.11 \cdot 10^{-16}$), (3) none of the estimated parameters reached any of the prescribed solution constraints.

365   ~~Location~~

3.1.1   ~~Cattle Lane (NSW, Australia)~~

3.2   Cattle Lane (NSW, Australia)

Cattle Lane is located on the Liverpool Plains, NSW, eastern Australia. Erosion of the basaltic Liverpool Ranges to the south produced a succession of unconsolidated silts, clays, sands, gravel and minor carbonate nodules within the Liverpool Plains. A
370 thick sequence of clay bound sediments overlie a gravel aquifer at 40 m. This aquifer has previously be shown to respond to loading by rainfall events (Timms and Acworth, 2005). The lithology of the 1 m screened interval was described by Acworth et al. (2015) as major basalt fragments mixed with coarse sand, shell and carbonate nodules. The site has previously been cored to 31.5 m depth, lithologically logged and geophysical surveyed, confirming that it is horizontally extensive (Acworth et al., 2015). Cross-hole seismics were also conducted by Rau et al. (2018) to the depth of 40 m (screened interval of bore BH30061
375 is 55 m depth, see Table 2), providing depth profiles of seismically inferred elastic variables that were used to constrain the pore pressure response to atmospheric tides analysis.

Borehole	Poroelastic results									
	θ	K_v^u	G	K^u	B	ν^u	ν	K	E	α
BH30061	0.39	3.8	0.3	3.5	1.0	0.46	-0.29	0.1	0.4	1
GW075409.1.2	0.27	9.7	2.6	6.3	1.0	0.32	-0.04	0.3	1.9	1
Thirlmere 2	0.19	16.6	2.6	13.1	0.9	0.38	-0.03	1.6	5.1	0.96
BH11	0.08	58.8	21.6	30.0	0.7	0.10	-0.21	8.1	34.2	0.81
BLM-1	0.06	64.2	19.1	38.7	0.8	0.16	-0.23	6.7	29.4	0.84

Table 4. Poroelastic results where θ is porosity (-), K_v^u is the vertical undrained bulk modulus, G is the shear modulus (GPa), K^u is the undrained bulk modulus, B is *Skempton*'s coefficient (-), ν^u is the undrained Poisson's Ratio (-), ν is Poisson's Ratio (-), K is Bulk Modulus (GPa), E is Young's modulus (GPa) and α is the *Biot-Willis* coefficient (-).

Further studies at this site include Acworth et al. (2016) and Acworth et al. (2017), which were precursors to Rau et al. (2018) in the investigation of pore pressure response to atmospheric tides, and Timms et al. (2018) on a core scale analysis of the site's laterally extensive and thick aquitard. Due to the sufficient M_2 response, time-series data of groundwater pressure heads measured between the 21/01/2016 and 14/04/2018 were used from the bore BH30061~~which is located at latitude −31.518340°, longitude 150.468332° and an elevation of 313 MASL (WGS84)~~. The groundwater pressure heads were collected using vented In-Situ Troll 700H series loggers at hourly intervals and sub-millimetre precision. Atmospheric pressure was measured by an In-Situ Baro Troll absolute gauge transducer.

The borehole BH30061 from Cattle Lane produced positive M_2 phase shifts (Table 2), with specific storage and hydraulic conductivity therefore being derived from the leaky model (Section 2.2.4). When applying the grain compressibility of quartz ($K = 42$ GPa) a *Biot-Willis* coefficient of 0.99 is obtained and hence justifies the assumption of incompressible grains ($\alpha \approx 1$) and the unconsolidated analytical model (Section 2.4.2).

The specific storage, hydraulic conductivity, porosity, shear modulus, and undrained Poisson's ratio from Cattle Lane are consistent with literature values for the sediment type (Bowles, 1996), and comply with previous estimates from higher in the stratigraphy at the same site obtained by cross-hole seismics (Acworth et al., 2015, 2016; Rau et al., 2018). The Young's modulus of 0.4 GPa deviates from the expected material range reported in the literature for an unconsolidated clay, sand and gravel mixture of between 0.025 and 0.2 GPa, although is reasonable when considering consolidation at 55 m depth and an in-situ derivation rather than laboratory tests (Bouzalakos et al., 2016). The Poisson's Ratio of -0.29 is the only parameter that deviates significantly from the expected range of 0.2 to 0.5. This will be discussed later.

3.2.1 ~~Thirlmere Lake (NSW, Australia)~~

3.3 Thirlmere Lake (NSW, Australia)

Thirlmere Lakes is located in the south-west of the Sydney Basin, NSW. Both bores are located in the quartz arenite Hawkesbury sandstone, which is about 100 m thick at the site. This sandstone is deposited by a braided river with the heterogeneous deposits showing overlapping and self incised fining up sequences, with over-bank deposited fines at paleo-channel boundaries (Miall and Jones, 2003). There is evidence that the upper portion of bore Thirlmere 2 passes through a geological fault damage zone, with drilling fluid losses recorded above the screened interval due to fractures (Impax, 2019). Other studies in the same lithology include Ross (2014), which investigated the potential for a bore field development within the Hawkesbury Sandstone, however, no publicly available studies exist for this lithology at the case site.

The time span and collection of the time-series data for the two bores differ. The time-series for GW075409.1.2 covers the time period from 03/07/2018 to 14/12/2018 and was downloaded from the *WaterNSW* real-time data portal with 15 min intervals. ~~The bore is located at latitude -34.230666^o, longitude 150.543996^o, elevation 314 MASL (Russell, 2012).~~ For this bore a coinciding barometric time-series data was obtained from a weather station approx. 500 m away. The bore Thirlmere 2 is located about 2 km from GW075409.1.2 and ~~is located at latitude -34.220836^o, longitude 150.536467^o, elevation 323 MASL~~ geo-coordinates are shown in Table ?? (Russell, 2012). The time-series data for Thirlmere 2 was collected for this study using a vented In-Situ Troll 700H series pressure transducer every 5 min between the 32/07/2019 and 29/10/2019. The coinciding barometric time-series was collected using a Solinst Baro-logger installed in the airspace of the borehole.

3.3.1 ~~Dodowa (Ghana)~~

~~Dodowa is located in the Shai Osudoku District in the southeastern part of the Greater Accra Region, Ghana. The local geology consists of the Togo Structural and Dahomeyan Structural units. The Togo being composed of a series of metamorphic and folded quartzites, phyllites and schists, and the Dahomeyan composed of altered belts of acid and basic gneisses. BH11 used within this paper is located in a Dahomeyan gneiss (Attoh et al., 1997). All units within the region appear highly weathered, resulting in an 5 m unconsolidated regolith, confining the underlying fractured igneous and metamorphic units.~~

~~BH11 was installed and previously studied by Foppen et al. (2020), including atmospheric tide analysis. The time-series for the water levels of BH11 was collected at 20 min intervals between the 18/10/16 and 07/06/2017 using Mini-Diver DI501; Schlumberger pressure transducers, with atmospheric pressure being recorded with a Mini-Diver DI500; Schlumberger barometric diver, located above ground at the site at an approximate latitude 5.881675, longitude −0.097244, elevation 88 MASL (Foppen et al., 2020).~~

3.3.1 ~~Death Valley (California, USA)~~

~~The Death Valley site is located in the western part of the USA on the border of Nevada and California at a position of; latitude 36.408130, longitude -116.471360 (WGS84), elevation 688 MASL. Bore BLM-1 is located in Paleozoic carbonate rock and~~
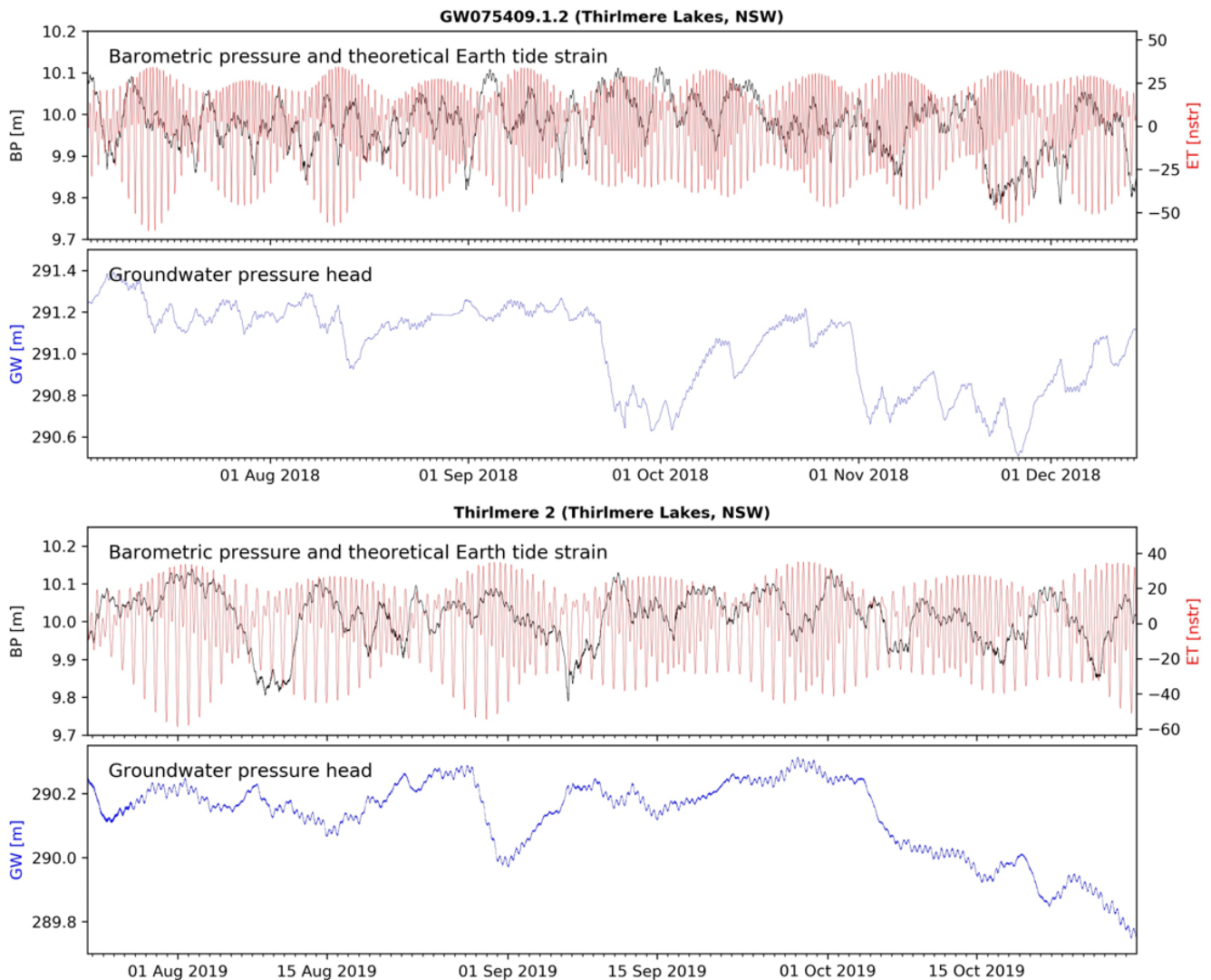
Figure 4. Time-series of barometric pressures (m), theoretical Earth tide nano-strain (nstr) from *PyGTide* for the corresponding groundwater levels from bores GW075409.1.2 and Thirlmere 2 from Thirlmere Lakes, NSW, Australia.

was left as an open well. The same time-series record was also used in Rau et al. (2020) and it is the same bore for which data was analysed in Cutillo and Bredehoeft (2011). Data was recorded at 15 min intervals using an In Situ Troll with a vented cable and an In Situ Barotroll. The time-series spans between the 25/06/2009 and 16/12/2009.

430   **3.4   Method application**

Groundwater pressure head and barometric pressure time-series were recorded at sub-hourly intervals at all sites (e.g. two sites shown in Figure 4) for at least three months which is longer than the ~28 days being suggested as the minimum requirement (Roeloffs, 1996). The theoretical Earth tide potential for the same duration and sampling frequency of each site was calculated using *PyGTide* (Rau, 2018). This required knowledge of the geo-position of the borehole (latitude, longitude and elevation in WGS84). Additional information required for the analysis, such as casing and screen radius's, screen depth and length, were also noted for each bore and were presented in Table 2. All time-series were detrended by a moving 3 day average using the *SciPy* detrend function, and the main tidal components were extracted using *HALS* (Section 2.1).

3.4 Hydro-geomechanical properties

The hydro-geomechanical properties for the field sites from the application of the method outlined in Section **??** are presented in Table **??**. The boreholes BH30061 and The borehole GW075409.1.2 from Cattle Lane and Thirlmere Lakes produced positive M_2 phase shifts (Table 2), with specific storage and hydraulic conductivity therefore being derived from the Pre-strain leaky model (Section **??**). All other bores had negative phase shifts and were processed using the Post-strain model (Section **??**) from Hsieh et al. (1987). BH30061 was the only data set processed using the proposed unconsolidated analytical model. If applying the assumed grain compressibility of quartz ($K = 42$ GPa) for BH30061, a *Biot-Willis* coefficient of 0.99 is obtained and hence justifies the assumption of incompressible grains ($\alpha \approx 1$) (Section 2.4.2). 2.4.4). Both the quartz sandstone bores returned *Biot-Willis* coefficients of 0.96 , and the gneiss bore 0.84, as such, these bores required which requires a value for the grain compressibility (Section 2.4.3).

Location Borehole $< K >$ S_s BE K_s θ K_v^u G K^u B ν^u ν K E α Cattle Lane BH30061 $5.4 \cdot 10^{-6}$ $1.7 \cdot 10^{-5}$ 0.11 ∞ 0.39 3.8 0.3 3.5 1.0 0.46 -0.29 0.1 0.4 1 Thirlmere GW075409.1.2 $4.8 \cdot 10^{-7}$ $3.2 \cdot 10^{-6}$ 0.38 ∞ 0.27 9.7 2.6 6.3 1.0 0.32 -0.64 0.3 1.9 1 Thirlmere Thirlmere 2 $1.9 \cdot 10^{-5}$ $2.8 \cdot 10^{-6}$ 0.35 42 0.19 16.6 2.6 13.1 0.9 0.38 -0.03 1.6 5.1 0.96 Dodowa BH11 $2.6 \cdot 10^{-6}$ $7.2 \cdot 10^{-7}$ 0.70 42 0.08 58.8 21.6 30.0 0.7 0.10 -0.21 8.1 34.2 0.81 Death Valley BLM-1 $4.3 \cdot 10^{-6}$ $6.7 \cdot 10^{-7}$ 0.62 42 0.06 64.2 19.1 38.7 0.8 0.16 -0.23 6.7 29.4 0.84 Results from case sites where; $< K >$ is hydraulic conductivity (m/s), S_s is the specific storage (1/m), BE is the barometric efficiency, K_s is the solid grain bulk modulus (assumed to be 42 GPa for consolidated systems, else ∞), θ is porosity (-), K_v^u is the vertical undrained bulk modulus, G is the shear modulus (GPa), K^u is the undrained bulk modulus, B is *Skempton*'s coefficient (-), ν^u is the undrained Poisson's Ratio (-), ν is Poisson's Ratio (-), K is Bulk Modulus (GPa), E is Young's modulus (GPa) and α is the *Biot-Willis* coefficient (-).

3.3.1 Cattle Lane (NSW, Australia)

The specific storage, hydraulic conductivity, porosity, shear modulus, and undrained Poisson's ratio from Cattle Lane are consistent with literature values for the sediment type (Bowles, 1996), and comply with previous estimates from higher in the stratigraphy at the same site obtained by cross-hole seismics (Acworth et al., 2015, 2016; Rau et al., 2018). The Young's modulus of 0.4 GPa deviates from the expected material range reported in the literature for an unconsolidated clay, sand and gravel mixture of between 0.025 and 0.2 GPa, although is reasonable when considering consolidation at 55 m depth and an

3.3.1 Thirlmere Lakes (NSW, Australia)

Estimates of hydro-geomechanical parameters (S_s of $3.2 \cdot 10^{-6}$ and $2.8 \cdot 10^{-6}$ $(1/m)$; k^f of $4.8 \cdot 10^{-7}$ and $1.9 \cdot 10^{-5}$ (m/s)) for the two sandstone bores are considered realistic for a quartz sandstone in this area. The higher k^f for Thirlmere 2 is believed to be indicative of enhanced hydraulic conductivity due to fractures. For this sandstone formation, SCA (2005, 2006) has previously reported S_s values of $2.49 \cdot 10^{-6}$ to $2.41 \cdot 10^{-4}$ $(1/m)$ and k^f of $1.15 \cdot 10^{-6}$ to $3.36 \cdot 10^{-6}$ (m/s) within this formation, including fracture networks (Ross, 2014). Geomechanical estimates of the shear modulus of 2.6 GPa marginally exceeds the expected range of $1 - 2$ GPa (Bertuzzi, 2014; Zhang et al., 2016; Zhang and Lu, 2018). Conversely, the bulk modulus and Young's modulus both fall within the expected ranges of 2.6 to 5.3 and 3 to 8 GPa, respectively. The estimated Poisson's ratios of -0.64 and -0.03 are low compared to values between 0.2 and 0.3 typically measured in the laboratory (McMillan et al., 2019).

3.3.1 Dodowa (Ghana)

The

3.4 Dodowa (Ghana)

Dodowa is located in the Shai Osudoku District in the southeastern part of the Greater Accra Region, Ghana. The local geology consists of the Togo Structural and Dahomeyan Structural units. The Togo being composed of a series of metamorphic and folded quartzites, phyllites and schists, and the Dahomeyan composed of altered belts of acid and basic gneisses. BH11 used within this paper is located in a Dahomeyan gneiss (Attoh et al., 1997). All units within the region appear highly weathered, resulting in an 5 m unconsolidated regolith, confining the underlying fractured igneous and metamorphic units.

BH11 was installed and previously studied by Foppen et al. (2020), including atmospheric tide analysis. The time-series for the water levels of BH11 was collected at 20 min intervals between the 18/10/16 and 07/06/2017 using Mini-Diver DI501 (Schlumberger pressure transducers), with atmospheric pressure being recorded with a Mini-Diver DI500 (Schlumberger barometric diver), located above ground (Foppen et al., 2020).

The gneiss bore returned *Biot-Willis* coefficients of 0.84, as such and therefore required a value for the grain compressibility (Section 2.4.3).

The hydrogeomechanical estimates of hydraulic conductivity of $2.6 \cdot 10^{-6}$ (m/s) and specific storage of $7.2 \cdot 10^{-7}$ $(1/m)$ are comparable with the values for the Togo Structural Unit from Foppen et al. (2020) derived from pumping and slug tests, which indicated ranges between 10^{-5} to 10^{-6} (m/s), and $2.3 \cdot 10^{-7}$ to $7.7 \cdot 10^{-8}$ $(1/m)$, respectively. The estimated porosity of 0.08 for BH11 slightly exceeds the range of 0.005 to 0.05 in Foppen et al. (2020). Comparison of elastic modulus is problematic for schists, as values are dependent on the original protolith and may vary significantly, and because schistose rock masses

are known for high values of anisotropy (Hoek and Diederichs, 2006). For example, Young's modulus for a schist, as in the screened interval of BH11, can vary significantly between 21 to 117 GPa depending on mineralogy and foliation orientation (Condon et al., 2020). Our estimated value of 34.2 GPa falls within this range. However, detailed mineralogy does not exist for this bore to allow a closer comparison with literature values.

3.4.1 ~~Death Valley (California, USA)~~

~~The~~

3.5 Death Valley (California, USA)

The Death Valley site is located in the western part of the USA on the border of Nevada and California. Bore BLM-1 is located in Paleozoic carbonate rock and was left as an open well. The same time-series record was also used in Rau et al. (2020) and it is the same bore for which data was analysed in Cutillo and Bredehoeft (2011). Data was recorded at 15 min intervals using an In Situ Troll with a vented cable and an In Situ Barotroll. The time-series spans between the 25/06/2009 and 16/12/2009.

As the Death Valley dataset has previously been analysed by Rau et al. (2020) who justified the use of confined ET, this method was used for the -1 degree phase shift of the Death Valley dataset which is located in the limit between pre and confined models.

The estimated hydraulic conductivity of $4.3 \cdot 10^{-6}$ (m/s), is in agreement with the Earth tide analysis derived value of $1.3 \cdot 10^{-6}$ (m/s) by Cutillo and Bredehoeft (2011). In contrast, the estimated specific storage value of $6.7 \cdot 10^{-7}$ $(1/m)$ is an order of magnitude smaller than the value of $7.3 \cdot 10^{-6}$ $(1/m)$ from Cutillo and Bredehoeft (2011). However, the specific storage and hydraulic conductivity values are both consistent with the values published by Rau et al. (2020) for the same data-set, using a method based on ET. The determined porosity (0.06) also aligns with the lower end of the range proposed by Cutillo and Bredehoeft (2011), and it is reasonable to assume the calculated Young's and shear modulus of 28.28 and 24.10 GPa are similarly plausible coinciding with literature lithological values (Parent et al., 2015). We note that the derived Poisson's ratio of -0.23 differs significantly from the value of 0.25 which was merely assumed in Cutillo and Bredehoeft (2011).

4 Discussion

4.1 Influences on the parameter estimation

While quantifying hydro-geomechanical properties it is beneficial to consider factors that influence the accuracy of parameter estimation. We note that absolute errors in the measurement of atmospheric or groundwater pressure are irrelevant for the presented methodology. This is because all parameters are based on the amplitude and phase of tidal components embedded within the measurements, i.e., the relative change characteristic of the harmonics. Here, the resolution of the measurement device directly determines detection and quantification of the responses to tidal forces. Schweizer et al. (2021) demonstrated that extraction of harmonics using HALS is accurate if the resolution of the pressure transducer is larger than twice the amplitude

of the tidal component under consideration. All instruments deployed in the field examples of this work comfortably fulfilled
this criterion. Schweizer et al. (2021) further noted that HALS outperforms the discrete Fourier transform, but also that an
objective error estimation for HALS is difficult~~and~~, depends on the nature of the residuals (difference between measurement
and model) and requires separate investigation. We consider that the accuracy of HALS is at least as good as that resulting
from fitting a conceptual model to measurements obtained during standard ~~hydraulic testing such as~~ aquifer testing.

Previous works have illustrated that quantifying BE by disentangling the groundwater response to EAT based on theoretical
Earth tides does not lead to additional uncertainty in parameter estimation since it evaluates the relative responses between
ET and GW (Acworth et al., 2016). However, this observation is valid only in a subsurface where the hydraulic conductivity
is $< K > \gtrsim 1 \cdot 10^{-5}$ m/s (Rau et al., 2020). The borehole water level response in lower $< K >$ environments is damped and
shifted compared to the pore pressure response outside the bore. A correction requires knowledge of both $< K >$ and S_s which
can be quantified using calculated ET strains. While this has been done before (Hsieh et al., 1987; Xue et al., 2016; Allègre
et al., 2016; Rau et al., 2020), there is no literature investigating its associated uncertainties.

4.2 Harmonic disentanglement allows estimation of the poroelastic parameter space

In this work, we make use of recent advances that allow quantitative disentanglement of the groundwater response to both Earth
and atmospheric tidal forces. Since ~~each mechanism~~ both mechanisms acts differently on the subsurface, the disentangled re-
sponses can be ~~merged through theoretical relationships. Unlike previous research, this~~ recombined to maximise the outcome.
This allows the solving of the complete poroelastic space for unconsolidated systems entirely based on time-series of mea-
sured groundwater pressure heads, atmospheric pressure, and ~~calculated~~ theoretical Earth tides. For consolidated systems, the
complete poroelastic space can also be solved through a system of nonlinear equations by assuming ~~the grain compressibility .~~
~~This approach has previously been~~ a value for grain compressibility and approach previously used in Rau et al. (2018). While
the results in Tables 3 and 4 provide reasonable values, except for the Poisson's ratio which is discussed separately below, they
are difficult to validate because independent measurements of poroelastic properties are rare.

~~A general agreement is held throughout the literature~~ The literature agrees that a negative phase shift between strain and
borehole water levels is representative of ~~an observed time lag caused by the slow flow of fluid from the formation into the bore~~
~~, in response to the tidal strain~~ confined conditions and horizontal flow between the formation and the bore (Bower, 1983; Hsieh
et al., 1987; Kümpel, 1997; Schulze et al., 2000). Conversely, no such agreement is held for positive phase shifts. Although
Section ~~??~~ 2.2.4 is based on the assumption that positive phase shifts relate to vertical flow ~~to the~~ between the borehole screen
and the water table, i.e. semi-confined conditions (Roeloffs et al., 1989), other explanations for positive phase shifts exist within
the literature. These include the influence of fracture transmissivity and length, the influence of ocean loading, heterogeneous
material properties and topographic effects (Roeloffs, 1996; Burbey, 2010). Here, positive phases from either vertical flow or
fracture flow describe a process in which pressure is able to be propagated rapidly, either to the water table or along a highly
transmissive fracture (Bower, 1983). Other mechanisms for phase shifts have also been explored in the broader literature, such
as Hanson and Owen (1982), who related fracture orientation (strike and dip) to either positive or negative phase shifts.

~~In this study,~~ Our results from various field sites show both positive and negative phases shifts~~were recorded at the various field sites~~. A comprehensive understanding of ~~negative and particularly positive~~ the causes and interpretation of phase shifts is still lacking within the scientific literature. Shi and Wang (2016) observed that negative phase shifts were indicative of predominantly horizontal groundwater flow in a completely undrained system, while a positive phase shift was indicative of a vertical hydraulic gradient in a semi-confined or unconfined system. The method by Hsieh et al. (1987) outlined above as the ~~post-stain~~ confined model (negative phase shift), which was used by Shi and Wang (2016), is based on the assumption of radial horizontal flow into a well. If a positive rather than negative phase shift is used as an input into the system of equations provided by Hsieh et al. (1987), the results will not be sensible. As such, a positive phase shift model is required. For this project the method provided by Wang (2000), and adapted by Xue et al. (2016) and Allègre et al. (2016), was implemented to account for vertical flow. ~~This method, as described in Section ??, was developed for a subsurface forcing by harmonic surface loading . Earth~~ Both Equation 13 and 14 were developed for harmonic loading (i.e. ocean or barometric loading) where strain is produced at the Earth's surface and propagated down (Wang and Davis, 1996). Earth tides do not act by surface loading but rather the mechanism is tidal dilatation, where gravitational forces act on mass across the vertical profile. ~~Although the method based on positive phase shifts has been successfully applied within the literature, the validity of this model has not yet been proven and further research is necessary.~~ This points to possible issues with the validity of simplified assumptions and conceptual models. Further research is required to test the applicability of analytical solutions based on simplified assumptions under real-world conditions.

~~Previous methods that utilised EAT for subsurface characterisation have always required the assumption of an elastic modulus, typically Poisson's ratio, to resolve additional geomechanical parameters of the subsurface. The outlined method in this study has removed this assumption by allowing the Poisson's Ratio to be calculated as part of the TSA. Primarily it is the prior estimate of the specific storage (also determined by TSA) which allows the Poisson's Ratio, along with all the other parameters, to vary within the theoretical ranges. As such, a relationship can be established between phase and amplitude in the methodologies presented here. Within the post-strain model (Section ??, Hsieh et al. (1988)) a decrease in phase (larger negative number) acts to decrease both the hydraulic conductivity and specific storage, whereas for an increase in the amplitude response the hydraulic conductivity remains stable (marginally increases) and the specific storage decreases. This can be summarised as:~~

$$\phi \downarrow = k^f \downarrow, S_s \downarrow; \; amp \uparrow = k^f \sim, S_s \downarrow$$

~~For the pre-strain model (Section ??, Wang (2000)) an increase in phase difference increases the hydraulic conductivity and decreases the specific storage, and an increase in the amplitude response increases the hydraulic conductivity but decreases the specific storage, summarised as:~~

$$\phi \uparrow = k^f \uparrow, S_s \downarrow; \; amp \uparrow = k^f \uparrow, S_s \downarrow$$

~~Within the compressible grains model (Section 2.4.3), an increase in amplitude response results in an increase in the shear modulus and a decrease in the porosity. Whereas an increase in barometric efficiency (i.e. smaller loading efficiency) results in~~

590 ~~an increase of both the shear modulus and porosity, summarised as:~~

$amp \uparrow = G \uparrow, \theta \downarrow \, ; \, BE \uparrow = G \uparrow, \theta \uparrow$

~~However, this is compounded with the change in the specific storage from either an increase or decrease in the amplitude response. Note the importance of this relationship and the effect of the S_2 disentanglement by Rau et al. (2020), where by comparison previous methods which calculated BE such as Jacob (1939) or Acworth et al. (2017) overestimate the barometric~~
595 ~~loading. This in turn results in an overestimation of the shear modulus, which would also affect the derivations of other parameters according to Equation 20.~~

4.3 Strain responses reveal subsurface heterogeneity and anisotropy

Combining ET and AT responses in the subsurface analysis is based on the principle that Earth and atmospheric tides induce strains with a different directionality. ET is fundamentally cubic, but is approximated as planar (tidal dilatation or strain)
600 (Schulze et al., 2000; Fuentes-Arreazola et al., 2018). However, Rojstaczer and Agnew (1989) stated that the use of the horizontal areal strain from Earth tides is a sufficient approximation for subsurface depths of up to thousands of kilometres which should cover depths that are relevant to groundwater systems. For ET, the ~~strain is experienced~~ well water levels must respond to strain in the vicinity of the well bore screen, although the ~~distribution of this stain radially (cylindrical or spherical)~~ sensitivity to strain that is more distant from the screen is ~~uncertain~~unknown. The subsurface strain response to Earth tide induced stress
605 depends on the elastic properties which are highly heterogeneous on a small scale. However, the pore pressure response as measured by a well intersects a larger volume and should therefore be representative of the theoretical values derived from Earth tide calculations.

Rojstaczer and Agnew (1989) ~~predict that the response of~~ predicted that the groundwater response to ET (areal strain) should be high for low porosity and compressibility. Similarly, for such conditions, the barometric efficiency should approach
610 one ($BE \rightarrow 1$, or equivalently $\gamma \rightarrow 0$). However, this ~~does not necessarily occur as can be seen~~ is not reflected in our results for Death Valley and Dodowa where the groundwater response magnitude to ET is large but ~~BE~~ BE is significantly smaller than unity. This phenomenon can be explained by the fact that ~~BE~~ BE is estimated vertically across a typical geological profile as a surface load, uni-axially compressing the subsurface. Here, consolidation generally increases with depth and we hypothesise that the AT response vertically integrates the material properties above the ~~monitoring point~~well screen, i.e. the
615 result is representative of the vertical heterogeneity in elastic properties encountered. The precise geometry of the representative volume from either ET or AT is currently unknown, but it is assumed to be equivalent. However, if this assumption is flawed and the representative volumes of ET or AT significantly differ, strain anisotropy may exist between these two forces and complicate their joint interpretation. Detailed field experimentation or coupled hydraulic-geomechanical modelling would be required to explore ~~such a phenomenon~~these processes.

4.4 ~~In-situ conditions explain~~ Possible reasons for discrepancy in poroelastic properties

Our results in ~~Table ??~~ Tables 3 and 4 largely comply with previously established values (Wang, 2000), except for the observation of negative Poisson's ratios. It is important to note that previous studies typically assume a literature value for Poisson's ratio when calculating geomechanical properties (Cutillo and Bredehoeft, 2011). Our new approach ~~is enabled through tidal disentanglement to remove~~ based on tidal disentanglement removes the need for such an assumption. However, the negative Poisson ratios are a surprising result and require explanation. Before discussing this, we confirm that considering the maximum possible influence from ocean tide loading does not lead to values that better comply with accepted knowledge.

It is theoretically possible for Poisson's ratio to range between negative one and positive half, i.e. $-1 \leq \nu \leq 0.5$ (Lakes, 1991; Lakes and Witt, 2002). Here, materials with a negative Poisson's ratio are described as auxetic, i.e. materials that become thicker parallel to the direction of the stress. The occurrence of auxetic behaviour in rocks was discussed by Gercek (2007), who summarised that as a Poisson's ratio becomes increasingly negative ($\nu \to -1$), the material ~~become~~ becomes highly resistant to shear deformations but easy to deform volumetrically. Ji et al. (2018) succinctly describe this relationship for conditions where the shear modulus is much greater than the bulk modulus, defined as $K < 2G/3$, and geologically is likely associated with highly anisotropic rocks. This ratio between the bulk and shear modulus is consistent with all results presented in this paper. As such, the negative Poisson's ratios are indicative of the subsurface laterally contracting while being vertically compressed, following the theory of linear poroelasticity.

~~Previous instances of negative~~ Negative Poisson's ratios for standard uniaxial core sample testing ~~have been recorded by Homand-Etienne and Houpert (1989) and Zhao et al. (2020)~~ in thermally induced micro-cracked granites have previously been reported (Homand-Etienne and Houpert, 1989; Zhao et al., 2020). However, ~~reporting of~~ auxetic behaviour in rock is ~~dominated by~~ dominantly found in studies involving low strains and low confining pressures. For example, in the Berra Sandstone, Handin et al. (1963) observed that small compressive strains (~~here, small was defined as~~ i.e., less than 200 Bar $\approx$ 2000 mH_2O or 20 MPa) for confining pressure conditions cause the ~~dilation~~ dilatation of pore spaces. Comparatively, observations of pore volumes remained constant for moderate strains (20 to 50 MPa) and reduced in volume for large strains (> 50 MPa). Ji et al. (2018) have recently examined auxetic behaviour over a broad range of lithologies and pressures. They concluded that negative Poisson's ratios are possible in crystalline igneous and metamorphic rocks (non-fractured) for confining hydrostatic pressures less than 5 MPa, and less than 200–300 MPa for more quartz-rich sedimentary rocks such as silt stones and sand stones. Further, Ji et al. (2018) observed that the porosity of sedimentary rocks plays an important role in controlling auxetic effects, similar to the nano-scale fabric in artificial auxetic materials (e.g. metallic foams).

The results in this paper are obtained in-situ for fully saturated, ~~undrained (confined)~~ likely undrained and confined conditions and caused by small magnitude strains~~, which are conditions that~~. Such conditions differ considerably from those used in traditional laboratory techniques for determining elastic moduli (i.e., E, G, K, ν). Compared to the conditions experienced during a compressive laboratory test, or those described above by (Ji et al., 2018), the strains caused by EAT are very small. For example, the loading variations caused by the atmospheric tidal component S_2 is typically less than $9 \cdot 10^{-5}$ MPa (0.1 mH_2O), and the confining pressure caused by an artesian standing water level of 100 mH_2O equates to a confining

pressure of only 0.98 MPa. Laboratory results are also well known for demonstrating bias in the sample strength, with the
655 strength decreasing with the sample's increasing physical size. It has been found that this occurs due to the incorporation of heterogeneities in the sample at larger scales, such as minor lithological changes or discontinuities due to fracturing or jointing ~~(Cundall et al., 2008; Masoumi et al., 2016)~~(Cundall et al., 2008; Masoumi et al., 2016).

Alternative ~~subjective~~ in-situ methods, such as seismic based methods (Rau et al., 2018), still derived positive Poisson's ratios when passing through the same heterogeneous material at the same confining pressures. However, elastic moduli have
660 previously been shown to be frequency dependent when saturated and under confining pressure (Wang, 1993; Tutuncu et al., 1998). Here, we hypothesise that the low frequency of the EAT induced stresses (2 cpd $\approx 2.3 \cdot 10^{-5}$ Hz), compared to seismic propagated waves (1 to 100 $Hz \approx 86,400$ to $8,640,000$ cpd), causes a highly relaxed response which allows sufficient time for pressure redistribution (Tutuncu et al., 1998). In contrast, the seismic frequency produces a localised un-relaxed or un-drained response as the seismic waves pass through the subsurface, where this effect has been shown to change with the frequency
665 (Pimienta et al., 2016). Both ~~(Tutuncu et al., 1998) and (Pimienta et al., 2016)~~Tutuncu et al. (1998) and Pimienta et al. (2016) provide evidence of decreasing Poisson's ratios with decreasing frequency when below the typical undrained response domain (< 10 $Hz \approx 864,000$ cpd).

For small strains, as relevant for this study, Zaitsev et al. (2017) have shown that the occurrence of negative Poisson's ratios is not as exotic as previously thought. Considering the context of Cundall et al. (2008), Gercek (2007) and Ji et al. (2018),
670 the negative Poisson's ratios derived by TSA in this paper seem plausible. ~~Here, we propose that these are due to an interplay of simultaneous conditions for the in-situ determination~~We propose possible hypotheses for conditions, such as the scale of the effective sample size, anisotropic strain responses from heterogeneities, low confining pressures, ~~and~~ the low frequency and small strains caused by EATs, and boundary effects. Meeting the requirements of a negative Poisson's ratio at these small strains defined by (Lakes, 1991) as non-affine deformation (non-uniform between scales), non-central forces, and in a state
675 of pre-existing strain (e.g., from overburden). The geomechanical derivations of this paper (Section 2.4) are based on linear poroelasticity. However, the auxetic responses observed by Ji et al. (2018) occurs both linearly and non-linearly within the negative Poisson's ratio space, depending on the confining pressure and the type of material (Zaitsev et al., 2017). Currently, no relationships between EAT and nonlinear poroelastic theory has been established within the literature. Future work in this space should therefore consider the integration of nonlinear geomechanics (Khan et al., 1991; Johnson and Rasolofosaon,
680 1996).

To the best knowledge of the authors no explicit or robust relationships exist in the literature between elastic moduli results obtained in the field to those estimated from the laboratory testing of core (Leriche, 2017). Similarly, no in-situ method currently exists that can derive elastic estimates ~~of thousands of cubic meters of material(e.g., meters around~~ representative of the large volume of material, such as surrounding a well bore screen~~)~~, as has been proposed for Earth tides (Zhang et al., 2019). ~~Over such~~
685 ~~a large volume,~~ It is likely that heterogeneity within almost any geological media will produce an anisotropic strain response to either Earth or atmospheric tides ~~. Such anisotropy~~ over such a large volume. Anisotropy may result in apparently atypical properties, ~~such as~~ for example negative Poisson's ratios, and should be investigated for the generic assumption common to most hydro- or geomechanical investigations of a homogeneous, isotropic aquifer of infinite lateral extent.

4.5 Implications for passive quantification of subsurface hydro-geomechanical properties

There are several uncertainties associated with the findings of this paper, with implications for passive quantification of subsurface hydro-geomechanical properties. These uncertainties and limitations of the method are as follows:

- Although subjective estimates have been attempted (Zhang et al., 2019), the size and scale of the volume of influence from either ET or AT are unknown. It is also possible that there is a difference between the size of influence for ET and AT. Further research is required to elucidate the zone of influence the derived properties are representative for, such as numeric modelling.

- Currently the poroelastic response to EAT is considered to be linear. However, rocks have previously been shown to respond in a nonlinear manner for undrained, tri-axially loaded laboratory settings, particularly at small strains (Johnson and Rasolofosaon, 1996; Zaitsev et al., 2017). As in-situ derivations of rock mass (or sediment) poroelastic values without the use of assumed primary values (E, G, K, ν) is relatively novel, the implication of assuming linearity for the analysis of in-situ properties remains unknown and unverified.

- The mechanism behind ~~pre-strain~~ leaky responses is believed to be due to a partial drained response in the subsurface. However, the exact causes of such responses are still unknown. In order for the validity of a positive phase shift model to be proven, a more comprehensive understanding of such mechanisms must be further developed.

- Skin and well bore storage effects have been assumed to be negligible in this paper. However, these two effects will alter the phase responses to either Earth or atmospheric tides, as was shown in the recent work by Gao et al. (2020). It is important to note that any phase uncertainties mainly influence the hydraulic conductivity values. However, additional consideration of skin and well bore storage effects will increase the accuracy and confidence in results.

- We note that there is very little literature reporting values let alone ranges of grain compressibility for mineralogy other than quartz, as has been discussed by Rau et al. (2018). Since this is the only real unknown, further work is required to elucidate the effect of grain compressibility uncertainty on the accuracy of the parameter estimation.

Passively characterising the subsurface ~~with in-situ analytical measurements may change the way in which the confined subsurface is understood~~ using the groundwater response to natural signals may improve our understanding of the subsurface. For example, the possibility of auxetic behaviour of subsurface materials in undrained conditions (i.e. hydraulically coupled) will have implications for assessing compaction from groundwater estimates, or the stability of slopes and cuttings. Here, the low strain elastic estimates from TSA may provide a lower bounding end-member for plausible ranges of properties. With further study, it may be possible to infer poroelastic properties at different confining pressures and frequencies or to provide a more accurate in-situ determination of geomechanical rock properties (e.g. specific storage, strength, etc.) prior to excavation and construction of civil and mining projects. However, further research is required to elucidate the scope of validity (space and time) and transferability of hydro-geomechanical properties derived from different methods.

5 Conclusions

The method developed in this paper provides a comprehensive approach to estimate in-situ hydro-geomechanical properties using Tidal Subsurface Analysis (TSA), i.e. from the monitored groundwater response to Earth and atmospheric tides (EAT). Our new method first objectively disentangles the groundwater response to Earth tides (ET) and atmospheric tides (AT) for the dominant response frequencies (M_2 and S_2). Secondly, the approach uses the amplitude and phase responses to ET and AT to determine the complete hydro-geomechanical parameter space: Specific storage, hydraulic conductivity, porosity, shear, Young's and bulk modulus, undrained and drained Poisson's Ratio, Skempton's and Biot-Willis coefficients. Unlike previous research, our new approach does not require an a priori estimate of the Poisson's ratio. However, the application to consolidated systems requires an estimate for the grain compressibility for which literature based values can be ~~used~~considered if available.

Application of our new method to five groundwater and barometric pressure records from four different hydrogeological settings delivers physically realistic results that are consistent with previous estimates. However, we reveal that the in-situ estimates of Poisson's ratio are consistently negative indicating auxetic behaviour. A closer look at the literature reveals that this is not unrealistic and can be attributed to an interplay between simultaneous in-situ conditions that differ from those of established laboratory tests. These include a larger effective sample size with scaling effects, anisotropic strain responses due to heterogeneities (e.g., micro-cracking), significantly lower confining pressures, and the small strains at low frequencies caused by the EATs.

Our approach allows estimation of the complete hydro-geomechanical parameter space in a passive way, i.e. from monitoring records of groundwater pressure head, measured atmospheric pressure and calculated ET. The primary advantage is that all parameters are determined for the same in-situ conditions and that the estimated values therefore should be internally consistent. The new method enables site-specific heterogeneity to be evaluated, as was shown by the two evaluated records from sandstone bores, providing hydro-geomechanical properties of the rock mass rather than small scale estimates on intact rock. This is a clear advantage to methods that require taking samples to the laboratory where replicating field conditions such as in-situ confining pressure and representative scale can be problematic. However, our method also raises the need for further research in key areas where significant uncertainties remain, for example the possibility for non-linearity of the poroelastic response to surface loading and Earth tide forces. Addressing the identified uncertainties could contribute towards improving subsurface monitoring and characterisation in both consolidated and unconsolidated systems.

Code and data availability. Data and code will be made available in a repository if this work is accepted for publication.

Author contributions. TCM and GCR conceived the idea for this paper. TCM and GCR analysed the data and made the figures. MSA and WAT contributed with datasets and suggested improvements.

Competing interests. The authors declare no competing interests.

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