

“Flooding in the Mekong Delta: Impact of dyke systems on downstream hydrodynamics” by Vo Quoc Thanh et al.

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Responses to Referee #1's comments

The authors use a 1D/2D hydrodynamic model that covers the Mekong Delta including its rivers, major canals and extending into the continental shelf in the surrounding ocean, to investigate the impact of protecting agricultural areas with high-dykes on the river hydrodynamics. They found that (a) High dykes (particularly those in Long Xuyen Quadrangle (LXQ), Plains of Reeds (PoR)) have wide-spread impact on the flow downstream (b) has impact on inland tidal effects. Recent literature is well covered. Reasonably well-written introduction. Language use is generally adequate (though there are a number of technical issues that need correcting.) However, the paper is not easy to read as it is organized in such a way that a lot of (seemingly unnecessary) material is mixed with the main narrative of the paper.

Authors' response: Thank you for the general comment on the paper. We will try to improve the paper structure to be easily readable.

1. What is the benefit of modelling the continental shelf? This is not an oceanographic/coastal engineering study. Your focus (as stated) was to investigate the impact of construction of high-dykes on the flow regime of the river system. You are also not considering highly dynamic ocean impacts like storm surge. What is the drawback of stopping the model at the river mouth and providing tidal boundary conditions with sea level there? You might have good reasons for this approach. If so, they need to be explained.

Authors' response: Regarding the modelling grid, we extended the modelling grid, including the shelf, in order to completely contain the river plume. Although the objective of this study is to investigate the impact of high-dyke constructions, we analyse hydrodynamics of the stations at the river mouths. In fact, the river discharge has contribution to sea levels at the estuary mouths (Kuang et al. 2017). Therefore, we included the shelf to investigate the impact and presented in the Table 4. If the modelling domain is limited at the river mouths, this impact would be excluded. As another reason, this approach is suitable toward salinity and sediment transport modelling.

2. One year of simulation is a short period to obtain meaningful results. I think it is important to cover at least several years of flow data as such data for this case study is available. Is there a barrier to doing that?

Authors' response: We partly agree with you that one year is a short period. However, it includes seasonal variation which is one of the main characteristics of the annual floods in the Mekong Delta. The model was used to compute for the floods in 2000 and 2001, but we analysed the flood in 2000.

It is possible to run a multiple year simulation, but it is quite difficult to select a suitable period because it contains different water years. We selected the flood in 2000 because it is one of the most severe floods recently. The highest water levels of the flood in 2000 is used as a reference for construction of flood prevention. Thus selecting a severe flood to evaluate impacts of high dykes could determine the maximum possible impacts of these construction on downstream hydrodynamics. To define a suitable period, it should consist of a low water flow year (2008), a moderate water flow year (2004) and high water flow year (2000) based on water levels at Tan Chau station. Another approach could use the long-term average hydrograph (MRC 2009) as the boundary conditions.

3. The point of departure (and justification for the methodology) of this paper seems to be the fact that previous studies could not able to predict the water level at the river mouth. If this is the sole justification to use a numerically expensive 2D model that includes continental shelf, the importance of obtaining those figures should be explained.

Authors' response: As we responded to the comment, the major reason to include the shelf is to investigate the possible changes of water levels at the river mouths. The previous studies are usually 1D models and their boundaries are defined at the river mouths so they are not able to calculate water level changes at the river mouth stations.

4. The point of doing a tidal harmonic analysis is unclear to me. Just testing the impact on the tidal range (amplitude) would have covered all the matter that is relevant to the central theme of the paper. Removing the tidal harmonic analysis part would shorten the paper - definitely would contribute to making it more readable and to the point.

Authors' response: Thanks for the suggestion to make the paper more readable. However, results of the tidal harmonic analysis are important because hydrodynamics in the Mekong Delta downstream are strongly influenced by tides. Thus amplitudes of the eight main tidal constituents are suitable indexes to indicate tidal propagation changes. Using these indexes can eliminate effects of the fluvial floods.

5. The authors should discuss the performance of the model. This is particularly important as many previous studies have used (much simpler) 1D modelling approach to arrive at similar results. How much is the computational effort? How does it compare with those reported in previous studies? What is the justification to use this modelling approach despite its expense (if that is the case)?

Authors' response: We will revise the "Model performance" section. The modelling approach in this study overcomes a limitation of the previous 1D models. This can help to understand hydrodynamics at the river mouths where rivers and oceans interact. We will compare the performance of this model with the previous models, but the model performance should reach a reasonable level. The model performance determined by the Nash-Sutcliffe efficiency (NSE). We compared the NSE values to the previous studies (e.g. Dang et al., 2018; Tran et al., 2017; Triet et al., 2017). NSE values of this model are slightly lower than the mentioned studies, but it is generally in the *good* category based on (Moriassi et al. 2007) evaluation. The reason to use this modelling approach because its advantages, as mentioned in the comment 3's response.

6. So many figures and many descriptions on model validation performance. This is an important topic to cover, but it is overdone in this case. Just one paragraph on how the model performed during validation and if absolutely necessary, one map showing validation results. Much of this can be moved to an appendix. In fact, it's best that they are presented as an online supplement rather than an Appendix, so as to keep the paper succinct and to the point.

Authors' response: Thank you for your suggestion. We will take this comment to revise the paper.

7. Scenarios need a better explanation. For example how much is protected with high dykes in "Dyke VMD" scenario? What is the basis?

Authors' response: We will revise the paper with more details. The basic is the base scenario of the flood 2000, without high dykes. (Duong et al. 2016) found that there is no high dyke in the Vietnamese Mekong Delta before 2000.

8. Water balance diagrams and descriptions are hard to understand. Please check the literature for much clearer ways of presenting these.

Authors' response: Thank you for your suggestion.

9. Lastly, it is important to place the findings within the context of other changes. Are these significant for example impact of climate change on upstream Mekong flow, dam construction, sea-level rise etc.? Some discussion on such issues is warranted.

Authors' response: Thank you for comments.

Comments on the manuscript

Thanks for comments and suggestions on the manuscript. We will correct them.

Authors' response: The number of the annual flood volume at Kraie is estimated by about 416 km³. The reviewer#1 suggests the flood volume of 475 km³ is an estimate of the whole Mekong River.

1. what is the problem of dyke ring with mixed heights (obviously the lowest point determines the level of protection)

2. Why it happens in this methodology (or what exactly happens: mixed height dyke rings misidentified as single height in the model?)

3. What is the relationship to ignoring small canals?

Authors' response: The lowest elevation of a dyke ring determines the level. The canal system in the Vietnamese Mekong Delta is dense, but the model considered the primary and secondary canals. The tertiary canals were excluded. Figure 1 presents an example of high dykes and low dykes. Unfortunately, the canal between low dykes and high dykes was excluded. The areas protected by

high dyke is smaller than those protected by low dykes in the blue polygon (Figure 1b), so we assume that the blue polygon is defined as a low dyke in modelling.



Figure 1. An example of high dykes and low dykes; (a) high dykes in orange and low dykes in green; (b) the blue line presents a low dykes in modelling. (After Triet et al. 2017)

is the kurtosis discussion necessary? This is useful if you do statistics of hundreds of hydrographs, but in this case we can clearly see that slipstream's are flatter by looking at the hydrographs.

Authors' response: Although the different shapes of hydrographs are recognised by looking, the kurtosis helps to determine the differences.

In this study's context, there is little value in doing a tidal harmonics analysis in my opinion. What you wanted to show is the high dyke development will affect the tidal impact on the river level in inland locations, while dykes do not affect it much in the coastal locations, as far as I could understand. A single graph showing the change in tidal range under different scenarios in various stations would show this adequately.

Authors' response: As we responded to the comment 4, the results of tidal harmonic analysis help to understand tidal propagation which is indicated by the diurnal constituents (K1, O1, P1 and Q1) and the semidiurnal constituents (M2, K2, N2, and S2). In my opinion, using these indexes to present tidal variation is better than tidal ranges at which water levels are influenced by the annual floods.

QLPH - is it a region or the project name? Please mark this area/infrastructure on a map (not at a new figure but in figure 4 or 1) and refer to it here.

Authors' response: QLPH is a project which was constructed for water management in the coastal Mekong Delta (Hoanh et al. 2012). It is on the Figure 1.

References

Dang, Duc Thanh, Thomas A Cochrane, Mauricio E Arias, and Van Pham Dang. 2018. "Future Hydrological Alterations in the Mekong Delta under the Impact of Water Resources Development, Land Subsidence and Sea Level Rise." *Journal of Hydrology: Regional Studies* 15(November 2017): 119–33. <https://doi.org/10.1016/j.ejrh.2017.12.002>.

Duong, V. H. T. et al. 2016. "Geographical Impact of Dyke Measurement for Land Use on Flood Water

in Geographical Impact of Dyke Measurement for Land Use on Flood Water in the Mekong Delta.” In 8th Eastern European Young Water Professionals Conference - IWA, , 308–17.

Hoanh, Chu Thai et al. 2012. “Modelling to Support Land and Water Management: Experiences from the Mekong River Delta, Vietnam.” *Water International* 37(4): 408–26.
<http://www.tandfonline.com/doi/abs/10.1080/02508060.2012.706797>.

Kuang, Cuiping et al. 2017. “River Discharge Contribution to Sea-Level Rise in the Yangtze River Estuary, China.” *Continental Shelf Research* 134(June 2016): 63–75.
<http://dx.doi.org/10.1016/j.csr.2017.01.004>.

Manh, N. V. et al. 2014. “Large-Scale Quantification of Suspended Sediment Transport and Deposition in the Mekong Delta.” *Hydrology and Earth System Sciences Discussions* 18: 3033–53. <http://www.hydrol-earth-syst-sci-discuss.net/11/4311/2014/>.

Moriasi, D. N. et al. 2007. “Model Evaluation Guidelines for Systematic Quantification of Accuracy in Watershed Simulations.” *Transactions of the ASABE* 50(3): 885–900.
<http://elibrary.asabe.org/abstract.asp??JID=3&AID=23153&CID=t2007&v=50&i=3&T=1>.

MRC. 2009. Annual Mekong Flood Report 2008. *Vientiane*.

Tran, D D, and J Weger. 2017. “Barriers to Implementing Irrigation and Drainage Policies in An Giang Province, Mekong Delta, Vietnam.” *Irrigation and Drainage*.

Tran, Dung Duc et al. 2018. “Assessing Impacts of Dike Construction on the Flood Dynamics in the Mekong Delta.” *Hydrology and Earth System Sciences Discussions* 22: 1875–96.
<https://www.hydrol-earth-syst-sci-discuss.net/hess-2017-141/>.

Triet, Nguyen Van Khanh et al. 2017. “Has Dyke Development in the Vietnamese Mekong Delta Shifted Flood Hazard Downstream?” *Hydrology and Earth System Sciences* 21(8): 3991–4010.

Van, P. D T et al. 2012. “A Study of the Climate Change Impacts on Fluvial Flood Propagation in the Vietnamese Mekong Delta.” *Hydrology and Earth System Sciences* 16(12): 4637–49.

Responses to Referee #2's comments

Overview

The paper describes the effects of the high dykes structures on the complex interaction of the high flows and tides in Mekong Delta. The authors use 1D-2D model Delft3d Flexible Mesh model on unstructured grid to simulate the hydrodynamics at the river branches, canals and the adjacent ocean shelf with certain improvements compared to the previous studies done on Mekong Delta.

No major concerns about the language use, however the manuscript would benefit from the proofread by a native speaker. The research question is clearly stated and addressed in the Discussion section.

Authors' response: Thank you so much for your suggestion.

The main concerns

The overall structure of the manuscript is sufficient, however the abundance of details (some of those are unnecessary in my opinion) makes it difficult to read. The modelling part of the study needs major revisions (see comments below). Moreover, the figures should be revised. Therefore, I suggest major revisions of content further on.

Authors' response: Thank you for your suggestion. We will revise them.

General comments

Introduction. *This section is somewhat well-structured and clear, however some information which is repetitive or not directly related to the study objective should be removed. For instance, speaking of the soil quality or challenges in agriculture may confuse the reader in the beginning. One sentence should be enough for it.*

Authors' response: Thank you for your suggestion. We will revise them. However, the soil quality and agriculture influence the high dyke development in the Vietnamese Mekong Delta because the high dykes are built to protect agricultural crops.

Methods. *The following aspects need to be addressed:*

- *Input data. The authors use SRTM data to run the 2D part of the model. SRTM is known to have rather large vertical and horizontal errors, however, there is no overview related to the data pre-processing, resolution used and possible errors coming from the input data accuracy. Some of the simulated water level changes are far smaller than the vertical accuracies of the terrain and bathymetry data. This aspect should be deliberately described in the Methods and Discussion section. The following paper might be useful: Hawker, L. P., Rougier, J., Neal, J. C., Bates, P. D., Archer, L., & Yamazaki, D. (2018). Implications of simulating global digital elevation models for flood inundation studies. *Water Resources Research*, 54. <https://doi.org/10.1029/2018WR023279>.*

Authors' response: We only use the SRTM data for the floodplains. The bathymetry of 2D part for rivers were extract from the 1D ISIS model. The floodplains of the Mekong Delta are flat (D D Tran and Weger 2017). The floodplain topography only influence during high flow season when the floodplains are inundated. Another reason is that the SRTM data for the Mekong Delta was efficiently used to simulate flood inundation (e.g. Dung Duc Tran et al. 2017; Triet et al. 2017). Therefore, we believe that the SRTM data are efficient to simulate the floods in the Mekong Delta.

- *High dyke geometry should be described (design return period, crest elevation, precise location etc.) and how they were incorporated into the computational mesh.*

Authors' response: The dyke geometry is various, as presented in the Figure 4 in the manuscript. The dyke geometries depend on the canal network. According to the Department of Agriculture and Rural Development of An Giang province, the high dykes are built, with their crest levels are higher than the peak water level of the flood 2000. Thus a high dyke in computation is defined as 2D dry grids. This modelling approach of floodplains and canals is based on field observations, as presented in Figure 2 and Figure 3.

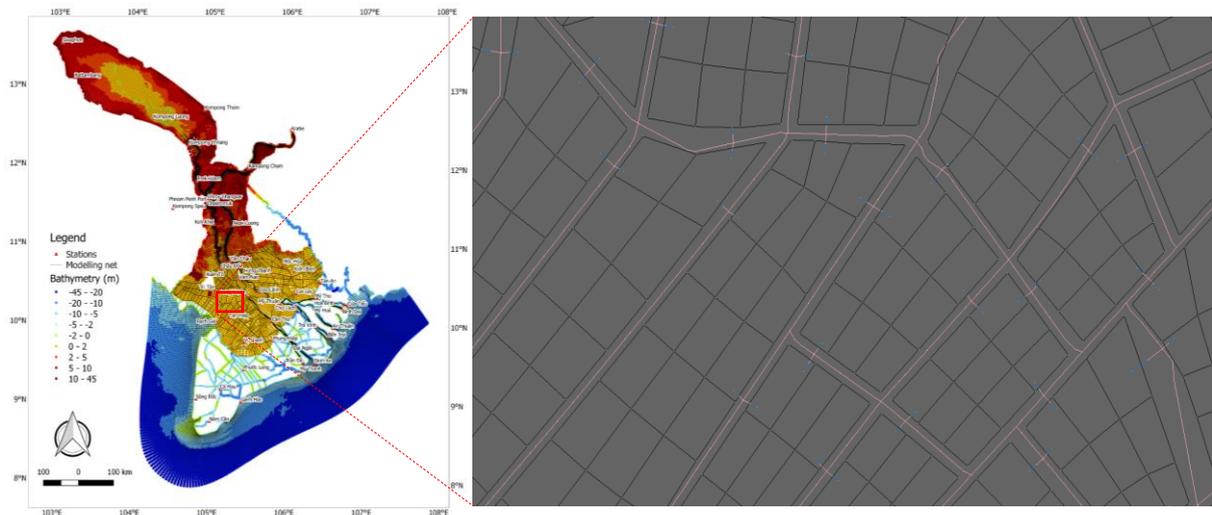


Figure 2. The modelling grid. The floodplains are defined as 2D grids (in black) and canals are defined as 1D networks (in pink). The connecting links of 1D network and 2D grids are in blue.



Figure 3. A high dyke and non/low dyke in An Giang province and their schematization in modelling. Photos by Vo Quoc Thanh 2012.

- *Simulation time, if possible, should be increased to more years. One year is not sufficient enough to track the system behaviour in given setting and may cause biases.*

Authors' response: As the literature review, the major factor influencing seasonal variation of fluvial flows is the Tonle Sap Lake storage. This feature is considered by using initial conditions which is simulated water levels of the previous flood.

- *The authors mention and illustrate throughout the manuscript high dyke as both, a flood protection measure and a dyke protected floodplain at the same time. This causes ambiguity and should be further specified/changed.*

Authors' response: We will correct it.

- *More details about the developed scenarios need to be added (see the dyke geometry).*

Authors' response: We will add more information about the developed scenarios.

Results. *The calibration/validation outcomes description should be substantially reduced, as it overloads the section. Computational time should be mentioned.*

Authors' response: Thank you. We will revise it.

Discussion. *I would recommend restructuring the discussion by answering the research questions in the same order as outlined in the Introduction section. It should be pointed out how the study benefited from using 2D code compared to 1D (MIKE11 or ISIS). The limitations related to data and methods used have to be put in a separate sub-section.*

Authors' response: Thank you for recommendations. We will revise it.

Conclusion. *Implications and future work should be added.*

Authors' response: Thank you for recommendations. We will revise it.

Figures. *Geographical names and symbols should be consistent throughout the manuscript.*

Authors' response: Thank you for recommendations. We will correct them.

P.2 line 17 – the sea level rise and land subsidence is an important point in the study area, the data about the future projections can be mentioned. However, it is important to point out why they are not considered in the paper (complexity, uncertainty, etc.).

Authors' response: Thank you for comments. We will revise it.

P.3 line 4 – sentence “These outcomes... “ belongs to Conclusion

P.3 line 15- Figure 4 comes right after Figure 1

P.4 line 3 – the reference seems to be outdated

P.4 line 12 – return period of the event should be specified

P.4 line 14 – sentence “The water...” is ambiguous

P.4 line 31 – units should be specified

Authors' response: Thank you for comments. We will correct them.

P.7 line 21 - the grid density is mentioned to be sufficient, however the grid cell sizes seem to be quite large in fact. It would be useful to have some more explanation for the readers who are not familiar with Delft3DFM model.

Authors' response: Thank you for comments. We will revise it.

P.7 line 32 – floodplain bathymetry topography

Authors' response: Thank you for comments. We will correct it.

Table 1 can be removed

Authors' response: Thank you for comments. We will correct it.

Figure 1. The map is hard to read. I would recommend changing the following: reduce thickness of canal network, make the flood zones boundaries and fill more pronounced. The outline of municipalities (the faded orange line) should be removed. It is better to avoid overlap of green and red colours if possible

Authors' response: Thank you for suggestion. We will edit the figure.

Figure 2 and 3. What is meant here by high dykes? Dyke protected floodplains or flood defence? Should be clearly stated. Is there a way to combine two figures in one or incorporate in the Figure 4?

Authors' response: We will clarify. The figure 2 and 3 present floodplain areas protected by high dykes until 2011. The information of Figure 2 and 3 was included in Figure 4.

Figure 4. One of the north arrows should be removed. Region boundaries are not visible

Authors' response: Thank you for comments. We will correct it.

Figure 7. Everything that is above the sea-level (0m a.s.l.) is topography

Authors' response: Thank you for comments. We will correct it.

Figure 8 and 9. In legend the sign ">=" should be corrected

Authors' response: Thank you for comments. We will correct it.

Figure 10 and 11. The legend which specified the thickness of red arrows should be added

Authors' response: Thank you for comments. We will correct it.

References

- Dang, Duc Thanh, Thomas A Cochrane, Mauricio E Arias, and Van Pham Dang. 2018. "Future Hydrological Alterations in the Mekong Delta under the Impact of Water Resources Development, Land Subsidence and Sea Level Rise." *Journal of Hydrology: Regional Studies* 15(November 2017): 119–33. <https://doi.org/10.1016/j.ejrh.2017.12.002>.
- Duong, V. H. T. et al. 2016. "Geographical Impact of Dyke Measurement for Land Use on Flood Water in Geographical Impact of Dyke Measurement for Land Use on Flood Water in the Mekong Delta." In 8th Eastern European Young Water Professionals Conference - IWA, , 308–17.
- Hoanh, Chu Thai et al. 2012. "Modelling to Support Land and Water Management: Experiences from the Mekong River Delta, Vietnam." *Water International* 37(4): 408–26. <http://www.tandfonline.com/doi/abs/10.1080/02508060.2012.706797>.
- Kuang, Cuiping et al. 2017. "River Discharge Contribution to Sea-Level Rise in the Yangtze River Estuary, China." *Continental Shelf Research* 134(June 2016): 63–75. <http://dx.doi.org/10.1016/j.csr.2017.01.004>.
- Manh, N. V. et al. 2014. "Large-Scale Quantification of Suspended Sediment Transport and Deposition in the Mekong Delta." *Hydrology and Earth System Sciences Discussions* 18: 3033–53. <http://www.hydrol-earth-syst-sci-discuss.net/11/4311/2014/>.
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- MRC. 2009. Annual Mekong Flood Report 2008. *Vientiane*.
- Tran, D D, and J Weger. 2017. "Barriers to Implementing Irrigation and Drainage Policies in An Giang Province, Mekong Delta, Vietnam." *Irrigation and Drainage*.
- Tran, Dung Duc et al. 2018. "Assessing Impacts of Dike Construction on the Flood Dynamics in the Mekong Delta." *Hydrology and Earth System Sciences Discussions* 22: 1875–96. <https://www.hydrol-earth-syst-sci-discuss.net/hess-2017-141/>.

- Triet, Nguyen Van Khanh et al. 2017. "Has Dyke Development in the Vietnamese Mekong Delta Shifted Flood Hazard Downstream?" Hydrology and Earth System Sciences 21(8): 3991–4010.*
- Van, P. D T et al. 2012. "A Study of the Climate Change Impacts on Fluvial Flood Propagation in the Vietnamese Mekong Delta." Hydrology and Earth System Sciences 16(12): 4637–49.*

Responses to Referee #3's comments

The study aims at evaluating the impact of high dykes on the hydrodynamic behavior of the Mekong river. The analysis is based on a 1D-2D model that reproduces the topographic characteristics of the river, as well as different dyke ring configurations. The work does not infer specific research questions but it is aimed at increasing the knowledge of the river dynamics and its behavior in the light of the recent construction of a complex dyke system. The manuscript is in general well written, even if it results sometimes heavy and difficult to follow due to many details regarding the study area. I have some doubts concerning the scientific contributions of such kind of studies, even though the publication could be justified by the importance of the study area and the relevance of the river dynamics investigated. That said, the current manuscript fails in specifying what are the added knowledge provided compared to previous investigations performed in the same area, and with the same objective (see e.g. Tran et al., 2018). Hereafter some major and minor comments that need to be addressed before considering the manuscript for publication.

Authors' response: Thank you for your comment. We will respond to your comments and revise the manuscript.

Comparing the previous studies, for instance (Dung Duc Tran et al. 2018; Triet et al. 2017), this study contributes to understanding possible impacts if the high dykes built to protect the entire VMD floodplains. Additionally, we found that the dyked floodplains in the LXQ and PoR not only influence water regimes on its directly linked Mekong' branch, but also on the other branches. We assessed impacts of high dykes on tidal propagation.

- One of the most relevant concern regarding the study is that it refers to only one year of data. The model has been calibrated and validated considering the 2000 and 2011 floods, respectively. After that, all the considerations regarding the river dynamics have been carried out referring to the event used for the calibration. However, Figure 5 clearly shows that the 2000 flood is different from the average condition. Thus, the question is: how representative is this event of the behaviors of the river network? The behavior of the different river branches and the way they interact depend on the specific contributions of the different basins. This to say that this analysis evaluates only a specific event, which might (is?) not be representative of the general river condition. As a matter of fact, previous studies investigating the same aspects (dyke effects) considered longer periods.

Authors' response: Thank you for your comment. The model was used to compute for the floods in 2000 and 2001, but we analysed the flood in 2000. It is possible to run a multiple year simulation, but it is quite difficult to select a suitable period. We selected the flood in 2000 because it is one of the most severe floods recently. The highest water levels of the flood in 2000 are used as a reference for construction of flood prevention. Thus selecting a severe flood to evaluate impacts of high dykes could determine the maximum possible impacts of these construction on downstream hydrodynamics.

- The difference relative to the study of Tran et al. (2018) is sometimes cited in the document. However, the Authors should better specify the differences and the added knowledge ensured by this

study. Also, are the results in line with previous findings? If not, how do you justify the difference? Does this study provide new information and knowledge relative to what was already known?

Authors' response: This study provides a new modelling approach for the Mekong Delta which can overcome the limitation of the existing 1D models in order to simulate hydrodynamics at the river mouths. In addition, this approach may consider effects of coastal processes (e.g. waves, storm surge). Compared to the study of Tran et al. (2018), we included impacts of high dykes in PoR and TransBassac and the results of this study show a similar increase of water levels. Besides, we found that the dyked floodplains in the LXQ and PoR not only influence water regimes on its directly linked Mekong' branch, but also on the other branches. Moreover, we investigated the impacts of high dykes on tidal propagation along the Mekong River.

- Differences in terms of water elevation are in most of cases very minimal and of the same magnitude of the error of the model. What is the representativeness of such results. How can you exclude that those limited variations obtained among different configurations do not depend on the model itself, or on the way it reproduces the interaction between river and dyke rings?

Authors' response: We found that the water volume stored in the Vietnamese Mekong Delta's floodplains is much smaller than the annual flood volume. Thus this causes the small changes among the cases. To avoid the errors, the model spin-up time covers the 1999 flood. By using the same model setup, except the high dyke configuration, we believe that the differences are caused by high dykes.

- The overall model description should be improved. The model structure covers a key role in the overall study and additional details should be reported. The distinction between high and small dykes is not clear unless you are familiar with the study area.

Please provide additional explanation about the model structure.

Authors' response: Thank you for your comment. We will revise it. We only consider impacts of high dykes and assume that low dykes and non-dykes allow floodplain inundation. This modelling approach of floodplains and canals is based on field observations, as presented in Figure 2 and Figure 3.

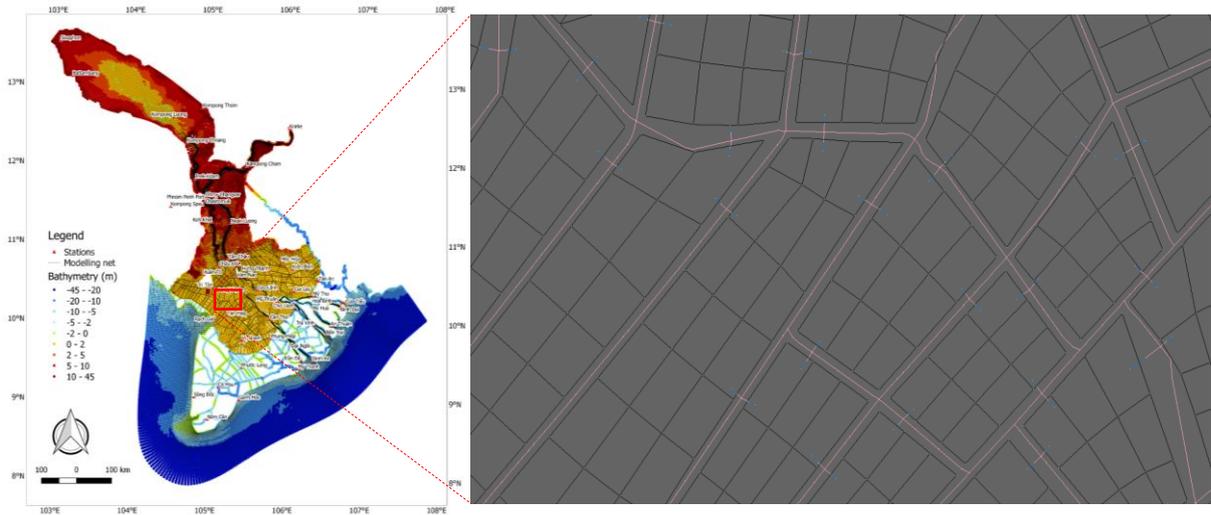


Figure 4. The modelling grid. The floodplains are defined as 2D grids (in black) and canals are defined as 1D networks (in pink). The connecting links of 1D network and 2D grids are in blue.



Figure 5. A high dyke and non/low dyke in An Giang province and their schematization in modelling. Photos by Vo Quoc Thanh 2012.

- Following all the hydrological details reported in the manuscript is sometimes difficult. Please always remind that a reader might not be familiar with the cited locations. All the cited locations should be identified within a Figure. For example, I would recommend adding a figure explaining the different geometric configurations and the dyke systems considered in the different cases. For example, what are the dyke system considered for the configuration LXQ, PoR, etc.?

Authors' response: Thank you for your suggestion. We will check the mentioned locations and locate in a map. We will revise the Section 2.3 *High dyke development scenarios* with more details.

- P2L9: which kind of hydraulic structures?

Authors' response: We introduced the hydraulic structures in general. The hydraulic structures are commonly high dykes in the flood-prone areas and sluice gates for salinity prevention in the coastal areas.

- P3L15: please put the figure in relation to their citation order. Figure 4 is cited in the text before figures 2 and 3.

Authors' response: We will correct it.

- P4L7-16: consider adding a scheme to better explain dykes interaction.

Authors' response: We revised the description.

- Figure 2 and 3: why the number of dykes is significant? Probably reporting their overall length is more relevant.

Authors' response: In my opinion, the high dykes are separated by the canal system. Thus the mean areas of floodplain protected by high dykes can reflex flood water conveyance. For example, in the same area of floodplain protected by high dykes, the smaller mean area has higher water conveyed capacity.

- P7L24: I think here you should refer to "dike ring". How do you manage, within the model, areas partially protected with high dykes and partially not?

Authors' response: In the case of a flood compartment containing a high dyke and a non-dyke, we used a ratio of areas of floodplains protected by the high dyke and the non-dyke. The larger area of floodplains will define the type of dykes.

- Section 2.3: I would suggest using the term "configuration" instead of "scenario". Scenario is usually referred to identify different hydrological conditions (e.g. events of different return periods), while configuration sounds more appropriate for taking into account different topographic characteristics of the river network. Please make those locations clear using a map.

Authors' response: Thank you so much for your suggestion. The study of (Dung Duc Tran et al. 2018) which was published in the HESS journal also use "scenario", so we prefer to use "scenario" to make it consistent.

- Sometimes the structure of the paper is "heavy". Please consider to simplify it by removing some sub-subsections, such as 2.4.1, 2.4.2...etc.

Authors' response: We will edit them.

- P11L8: any discussion about the calibrated Manning coefficients: are they reasonable? Are they in agreement with those of previous studies?

Authors' response: As mentioned in the calibration method section, we started to calibrate the model with calibrated roughness values from Manh et al. (2014) and Van et al. (2012). Thus the

calibrated roughness values are in agreement with these studies. However, there are slight differences in the coastal areas. We added more details in section 3.1.

- P13L31: *Is it relevant reporting a difference of 0.6 cm? How reliable is this estimation?*

See also my previous comment on that.

Authors' response: This increase is reasonable because this is yearly mean increase.

- *Figure 1: green areas should be reported as "high dyke protected areas". The same in Figure 4.*

Authors' response: Thank you for your suggestion. We will edit it.

- *Figure 7: check the unit of measure: m a.s.l. ?*

Authors' response: Thank you for your suggestion. We will edit it.

- *Figure 10-11: is the arrow dimension proportional to the flow? In case specify or add a legend.*

Authors' response: We will add a legend.

- *Figure 13: I was not able to find some of these stations in a map. Please add a reference to a map where those stations are visible.*

Authors' response: Thank you for your suggestion. We will add it.

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Flooding in the Mekong Delta: Impact of dyke systems on downstream hydrodynamics

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Abstract. Building high dykes is a common measure to cope with floods and plays an important role in agricultural management in the Vietnamese Mekong Delta. However, the construction of high dykes causes considerable changes in hydrodynamics of the Mekong River. ~~Therefore, this paper aims to assess the impacts of the high dyke system on water level fluctuations and tidal propagation on-in the Mekong River branches using a modelling approach. We developed a coupled 1D-2D In order to consider interaction between rivers and seas, an unstructured modelling grid was generated, with 1D-2D coupling, with Delft3D Flexible Mesh software. The model domain covered the Mekong Delta and extending to the East (South China Sea) and West (Gulf of Thailand) seas, while scenarios included the presence of high dykes in the Long Xuyen Quadrangle (LXQ), Plains of Reeds (PoR) and TransBassac regions. The model was manually calibrated for the flood-year 2000 high flow season, of the year 2000. To assess the role of floodplains, scenarios consisting of high dykes built in different regions of the Long Xuyen Quadrangle (LXQ), Plains of Reeds (PoR) and TransBassac were carried out. Results show that inclusion of high dykes varies the percentages of seaward outflow through the different Mekong branches and slightly redistributes flow over the low flow and high flow seasons. The LXQ and PoR high dykes result in an increase of daily mean water levels and a decrease of tidal amplitudes in their adjacent river branches. Moreover, the different high dyke systems not only have an influence on the hydrodynamics in their own branch, but also on other branches because of the connecting channel of Vam Nao. These conclusions also hold for extreme flood scenarios of 1981 and 1991 that had larger peak flows but smaller flood volumes. Peak flood water levels in the Mekong Delta in 1981 and 1991 are comparable to the 2000 flood since peak floods decrease and elongate due to upstream flooding in Cambodia. Future studies will focus on sediment pathways and distribution as well as climate change impact assessment.~~

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1 Introduction

- 30 Rivers are the major source of fresh water supply for human use (Syvitski and Kettner, 2011). In addition the fresh water supply is an important resource for ecosystems. When river discharge exceeds ~~its~~ ~~their~~ ~~s~~² bank full discharge, ~~its~~ ~~their~~ floodplains

inundate. The fluvial floods bring both advantages and disadvantages to local residents. Floods are the main source of fresh water supply and deliver sediments as a natural and valuable fertilizer source for agricultural crops (Chapman and Darby, 2016). This is an important process in the Mekong Delta since the majority of local citizens ~~is~~are farmers. In contrast, extreme floods may damage crops and infrastructure.

5 In order to maintain agricultural cultivation during ~~flood~~the high flow seasons, dyke rings have been built to protect agricultural crops in the Vietnamese Mekong Delta (VMD). As a result, the river system in the VMD has significantly changed, especially after the severe floods in 2000 (Biggs et al., 2009; Renaud and Kuenzer, 2012). A dense canal system has been created in flood-prone areas to efficiently drain flood waters from the Long Xuyen Quadrangle and the Plains of Reed to the West Sea (Gulf of Thailand) and to the Vamco Rivers, respectively (Figure 1).

10 Recently large hydraulic structures have been built not only in the flood-prone areas but also in the coastal areas to protect cropping systems from saline intrusion. Therefore, hydrodynamic processes have considerably changed. Understanding the prevailing hydrodynamics is essential for sustainable water management in these regions.

The high dyke system is intended to reduce local natural flood hazards, but may alter the hazard downstream (Triet et al., 2017). Besides, they also increase potential risk due to dyke breaks. Following different approaches, Tran et al. (2017) found

15 that the high dyke system in the upstream of VMD causes an increase of the peak water levels in the downstream areas. However, water levels at these downstream stations are highly dominated by tidal motion. In fact, tides may result in an increase of water levels in the central VMD. Thus an analysis of tidal fluctuation is needed to investigate water level changes

on the Mekong River. ~~They evaluated that~~ ~~the~~ The high dyke system may be an important factor, but ~~that~~ sea level rise in combination with land subsidence enhances peak water levels at the central stations to a larger extent (Triet et al., 2017). ~~Thus~~

20 ~~an analysis of tidal fluctuation is needed to investigate water level changes on the Mekong River.~~ The high dyke system influences not only on the downstream hydrodynamics ~~due to~~ by reducing inundated floodplain areas, but also on fluvial sediment deposition on floodplains.

There is a number of large-scale numerical models used for simulating the annual floods, suspended sediment transport and evaluating impacts of dyke construction in the Mekong Delta (Manh et al., 2014; Tran et al., 2018; Triet et al., 2017; Van et

25 al., 2012; Wassmann et al., 2004). For instance, Tran et al. (2017) investigated the impacts of the upstream high dyke system on the downstream part of the VMD. By using a MIKE hydrodynamic model for Mekong Delta, they found that the high dykes system in the LXQ can reduce the discharge of the Song Tien, diverting around 7% of the total volume to Song Hau. In addition, the yearly discharge variations have slight effects on the peak of water levels at Can Tho station, while Triet et al.

(2017) found that the high dyke system caused an increase of the flood peaks from 9 to 13 cm at the central VMD stations.

30 Moreover, Triet et al. (2017) show that the development of the dyke system upstream of the VMD reduces flood retention in this area, leading to a rise of 13.5 cm and 8.1 cm in the peak water levels in the downstream part of the VMD at Can Tho and My Thuan, respectively.

The studies mentioned previously evaluated the impact of the high dykes in the LXQ and the high dykes developed until 2011. The impact of the other floodplain regions needs to be considered, including the LXQ, PoR and TransBassac. Additionally,

(Manh et al., 2014; Tran et al., 2018; Triet et al., 2017) used a common version one-dimension model of MIKE11 for the Mekong Delta and the downstream boundaries are defined at the Mekong's river mouths. However, Kuang et al. (2017) found that river flows can contribute to a rise of water level at the river mouths. Thus, in the present study another modelling approach is used in order to address these issues.

- 5 This study aims at assessing the impacts of the high dyke system on water level fluctuation and tidal propagation on the Mekong River branches. An unstructured, combined 1D-2D grid, ~~covering the Mekong Delta and extending to the East (South China Sea) and West seas,~~ is used to simulate the flood dynamics in 2000. The model domain covers the Mekong Delta and extends from Kratie in Cambodia to the East (South China Sea) and West seas. Simulated scenarios present the impact of high dykes in different floodplain regions and the entire VMD. ~~The specific objectives are: These outcomes will benefit sustainable~~
- 10 ~~water management and planning in the VMD:~~
- To develop a calibrated and validated hydrodynamic model using Delft3D Flexible Mesh that is able to simulate the annual floods in the Mekong Delta;
 - To analyse spatial-temporal distribution of the Mekong River's flows for different extreme river flow scenarios; and
 - To evaluate how the development of high dykes which are built to protect floodplains, influences the downstream
- 15 hydrodynamics, particularly with respect to tidal propagation.

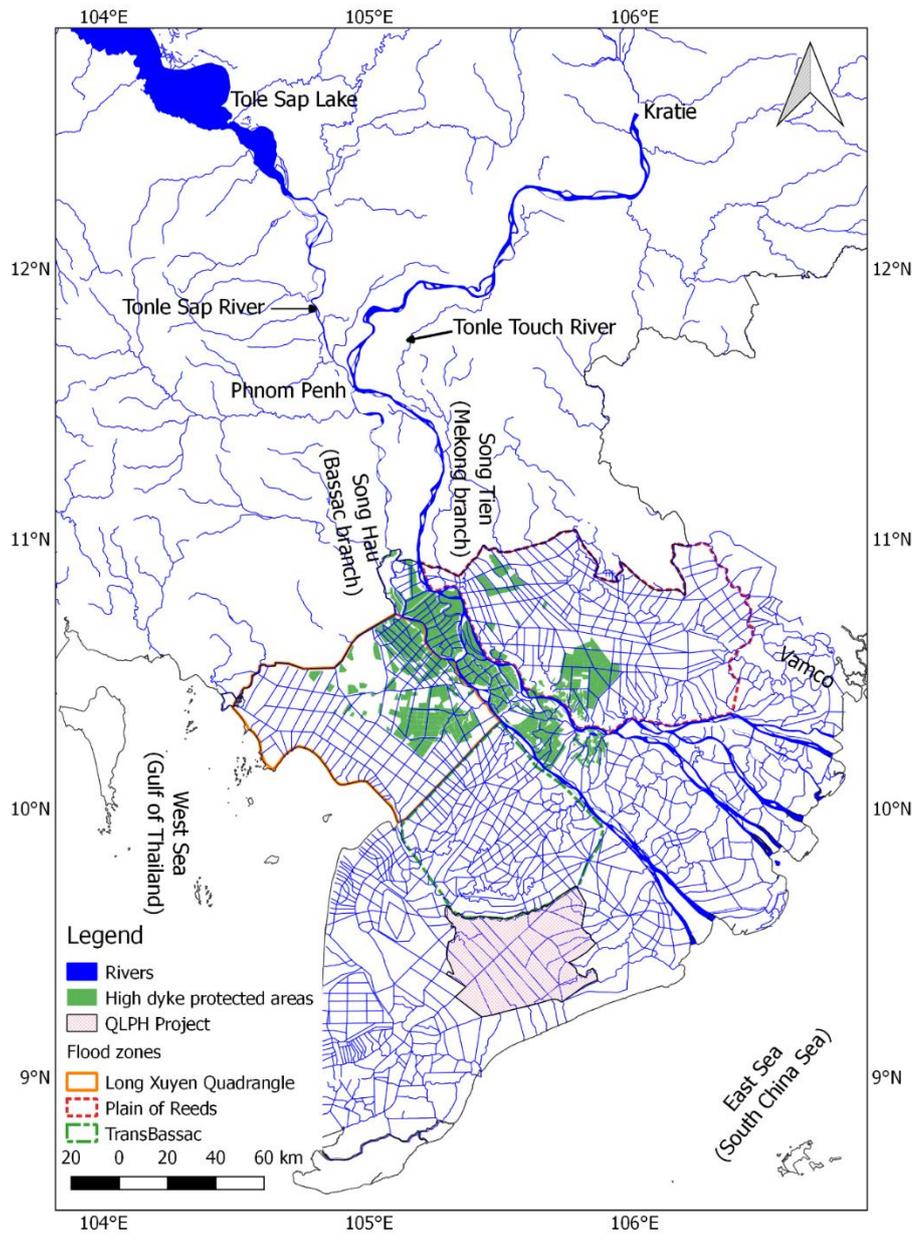


Figure 1. Location of the Mekong Delta.

1.1 The Mekong Delta

The Mekong is one the largest rivers in the world (MRC, 2010). It starts in Tibet, China, flowing through five riparian countries
 5 and reaches the ocean via originally nine branches but now seven estuaries. It has a length of 4800 km and a total draining

catchment area of 795,000 km² (MRC, 2005). The Mekong Delta starts from Phnom Penh (Figure 1), where the Mekong river is separated into two branches, namely Mekong and Bassac (Gupta and Liew, 2007; Renaud et al., 2013). The Mekong Delta is formed by sediment deposition from the Mekong River, which provides a yearly amount of 416 km³ of water and 73 Mt/year of sediment at Kratie, mainly distributed in the wet-high flow season (Koehnken, 2014; MRC, 2005). The Mekong Delta has a complex river network, especially in the Vietnamese part. The Mekong Delta's river network is illustrated in Figure 2-Figure 4. It has resulted from extensive man-made canal development from 1819 onwards (Hung, 2011).

Regarding land resources, the VMD area is about 4 million ha in which three-quarters is used for agricultural production (Kakonen, 2008). The livelihoods of the local citizens are mainly based on agriculture and aquaculture. The river infrastructure has been developing for the priority of agriculture. It provides just over one half of rice yields in Vietnam and provides up to approximately 90% of exported rice yields from Vietnam (GSOVN, 2010). However, the rice cultivation is highly influenced by annual floods (MRC, 2009a).

The most intensive agricultural production in the VMD is found in An Giang province (Figure 2 ~~Figure-4~~). Although it is also a flood-prone area, the inundation periods are slightly shorter due to flood withdraw to the West Sea. In the deep flooded zones (Long Xuyen Quadrangle and Plain of Reeds), the high dykes were densely built downstream of these zones. A reason for this is that the downstream LXQ and PoR areas experience low flood peaks so the dyke rings do not need to be heightened as the upstream LXQ and PoR.

The Mekong Delta is dominated by a tropical monsoonal climate. There are two dominant monsoons. The southwest monsoon is from May to October, coinciding with the wet-high flow season. The other, drier monsoon period is from November to March, followed by a transition period (MRC, 2010). The mean temperature is approximately 26.5° C. Although the climate is seasonally changing, monthly averaged temperature differences are 4°C between the hottest and coldest months (Le Sam, 1996). However, seasonal rainfalls are drastically different in terms of time and space. ~~Wet-~~The high flow seasons contribute approximately 90% of the total annual rainfall intensity, whereas the ~~dry-~~low flow season (from December to April) account for 10% of the total rainfall. The yearly mean rainfall is about 1600 mm in the VMD. The highest rainfall is found in the western coastal area of the Mekong Delta, ranging between 2000 and 2400 mm. The eastern coast receives about 1600 mm of rainfall, while the lowest rainfall is in the centre of the VMD (Le Sam, 1996; Thanh et al., 2014).

1.2 High dyke development in the Vietnamese Mekong Delta

The Mekong Delta has been modified extensively over the last two decades after the hugely damaging flood of 2000. Noticeable change is the hydraulic infrastructure, especially the dyke development. Before the dykes were built, a dense canal network was developed to drain floods to the West Sea and to clean acid sulphate soils.

Depending on the dyke function, the dykes can be classified into two categories. The low dykes are built to protect the rice harvest of Summer-Autumn crops in August. This is the rising phase of the annual floods. The low dykes allow flood overflows and inundation of floodplains so the crests of the low dykes are designed to just equal the maximum water level in August.

The high dykes are constructed in order to completely prevent the annual floods and enable intensive agricultural production. Generally the high dykes are designed at a crest level of 0.5 meter above the year 2000 flood peak. The flood 2000 was a severe flood which has a 50-year recurrent interval in terms of flood volumes (MRC, 2005). In An Giang, there are two ~~kinds~~ types of high dykes, ~~consisting of usual high dykes and 2-layer high dykes~~. The ~~former first type kind~~ of high dykes has a single dyke ring only. The ~~water-hydro~~ dynamics just outside of the dyke ring are dominated by floods. These dykes have a straightforward flood plain protection function but a high risk of breaching. The ~~latter other type~~ contains some first-type high dykes which is protected by a large overall dyke. The hydrodynamics outside of these high dykes are controlled by structures (sluice gates) in the overall dyke. highly protects the field inside of the high dykes. They consist of an overall dyke ring and their sub dykes. Water levels in the high dyke rings are controlled by sluice gates.

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15 Several studies have mapped the high dykes in the VMD by using remotely sensed images (e.g. Duong et al., 2016; Fujihara et al., 2016; Kuenzer et al., 2013). By this method, the high dykes are identified via flooded and non-flooded areas. However, these results are easily affected by water management of the high dyke rings. For example, in An Giang, the high dyke areas are managed according to the rule of the 3+3+2 cropping cycle. In other words, these areas are cultivated for eight (3+3+2) agricultural crops in 3 years and allowed to inundate during part of the year once every 3 years. Thus the results need to be verified with observations for reliability of the maps.

High dykes were hardly constructed in the VMD before 2000 (Duong et al., 2016). The year 2000 historical flood, particularly, caused enormous damage to infrastructure and residents' properties. After the flood event, the local authorities planned and built a cascade of high dykes in order to protect the residents and cultivations which are the major livelihood in this region. In addition, the VMD has great potential for intensification of agricultural production. Until 2009 the area protected by ~~of~~ high dykes was about 1,222 km², covering around 35% of the An Giang province area and this percentage increased to over 40% (about 1,431 km²) in 2011. Dong Thap has a much lower coverage of about 30%, corresponding to an area of 990 km². Dong Thap has deep flood-prone ~~inundated~~ areas and its soil contains high concentration of sulphates, causing low potential for agriculture (Kakonen, 2008).

Figure 2 ~~and Figure 3~~ presents numbers and areas of high dykes by districts in An Giang and Dong Thap provinces until 2011. In 2011, An Giang and Dong Thap numbers of high dykes ~~compartments~~ were 329 and 657, respectively. The total area protected by high dykes ~~area~~ in An Giang was larger than in Dong Thap (about 14 compared to 10 km² respectively). As a result, the mean area of a high dyke in An Giang is larger than in Dong Thap. In fact, high dykes are located mainly along the banks of the Song Tien and Song Hau (Figure 2) where the soils are alluvial (Nguyen et al., 2015).

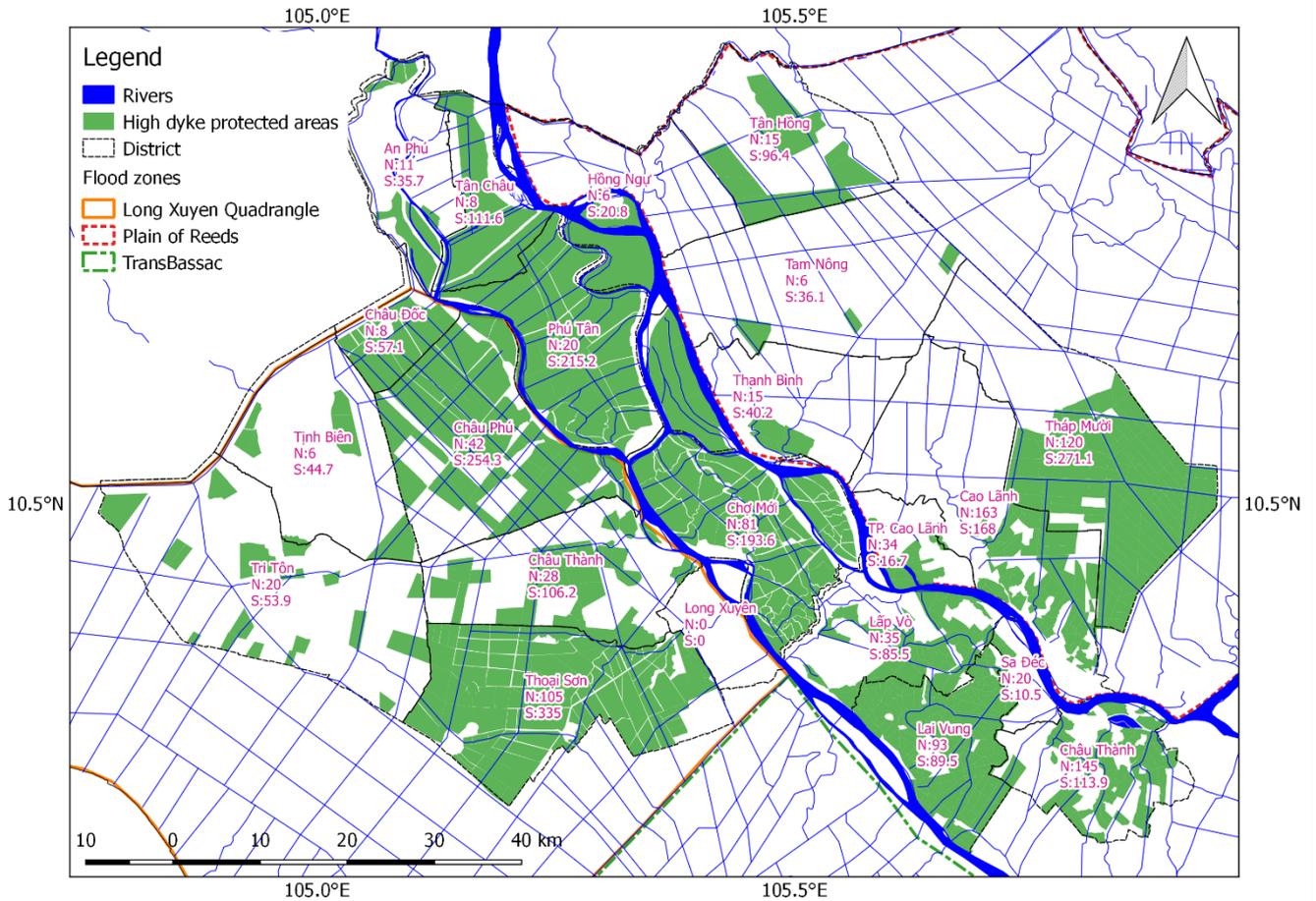


Figure 2. Numbers of high dykes and its areas by districts in Dong Thap province.

Figure 3. Numbers of high dykes and its areas by districts in An Giang province.

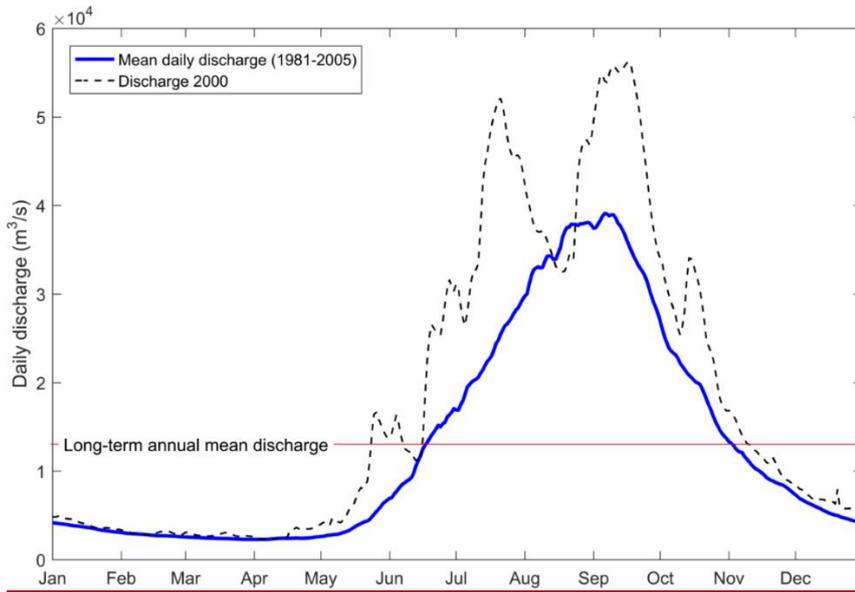
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Figure 4. Spatial distribution of high dykes which were built until 2011. The high dykes are presented by districts. The number of high dykes (N) and their protected areas (S) in km².

1.3 Flood dynamics in the Mekong Delta

The Mekong Delta is spatially separated into inner and outer parts. The former is dominated by fluvial processes, while the latter is dominated by marine processes, including tides and waves (Ta et al., 2002). ~~The inner delta is low lying and flat.~~ The Mekong River supplies approximately 416 km³ of water volume annually, or on average 13,200 m³/s through Kratie (MRC, 2005). Figure 3 shows that water discharge varies from 1700 m³/s to 40,000 m³/s between ~~dry low flow~~ and ~~wet high flow~~

seasons (Frappart et al., 2006; Le et al., 2007; MRC, 2009b; Wolanski et al., 1996). During the ~~flood season~~ high flow season high water discharge causes inundation in the delta floodplains in Cambodia and Vietnam. The annual floods in the Mekong Delta can be indicated by their peaks and volumes. The analysis of flood peaks and volumes at Kratie from 1961 to 2017 shows that the floods in 1991 and 2000 was extreme (Figure 4).



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Figure 3. Temporal distribution of daily water discharge at Kratie (available data from Darby et al., 2016).

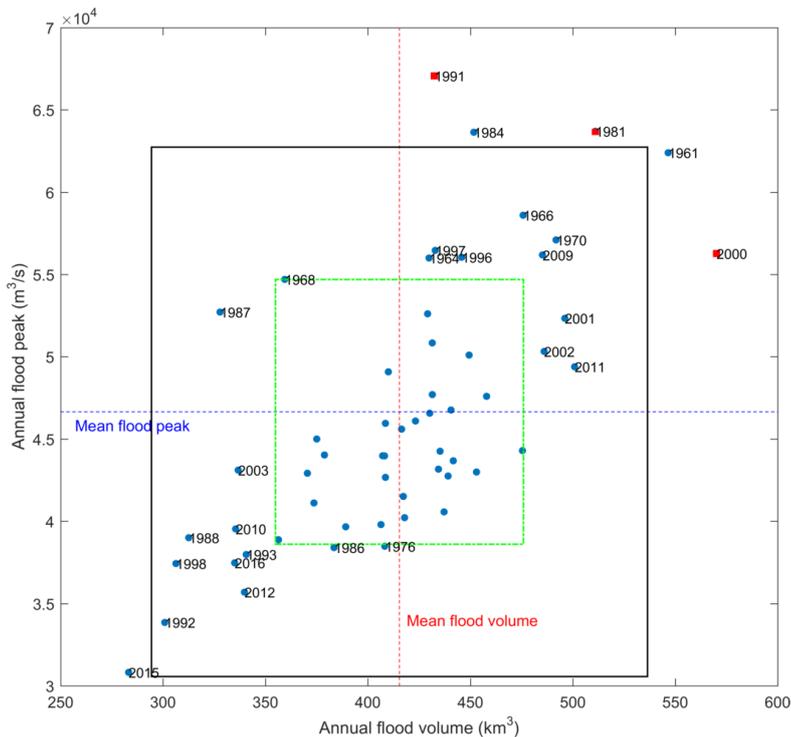


Figure 4. The annual flood peaks and volumes at Kratie from 1961 to 2017. The green and black boxes indicate significant (mean \pm standard deviation) and extreme (mean \pm 2 x standard deviation) drought or flood years, respectively.

From Kratie to Phnom Penh, the hydrodynamics of the Mekong River are dominated by fluvial flows. The river banks are lower than water levels in ~~flood season~~the high flow seasons, leading to water overflowing into the floodplains. The floodplains on the west side convey water to the Tonle Sap River while the flood water flows into the Tole Touch River on the east side (Figure 1). The floodplains on the west side receive less water than those on the east side, with water volumes of 24.7 and 35.4 km³, respectively. The peak discharges of the Mekong River to the left and right floodplains are approximately 5,400 and 7,800 m³/s (Fujii et al., 2003). These floodplains in combination with the Tonle Sap River cover about half of the Mekong's peak discharge.

At Phnom Penh, the Mekong is divided into two branches (Mekong and Bassac). In addition the Mekong River confluences with the Tonle Sap River. The Tonle Sap Lake is the largest freshwater body in Southeast Asia and it has a crucially important role in controlling water levels in the Mekong ~~inner part~~Delta. Its surface area would cover an area of 3,500 km² during ~~dry~~low flow seasons and is about four times larger during ~~wet~~high flow seasons (MRC, 2005). The water volume of the lake is up to 70 km³ in the ~~flood season~~high flow season (MRC, 2005). The Tonle Sap Lake has a function as a natural flood retention basin of the Mekong River, leading to a reduction of annual variations of water discharge flowing into the Delta. The flood flows to the Lake and reverses back during low flows to the Mekong River at the Phnom Penh confluence. Figure 5 shows long-term daily average water discharge flowing in and out of the Tonle Sap Lake at the Prek Kdam station. When water levels at Kampong Luong increase, reaching the peak of over 9 meters, the Lake supplies water to the Delta, increasing Mekong River flows ~~afterward~~after the flood season and helping to reduce saline intrusion in the coastal areas during the low flow~~dry~~ seasons. From May to September, Mekong water feeds into the Tonle Sap Lake. From October until the following April it drains back into the Mekong.

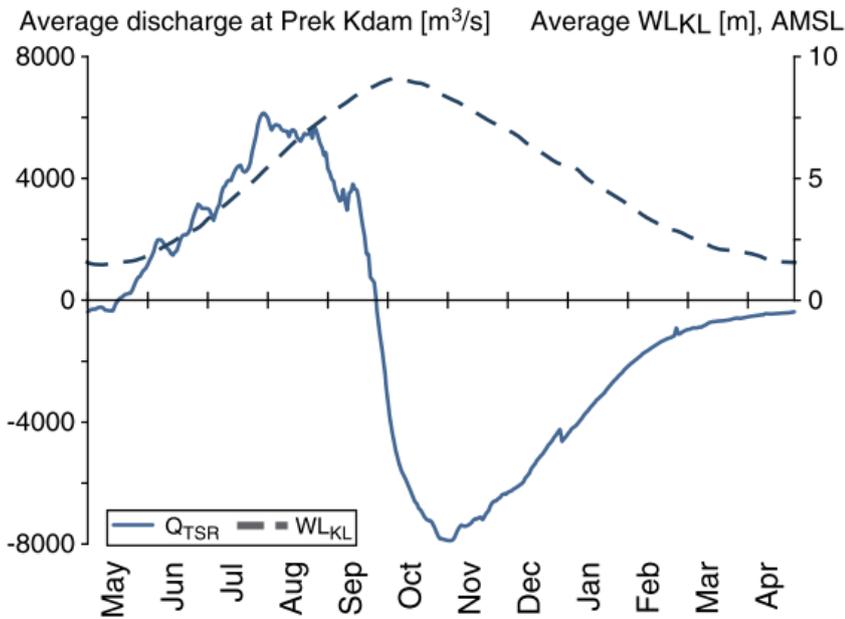


Figure 5. Daily averaged (from 1997 to 2004) water discharge hydrograph at Prek KDam and water level variation at Kampong Luong (Kummu et al., 2014). The solid line presents river flow coming in the Tonle Sap Lake at the Prek Kdam station, while the dashed line shows water levels at Kampong Luong station.

5 From Phnom Penh to the Cambodia-Vietnam (CV) border, the Mekong River flows mainly through the Mekong branch, reaching up to 26,800 m³/s during flood peaks (Fujii et al., 2003). During the flood peaks, the floods discharge onto the VMD through the Mekong, Bassac branches and the floodplains overflow by 73%, 7% and 20% of the total discharge, respectively (Figure 1).

In the VMD, the Mekong River flow diverts partly from the Song Tien (Mekong branch) to Song Hau (Bassac branch).

10 Regarding flood distribution, water discharges at Tan Chau (Song Tien) and Chau Doc (Song Hau) are estimated to be 80% and 20% of the total flood flow, respectively. However, the connecting channel of Vam Nao leads to a relative balance between the Song Tien (at My Thuan) and the Song Hau (at Can Tho) downstream (Figure 6). At these stations water levels are strongly dominated by tides of the East Sea. Water levels in the coastal VMD fluctuate by tides from both the East Sea and West Sea, but the tidal range of the East Sea is much higher than that of the West Sea. Therefore, the East Sea's tides play a more important role and become the main dominant factor controlling hydrodynamics in the VMD coastal areas.

15

2 Methodology

This section introduces the methodology of our study. Section 2.1 describes the model setup. Section 2.2 provides the model calibration and validation. Sections 2.3 and 2.4 elaborate on the scenarios of high dyke development and further analysis of these scenarios, respectively.

2.1 Model description and setup

2.1.1 Software description

The hydrodynamic model applied in this study is the Delft3D Flexible Mesh (DFM) Model Suite which has been developed by Deltares (deltares.nl). DFM is a multi-dimensional model which includes one, two and three dimensions in the same setup.

- 5 It solves the two- and three-dimensional shallow water equations (Kernkamp et al., 2011). These equations describes mass and momentum conservation (Deltares, 2018).

$$\frac{\partial h}{\partial t} + \nabla \cdot (h\mathbf{u}) = 0$$
$$\frac{\partial h\mathbf{u}}{\partial t} + \nabla \cdot (h\mathbf{u}\mathbf{u}) = -gh\nabla\zeta + \nabla \cdot (vh(\nabla\mathbf{u} + \nabla\mathbf{u}^T)) + \frac{\tau}{\rho}$$

Where $\nabla = \left(\frac{\partial}{\partial x}, \frac{\partial}{\partial y}\right)^T$, ζ is the water level, h the water depth, \mathbf{u} the velocity vector, g the gravitational acceleration, v the

- 10 viscosity, ρ the water mass density and τ is the bottom friction.

DFM allows computation on unstructured grids so it is suitable for regions with complex geometry (Achete et al., 2015), including combinations of 1D, 2D and 3D grids. This feature is efficient for taking into account small canals. Therefore, in this study DFM is selected for simulating floods dynamics in the Mekong Delta which comprises a dense river network and highly variable river widths, dykes and flood plains.

15 2.1.2 Model setup

~~The VMD witnessed 3 large floods continuously from 2000 to 2002 based in the flood classification of the Tan Chau's flood peaks. Thus the 2000 and 2001 floods were chosen to calibrate and validate the model respectively. Another reason for selecting the 2000 flood is that datasets for this flood are comprehensive.~~ The model in this study was improved from the model used by (Thanh et al., (2017)). In the present configuration, the model uses a depth-averaged setting.

20 Grid generation and improvement

The unstructured model was constructed with an approach of multi-scale modelling; specifically, it consists of a combination of 1-D (canals) and 2-D (the main branches of the Mekong River, its floodplains and shelf) parts. The approach shows efficiency in the case of complex geometry such as the entire Mekong delta. To capture the hydrodynamics of the main branches and estuaries of the Delta, the main channels are represented in enough horizontal detail to resolve the flow patterns

25 over channels and shoals and at the main bifurcations and confluences. Regarding the shelf, the model extended to approximately 80 km from the coastline of the Delta to fully contain the river plume (Figure 6).

The ~~unstructured~~ grid includes the river system of the Mekong River from Kratie to the East Sea and its shelf. The mainstream of the Mekong River, the subaqueous delta and floodplains are represented by 2D cells while the primary and secondary canals are modelled ~~in as~~ 1D networks. The 2-D cells are a combination of curvilinear (in the main channels) and triangular grid cells.

- 30 The grid creation was introduced and recommended by (Bomers et al., (2019) and; Kernkamp et al., (2011)). The grid/element sizes resolution varies vary from about 0.1 km in rivers to 3 km in the delta shelf. The lengths of grid are various depending

on river geometry. The lengths of cells are generally around 700 m on the Mekong River mainstreams and reduce to approximately 200 m at river bifurcations and confluences. The larger cells of the Tonle Sap Lake, floodplains and Sea are up to 2000 m. The uniform length of 1D segments is 400 m. The grid quality is critical to accurate simulations, so the grid has been made orthogonal, smooth and sufficiently dense, to orthogonality values of less than 10%.

- 5 From the survey data, it can occur that a dyke ring in the model can consist of high dykes and low dykes together. This situation may occur because the model only includes the main rivers and secondary canal network, but tertiary and small canals are not included. In order to determine whether the dykes are fully protected or partly protected, the ratio of high dyke area and low dyke/non-dyke area is calculated. If the ratio is higher than or equal to 1, the dyke is recognised as a high dyke. If not, it is determined as a non-high dyke. In the modelling approach, a high dyke is not allowed water flow from linked canals to its protected floodplains and we did not consider overflows the crest of dykes.

10 The VMD witnessed 3 large floods continuously from 2000 to 2002 based in the flood classification of the Tan Chau's flood peaks. Thus the 2000 and 2001 floods were chosen to calibrate and validate the model respectively. Another reason for selecting the 2000 flood is that datasets for this flood are comprehensive.

Bathymetry data

- 15 For modelling the flood dynamics in the Mekong Delta, bathymetry is a key element. However, available data of the Mekong Delta is limited. For river bathymetry, cross-sectional data has been used that was collected by the Mekong River Commission and used to develop the 1D hydrodynamic model (ISIS) to simulate fluvial flood propagation (Van et al., 2012). To use these profile data for 2D modelling, the cross-sectional data were interpolated to river bathymetry for the main branches while the primary and secondary canals directly used the cross-sectional data from the 1D ISIS network model. The bathymetry of the sea area is extracted from ETOPO1 (Amante and Eakins, 2009). ~~For The floodplains bathymetry/topography, it is attached~~
20 obtained from the freely available digital elevation models of SRTM90m (Reuter et al., 2007). Although SRTM is not a high quality digital elevation model, it was reasonably used for flood modelling in the Mekong Delta (e.g. (Dung et al., 2011; Tran et al., 2018; Triet et al., 2017). ~~(Shuttle Radar Topography Mission). The bathymetry of the sea area is extracted from ETOPO1 at a resolution of approximately 1 km.~~

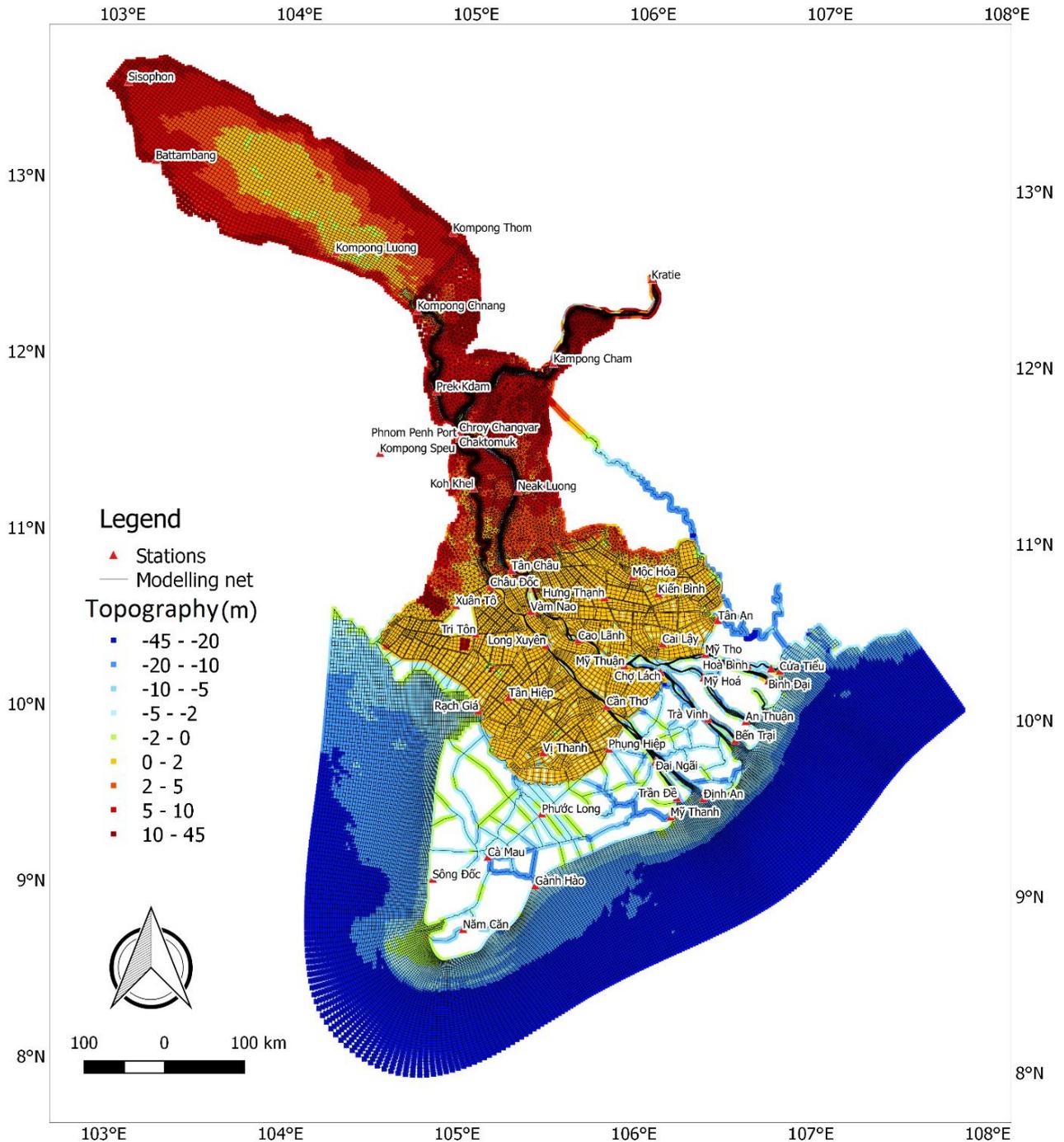


Figure 6. Mekong Delta modelling grid and River-river bathymetry-interpolated topography from 1D ISIS cross-section data-interpolation and shelf bathymetry-topography of the Mekong Delta.

Boundary conditions

Open boundaries are defined as water discharge (at Kratie) and water levels (the Sea). The measured water discharges were used for the upstream boundary at Kratie and were collected from the Mekong River Commission. The latter were defined as astronomical tidal constituents and extracted from a global tidal model (TPXO, Egbert and Erofeeva, 2002). Besides, in order to allow alongshore transport, the North cross-shore boundary is defined as Neumann boundary which is driven by the longshore water level gradient.

Initial conditions

Water levels in the Mekong Delta vary highly in space due to large-scale flood retention. The model takes a long time to capture the system behaviour, especially to arrive at the correct flood storage of the Tonle Sap Lake. The Tonle Sap Lake plays a significant role in controlling upstream discharge in the ~~dry-low flow~~ season. Therefore, the model was spun up over the year 1999; simulated results at the end of this year were used as initial conditions for the year 2000 simulation.

2.2 Model calibration and validation

The years of 2000 and 2001 were chosen for calibrating and validating the model respectively. The model calibration parameter is the roughness coefficient. This parameter is also selected for calibration without any sensitivity analysis since it is commonly used for calibrating hydrodynamic model (Manh et al., 2014; Wood et al., 2016). In this study, the 'trial and error' method is used for calibration. The roughness coefficients are extracted from the previous calibrated models, including ISIS (Van et al., 2012) and MIKE11 (Manh et al., 2014), in order to speed up the calibration process. The model was calibrated against measured data, with the objective function of Nash-Sutcliffe efficiency (NSE). NSE is a normalised statistical indicator that used comparison of residual variance and measured data variance (Nash and Sutcliffe, 1970) and calculated as:

$$E = 1 - \frac{\sum_{t=1}^T (Q_m^t - Q_o^t)^2}{\sum_{t=1}^T (Q_o^t - \bar{Q}_o)^2}$$

where \bar{Q}_o is the mean of observed discharges, Q_m^t is simulated discharges, and Q_o^t is observed discharge at time t.

In this study, we used different temporal intervals of observation data. The daily data are used in the Cambodia Mekong Delta (CMD) and hourly in the VMD. The reason is that hydrodynamics in the CMD are unlikely affected by tides, particularly in ~~flood season~~high flow seasons; while hydrodynamics in the VMD are strongly dominated by tides even in the ~~flood season~~high flow seasons, so the hourly data are better for representing tidal fluctuation.

NSE is commonly used for evaluating hydrological models. Model performance is acceptable if NSE is higher than 0 (Moriassi et al., 2007). NSE is higher than 0; this mean the simulation is a better predictor than the mean observation. The NSE of 1 corresponds to a perfect match of modelled results to the observed data. The hydrodynamic model is defined as well calibrated if NSE in terms of water levels and discharges is higher than 0.5. ~~Table 1 presents different categories of model performancee based on NSE values~~ (Moriassi et al., 2007): classified model performance based on NSE, as very good (NSE > 0.75), good (0.75 ≤ NSE ≤ 0.65), satisfactory (0.65 ≤ NSE ≤ 0.5) and unsatisfactory (NSE < 0.5).

Table 1. NSE categories for evaluating model performance (Moriassi et al., 2007).

In addition, we used an index of bias in order to recognize if the model has systematic under- or over estimates of water levels. In this study, a commonly used bias measure that is mean error is used to represent systematic error of the model (Walther and Moore, 2005). The bias is computed based on the following equation.

$$Bias = \bar{S} - \bar{O}$$

Where \bar{S} is the simulated yearly mean and \bar{O} is the observed yearly mean. The Bias is calculated for water levels over the year 2000.

2.3 High dyke development scenarios

10 To investigate the roles of different floodplains in the VMD and impact of these floodplains on the VMD's hydrodynamics and downstream tidal propagation, we developed scenarios that include contributions of each floodplains' water retention. These scenarios used the hydrograph of the flood 2000 which is an extremely wet year in order to estimate the maximum impacts of high dykes. The results of a statistical analysis of flood peaks and volumes encourage to choose the flood 2000 (Figure 4).

15 The hydrodynamic forcing is the same in these scenarios; the only difference is development of high dykes. Scenario 1 (Base): This is the base scenario of the flood 2000, without high dykes. The floodplains in the VMD were not protected by high dykes before 2000 (Duong et al., 2016), so no high dyke is considered in this scenario.

Scenario 2 (Dyke 2011): Including the high dyke system in 2011 as illustrated in Figure 42. The number of high dykes and the protected floodplain areas are described in Section 1.2.

20 Scenario 3 (Dyke LXQ): High dyke system developed only in the LXQ. The floodplain area protected by the high dykes in LXQ is approximately 3,034 km².

Scenario 4 (Dyke PoR): High dyke system developed only in the PoR. The PoR is a deep inundation region in the high flow season (Kakonen, 2008). In this scenario, the high dykes in PoR protects a floodplain area of around 5,020 km².

25 Scenario 5 (Dyke TransBassac): High dyke system developed only in the Trans Bassac region. This region is a shallow inundated area of 3,152 km².

Scenario 6 (Dyke VMD): High dyke system totally developed over the VMD's floodplains. This scenario is to investigate the possible impacts of high dykes if they are built to protect the entire VMD floodplains. The total area of floodplains, considering in the model, is about 13,059 km².

2.4 Analysis of simulations

30 **-Tidal harmonic analysis**

The peak water level is a good index to indicate extreme events in the flooded areas. Tran et al. (2017) and Triet et al. (2017) used the flood peaks to assess the impact of high dykes in the VMD. However, the VMD coastal area is drastically dominated by tides. As a result, amplitudes of tidal constituents are good indices for presenting average variations of water levels in coastal areas. The water levels at the stations along the Song Tien and Song Hau were analysed over the whole year 2000 by T_TIDE (Pawlowicz et al., 2002).

$$x(t) = b_0 + b_1 t + \sum_{k=1}^N (a_k e^{i\sigma_k t} + a_{-k} e^{-i\sigma_k t})$$

where N is a number of tidal constituents. We analysed the 8 main tidal constituents. Each constituent has a frequency σ_k which is known, and a complex amplitude a_k which is not known. $x(t)$ is a time series. a_k and a_{-k} are complex conjugates.

-Water balance calculation

To understand flow dynamics, the water balance analysis is conducted by using hourly discharge data of simulations. The targeted stations for this analysis are located on the Mekong's mainstreams and boundaries of the flood-prone zones.

$$V_{in}^t = \sum_t Q * dt$$

where V_{in}^t is total water volume flowing in the targeted regions in accordance with the Mekong flow's direction. Q is hourly simulated discharge. dt is the temporal interval. t is selected periods of an entire year and seasons.

3 Results

In this section we present results of model performance and analysis. The model performance in calibration and validation is indicated by NSE values (section 3.1). The results of spatial distribution and temporal variation are shown in section 3.2. In addition, section 3.3 presents impact of high dykes on water levels and tidal propagation.

3.1 Model calibration and validation

The overall model performance is generally satisfactory good enough for simulating flood dynamics in the Mekong Delta. Figure 6Figure 8 and Figure 7Figure 9 show the NSE values of water levels and discharges respectively. For water level calibration, there are up to 36 stations used for calibration and the majority of these stations has NSE values high than the satisfactory level of 0.5. The model performance shows its stability in validation as NSE values are higher than 0.7. Table 2 presents calculated bias of water levels at stations in the Mekong Delta. Generally, the model slightly overestimates water levels. The large biases were found in the CMD, with the largest bias of around 1 m at Kratie. The absolute values of biases decrease to smaller than 0.2 m at the stations in the VMD. Particularly, the biases at the middle and coastal VMD stations are smaller than 0.1 m (Appendix A).

The annual flood flows through the VMD by the Mekong mainstreams and over floodplains so discharge data of stations on these are employed in calibration. There are 11 stations on the mainstreams and across the CV border used in calibration. Simulated and measured discharges at these stations have a good agreement and this is indicated by high NSE values. As a result, the Manning roughness coefficients of the Mekong River reaches and its floodplains after calibration and validation are illustrated in Table 1. The range of roughness coefficients is relatively similar to the previous modelling efforts (Dang et al., 2018a; Manh et al., 2014; Tran et al., 2018; Triet et al., 2017; Van et al., 2012).

Based on the mentioned classification, NSE at 33 of 36 stations are higher than the acceptable value of 0.5 in which 23, 4 and 6 stations are classified as very good, good, and satisfactory categories, respectively. The 3 unsatisfactory stations are at Ca Mau, Phuoc Long and Rach Gia. Regarding to model validation, there are only 14 stations used for validation due to availability of observed data. Over the validation period, these stations have NSE values in the same groups with calibration. They are in good and very good classes, but the values at My Thuan stations are increased from 0.69 in calibration to 0.74 in validation.

Figure 7. NSE values of water levels at the gauging stations in the Mekong Delta. The calibration (the year 2000) and validation (the year 2001) are presented on the left and right maps respectively.

Table 2. Calculated bias for water level calibration at stations in different regions.

The annual flood flows through the VMD by the Mekong mainstreams and over floodplains so discharge data of stations on these are employed in calibration. There are 11 stations on the mainstreams and across the CV border used in calibration. Nine out of 11 stations have NSE values in very good category. The two stations of across the border, namely Right Border (to the PoR) and Left Border (to the LXQ), are in satisfactory and unsatisfactory categories, respectively; however, the NSE values are 0.49 at Left Border and 0.54 at Right Border, fluctuating around the acceptable criteria. Data at these stations are not available for validation. All stations used for validation are in very good group. Compared to calibration, NSE values in validation are relatively stable, except My Thuan station which has an increase from 0.84 to 0.95. As a result, the Manning roughness coefficients of the Mekong River reaches and its floodplains after calibration and validation are illustrated in Table

Figure 79. NSE values of water discharges at the gauging stations in the Mekong Delta. The calibration (the year 2000) and validation (the year 2001) are presented on the left and right maps respectively.

Table 1. Calibrated values of Manning roughness coefficient.

<u>River reaches/floodplains</u>	<u>Manning roughness coefficient</u>	<u>River reaches/floodplains</u>	<u>Manning roughness coefficient</u>
<u>Kratie to Phnom Penh</u>	<u>0.031</u>	<u>The Tonle Sap Lake and River</u>	<u>0.032</u>

<u>Cambodia floodplains</u>	<u>0.036</u>	<u>Phnom Penh to Vam Nao (Song Hau)</u>	<u>0.033</u>
<u>Phnom Penh to Tan Chau</u>	<u>0.031</u>	<u>Vam Nao to Can Tho (Song Hau)</u>	<u>0.027</u>
<u>Tan Chau to My Thuan</u>	<u>0.029</u>	<u>Can Tho to Song Hau mouths</u>	<u>0.021</u>
<u>VMD floodplains</u>	<u>0.018</u>	<u>VMD channels</u>	<u>0.027</u>
<u>My Thuan to Song Tien mouths</u>	<u>0.023</u>	<u>Continental shelf</u>	<u>0.016</u>

3.2 Spatial distribution and temporal variation~~distribution~~ of water volume in the VMD

3.2.1 Spatial distribution

Water enters the VMD by three ways: Song Tien, Song Hau and flows across the CV border. Figure 8 presents spatial distribution of water volume in the VMD. The VMD received around 580 km³ in 2000, with volume of 405, 83, 61 and 31 km³ through the Song Tien, Song Hau, the right and left CV border, respectively. The Song Tien diverts a considerable amount of 152 km³ water to the Song Hau by the Vam Nao canal. This is the major mechanism to balance the flows seaward between the Song Tien and Song Hau. In fact, the streamflows are relatively equal between the Song Tien and Song Hau, with amounts of 247 (at My Thuan) and 235 km³ (at Can Tho) respectively. The Song Tien is drained by its five estuary branches, while the Song Hau only has two branches. The Song Hau flows into the East Sea discharging 162 and 69 km³ via the Dinh An and Tran De branches respectively. The Song Tien's estuary branches, namely Cung Hau, Co Chien, Ham Luong, Dai and Tieu, drain a similar volume to the East Sea, with a range of 54-63 km³, except for the Tieu branch discharging only 34 km³.

Besides the mainstreams of the Mekong River, floodplains have a substantial role in changing hydrodynamics in the VMD. Hence, we analysed the water balance on the three main flood-prone areas, consisting of the LXQ, PoR and TransBassac. Among of these, the PoR harbours the largest amount of floodwater, so it is a main flood storage of the VMD. Water flows into the PoR primarily across the eastern part of the CV border in the Delta area. In fact, this way conveyed approximately 61 km³ in 2000. The simulated results show a volume deficit of 29 km³ from the West and South boundary of PoR that is drained to the Song Tien. The South of PoR drain a volume of around 38 km³ to the Soai Rap estuarine branch by the Vamco River. Analysing the water balance of the LXQ shows that it receives water from the North and East sides, while it drains water to the West and South sides. The yearly inflow of the LXQ is about 44 km³, with amounts of 31 and 13 km³ from the North and East boundaries, respectively. It is found that a similar amount of water drains out of the LXQ. The LXQ mainly releases water to the West bounday (32 km³) into the West Sea, followed by the South boundary (13 km³). The drained amount of 11 km³ from the South LXQ mostly enters the TransBassac floodplains. An additional source into this TransBassac region is from the Song Hau, with yearly volume of 6 km³. The sum of inflows is drained by the South canals of this region.

The principal dynamical characteristic of the Mekong Delta floods is their seasonal variation. Figure 8 and Figure 9 illustrate the seasonal variation of water volume and volume percent (compared to the yearly and seasonal total entering volumes at Kratie) respectively. Obviously, the flows in the flood season~~high flow season~~ is significantly higher than those in the ~~dry~~low

- flow season. The flood flows contribute up to 53-65% of the annual flows throughout the mainstreams and the percentages increase to over 80% on the floodplains. The Mekong River flowing into the VMD in 2000 is about 97% of the total flow at Kratie. However, the water volume coming in the VMD is higher than the entry volume at Kratie in the dry-low flow season. In the dry-low flow season, there are slight discrepancies of water volumes in the segments of the Mekong River, e.g. from
- 5 Tan Chau to My Thuan. A part of the discrepant proportion is stored in the river segment. As evidence, water levels at Tan Chau at the beginning of the dry-low flow season is about 2 m, and increase to 3.5 m at the beginning of the flood-season-high flow season.

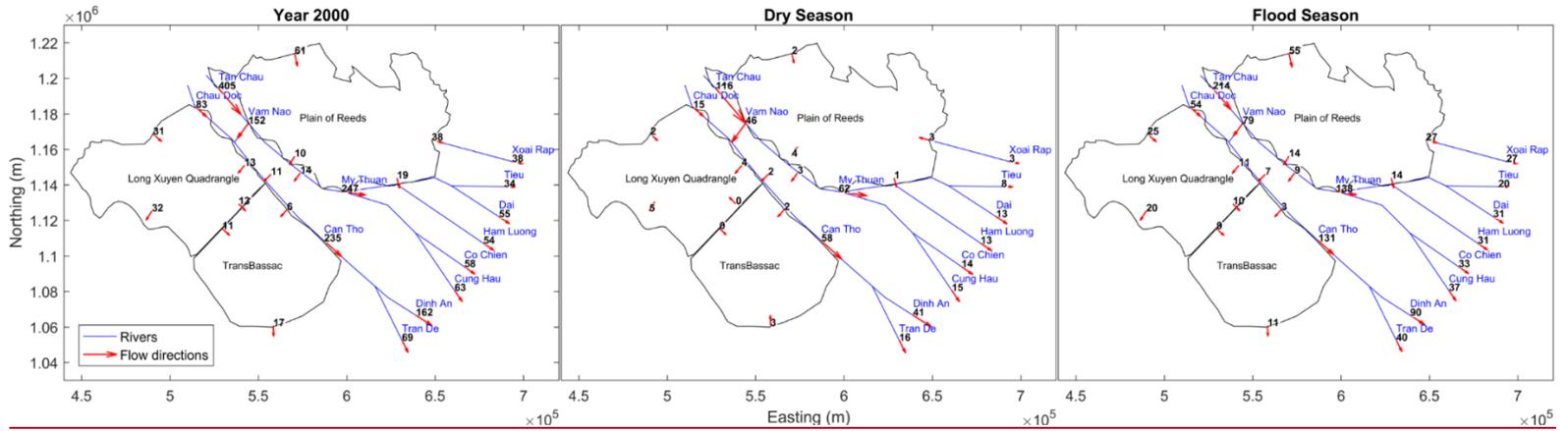
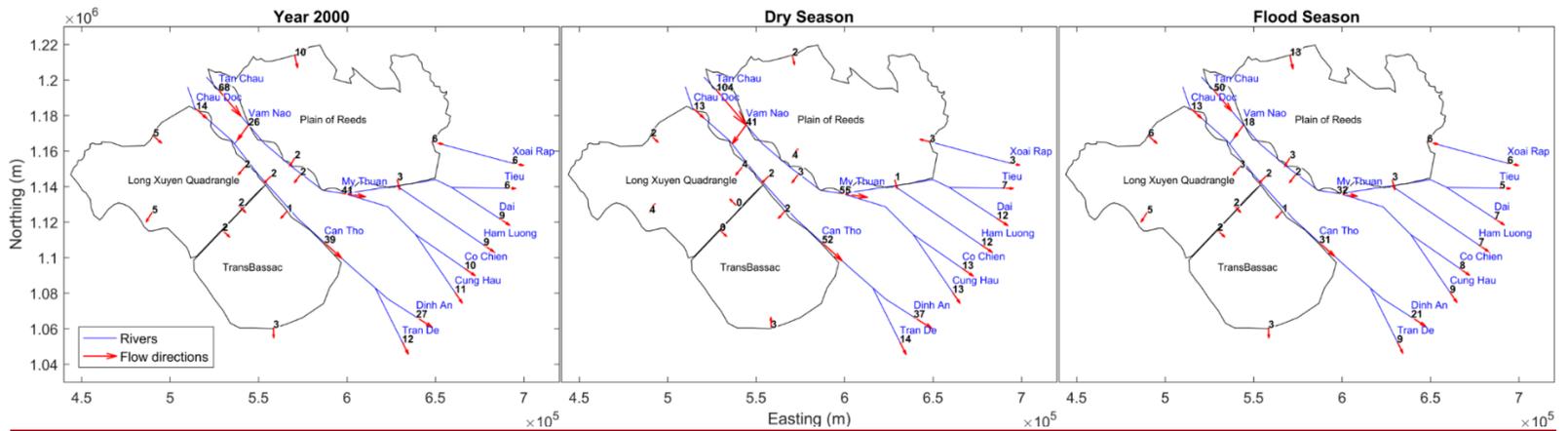


Figure 8. Spatial distribution of water volume (km^3) throughout the VMD in 2000. The **dry-low flow** season is calculated from 01/Jan to 30/Jun and the **flood-season high flow season** is from 01/Jul to 30/Oct.



5 Figure 9. Percentages (compared to total water volume at Kratie) of water distribution throughout the VMD in 2000.

3.2.2 Temporal ~~distribution~~variation

Figure 10 presents simulated fortnightly average discharges at in- and out-flows of the Mekong branches and cumulative water storage in the main VMD's floodplains from Apr-2000 to Apr-2001. The yearly average inflows of the Mekong branches at Tan Chau and Chau Doc are approximately 13,000 and 2,700 m³/s, respectively. Playing a great role in water diversion between the two Mekong branches, the Vam Nao canal makes water discharges on the Song Tien and Song Hau more balanced seaward of the Vam Nao canal (Figure 8). Consequently, the water discharges at My Thuan (Song Tien) and Can Tho (Song Hau) stations become almost similar, with annually average amounts of about 7,900 and 7,500 m³/s respectively. The proportion at Can Tho station is simultaneously drained through the Song Hau mouths. The total outflow of the Song Tien is slightly greater than at My Thuan due to added flows from the southern PoR, discharging of around 8,400 m³/s.

The water discharges on the Song Tien and Song Hau are highly variable over time. As shown for the results of discharge variations, the ~~flood season~~high flow season is from the beginning of July to the end of October and the remaining period is defined as the low flow~~dry~~ season. The largest seasonal difference is at Tan Chau, with the maximum and minimum discharges of about 21,000 and 4,500 m³/s in the ~~flood~~high flow and low flow~~dry~~ seasons, respectively. The flood flow at Chau Doc reaches a peak of 5,600 m³/s while the lowest flow is only 500 m³/s in the low flow~~dry~~ season. However, the ~~flood~~high and ~~dry~~low flows on the Song Hau at Can Tho increase to over 14,100 and 2,200 m³/s, respectively. A similar fluctuation is found at the Song Hau's mouths. On the Song Tien, the flood discharge at My Thuan is just 14,800 m³/s and slightly rise to 17,000 m³/s at the Song Tien mouths, but the ~~dry~~low flows are similarly of 2,400 m³/s at these stations.

The hydrographs at the upstream VMD are flatter than those of the downstream. The hydrograph shapes are indicated by their kurtoses and illustrated in Figure 10. The kurtosis index is a measure of the peakedness of the distribution. Downstream, the hydrographs are narrower at Can Tho, My Thuan and outflows of the Song Tien and Song Hau, with kurtosis higher than 1.5. One of the noticeable points is that during the ~~flood season~~high flow season flows at Can Tho and My Thuan stations at the beginning are relatively lower than at the end, while the flood flows are stable throughout the ~~flood season~~high flow season at Tan Chau and Chau Doc stations. This clearly shows how the early flood peak is stored in the major floodplains of the VMD. Figure 10 depicts the cumulative volumes in the major floodplains. At the beginning of the ~~flood season~~high flow season, these floodplains are almost empty. By early October storage increases to 11, 8 and 2 km³ in the PoR, LXQ and TranBassac, respectively. When these floodplains are filled, the flood flows at Can Tho and My Thuan reach their maxima during the year.

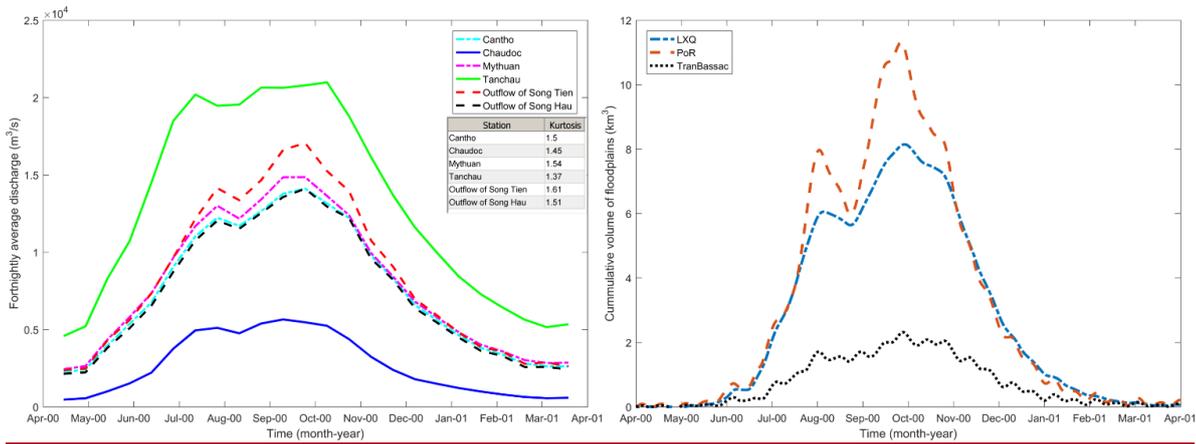


Figure 10. Fortnightly average discharges at stations along the Mekong branches (left) and cumulative water volumes of the major floodplains in the VMD (right).

3.3 Water level changes under high dyke development

- 5 Figure 11 shows that including high dykes increases daily mean water levels on Song Hau (Chau Doc, Long Xuyen and Can Tho) and on Song Tien (Tan Chau, Cao Lanh and My Thuan), especially in the flood season/high flow season. The highest increase was found at Chau Doc and Tan Chau stations while increases decline more seaward.

3.3.1 Daily water levels

10 On the Song Hau the dyked floodplains in the LXQ, PoR and TranBassac cause increases of 12.3, 6.1 and 1.1 cm of annual mean water levels at Chau Doc station, respectively. However, Table 2 shows that the effect of the PoR dykes on water level at Long Xuyen and Can Tho is larger than that of the LXQ dykes. With the high dykes built until 2011, the yearly averaged water levels would increase by 10.2 cm (Chau Doc), 1.5 cm (Long Xuyen) and 0.2 cm (Can Tho). If the high dykes would be extended over the VMD (Scenario 6), the yearly mean water levels would rise up to 22.3 cm at Chau Doc station.

15 Generally, water levels on the Song Tien are less affected by the high dykes. Among the considered floodplains, the PoR has the highest effect on Song Tien's water levels since they are directly connected. For example, the yearly mean water level at Tan Chau increases by about 8.8 cm, but only 0.6 cm at Cao Lanh station. Interestingly, the PoR slightly reduces water levels at My Thuan due to reducing conveyed capacity of floodwater from the CV border. Although the LXQ is not directly linked to the Song Tien, it causes rising water levels by around 3.6 and 1.1 at Tan Chau and My Thuan, respectively. As the high dykes were covering 2,421 km² until 2011, the mean water levels are projected to rise by approximately 0.6 cm at My Thuan
 20 and up to 6.1 cm at Tan Chau. In addition, the mean water level at Tan Chau could increase by 16.9 cm if the VMD's floodplains are fully dyked.

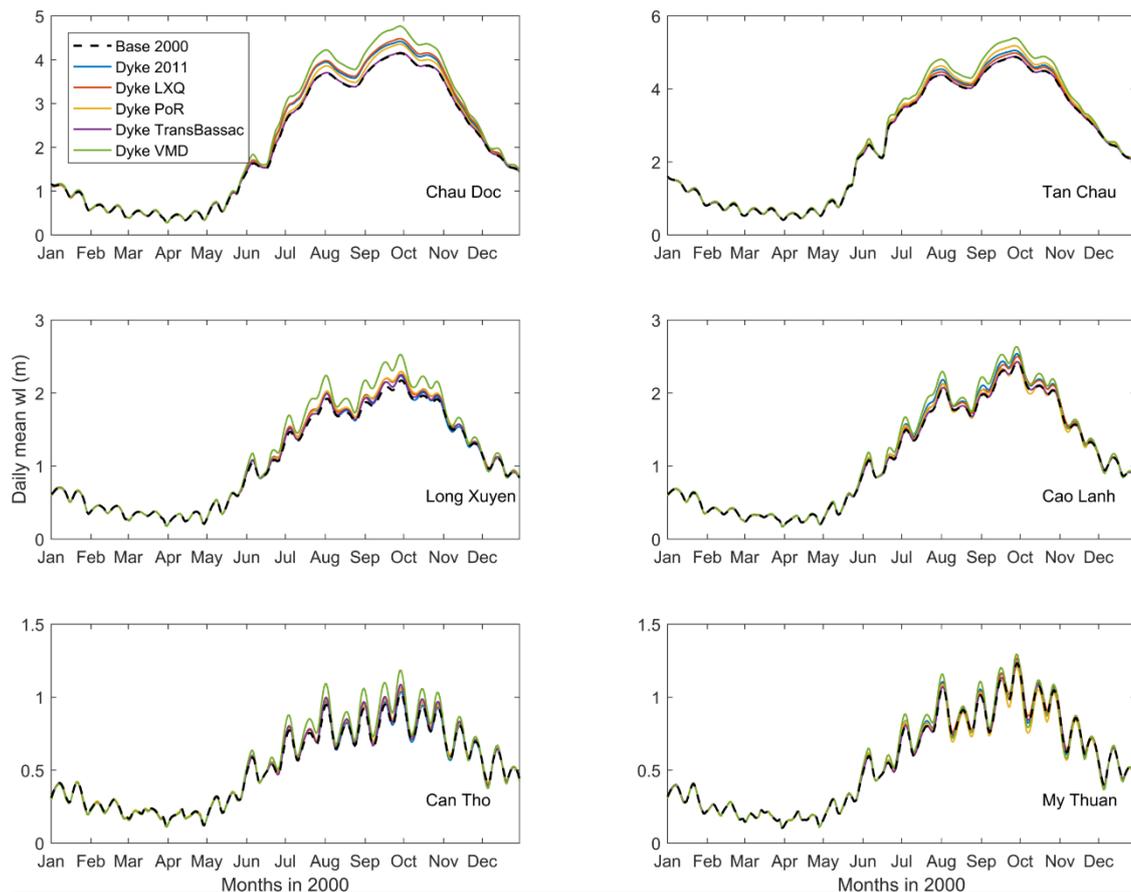


Figure 11. Daily mean water level variations at selected stations along the Song Hau (left) and Song Tien (right) under different scenarios of high dyke development.

Table 2. Increases of yearly mean water levels (in cm) over the year 2000 at the selected stations along the Song Tien and Song Hau under different scenarios of high dyke development.

Station \ Scenario	Song Hau				Song Tien			
	Chaudoc (cm)	Longxuyen (cm)	Cantho (cm)	Dinhan (cm)	Tanchau (cm)	Caolanh (cm)	Mythuan (cm)	Bentrai (cm)
Dyke 2011	10.2	1.5	0.2	5.0 30.0	6.1	3.4	0.6	0.018 2
Dyke LXQ	12.3	3.2	1.0	1.4 20.0	3.6	2.6	1.1	0.028 2
Dyke PoR	6.1	3.6	1.2	0.015 10.2	8.8	0.6	-0.8	0.019 10.2

Dyke TransBassac	1.1	1.9	0.7	0.017 $\times 10^{-2}$	0.8	0.7	0.3	0.014 $\times 10^{-2}$
Dyke VMD	22.3	9.2	3.0	0.046 $\times 10^{-2}$	16.9	6.6	1.4	0.046 $\times 10^{-2}$

3.3.2 Tidal amplitudes

The hydrodynamics in the Mekong Delta are significantly influenced by tides from the East Sea. A tidal harmonic analysis is conducted over the year 2000 to explore possible changes of the main tidal constituents. Figure 12 depicts the projected changes of tidal amplitudes along the Song Tien and Song Hau starting from the river mouths to approximately 195 km landward under high dyke development.

Tidal amplitudes at the river mouths are unlikely to change. However, differences become significant more inland. At Chau Doc, the LXQ causes the largest increase of tidal amplitudes compared to the other zones. It slightly increases M2 and K1 tidal amplitude by about 13% and 15% respectively. The TransBassac area has a main role in tidal amplitude change from Long Xuyen to Can Tho. Its dyked floodplains lead to an increase of the tidal amplitudes with 8 to 13%. Additionally, the M2 and K1 amplitudes could rise close to 28, 27 and 12% at Chau Doc, Long Xuyen and Can Tho, respectively. In contrast, high dykes in the PoR result in a marginal reduction of the amplitudes on the Song Hau. Similarly, the LXQ and TransBassac cause slight decreases of tidal variation on the Song Tien. High dykes constructed on the PoR leads to higher tidal amplitudes on the Song Tien, with increases of about 6%. These increases could reach to 28% at Tan Chau, 11% at Cao Lanh and 4.4% at My Thuan.

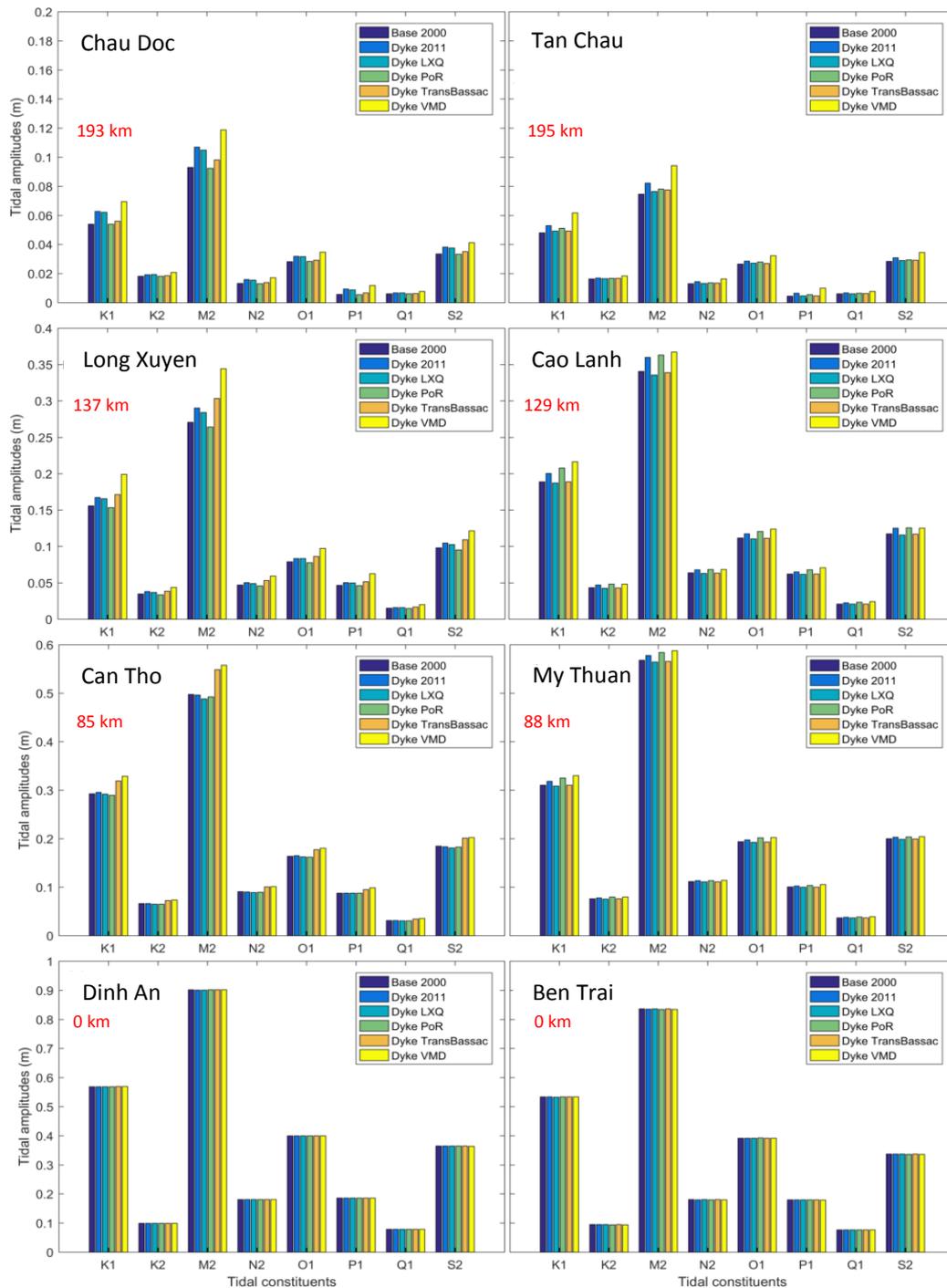


Figure 12. Tidal amplitudes of the 8 main constituents at the selected stations along the Song Tien (right) and Song Hau (left) from the river mouths to about 195 km landward in the scenarios of high dyke development.

4 Discussion

4.1 Model performance

The calibration presented in this study considered a larger number of stations compared to previous studies (e.g. Thanh et al., 2017; Tran et al., 2018; Triet et al., 2017; Van et al., 2012). These stations are mainly located in the VMD (). The majority of stations on the main Mekong River's branches have a NSE higher than 0.8 (good). In contrast, the stations located further away from the main stream have a lower NSE. The NSE values of Phuoc Long and Ca Mau stations are lower than the acceptable level because the water levels at these stations are highly influenced by local infrastructure, specifically the Quan Lo Phung Hiep Project (QLPH, see Figure 1). The QLPH is built to protect this area from saline intrusion. Flows entering QLPH are controlled by a series of sluice gates mainly located along the coast to ~~avoid-prevent~~ saline intrusion into areas for rice cultivation and control fresh water sources. We did not consider these sluice gates in the model, because they do not have a fixed operation schedule, but one that is based on crop calendars and in-situ hydrodynamics (Manh et al., 2014). For example, observed water level at Phuoc Long station is relatively unchanged at 0.2 m over the year 2000, while the model estimates that water levels at this station have semi-diurnal variations between -0.2 m and 0.6 due to tidal effects of the East Sea. During validation, a better fit was found at My Thuan station, while the others stations have comparable NSE values. As such, we are confident that the model is capable of capturing hydrodynamics in the Mekong Delta accurately.

The modelling approach in this study overcomes a limitation of previous 1D models which define their boundaries at the river mouths. The boundary conditions (usually water levels) at these locations are not always available because the water level measurement system of the VMD is not installed in all river mouth locations. Imposing a simple tidal forcing is not justifiable because river flow will impact on the mean water level as well as tidal characteristics in river mouths. Our model grid, which considers part of the shelf, allows for a proper description of these dynamics.

4.2 Spatio-temporal distribution of water volume in the VMD

The total net water volume flow through the Mekong Delta at Kratie is approximately 600 km³ in 2000 where the annual flood contributed about 480 km³. This is considerably higher than the average volume of 330 km³. However, the annual flood peak in 2000 is just slightly higher than the mean flood peak of 52.000 m³/s (MRC, 2009a). Thus, the 2000 flood is characterized by a broader than usual hydrograph.

Several studies have investigated the distribution of flood volume in the Mekong Delta (e.g. Manh et al. (2014), Nguyen et al. (2008) and Renaud and Kuenzer (2012)). Manh et al. (2014) calculated flood volume distribution for the floods between 2009 and 2011 in the upper VMD and concluded that the flood distribution is marginally changed over the mentioned period. However, they did not estimate flow distribution through the river mouths. We found a similar pattern of flood volume distribution on the mainstreams, but our model estimated a larger discharge across the VC border to the VMD. A possible explanation is that the 2000 flood is considerably larger than the floods during the 2009-2011 period. Table 3 shows a

comparison of the VMD's outflows of the current study and five other model as summarized by Nguyen et al. (2008). There is only a small variation among the used models which is attributed to different topographical data and boundary conditions (Nguyen et al., 2008). The flow distribution of the current study falls within the range of variation of the other five models, though it differs in some important branches such as Song Tien and Song Hau, below Vam Nao.

5 **Table 3. Distribution of water discharge throughout the river mouths (after Nguyen et al., 2008).**

<u>Model name</u>	<u>The Song Tien below Vam Nao (%)</u>	<u>The Song Hau below Vam Nao (%)</u>	<u>Co Chien (%)</u>	<u>Cung Hau (%)</u>	<u>Dinh An (%)</u>	<u>Tran De (%)</u>	<u>Ba Lai (%)</u>	<u>Ham Luong (%)</u>	<u>Tieu (%)</u>	<u>Dai (%)</u>	<u>Others (%)</u>
<u>NEDECO 1974</u>	<u>51</u>	<u>49</u>	<u>13</u>	<u>15</u>	<u>28</u>	<u>21</u>	<u>0</u>	<u>15</u>	<u>2</u>	<u>6</u>	<u>0</u>
<u>VNHS 1984</u>	<u>55</u>	<u>45</u>	<u>13</u>	<u>18</u>	<u>27</u>	<u>18</u>	<u>0</u>	<u>17</u>	<u>1</u>	<u>6</u>	<u>0</u>
<u>SALO89 1991</u>	<u>44</u>	<u>54</u>	<u>12</u>	<u>8</u>	<u>26</u>	<u>24</u>	<u>2</u>	<u>14</u>	<u>5</u>	<u>2</u>	<u>8</u>
<u>Nguyen Van So 1992</u>	<u>=</u>	<u>=</u>	<u>11</u>	<u>12</u>	<u>19</u>	<u>16</u>	<u>1</u>	<u>14</u>	<u>1.5</u>	<u>6</u>	<u>20</u>
<u>VRSA 1993</u>	<u>50</u>	<u>44</u>	<u>11</u>	<u>5</u>	<u>18</u>	<u>18</u>	<u>0</u>	<u>9</u>	<u>2</u>	<u>8</u>	<u>29</u>
<u>This study</u>	<u>41</u>	<u>39</u>	<u>10</u>	<u>11</u>	<u>27</u>	<u>12</u>	<u>0</u>	<u>9</u>	<u>6</u>	<u>9</u>	<u>14</u>

Note: in this study the percentages are calculated based on the total volume at Kratie.

The water distributions slightly vary over the floodhigh flow and drylow flow seasons. The largest changes are found in the discharges onto the floodplains. For instance, water volumes are highly seasonal at the CV border stations. The water flows in the drylow flow season contribute to 2 - 6% of the yearly flows at these stations. The relative percentages of the Mekong flow, existing via the Song Hau estuaries in the drylow flow season are higher than those in the floodhigh flow season while the percentages at the Song Tien estuaries are relatively constant.

There are several studies which investigate the roles of the Tonle Sap Lake in regulating the flood regimes on the Mekong River (Fujii et al., 2003; Kummur et al., 2014; Manh et al., 2014). Kummur et al. (2014) estimated that the Tonle Sap Lake is capable of reducing about 20% of the Mekong mainstream discharge and its greatest storing volume is in August, with an amount of around 15 km³ from the Mekong River flows. The highest monthly released volume occurs in November and peaks at nearly 20 km³. Consequently, the Tonle Sap Lake has a crucial role on the ~~seasonal~~ temporal scale in regulating the Mekong River flows. The VMD floodplains have a different role compared to the Tonle Sap Lake in changing the Mekong mainstream flows. They mainly store early flood waters in August. This leads to reduce flood flows at downstream stations along these floodplains. These stations reach the peak discharges when the VMD floodplains are nearly fully filled. Therefore, the peak flows at the downstream stations occur in October. These results are consistent with the analysis of Dang et al. (2018).

4.3 Impact of high dyke development

~~The Mekong Delta is presently facing~~ There are several threats ~~to the current status of the Mekong Delta~~, such as the impact of hydropower dams, sea level rise, delta land subsidence, and hydraulic infrastructure (Dang et al., 2018a; Kondolf et al., 2018). Impacts of these threats are highly ~~depends-various in terms of timescales on temporal scales~~. Among of these, hydraulic infrastructure (especially high dykes) has considerable influence on the hydrodynamics in the region on a short ~~temporal~~ timescale. The high dykes in the VMD are built to protect agricultural land during floods. As a result, flood discharges on the rivers increase and hydrodynamics in the VMD change. Specifically, the results indicate that lack of flood retention in the LXQ leads to an increase of water levels on the Song Hau, with a downward trend of increases from Chau Doc to Can Tho. This rising pattern is found by Tran et al. (2017) as well, albeit with different magnitudes because of different ~~years-comparison~~. They compared the peak water levels while we used daily mean water levels for comparison. Tran et al. (2017) found that the water level peaks would be drastically higher if the high dykes were built. These peaks have specially increased in the upper VMD (e.g. by 66 cm at Chau Doc and only 4 cm at Can Tho).

Interestingly, the high dykes in the PoR has slightly stronger impacts on water levels at Long Xuyen station than those in the LXQ. The reason for that is an increase of water levels on the Song Tien, causing an increase of water diversion from the Song Tien to the Song Hau. Because of the connecting canal of Vam Nao, the PoR floodplains have not only influenced water level fluctuation on the Song Tien, but also on the Song Hau. In addition, the LXQ floodplains affect both the Song Tien and Song Hau water levels. Nevertheless, the increasing levels on the Song Tien remain slightly lower than the levels on the Song Hau since the Song Tien has more river mouths and has a higher conveyance capacity in comparison to the Song Hau.

Recent studies on impact of high dykes in the VMD (e.g. Tran et al. (2017) and Triet et al. (2017)) only compared the maximum water levels. However, we found that the high dykes also resulted in reduction of minimum water levels. This mean the high dykes have effects on tidal fluctuation on the main branches. We analysed tidal amplitudes of the 8 main constituents over the year 2000 in order to quantify how water levels on the main branches changed. Noticeably, the complete implementation of the high dyke system over the VMD floodplains can cause increases of about 12% and 4% tidal amplitudes at Can Tho and My Thuan stations, respectively. Additionally, high dykes in the PoR directly adjacent to Song Tien cause tidal amplitude reduction on Song Hau and vice versa. The reason is that river water cannot flow into the floodplains, leading to an increase of river discharge in the main streams. This increased river discharge causes significant M2 amplitude reduction (Guo et al., 2016). The amplitudes and mean water level at the river mouth stations are unlikely to change under high dyke development (Table 2). The reason is that flood retention loss due to ~~embankments-floodplain areas protected by the high dykes~~ has insignificant changes on the water discharge at that location. In contrast and as an example, Kuang et al. (2017) found that if the water discharges from the Yangtze River upstream would increase by 20,000 m³/s, the ~~sea-level~~ water levels at the mouth could rise approximately 1 cm. An explanation is that the water discharge change due to high dyke development is not larger enough to increase water levels at the river mouths.

The impact of high dyke development on the downstream hydrodynamics are considered different and less than those of hydropower dams, climate change and sea level rise. (Dang et al., 2018a) reveal that hydropower development increases the monthly water levels at Tan Chau by 0.4 m and at My Thuan by 0.05 m in a wet year. Sea level rise causes a gradual but drastic increase of water levels. For instance, the water level at My Thuan could increase by 0.3 m under sea level rise of 0.38 m.

5 However, the high dyke development in the VMD causes another different effect on the floodplains. It prevents flood waters from entering to the floodplains and this excludes sediment deposition on these floodplains. Sediment deposition on the floodplains would benefit agricultural production. (Chapman et al., 2016) estimated that annual sediment deposition in An Giang would be worth USD 26 million.

4.4 Flood discharges and volumes scenarios

- 10 In order to investigate the impact of the distribution of hydrological conditions, we selected three years of 1981, 1991 and 2000 for comparison since these include the extreme floods in terms of the flood peaks and highest volumes. The simulations considered conditions with and without the high dykes. Figure 13 shows that the water levels in the VMD stations are comparable for all three years, although the 1991 peak flood at Kratie is about 20% larger than the 2000 peak flood. Even more, the 2000 year flood with largest flood volume leads to highest water levels. Flood volume is thus more important than
- 15 flood peak flows for extreme flood conditions. The reason for this is that high flood peaks flood the area and fill the Tonle Sap Lake in Cambodia so that the peak flood flow decreases and elongates downstream (Triet et al., 2017). In addition Figure 13 shows that the water level difference for scenarios with and without high dykes is similar and decreases downstream for all three years.

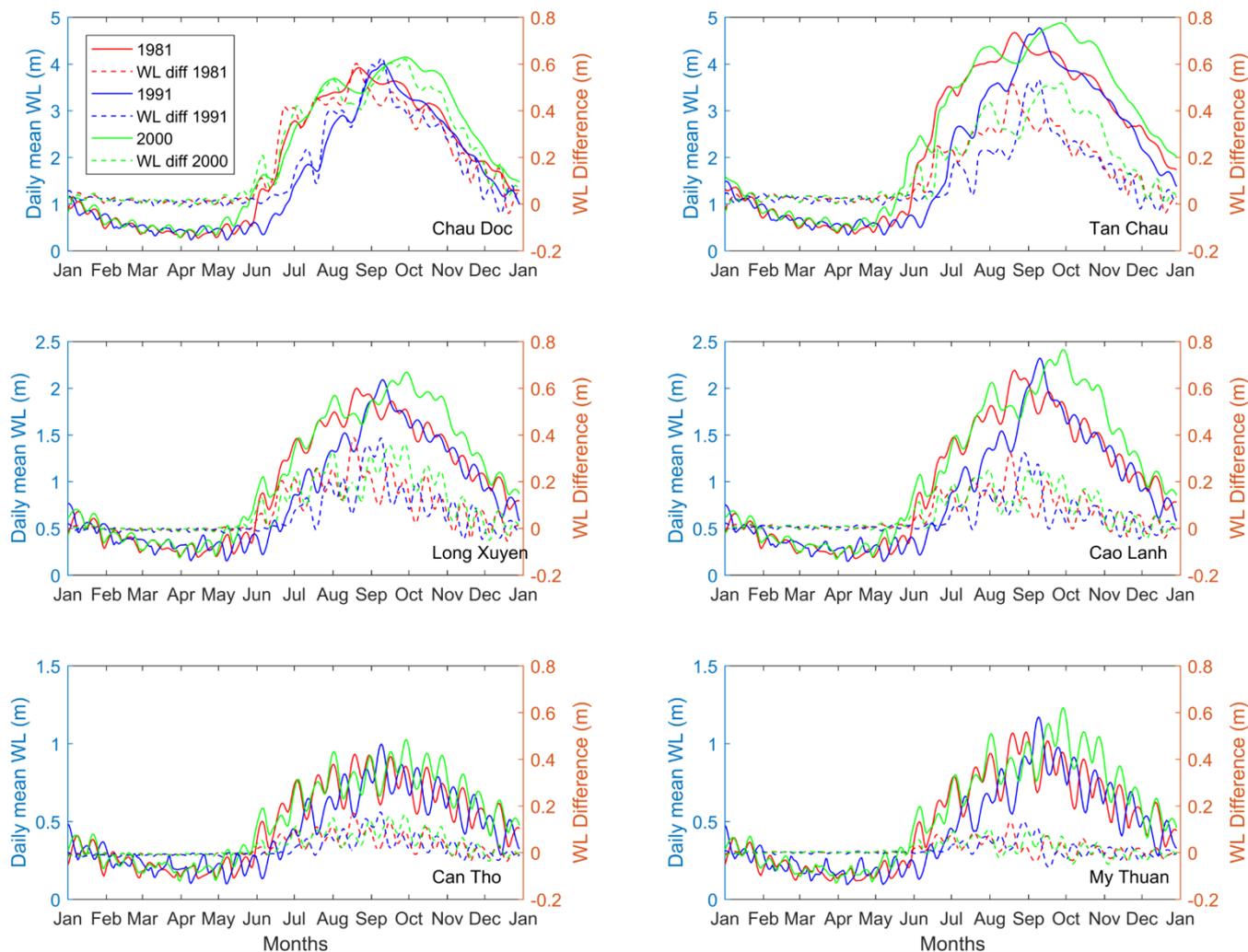


Figure 13. Simulated daily mean water levels in high-flood years of 1981, 1991 and 2000 at selected stations on the Mekong River. The dashed lines indicate water level differences due to impacts of high dykes.

5 Conclusions

- 5 In this study, we applied a process-based model (DFM) in order to simulate hydrodynamics in the entire Mekong Delta from Kratie to ~~the shelf~~~~extended-sea~~ areas. The model was calibrated by a ~~comprehensive~~ dataset of water levels and discharge ~~at~~ 36 stations over the Mekong Delta. ~~As a result, t~~The model shows a good agreement between simulations and observations. This model is an improved version of the model used by Thanh et al. (2017), by taking into account the Cambodian and Vietnamese floodplains and the dense river/canal network in the VMD. Nevertheless, it does not contain tertiary rivers/canals and hydraulic structures for salinity regulation.
- 10

We found that the change in river seasonal flow distribution throughout the Mekong's mouths is insignificant, except at the Dinh An mouth which has a slight increase in the drylow flow season. In contrast, the Mekong River network discharging to the sea through the Soai Rap mouth and the West LXQ dramatically dropped in the drylow flow season compared to the flood high flow season due to overflow reduction at the CV border.

- 5 This study found that the dyked floodplains in the LXQ and PoR not only influence water regimes on its directly linked Mekong' branch, but also on the other branches. The LXQ high dykes cause an increase in daily mean water levels, but a decrease in tidal amplitudes on the Song Tien (after the connecting channel of Vam Nao). A similar pattern is also found for the interaction between the PoR high dykes and the Song Hau. The high dykes built in the PoR, LXQ and TransBassac regions have a demonstrated impact on water levels at Tan Chau, Chau Doc and Can Tho, respectively. These outcomes will benefit
- 10 sustainable water management and planning in the VMD.

Appendix: Model calibration

For water level calibration, there are up to 36 stations used for calibration. Based on the mentioned classification, NSE at 33 of 36 stations are higher than the acceptable value of 0.5 in which 23, 4 and 6 stations are classified as very good, good, and satisfactory categories, respectively (Figure A1). The 3 unsatisfactory stations are at Ca Mau, Phuoc Long and Rach Gia.

- 15 Regarding to model validation, there are only 14 stations used for validation due to availability of observed data. Over the validation period, these stations have NSE values in the same groups with calibration. They are in good and very good classes, but the values at My Thuan stations are increased from 0.69 in calibration to 0.74 in validation.

For water discharges calibration, there are 11 stations on the mainstreams and across the CV border used in calibration. Nine out of 11 stations have NSE values in very good category (Figure A2). The two stations of across the border, namely Right

- 20 Border (to the PoR) and Left Border (to the LXQ), are in satisfactory and unsatisfactory categories, respectively; however, the NSE values are 0.49 at Left Border and 0.54 at Right Border, fluctuating around the acceptable criteria. Data at these stations are not available for validation. All stations used for validation are in very good group. Compared to calibration, NSE values in validation are relatively stable, except My Thuan station which has an increase from 0.84 to 0.95.

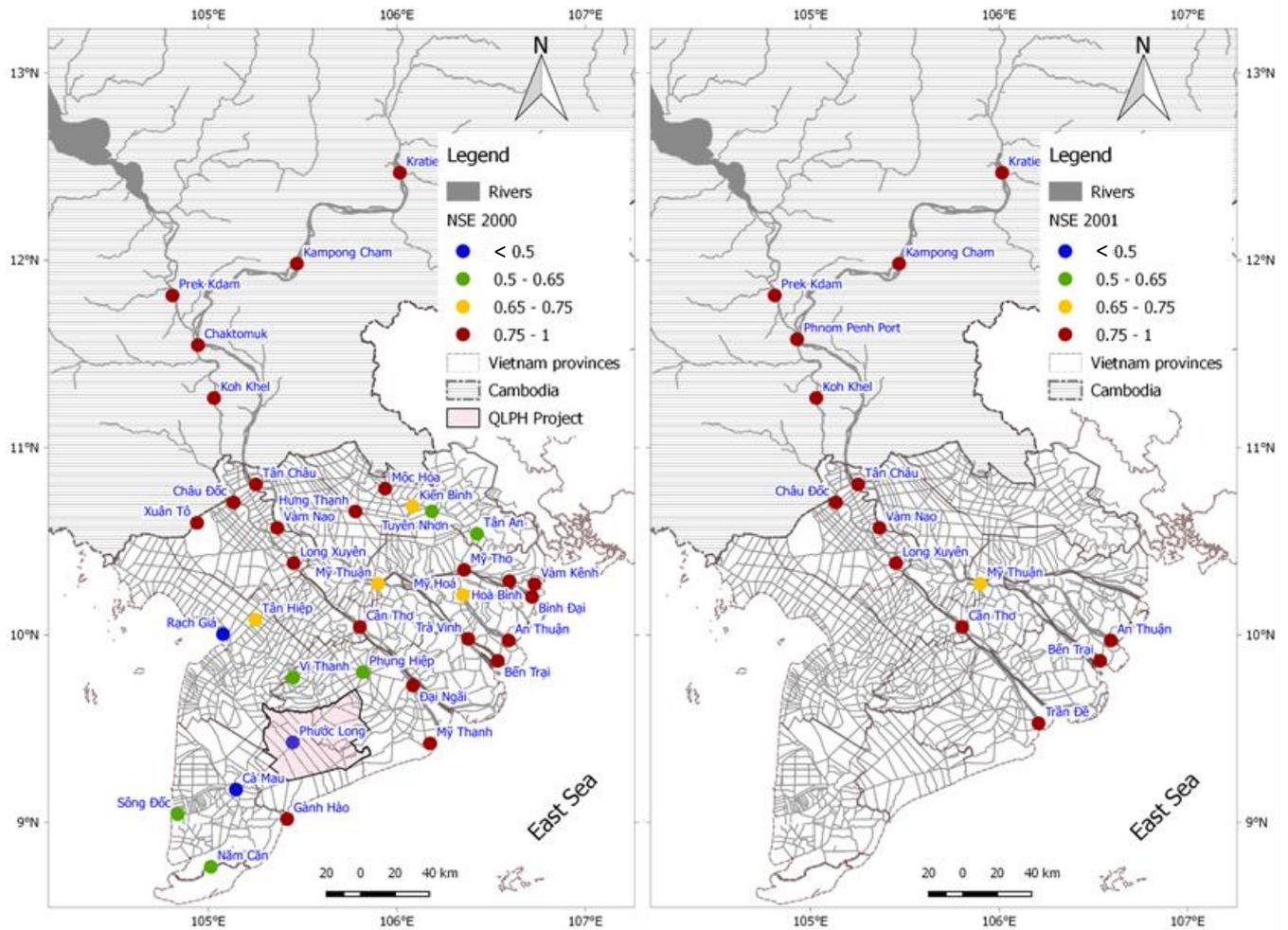


Figure A1. NSE values of water levels at the gauging stations in the Mekong Delta. The calibration (the year 2000) and validation (the year 2001) are presented on the left and right maps respectively.

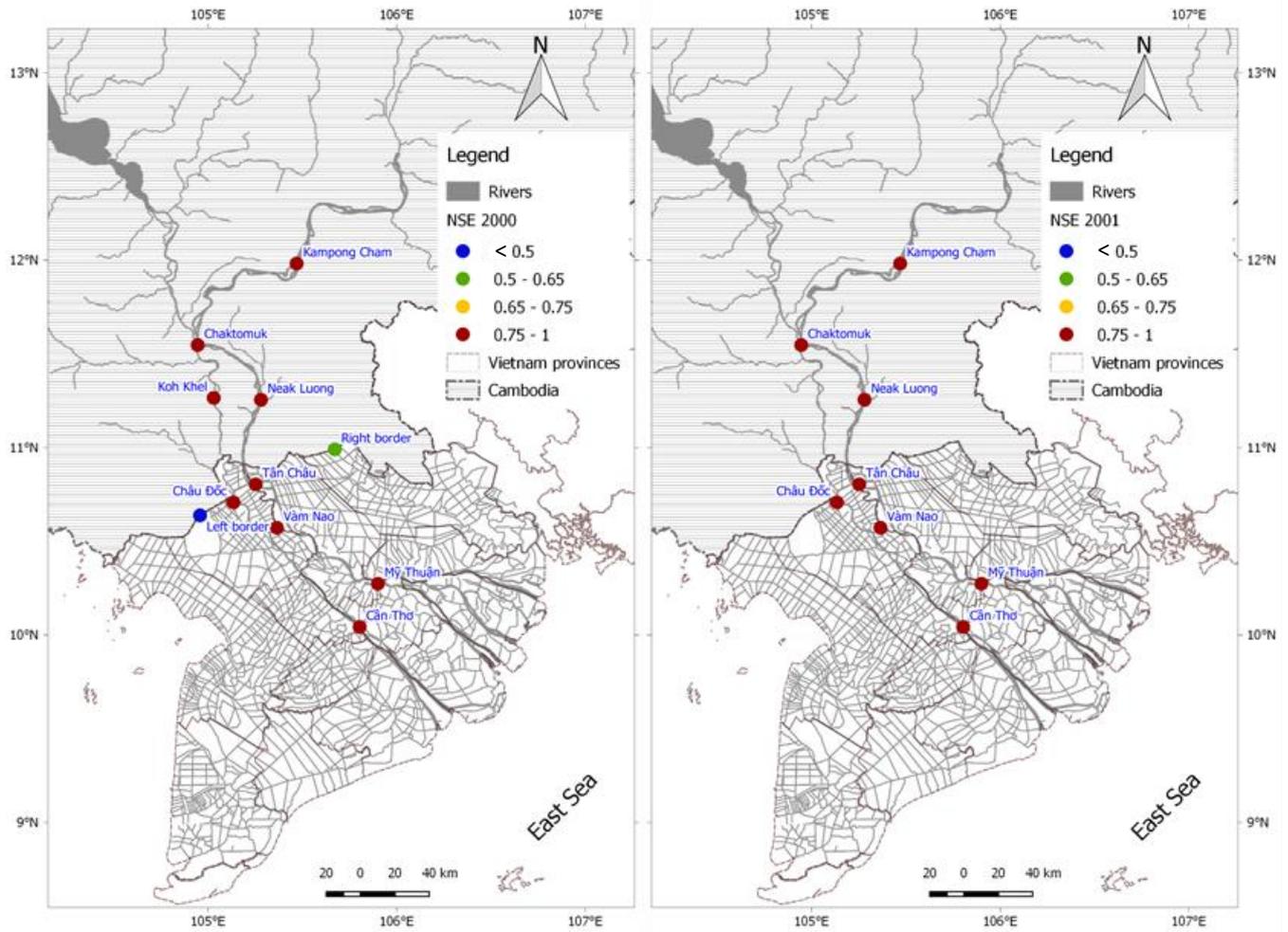


Figure A2. NSE values of water discharges at the gauging stations in the Mekong Delta. The calibration (the year 2000) and validation (the year 2001) are presented on the left and right maps respectively.

Table A1. Calculated bias for water level calibration at stations in different regions.

<u>CMD</u>	<u>Bias</u> (m)	<u>Upstream</u> <u>VMD</u>	<u>Bias</u> (m)	<u>Middle VMD</u>	<u>Bias</u> (m)	<u>Coastal</u> <u>VMD</u>	<u>Bias</u> (m)	<u>Coastal</u> <u>VMD</u>	<u>Bias</u> (m)
<u>Kratie</u>	<u>1.02</u>	<u>Tan Chau</u>	<u>0.12</u>	<u>Kien Binh</u>	<u>-0.03</u>	<u>An Thuan</u>	<u>-0.02</u>	<u>Nam Can</u>	<u>-0.24</u>
<u>Kampong</u> <u>Cham</u>	<u>0.54</u>	<u>Chau Doc</u>	<u>-0.20</u>	<u>My Thuan</u>	<u>-0.08</u>	<u>Ben Trai</u>	<u>-0.04</u>	<u>Phung Hiep</u>	<u>-0.07</u>
<u>Prekdam</u>	<u>0.92</u>	<u>Vam Nao</u>	<u>-0.10</u>	<u>Can Tho</u>	<u>0.02</u>	<u>Binh Dai</u>	<u>0.00</u>	<u>Phuoc Long</u>	<u>-0.06</u>
<u>Chaktomuk</u>	<u>-0.15</u>	<u>Xuan To</u>	<u>0.09</u>	<u>Hung Thanh</u>	<u>0.00</u>	<u>Ca Mau</u>	<u>-0.18</u>	<u>Rach Gia</u>	<u>-0.08</u>

<u>Kohkhel</u>	<u>-0.51</u>	<u>Moc Hoa</u>	<u>0.04</u>	<u>Tuyen Nhon</u>	<u>0.06</u>	<u>Dai Ngai</u>	<u>-0.02</u>	<u>Song Doc</u>	<u>-0.03</u>
-	-	<u>Long Xuyen</u>	<u>-0.12</u>	-	-	<u>Ganh Hao</u>	<u>-0.07</u>	<u>Tan An</u>	<u>0.08</u>
-	-	-	-	-	-	<u>Hoa Binh</u>	<u>0.09</u>	<u>Tan Hiep</u>	<u>-0.10</u>
-	-	-	-	-	-	<u>My Hoa</u>	<u>0.00</u>	<u>Tra Vinh</u>	<u>0.05</u>
-	-	-	-	-	-	<u>My Thanh</u>	<u>0.02</u>	<u>Vam Kenh</u>	<u>0.07</u>
-	-	-	-	-	-	<u>My Tho</u>	<u>0.02</u>	<u>Vi Thanh</u>	<u>0.00</u>

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