

Authors' responds:

HESS-2017-85: Xu et al., Numerical modeling and sensitivity analysis of seawater intrusion in a dual-permeability coastal karst aquifer with conduit networks, Hydrol. Earth Syst. Sci. Discuss.,

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Editor:

In particular, I expect that the Authors improve the paper, by taking into account and properly answering both the reviewers' comments and the questions that I list below.

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1) The presentation and the language should be improved. I provide several suggestions in the attached marked version of the manuscript, where some of the most obscure (if not incorrect) sentences are annotated.

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Response: Thanks for providing the important suggestions in the languages. We have corrected the grammar issues and rephrase the sentences. The Morris methodology and global sensitivity analysis results sections are rewritten to address some issues from the reviewers.

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2) From the physical point of view the choice of a 2D model is not supported by solid arguments. In fact, a 2D model assumes that the quantities are constant along the direction perpendicular to the modelled cross section, i.e., parallel to the coast line. Is this realistic? Does the conduit network have a large extension along that direction? How are the results affected from 3D flow and transport in the porous matrix?

25

Response: We admit the 2D model has some limitations on simulating the seawater intrusion in the entire aquifer. The conduit network does not have a large extension parallel to the coastline, however, relatively high hydraulic conductivity layers are found at nearly at the same depth as the conduit network. Overall, we only try to simulate the cross section that containing the conduit segment, not the other parts of the aquifer. The editor is right that the exchanges between the conduit and surrounding porous medium are computed in this 2D model, however, the effects of flow and transport along the direction perpendicular to the cross section is ignored in this study.

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3) I fully agree with the remark by Referee #2 that the discussion of the results is very long and difficult to follow.

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Response: The discussion of global sensitivity analysis result is shortened. We delete the redundant results which has been presented in the previous local sensitivity analysis subsection, and only discussed the important parameters and major findings in this study.

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4) I fully agree also with the remark by Referee #2 about CSS and weighting.

Response: We have clarified the CSS and weighting in the responses to Referee #2 (see below) and also in the revised manuscript.

Anonymous Referee #1

50 With the resubmitted version of their manuscript, Xu et al provide a significantly improved version of their work. The manuscript was shortened to add more focus. They provide a sound explanation why they chose to apply 2 methods for sensitivity analysis. A better link between previous research at the study site and this study was established and the elaboration of parts of the sensitivity analysis methods was improved.

55 I agree with the authors that parameter correlation does not have to be addressed in the frame of the local sensitivity analysis as (1) local parameter analysis is by definition very limited in identifying parameter interactions and (2) parameter interactions are explicitly addressed by Morris's method, which is by definition a "global" parameter sensitivity analysis scheme.

60 Some confusion came up addressing my comment on including more comparison to the work of others. Indeed, this was partially addressed by including Shoemaker et al. (2004) in the discussion but the link to studies that used sensitivity in karst modelling is still weak. The authors indicate that they are aware of just few karst studies using sensitivity analysis but a quick scan of the literature revealed couple of studies using local (Oehlmann et al., 2014), regional (Chang et al., 2017; Hartmann et al., 2015) and global (65 sensitivity analysis (Chen et al., 2017; Hartmann et al., 2013) in karst modelling for both lumped and distributed modelling approaches.

70 Including some of these (or similar) references to their manuscript will strengthen the state of the art of karst sensitivity analysis as well as the discussion. After these very minor suggestions have been applied I would feel very confident recommending this study for publication in Hydrology and Earth System Sciences.

75 **Response: Thanks for providing these related references. We agree with the reviewer and include these latest references in the introduction section to provide a more complete description of karst sensitivity analysis.**

Anonymous Referee #2

80 The paper has significantly improved in clarity. My impression is that the discussion of the results is pretty long and articulated and would benefit a lot from a summary table or even a summary as bullet points. It is still difficult to follow it.

85 **Response: We have shortened the discussion part, particular the global sensitivity analysis subsection, by deleting the redundant discussion that has been addressed in the local sensitivity analysis results. The major findings in this study are summarized as bullet points in the conclusion section.**

90 I am providing below some comments on the response letter and some minor comments on the new manuscript.

Line 137 in the response letter: I believe there is some misunderstanding regarding local sensitivity analysis and how they should be used. The weighting is based on the variance-covariance matrix of the data errors. This is different than the correlation matrix of the parameters. The answer of the authors regarding omega is not correct.

95

Response: We agree with the reviewer here. We have clarified that the omega is set as the variable-covariance matrix of parameters as the data errors in the revised manuscript.

The CSS uses the weighting, so it only accounts for the variance-covariance of the data errors. I think this is the point of the authors, and I agree – but this is only because CSS by themselves do not tell the whole story and need to be used combined with other measures. To get a bigger picture, it is important to consider them with the parameter correlations, which UCODE does calculate. UCODE calculates also leverage that is another measure including information on correlations.

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Response: We agree with the reviewer that UCODE computes the parameter correlations, and CSS does not consider parameter correlation but data correlation. We add the parameter correlations computed from UCODE and discuss the meaning of correlations in subsection 4.1.3 in the revised manuscript.

110

Their final statement that says the conventional local sensitivity analysis (e.g., the one used in UCODE and also in this study) does not address parameter correlation is not correct. UCODE does calculate the variance-covariance matrix for the parameters and also the correlations. Whether it is chosen to use them or ignore them, this is the authors' choice. It is not a shortcoming of UCODE, but of the way it is used. Relying only on CSS, does not allow getting the full picture. It is true that user needs to make the interpretations by considering these together, so the problem is NOT “addressed” automatically. Guidelines on how to use CSS, parameter correlation coefficients and also leverage at the same time are provided in Hill and Tiedeman, 2007. The most informative way of going through all the measure is following the sensitivity analysis developed for the synthetic exercise presented in the book. I can provide more references upon request.

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Response: We agree that the CSS only does not provide a full picture of parameter sensitivities. We add the parameter correlations from UCODE in this revised manuscript.

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Line 270 in the response letter: same comment as above
In the new manuscript:
Line 31: What does WKP stand for?

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Response: WKP stands for the Woodville Karst Plain. We change the wording in the revised manuscript.

Line 30-35: the justification of why Local and Global analysis are performed is very weak

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Response: In the revised manuscript, we have clarified the objectives of this study and the importance of sensitivities analysis in the last two paragraphs of the introduction section.

140 Line 268-270: check the sentence

Response: Revised.

145 Line 382: parameters eventually to be adjusted during calibration

Response: I'm not sure if I understand the reviewer's question correctly. Obviously, the parameters are adjusted during calibration to compute the salinity and head simulations. However, we didn't do model calibration in this study due to the lack of observational data. The parameters (actually, boundary conditions) to be adjusted in the scenario analysis is determined by the local sensitivity analysis.

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Line 600: it would be good to understand the connection between the sensitivity analysis and this has been used to design scenarios

155

Response: The local sensitivity analysis results are used to design the scenarios. The two most important parameters and boundary conditions identified in local sensitivity analysis, including the salinity and sea level at the submarine spring, are evaluated in the scenarios. The scenarios also use all the parameter values evaluated in the local sensitivity analysis. We have clarified the connection in section 5.

160

Line 601: remove one "the"

Response: Revised.

165

Line 742: any idea of why this is happening?

Response: The size of grid cells and advective velocity are two major factors of numerical dispersion, which can be reduced by decreasing the grid cell size (increasing the spatial resolution) and/or decreasing the advective velocity. In this study, the advective velocity is relatively fast associated with the large hydraulic conductivity. On the other hand, the computational time/cost will be a trouble for sensitivity analysis when increasing spatial resolution, since the model will run many times. We add some explanation in the last paragraph of conclusion section.

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**Numerical modeling and sensitivity analysis of seawater
intrusion in a dual-permeability coastal karst aquifer with
conduit networks**

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200 **Abstract**

Long distance seawater intrusion ~~has been~~ widely observed through the subsurface conduit system ~~in coastal karst aquifers, as groundwater contaminant~~. In this study, ~~seawater intrusion in dual-permeability karst aquifer with conduit networks is studied by~~ a two-dimensional density-dependent flow and transport SEAWAT model. Local and global sensitivity ~~analyses~~ are used to evaluate the effects of boundary conditions and hydrological characteristics ~~on modeling seawater intrusion in karst aquifer~~, including hydraulic conductivity, effective porosity, specific storage and dispersivity of the conduit ~~network~~ and ~~of~~ the porous medium. The local sensitivity evaluates the parameters sensitivities for modeling seawater intrusion ~~specifically~~ in the Woodville Karst Plain (WKP). The global sensitivity analysis provides a more comprehensive interpretation of parameter sensitivities, ~~such as~~ the non-linear relationship between simulations and parameters, ~~and/or parameter interactions~~. The conduit parameters ~~and boundary conditions~~ are important to the simulations in the porous medium, because of the ~~dynamical exchanges~~ between the two systems. Therefore, salinity and head simulations in the karst features, such as the conduit system and submarine springs, are critical for understanding seawater intrusion in a coastal karst aquifer. In the continuum SEAWAT model, ~~the sensitivity of hydraulic conductivity is not accurately evaluated, since the~~ conduit flow velocity ~~is not accurately calculated by Darcy's equation as a function of~~ head difference and hydraulic conductivity. In addition, dispersivity is no longer an important parameter in advection-dominated karst aquifer with conduit system, compared to the sensitivity results in a porous medium aquifer. Finally, the extents of seawater intrusion are quantitatively evaluated ~~and measured~~ under

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~~different~~ scenarios ~~by~~ changing the important parameters identified from sensitivity results, including salinity at the submarine spring with rainfall recharge, sea level rise and longer simulation time under an extended low rainfall period.

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245 Key Words: Seawater intrusion; Coastal karst aquifer; Variable-density numerical model; Dual-permeability karst system; Sensitivity analysis

1. Introduction

250 Many serious environmental issues have been caused by seawater intrusion in the coastal regions, such as soil salinization, marine and estuarine ecological changes, and groundwater contamination (Bear, 1999). Werner et al. (2013) pointed out that climate variations, groundwater pumping, and fluctuating sea levels are important factors to the mixing of seawater and freshwater in the aquifer. Custodio (1987) and Shoemaker (2004) summarized the control factors of seawater intrusion into a coastal aquifer, including the geologic and lithological heterogeneity, localized surface recharge, paleo-

255 hydrogeological conditions and anthropogenic influences. ~~Particularly, seawater intrusion~~ in a coastal aquifer is significantly impacted by sea level rise, which has been recognized as a serious environmental threat in ~~the 21st~~ century (Voss and Souza, 1987; Bear, 1999; IPCC, 2007). ~~In~~ Ghyben-Herzberg relationship, a small rise of sea level would cause 260 extended seawater intrusion, and significantly moves the mixing interface position further landward in a coastal aquifer (Werner and Simmons, 2009). ~~For example,~~ Essink et al. (2010) ~~systematically studied the exacerbated~~ seawater intrusion under sea level rise and global climate change. Likewise, high tides associated with hurricanes or tropical storms

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have been found to temporarily affect the extent of seawater intrusion in a coastal aquifer (Moore and Wilson, 2005; Wilson et al., 2011).

275 Modeling seawater intrusion in a coastal aquifer requires a coupled density-
dependent flow and salt transport groundwater model. The simulated salinity is computed
by the groundwater velocity field from flow modeling, and salinity in turn determines
water density and affects the simulation of flow field. Several variable-density numerical
models have been developed and widely used to study seawater intrusion, including
280 SUTRA (Voss and Provost, 1984) and FEFLOW (Diersch, 2002). SEAWAT is a widely
used density-dependent model, which solves flow equations by finite difference method,
and transport equations by three major classes of numerical techniques (Guo and
Langevin, 2002; Langevin et al., 2003). Generally speaking, most variable-density
models are numerically complicated and computational expensive, which require smaller
285 timestep and implicit procedure for solving flow and transport equations iteratively many
times in each timestep (Werner et al., 2013).

On the other hand, a karst aquifer is particularly vulnerable to groundwater
contamination including seawater intrusion in a coastal region, since sinkholes and karst
windows are usually connected by well-developed subsurface conduit networks. Some
290 karst caves are found open to the sea and become submarine springs below the sea level,
connected with the conduit network as natural pathways for seawater intrusion. Fleury et
al. (2007) reviewed the studies of freshwater discharge and seawater intrusion through
karst conduits and submarine springs in coastal karst aquifers, and summarized the
important control factors, including hydraulic gradient of equivalent freshwater head,
295 hydraulic conductivity, and seasonal precipitation variation. For example, seawater

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305 ~~intrudes through the conduit network as~~ preferential flow ~~and contaminates~~ the ~~fresh~~
groundwater ~~resources~~ in a coastal karst aquifer (Calvache and Pulido-Bosch, 1997). As
an indicator of rainfall and regional freshwater recharges, salinity at the outlet of conduit
system is diluted by freshwater discharge during a rainfall season, but remains constant as
saline water during a low rainfall period (Martin and Dean, 2001; Martin et al., 2012).

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Modeling groundwater flow in a dual-permeability karst aquifer is a challenging
issue since groundwater flow in a karst conduit system is often non-laminar (Davis, 1996;
310 Shoemaker et al., 2008; Gallegos et al., 2013). Several discrete-continuum numerical
models, such as MODFLOW-CFPM1 (Shoemaker et al., 2008) and CFPv2 (Reimann et
al., 2014; Reimann et al., 2013; Xu et al., 2015a; Xu et al., 2015b), have been developed
to simultaneously solve the non-laminar flow in the conduit, the Darcian flow in a porous
medium and the exchanges between the two systems. However, these constant-density

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315 karst models ~~have limitations in simulating~~ the density-dependent seawater intrusion
processes in a coastal aquifer. ~~The~~ VDFST-CFP, developed by Xu and Hu (2017), is
~~based on~~ a density-dependent discrete-continuum modeling approach to study seawater
intrusion in a coastal karst aquifer with conduits. However, VDFST-CFP is not able to

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320 ~~simulate the seawater intrusion processes~~ addressed in this study, ~~due to~~ the computational
constraints ~~and the numerical method limitations~~ associated with the aquifer geometry
and the domain scale. ~~Therefore, the variable-density SEAWAT model is still applied in~~
this study, in which Darcy's equation is used to compute flow not only in the porous
medium, but also in the conduit with large values of hydraulic conductivity and effective
porosity.

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335 Since simulating seawater intrusion in karst aquifer is challenging, sensitivities
 analysis is important to provide guideline for understanding the hydrology model, data
 collection and groundwater resources management. Several sensitivity studies have
 evaluated the parameters in karst aquifers. Kaufmann and Braun (2000) reported that
 boundary conditions and sink recharges are important to the preferential flow path in a
 340 karst aquifer. Scanlon et al. (2003) also confirmed that recharge is important to karst
 spring discharge. Regional sensitivity analysis (RSA) has been widely used to show that
 relationship of karst spring discharge with different hydrological processes in a local
 karst catchment (Chang et al., 2017), Chen et al. (2017) and Hartmann et al. (2015)
 applied Sobol's global sensitivity method to evaluate parameters using different objective
 345 functions under different hydrodynamic conditions. However, very few studies have
 addressed the parameter sensitivities of seawater intrusion in a coastal karst aquifer.
 Shoemaker (2004) performed a sensitivity analysis of the SEAWAT model for seawater
 intrusion to a homogeneous porous aquifer, concluded that dispersivity is an important
 parameter to the head, salinity and groundwater flow simulations and observations in the
 350 transition zone. Shoemaker (2004) also concluded that salinity observations are more
 effective than head observations, and head and salinity simulations and observations are
 more sensitive to parameters at the "toe" of the transition zone. The sensitivity results in
 this study confirm some conclusions in Shoemaker (2004), and highlight the significance
 of conduit network on seawater intrusion in a coastal karst aquifer with interaction
 355 between a karst conduit and a porous medium.

The parameter sensitivities are evaluated to address the impacts of the two major
 challenges in this study, as the density-dependent flow and transport coupled seawater

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intrusion processes, and the dual-permeability karst system. This study aims to strengthen the understanding of the roles of model parameters and boundary conditions in simulating

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seawater intrusion in the coastal karst region. To our knowledge, this is the first attempt to assess the parameter sensitivities for seawater intrusion to a vulnerable dual-

permeability karst aquifer. The rest of the paper is arranged as follows: the details of local and global sensitivity analysis methods are introduced in Sect. 2. The model setup, hydrological conditions, model discretization, initial and boundary conditions are

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discussed in Sect. 3. The results of local and global sensitivity analysis are discussed in Sect. 4. The scenarios of seawater intrusion simulation with different boundary

conditions and simulation time are presented in Sect. 5. The conclusions are made in Sect.

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6.

2. Methods

390 The governing equations used in the SEAWAT model can be found in the Guo
and Langevin (2002), including the variable-density flow equation with additional
density terms, and the advection-dispersion solute transport equation. The local and
global sensitivity methods used in this study are briefly introduced below. Note that the
sensitivity analysis does not necessarily need field observations, but only evaluates the
395 model simulations with respect to parameters instead. Field observational data, especially
head and salinity measurements in the conduit, are seldom available considering the
difficulties of sensor installation in the deep subsurface conduit network. Model
calibration is beyond the scope of this study, due to the lack of observational data in the
Woodville Karst Plain (WKP).

400

2.1 Local sensitivity analysis

In this study, UCODE_2005 (Poeter and Hill, 1998) is used in the local sensitivity
analysis to evaluate the derivatives of model simulations with respect to parameters at the
specified values (Hill and Tiedeman, 2006). The forward difference approximation of
405 sensitivity is calculated as the derivative of the i th simulation respect to the j th model
parameters,

$$\left. \frac{\partial y'_i}{\partial x_j} \right|_b \approx \frac{y'_i(x + \Delta x) - y'_i(x)}{\Delta x_j} \quad 1)$$

where y'_i is the value of the i th simulation; x_j is the j th estimated parameter; x is a vector
of the specified values of estimated parameter; Δx is a vector of zeros except that the j th
parameter equals Δx_j .

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Since parameters can have different units, scaled sensitivities are used to compare the parameter sensitivities. In UCODE 2005, a scaling method is used to calculate the dimensionless scaled sensitivities (DSS) by the following equation,

$$dss_{ij} = \left(\frac{\partial y'_i}{\partial x_j} \right) \Big|_x |x_j| \omega_{ii}^{1/2} \quad 2)$$

where dss_{ij} is the dimensionless scaled sensitivity of the i th simulation with respect to the j th parameter; ω_{ii} is the weight of the i th simulation, based on the variance-covariance matrix of the parameters.

The DSS values of different simulations with respect to each parameter are accumulated as the composite scaled sensitivities (CSS), which reflect the total amount of information provided by simulation for the estimation of one parameter. The CSS of the j th parameter is evaluated via:

$$css_j = \sum_{i=1}^{ND} \left[(dss_{ij})^2 |x_j| / ND \right]^{1/2} \quad 3)$$

where ND is the number of simulated quantities, as the head and salinity simulations in this study.

2.2 Morris method for global sensitivity analysis

The local sensitivity analysis is conceptually straightforward and easy to perform without expensive computational cost, however, only calculates the parameter sensitivities at one specified value for each parameter instead of the ranges. In addition, the local sensitivity indices are based on the first order derivative only, assuming a linear relationship of simulated quantities with respect to parameters.

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The global sensitivity analysis evaluates the non-linear relationship of parameters with simulations, and/or involved in interaction with other factors. Morris method is applied in this study to evaluate the global parameter sensitivities (Morris, 1991). The design of Morris method is made by individually randomized “one-step-at-a-time” (OAT) experiment, which perturbs only one input parameter and computes a new simulated output in each run. The Morris method is composed of a number r of local changes at different points of the possible range values. In each parameter, a discrete number of values called levels are chosen within the parameter ranges.

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In Morris method, the k -dimensional vector x of the model parameters has components x_i to be divided into p uniform intervals. The global parameter sensitivity is evaluated from the difference of simulation results by changing one parameter at a time, which is called an elementary effect (EE), d_i , defined as,

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$$d_i = \frac{1}{\tau_y} \frac{[y(x_1^*, \dots, x_{i-1}^*, x_i^* + \Delta, x_{i+1}^*, \dots, x_k^*) - y(x_1^*, \dots, x_k^*)]}{\Delta} \quad 4)$$

where Δ is the relative distance in the parameter coordinate; τ_y is the output scaling factor; $\{x_i^*\}$ is the parameter set selected in a sampling method.

To compute the EE for the k parameters, $(k+1)$ simulations will run with perturbation of each parameter, which is called one “path” (Saltelli et al., 2004). An ensemble of EEs is generated with multiple paths of parameter set. The total number of model run is $r(k+1)$, where r is the number of paths.

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Two sensitivity measures are proposed by Morris method to approximate parameter sensitivities: the mean μ estimates the overall influence of the factor on the output, and the standard deviation σ estimates the non-linear effect between input and

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output, and/or the parameter interactions (Saltelli et al., 2004). The mean μ and standard deviation σ of the EEs are evaluated with the r independent paths in the Morris method,

$$\mu = \sum_{i=1}^r d_i / r \quad 5)$$

$$\sigma = \sqrt{\sum_{i=1}^r (d_i - \mu)^2 / r} \quad 6)$$

In this study, the EEs for the method of Morris are not generated by Monte Carlo random sampling, which usually needs extremely large numbers (>250) of paths for the 11 parameters in this study and takes a very long time to complete sensitivity computation without parallelization. To save the running time and computational cost, the more efficient trajectory sampling is developed by Saltelli et al. (2004), which becomes a widely-used method to generate the ensemble of EEs for Morris method but ensure the confidence of global sensitivity results. In trajectory method, the choice of parameter p is usually even, and Δ equals to $\pm p / [2(p - 1)]$, either positive or negative. The trajectory method starts by randomly selecting a "seed" value x^* for the vector x . Each component x_i of x^* is randomly sampled from the set $(0, 1/(p-1), 2/(p-1), \dots, 1)$. The randomly selected vector x^* is used to generate the other sampling points but not one of them, which means that the model is never evaluated at vector x^* . The first sampling point, $x^{(1)}$, is obtained by changing one or more components of x^* by Δ . The second sampling point, $x^{(2)}$, is generated from x^* but differs from $x^{(1)}$ in its i th component that has been either increased or decreased by Δ , but conditioned on the domain, and the index i is randomly selected in the set $\{1, 2, \dots, k\}$. In other word, $x^{(2)} = (x_1^{(1)}, \dots, x_{i-1}^{(1)}, x_i^{(1)} \pm$

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$\Delta, x_{i+1}^{(1)}, \dots, x_k^{(1)}$). The third sampling point, $x^{(3)}$, differs from $x^{(2)}$ for only one component j , for any $j \neq i$, will be $x_j^{(3)} = x_j^{(2)} \pm \Delta$. A succession of $(k+1)$ sampling points $x^{(1)}, x^{(2)}, \dots, x^{(k+1)}$ is produced in the input parameters space called a trajectory, with the key characteristic that two consecutive points differ in only one component. Note that the choice of components x^* to be increased or decreased is conditioned on that x_i still being within the domain. In the trajectory sampling, any component i of the “base” vector x^* has been selected at least once by Δ in order to calculate one EE for each parameter.

Once a trajectory has been constructed and evaluated by Morris method, an EE for each parameter $i, i = 1, \dots, k$, can be computed. If $x^{(l)}$ and $x^{(l+1)}$, with l in the set in $(1, \dots, k)$, are two sampling points differing in their i th component, the EEs associated with the parameter i is computed as,

$$d_i(x^{(l)}) = \frac{|y(x^{(l+1)}) - y(x^{(l)})|}{\Delta} \quad (7)$$

A random ensemble of r EEs is pre-selected at the beginning of sampling, but the starting point of each trajectory sampling is also randomly generated. In other words, the points belonging to the same trajectory are not independent, but the r points sampled from each distribution belonging to different trajectories are independent.

3. Model development

3.1 Study site

The numerical model developed in this paper is based on the parameter values of porous medium and conduit measured in the aquifer at the Woodville Karst Plain (WKP). The Spring Creek Springs (SCS) is a system, consisting of 14 submarine springs located

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in the Gulf of Mexico (Fig. 1). SCS is an outlet of the subsurface conduit network and the entrance of seawater intrusion, exactly located at the shoreline beneath the sea level.

Davis and Verdi (2014) ~~described~~ a groundwater cycling conceptual model to explain the hydrogeological conditions in the WKP. ~~In this conceptual model of seawater and freshwater interaction,~~ seawater intrudes through subsurface conduit networks during low precipitation periods, while rainfall recharge ~~dilutes and~~ pushes the intruded seawater out ~~from the submarine spring during high rainfall periods, usually~~ after a heavy storm event.

Later on, ~~the~~ conceptual model is quantitatively simulated by a constant-density CFPv2 numerical model ~~in~~ Xu et al. (2015b). Tracer test studies and cave diving investigations indicate that the conduit system starts from the submarine spring and extends 18 km landward connecting with an inland spring called Wakulla Spring, although the exact locations of the subsurface conduits are unknown and difficult to explore (Kernagis et al., 2008; Kincaid and Werner, 2008). Evidence shows that seawater intrusion has been observed through subsurface conduit system for more than 18 km in the WKP (Xu et al., 2016). In addition, Davis and Verdi (2014) also point out that sea level rise at the Gulf of Mexico in the ~~20th~~ century could be a reason for increasing discharge at an inland karst spring (Wakulla Spring) and decreasing discharge at SCS, when the hydraulic gradient between the two springs is directed towards the Gulf.

(Insert Fig. 1 here)

In this study, a two-dimensional SEAWAT model is set up to simulate seawater intrusion ~~via the SCS~~ through the major subsurface conduit network in the WKP (Fig. 1). Figure 2 presents the cross section schematic figure in a coastal karst aquifer with a conduit network and a submarine spring opening to the sea. The model spatial domain is

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not a straight line from the SCS to Wakulla Spring, but the cross section along the major conduit pathway of seawater intrusion between the two springs. The conduit geometry in the model is set as 18-km long and 91-meter deep with the height of 10 meters in the horizontal part, and the width of 50 meters in the vertical part.

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(Insert Fig. 2 here)

The 2D model has some limitations on simulating seawater intrusion in the entire aquifer, usually assuming that the quantities are constant parallel to the shoreline.

However, this study only aims to evaluate the parameter sensitivities on modeling seawater intrusion in the coastal karst aquifer through the conduit network, salinity plume in the porous medium and the exchanges between the two systems are simulated within the vertical cross section. The simulation of seawater intrusion in the direction that perpendicular to the cross section and 3D flow and transport in the porous matrix are ignored and beyond the scope of this study. The assumption of two-dimensional model is

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reasonable and sufficient for understanding the parameter sensitivities, since the horizontal exchange flux between the conduit and the surrounding porous medium is relatively small, considering that the exchange permeability on the conduit wall is much smaller than the large conduit hydraulic conductivity. In addition, most SEAWAT models are setup for two-dimensional cross section with finer-resolution vertical discretization.

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3.2 Hydrological parameters

Table 1 presents the hydrological parameter values of the Upper Floridan Aquifer (UFA) in the WKP and boundary conditions used in the model. These parameters have

625 been calibrated in the regional-scale groundwater flow and solute transport models by
Davis et al. (2010), and then been applied in many previous modeling studies (Gallegos
et al., 2013; Xu et al., 2015a; Xu et al., 2015b). It should be pointed out that **model
calibration has not been conducted in this study**, since the **head and salinity** observational
field data are insufficient **particularly in the conduit**, considering the difficulties of
630 monitoring devices installation in the subsurface conduit. The parameter values **in Table
1** are evaluated in the following local sensitivity analysis and **then** applied in the seawater
intrusion scenarios in Sect. 5.

(Insert Table 1 here)

The values of hydrological parameters (hydraulic conductivity, specific storage
635 and effective porosity) in the conduit are generally greater than those **of** surrounding
porous medium. Hydraulic conductivity of the porous medium is assigned as 2286 m/day,
and as large as 610,000 m/day **for** the conduit system. **Note that** even the hydraulic
conductivity of porous medium in the study region is larger than most alluvial aquifers,
due to numerous small fractures and relatively large pores existed in the karst aquifer
640 **associated with the** dissolution of carbonate rocks. Specific storage and effective porosity
in the porous medium are assumed as 5×10^{-7} and 0.003, respectively. Specific storage
and effective porosity are 0.005 and 0.300 in the conduit layer, respectively. The
longitudinal dispersivity is estimated as 10 m in the porous medium, but is assumed a
very small value (0.3 m) in the conduit, because advection is **dominating** and dispersion
645 is negligible in the solution of transport in the conduit.

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3.3 Spatial and temporal discretization

The grid discretization and boundary conditions of the two-dimensional SEAWAT numerical model are shown in Fig. 3, with 140 columns and 37 layers in the cross section. Guo and Langevin (2002); Werner et al. (2013) pointed out that ~~fine-~~ ~~resolution~~ vertical grid is required for accurately modeling the density-dependent flow and solute transport. The vertical thickness of each grid cell is set uniformly as 3.048 m (10 ft) in this study, significantly smaller than the ~~large thickness of 152 m~~ in many previous constant-density modeling studies in the WKP, for example, Davis and Katz (2007); Davis et al. (2010); Xu et al. (2015a); Gallegos et al. (2013); Xu et al. (2015b).

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(Insert Fig. 3 here)

~~Based on the field scale,~~ the horizontal discretization for each cell is set uniformly as 152 m, except columns #22 and #139, which are 15.2 m as the vertical conduit network connecting the submarine spring (SCS) and inland spring (Wakulla Spring), respectively. The sizes of spring outlets and the conduit ~~network~~ are based on the observational field data and the calibrated values from the previous ~~modeling~~ studies (Gallegos et al., 2013). ~~For model simplicity,~~ the ~~size~~ of horizontal conduit network is assumed constant in this study. The outlet of vertical conduit system is the submarine spring (SCS) located at the shoreline at column #22. The conduit system starts from the submarine spring, descends downward to layer #29 (nearly 100 m below sea level), horizontally extends nearly 18 km from column #22 to column #139, and then rises upward to the top through column #139. Seawater intrudes at the SCS on the first layer of column #22, and then flows vertically downward into the conduit system. The inland spring is simulated by the DRAIN package as general head boundary condition in the

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SEAWAT model. All layers are simulated as confined aquifer since the conduit is fully saturated, which are consistent to the previous numerical models used in Davis et al. (2010); Xu et al. (2015a); Xu et al. (2015b) in the WKP.

690 A transient 7-day stress in the SEAWAT model is evaluated throughout this study, ~~expect the scenarios of longer simulation time for evaluating seawater intrusion~~ under an extended low rainfall period in Sect. 5.4. The timestep of flow model is set as 0.1 days, and the timestep of transport model is determined by SEAWAT automatically.

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695 3.4 Initial and boundary conditions

The initial condition of head is constant within each layer, set as 0.0 m as the present-day sea level for the cells from the boundary on the left (column #1) to the shoreline (column #22), and gradually rises to 1.52 m ~~at inland boundary on the right,~~ determined by the elevation of Wakulla Spring. Note that the head values are written in the input files of SEAWAT model instead of equivalent freshwater head. The initial conditions of salinity are assumed as a constant value of 35.0 PSU (Practical Salinity Unit), ~~assuming no freshwater dilution~~ at the sea boundary and the leftmost 10 columns. The seawater/freshwater mixing zone is assumed from 35 PSU at column #11 to 0 PSU at column #45, with a gradient of 1.0 PSU per column. Salinity is set uniformly as 0.0 PSU from column #46 to the inland boundary on the right, as uncontaminated freshwater before seawater intrudes. Several testing cases have been made to confirm that the initial conditions ~~do not significantly~~ affect the modeling results.

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The boundary conditions are also presented in Fig. 3. The less-permeable confining unit of the UFA base is simulated at the bottom of model domain as no-flow

715 boundary condition. The constant head and concentration inland boundary condition on
the right is 1.5 m as the elevation of inland spring, and 0.0 PSU as uncontaminated
freshwater. The seawater boundary on the left is 3.38 km away from the shoreline, set as
0.0 m constant head as the present-day sea level and 35.0 PSU constant concentration as
seawater without mixing. The boundary conditions of head and salinity at the submarine
720 spring (column #22, layer #1) are adjusted and evaluated in the scenarios of different sea
level, salinity and rainfall conditions in Sect. 5.

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4. Sensitivity Analysis

Sensitivity analysis evaluates the uncertainties of salinity and head simulations
725 with respect to eleven parameters, helps to understand the effects of variations and
interactions of aquifer parameters and boundary conditions on simulations. The symbols
and definitions of the eleven parameters are listed in Table 1, as well as the values
computed in the local sensitivity analysis, and the parameter ranges evaluated in the
global sensitivity analysis (Table 1). There are six parameters in the groundwater flow
730 model, including hydraulic conductivity (HY_P and HY_C), specific storage (SS_P and
SS_C) of the conduit and of the porous medium, recharge rate (RCH) and the sea level at
the submarine spring (H_SL). The other five parameters, including effective porosity
(PO_P and PO_C), dispersivity (DISP_P and DISP_C) of the conduit and the porous
medium, and the salinity at the submarine spring (SC), are in the solute transport model.

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4.1 Local sensitivity analysis

In the local sensitivity analysis, the CSSs of parameters with respect to head and salinity simulations are calculated at several locations along the conduit network and the porous medium, respectively. The CSSs are computed for the parameter values in the maximum seawater intrusion benchmark case in Sect. 5.1, which is developed to quantitatively evaluate the extent of seawater intrusion specifically in the WKP after a 7-day low precipitation period. The parameters to be adjusted and evaluated in the scenarios are also determined based on the local sensitivity result.

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Parameter sensitivities are computed at several locations, from column #25 to column #75 with an interval of 5 cells along the horizontal conduit (layer #29), where column #25 is close to the shoreline as fully contaminated by seawater, and column #75 is assumed as the uncontaminated freshwater aquifer. The parameter sensitivities of simulations in a porous medium are evaluated at layer #24, 15.2 m (50 ft) or 5 layers above the conduit layer, from column #25 to column #75 with an interval of 5 cells along the horizontal direction.

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4.1.1 Local sensitivity analysis of simulations in the conduit

Figure 4 shows the arithmetic mean of CSSs computed in the evaluated locations along the conduit layer. The largest CSS value indicates that salinity at the submarine spring (SC) is the most important parameter to both salinity and head simulations. Hydraulic conductivity, specific storage and effective porosity of the conduit (HY_C, SS_C and PO_C), as well as the sea level at the submarine spring (H_SL) are also important parameters. Simulations are not sensitive to hydraulic conductivity, specific

770 storage and effective porosity of the porous medium (HY_P, SS_P and PO_P), recharge
rate (RCH) and dispersivity (DISP_C and DISP_P). Generally speaking, the parameter
sensitivities with respect to head simulations are similar and consistent with salinity
simulations.

(Insert Fig. 4 here)

775 The boundary conditions of the conduit system, including salinity and sea level at
the submarine spring (SC and H_SL), are important in modeling seawater intrusion in the
WKP. Seawater enters the conduit system at the submarine spring, and intrudes landward
through the subsurface conduit system. The most important parameter is identified as the
780 salinity at the submarine spring (SC), which ~~affects~~ the equivalent freshwater head in
terms of water density at the inlet of conduit system, and affects flow simulation within
the conduit system. ~~The salinity at the submarine spring (SC) is determined by freshwater~~
mixing and dilution from the conduit network, in other words, is controlled by the rainfall
recharges and freshwater discharge from the aquifer to the sea. In this study, rainfall
recharge is represented by salinity at submarine spring with freshwater dilution ~~instead of~~
785 ~~the recharge~~ flux on the surface (RCH), ~~which~~ is not an important parameter, ~~and~~ not
applicable to represent the total rainfall recharge in ~~the~~ two-dimensional SEAWAT
model. On the other hand, the sea level at the submarine spring (H_SL) has an
intermediate CSS, ~~indicating that it is also important in~~ flow field and salinity transport
simulations. However, ~~sea level~~ is not as important as the salinity at the submarine spring
790 (SC). In other words, the extent of seawater intrusion in the conduit is more sensitive to
rainfall recharge and freshwater discharge represented by the parameter SC, rather than
the sea level and/or tide level variations.

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805 Dispersivity is usually an important parameter in the sensitivity analysis of
transport modeling in a porous medium aquifer (Shoemaker et al., 2004). However, the
conduit and porous medium dispersivities (DISP_C and DISP_P) are not evaluated as
important parameters in the dual-permeability model, in this study. Advection is
dominating in the transport of seawater in the high permeability conduit network, while
dispersion is negligible in such high velocity flow condition. Moreover, the dispersion
810 solution and dispersivity sensitivities in the conduit are inaccurately calculated when
conduit flow becomes turbulent. On the other hand, the numerical dispersion is
significantly greater than the physical dispersion in the conduit. The Peclet number can
be as great as 2500, far beyond the theoretical criteria (<4) for solving the advection
dispersion transport equation by finite difference method (Zheng and Bennett, 2002).
815 Dispersivity sensitivities have large uncertainty in this study, indicating that the
continuum SEAWAT model is not applicable to accurately compute the salinity
dispersion in the conduit. An experiment of deactivating the DSP (dispersion) package in
SEAWAT confirms that dispersion is negligible within the conduit network in this study,
and the mixing is mostly due to the numerical dispersion instead of the solution of
820 dispersion equation in this study.

The parameters with the six largest CSS are presented in Fig. 5, with respect to
the combination of head and salinity simulations in the evaluated locations along the
conduit network, from column #25 to column #75. The largest CSS values are found at
either column #50 or #55 within the conduit, matches with the position of
825 seawater/freshwater mixing zone along the conduit network in the maximum seawater
intrusion case (Sect. 5.1). The largest CSS values are found at the mixing zone than

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anywhere else for all parameters, because head and salinity simulations only change significantly near the mixing zone but remain constant in other locations.

840 (Insert Fig. 5 here)

4.1.2 Local sensitivity analysis of simulations in the porous medium

Figure 6 shows the arithmetic mean of CSSs computed in the evaluated locations in the porous medium (layer #24). The largest CSS value indicates that salinity at the submarine spring (SC) is also the most important parameter with respect to simulations in the porous medium, although it is a boundary condition of the conduit system. However, some parameter sensitivities exhibits different pattern compared to from the results of simulations in the conduit. The hydraulic conductivity and effective porosity of both the conduit and porous medium (HY_C, HY_P, PO_C & PO_P), specific storage of the conduit (SS_C) and dispersivity of the porous medium (DISP_P), have intermediate CSS values. The CSS values at different evaluated locations along the layer of porous medium are plot in Fig. 7, except the three unimportant parameters. Similar to the sensitivity analysis of simulations along the conduit, the largest CSSs are found at either column #35 or #40, which is the mixing zone position in the porous medium in the maximum seawater intrusion case (Sect. 5.1).

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(Insert Fig. 6 and 7 here)

The important rules of the boundary condition and hydrological parameters of the conduit system on simulations in the porous medium are highlighted in the local sensitivity analysis. Salinity at the submarine spring (SC) remains the most important parameter and determines the seawater intrusion plume in the porous medium. The

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conduit parameters, such as hydraulic conductivity, effective porosity and specific storage (HY_C, PO_C and SS_C), are also important to the simulations in the porous medium. The CSSs of conduit parameters indicate that groundwater flow and seawater transport through the conduit system have significant impact on the simulations in the surrounding porous medium. In summary, simulations in the porous medium are sensitive to both the conduit and porous medium parameters, highlight the interaction between the two domains in simulating seawater intrusion in the dual-permeability WKP coastal karst aquifer. As a result, simulations and observations of salinity and head in the conduits and other karst features have significance on calibrating numerical models and values for understanding seawater intrusion.

4.1.3 Parameter correlations

The correlation coefficients and covariance matrix of all parameters are calculated and presented in Fig. 8. The white and black colors represent positive and negative parameter correlations, respectively. Generally speaking, hydrological parameters of porous medium are positively correlated with the other parameters of porous medium, but negatively correlated with conduit parameters, and vice versa. On the other hand, hydraulic conductivity, specific storage and porosity have similar correlation pattern among all evaluated parameters, while the correlation of dispersion is different than others. For example, hydraulic conductivity (HY_P) has strong positive correlation with specific storage (SS_P) and porosity (PO_P), however, has negatively correlated with dispersivity (DISP_P). The correlations of conduit parameters exhibit similar relationship as well. The results can be explained as that larger hydraulic conductivity would result in higher seepage velocity in either conduit or porous medium by the Darcy's Law;

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therefore, salt transport comes from the submarine springs also results in higher salinity in both the conduit porous medium domains. However, larger dispersivity could decrease the peak values of salinity concentration but enlarge contaminant plumes due to stronger dispersion and diffusion.

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4.2 Global sensitivity analysis

The local sensitivity analysis analyzes the parameter sensitivities specifically for the seawater intrusion in the WKP, as the maximum seawater intrusion case in Sect. 5.1.

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However, local sensitivity result is lack of representative for the entire parameter ranges, and higher-order derivatives of simulations. The global sensitivity analysis is essential to provide a comprehensive understanding of the relationship between simulations and parameters for modeling seawater intrusion to a coastal karst aquifer.

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The derivatives of simulations with respect to the selected parameters in Figure 9, clearly indicate local sensitivity results are not representative in the entire parameter range. For example, both head and salinity simulations in the conduit are nearly constant to the variation of an unimportant parameter (DISP_P) in the local sensitivity study.

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However, simulations are non-linear to salinity at the submarine spring (SC). Parameter SC is identified as the most important parameter in the local sensitivity analysis, partially because that the CSS value is computed at the largest derivative value where salinity is 35 PSU.

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(Insert Fig. 9 here)

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The locations in the conduit and porous medium systems with the largest CSS values from the local sensitivity analysis are evaluated in the global sensitivity analysis.

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Parameter sensitivities are computed at the locations with largest CSS values in the previous local sensitivity analysis, specifically, column #50, layer #29 in the conduit and column #35, layer #24 in the porous medium, respectively. The Trajectory sampling method developed by Saltelli et al. (2004) is introduced in Sect. 2.2 and applied in the global sensitivity analysis, with the recommended choice of $p = 4$ and $r = 10$ by Saltelli et al. (2004).

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4.2.1 Global sensitivity analysis of simulations in the conduit

In the global sensitivity analysis, the mean and standard deviation of the EEs for salinity simulation in the conduit (column #50, layer #29) are presented in Fig. 10a.

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Consistent with the local sensitivity analysis, the largest mean value of EEs indicates that parameter SC is the most important parameter to salinity simulations. Parameter SC also

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has the largest standard deviation of the EEs due to the non-linear relationship between salinity simulation and parameter SC shown in Fig. 9, in which the derivatives vary with

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different parameter values. The hydraulic conductivity and effective porosity of the conduit (HY_C and PO_C), as well as sea level (H_SL), are all important to salinity simulation with relatively large mean and standard deviation values of EEs. Generally speaking, the local and global sensitivity study results for salinity simulation in the conduit are similar, however, the standard deviation of EEs provides additional

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information of parameter sensitivities in the global sensitivity study.

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(Insert Fig. 10 here)

The global sensitivities for head simulations with respect to parameters are more complicated than salinity simulations (Fig. 9b). The mean and standard deviation of EEs

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for head simulations are smaller than those for salinity simulations, consistent with the conclusion of Shoemaker (2004) that salinity simulation is more effective than head. The two largest mean values of EEs show that the specific storage (SS_C) and effective porosity (PO_C) of the conduit are the two most important parameters. As mentioned in the local sensitivity analysis, parameters in transport model are also important to the head simulation in a coupled density-dependent flow and transport model. For example, effective porosity is important in head simulation since the solution of salinity transport in turn determines the density and impact flow calculation in the model, particularly in the study of density-dependent seawater intrusion. In addition, head simulations are also sensitive to the boundary conditions of salinity in the transport model, since equivalent freshwater head is a function of density in terms of salinity in the coupled variable-density flow and transport model for simulating seawater intrusion. Different from salinity simulation, salinity at the submarine spring (SC) no longer has the largest mean of EEs. However, the standard deviation of EEs for parameter SC is still the largest due to the non-linear relationship to head simulation shown in Fig. 9.

One of the major finding in the global sensitivity analysis is that the hydraulic conductivity of the conduit (HY_C) has smaller means and standard deviations of EEs than the other two parameters (PO_C and SS_C), and no longer becomes the most important parameter as shown in the previous local sensitivity analysis. This is different from the common knowledge and empirical experience in hydrogeological modeling, but is actually reasonable in karst aquifer with the non-laminar conduit flow. In the SEAWAT model, Darcy equation is used to calculate the flow velocity in the whole model domain including the conduit system, however, is only accurate for laminar

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seepage flow in the porous medium. Groundwater flow is usually non-laminar even turbulent in the conduit system, when the conduit flow rate is non-linear to head gradient and hydraulic conductivity. The simulation of conduit flow is beyond the applicability of Darcy equation in SEAWAT model, with relatively large error and uncertainty in the relationship between hydraulic conductivity and head simulation. Then, the uncertainty of hydraulic conductivity sensitivities can be large and difficult to be accurately measured.

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4.2.2 Global sensitivity analysis of simulations in the porous medium

The hydraulic conductivity of the porous medium (HY_P) and salinity at the submarine spring (SC) are identified as the two most important parameters for salinity simulations in the porous medium (Fig. 11a). Compared to parameter HY_P, parameter SC has much larger CSS value at 35.0 PSU with the largest derivative in the local sensitivity analysis (Fig. 6), and also larger standard deviation of EE in the global sensitivity analysis. Local sensitivity analysis overestimates the sensitivity of parameter SC within the range, and global sensitivity analysis provides a more comprehensive understanding of the physical meaning of parameter SC, for example, variability of rainfall recharges and freshwater discharge. As the boundary condition of conduit system, salinity at the submarine spring (SC) determines the equivalent freshwater head at the inlet of seawater intrusion and affects simulations in the conduit, and also the surrounding porous medium via exchanges between the two systems. The global sensitivity results highlight the significance of conduit and porous medium interactions in a dual-permeability aquifer. Similar to salinity at the submarine spring (SC), dynamic interactions between the conduit and the porous medium in this study are clearly shown

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1100 ~~in~~ the relatively large mean of EEs for sea level (H_SL), effective porosity and specific storage of the conduit (PO_C and SS_C). ~~Effective porosity is important for head simulations in this study, since the density-dependent flow and transport models are coupled for simulating seawater intrusion.~~
 (Insert Fig. 11 here)

1105 On the other hand, parameter sensitivities for simulations in the porous medium ~~are different from the sensitivities for simulations~~ in the conduit. The porous medium hydraulic conductivity (HY_P) is an important term in the flow equation for solving head and advective velocity for the transport equation (Fig. 11b), similar to most sensitivity result of hydrological modeling for flow in a porous medium. ~~For the~~ simulations in the conduit, ~~effective porosity and specific storage of the conduit (PO_C and SS_C) are more~~ important than hydraulic conductivity (HY_C), because of the ~~large~~ uncertainty in conduit flow ~~computation~~ by Darcy's equation in the continuum SEAWAT model.

1110 **5. Seawater Intrusions Scenarios**

1115 In this section, the ~~extents~~ of seawater intrusion are quantitatively measured and evaluated under ~~different~~ scenarios of boundary conditions, which are identified as the important parameters in the local sensitivity analysis. In each scenario, only one parameter is adjusted and others ~~are constant as~~ the maximum seawater intrusion benchmark case ~~in Sect. 5.1.~~

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5.1 The maximum seawater intrusion benchmark case

1180 The local sensitivity analysis computes the sensitivities of parameter values in the maximum seawater intrusion benchmark case, which assumes the head and salinity boundary conditions are 0.0 m as the present-day sea level, and 35.0 PSU as seawater without dilution at the conduit system outlet, respectively. Salinity and sea level at the submarine spring (SC and H_SL) are identified as two important parameters and then adjusted in the following two scenarios. In this case, the longest distance of seawater intrusion is simulated with the assumption that freshwater recharge is negligible, and the outlet of conduit system is filled with undiluted seawater. Figure 12 presents the simulated salinity and head profile in the cross section after a 7-day simulation.

(Insert Fig. 12 here)

1190 According to the Ghyben-Herzberg relationship, landward seawater intrusion is on the bottom of the aquifer beneath the seaward freshwater on the top. The equivalent freshwater head at the submarine spring is calculated as 2.29 m (7.5 ft) when salinity is 35.0 PSU at the submarine spring, and undiluted seawater is filled within the 91 meters deep submarine cave and conduit network. The equivalent freshwater head at the submarine spring is higher than the 1.52 m (5.0 ft) constant head at the inland spring, 1195 diverts the hydraulic gradient landward and causes seawater to intrude into the aquifer. Seawater intrudes further landward through the highly permeable conduit network, also contaminates the surrounding porous medium via exchange on the conduit wall. The seawater/freshwater mixing zone in the deep porous medium beneath the conduit is only slightly behind the seawater front in the conduit, because high-density saline water easily 1200 descends from the conduit and flows downward. The area with relatively smaller salinity

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to the left of the vertical conduit network near shore is due to the freshwater discharge dilution from the aquifer to the sea, since the equivalent freshwater head is only 2.29 m at the submarine spring but remains 0 m in other areas. The mixing zone position in the conduit, defined as the location with salinity of 5.0 PSU, is measured at nearly 5.80 km landward from the shoreline. The width of mixing interfaces, defined as the distance between the locations with salinity of 1.0 PSU and 25.0 PSU, are roughly the same as 7 grid cells or 1.13 km in both the conduit and porous medium.

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5.2 Salinity variation at the submarine spring (SC)

Sensitivity analysis indicates that the salinity at the submarine spring (SC) is generally the most important parameter for simulations in both the conduit and the porous medium. Salinity at the submarine spring is diluted by large amount of rainfall recharge and freshwater discharge after a significant precipitation event, but remains highly saline after an extended low rainfall period, as shown in the maximum seawater intrusion benchmark case in Sect. 5.1. The equivalent freshwater head at the submarine spring is 2.29 m when salinity is 35.0 PSU, proportionally decreases to 0.0 m, where salinity is 0.0 PSU and freshwater is filled within the conduit system. The impact of freshwater recharge on seawater intrusion is evaluated in four scenarios with salinity levels of 0.0 PSU, 10.0 PSU, 20.0 PSU and 30.0 PSU at the submarine spring (Fig. 13). The mixing zone in both the conduit and porous medium are measured at 4.0 (4.5) km away from the shoreline in the cases of salinity of 10.0 (20.0) PSU at the submarine spring. Compared to the maximum seawater intrusion benchmark case, rainfall recharge and freshwater discharge dilute seawater intrusion and move the interface significantly seaward. The

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mixing zone is very close to the shoreline when salinity is 0.0 PSU at the submarine spring and seawater intrusion is blocked by large amount of freshwater dilution. The shape of mixing interface is similar to the maximum seawater intrusion benchmark, but the width of mixing interface is wider due to the slower advective flow with smaller or even reversed hydraulic gradient from the aquifer to the sea. In the scenarios of freshwater dilution, the solution of dispersion becomes more accurate and important in salinity transport with slower groundwater seepage flow. Generally speaking, a heavy rainfall event dilutes the intruded seawater and moves the mixing interface seaward.

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5.3 Sea level variation at the submarine spring (H_SL)

In addition to salinity, sensitivity analysis indicates that sea level at the submarine spring is also an important parameter. IPCC (2007) predicted an approximation of 1.0 m sea level rise by the end of 21st century, which has significant impacts on seawater intrusion in a coastal karst aquifer. The extents of seawater intrusion in the conduit and porous medium under 0.91 m and 1.82 m sea level rise conditions are quantitatively evaluated in this study (Fig. 14). Salinity at the submarine spring remains 35.0 PSU, but the head at the submarine spring increases to simulate rising sea level. The simulated salinity profiles show that the width and shape of the mixing zone are similar to the results in the maximum seawater intrusion benchmark. However, the mixing zone is intruded landward along the conduit to almost 7.08 km from the shoreline with 0.91 m sea level rises, which is 1.28 km further inland than the simulation under present-day sea level. In the other extreme case of 1.82 m sea level rise, seawater intrudes additional 0.97

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1295 km further inland along the conduit than the simulated result with 0.91 m sea level rise,
or 2.25 km further landward than the simulation under present-day sea level. Compared
1300 to the porous alluvial aquifer, seawater intrudes further landward through the conduit
network in the a dual-permeability karst aquifer under sea level rise. This scenario
confirms the concerns of severe seawater intrusion in the coastal karst aquifer under sea
level rise, also highlights the values of conduit system as the major pathway for long-
distance seawater intrusion. In addition, sea level rise might have great impacts on the
regional flow field and hydrological conditions in a coastal aquifer. Davis and Verdi
(2014) reported an increasing groundwater discharge at the inland Wakulla Spring in the
WKP associated with the rising sea level in the past decades. The relationship between
spring discharge and sea level was quantitatively simulated by a CFPv2 numerical model
1305 in Xu et al. (2015b), however, beyond the scope of this study.

(Insert Fig. 14 here)

5.4 Extended low rainfall period

1310 The elapsed time in simulations are set constant in the sensitivity analysis and the
previous scenarios for consistent comparison purposes. However, extents of seawater
intrusion under scenarios of extended low rainfall periods are presented in Fig. 15, with
the extended simulated time of 14, 21 and 28 days. The boundary conditions of salinity
and sea level at the submarine spring remain 35.0 PSU and 0.0 m, respectively, as the
maximum seawater intrusion benchmark.

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1330 Seawater persistently intrudes through both the conduit and the porous medium domains during the extended low rainfall period, since the 2.29 m equivalent freshwater head at the submarine spring is higher than the inland freshwater boundary. Compared to the maximum seawater intrusion benchmark with a stress period of 7-day elapsed time in simulation, the mixing zone position moves additional 1.29 km landward in the conduit and the surrounding porous medium in the 14-day simulation. In the predictions of 21 (28)-day extended low rainfall period, the mixing zone finally arrives at 7.56 (7.89) km from the shoreline. Above all, seawater intrudes further inland through conduit network during an extended low rainfall period, contaminates fresh groundwater resources in the aquifer and becomes an environmental issue in coastal regions.

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1340 **6. Conclusion**

In this study, a two-dimensional SEAWAT model is developed to study seawater intrusion in a dual-permeability coastal karst aquifer with a conduit network. Local and global sensitivity analyses are used to evaluate the parameter sensitivities, and then help understand the roles of karst features in seawater intrusion. Some major conclusions from sensitivity analysis are summarized here,

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1) The global sensitivity analysis is important to accurately estimating the parameter sensitivities in wide ranges, due to the parameter interactions and non-linear relationship between simulations and parameters shown in Fig. 9, since local sensitivity analysis only evaluates at one specified parameter value. Different from other karst studies, head simulations are sensitive to boundary conditions and parameters of transport equation, since the solution of salinity in terms of

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density affects the equivalent freshwater head calculation in the coupled density-dependent flow and transport SEWAT model.

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2) Overall, salinity at the submarine spring (SC) is the most important parameter.

The boundary conditions and hydrological parameters of the conduit system are important to not only the simulations in the conduit, but also the porous medium via exchanges between the two systems. The submarine spring and conduit

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system are the major entrance and pathway, respectively, for seawater intrusion in the coastal karst aquifer. Sensitivity analysis indicates that the simulations in the conduit are particularly important for understanding the hydrogeological processes in the dual-permeability karst aquifer, and field observational data within the conduit system are necessary for the model calibration.

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3) Different from the previous studies in Shoemaker (2004), dispersivity is no longer an important parameter for simulations in the conduit. Advection is dominant but dispersion is negligible in salinity transport under the conditions of turbulent flow in the conduit, and also the relatively fast seepage flow in the surrounding porous medium. The interaction between conduit and porous medium significantly

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change the flow field and affect the applicability of transport model. In the simulated salinity profile, mixing process is mostly due to numerical dispersion instead of the solution of dispersion equation, since the Peclet number is extremely large in the domain and beyond the criteria of solving transport equation by finite difference method.

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4) Hydraulic conductivity is no longer an important parameter for simulations in the conduit. Conduit flow is usually non-laminar and beyond the range of Darcy

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1405 equation used in SEAWAT model, which assumes a linear relationship between specific discharge and head gradient. Therefore, the uncertainty and sensitivity of conduit permeability is difficult to be accurately evaluated by hydraulic conductivity in the continuum model.

The extents of seawater intrusion and width of mixing interface are quantitatively
1410 measured with different salinity and sea level at the submarine spring, which are identified as important parameters in the sensitivity study. In the maximum seawater intrusion benchmark case with salinity and head as 35.0 PSU and 0.0 m at the submarine spring, respectively, the mixing zone in the conduit moves to 5.80 km from the shoreline with 1.13 km wide after a 7-day low rainfall period. Rainfall and regional recharges
1415 dilute the salinity at the submarine spring (SC), and significantly shift the mixing zone position seaward to 4.0 (4.5) km away from the shoreline with salinity of 10.0 (20.0) PSU.

Compared to the benchmark, seawater intrudes additional 1.29 (2.25) km further landward along the conduit under 0.91 (1.82) m sea level rise at the submarine spring (H_SL). In addition, the impacts of extended low rainfall on seawater intrusion through
1420 conduit network are also quantitatively assessed with longer elapsed time in simulation. The mixing zone moves to 7.56 (7.89) km from the shoreline, after a 21 (28)-day low precipitation period.

In summary, the modeling and field observations in the karst features, including the subsurface conduit network, the submarine spring and karst windows, are critical for
1425 understanding seawater intrusion in a coastal karst aquifer, and important for model calibration. The discrete-continuum density-dependent flow and transport model, for example, the VDFST-CFP in Xu and Hu (2017), is important to accurately simulate

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1435 seawater intrusion and assess parameter sensitivities in the coastal karst aquifer with
conduit networks. ~~Advanced numerical methods and/or high-performance computing~~ are
expected to solve the issue of Peclet number limitation in this study, and ~~reduce the~~
~~uncertain~~ of dispersion ~~solution with higher resolution~~.

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Competing interests

1440 The authors declare that they have no conflict of interest.

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Table 1. The symbols and definitions of parameters used in this study, the specified evaluated values in local sensitivity study and evaluation ranges (the lower and upper constraints) of each parameter in global sensitivity analysis.

Parameter	Definitions	Lower	Upper	Evaluated value	Unit
HY_P	Hydraulic conductivity (porous medium)	1.524	4.572	2.286	($\times 10^3$) meters/day
HY_C	Hydraulic conductivity (conduit)	3.048	9.144	6.096	($\times 10^5$) meters/day
SS_P	Specific storage (porous medium)	4.00	6.00	5.00	($\times 10^{-7}$) dimensionless
SS_C	Specific storage (conduit)	0.03	0.07	0.05	dimensionless
RCH	Recharge rate on the surface	0.00	0.03	0.01	meters/day
H_SL	Sea level at the submarine spring	-0.305	0.914	0.305	meters
PO_P	Porosity (porous medium)	0.001	0.005	0.003	dimensionless
PO_C	Porosity (conduit)	0.200	0.400	0.300	dimensionless
SC	Salinity at the submarine spring	0.0	35.0	35.0	PSU
DISP_P	Longitudinal dispersivity (porous medium)	6.10	12.20	10.00	meters
DISP_C	Longitudinal dispersivity (conduit)	0.15	0.60	0.30	meters

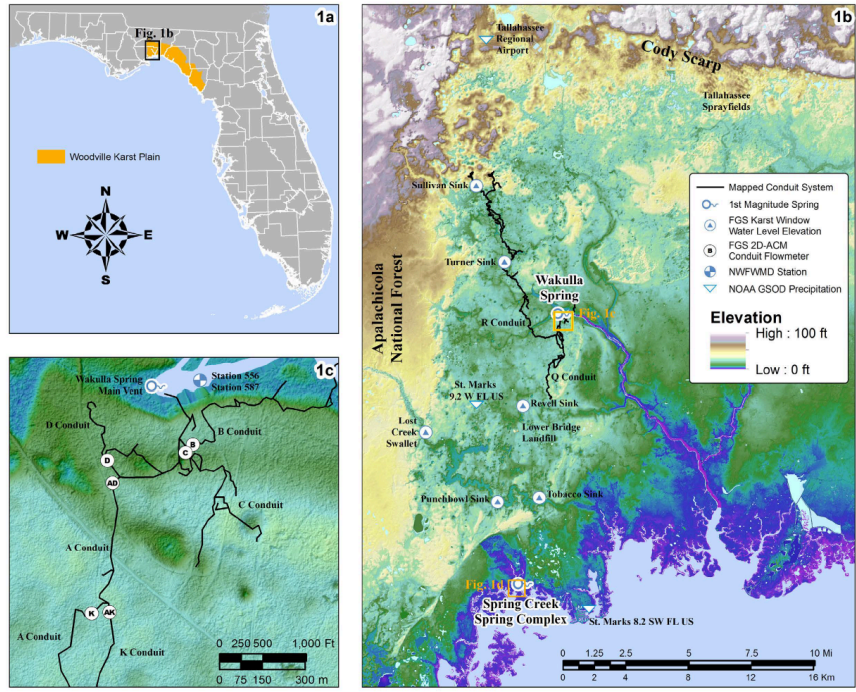


Figure 1. a) Locations of the Woodville Karst Plain (WKP) and the study site; b) The map of the Woodville Karst Plain showing the locations of features of note with the study; c) The detail of cave system near Wakulla Springs. Modified from Xu et al., (2016).

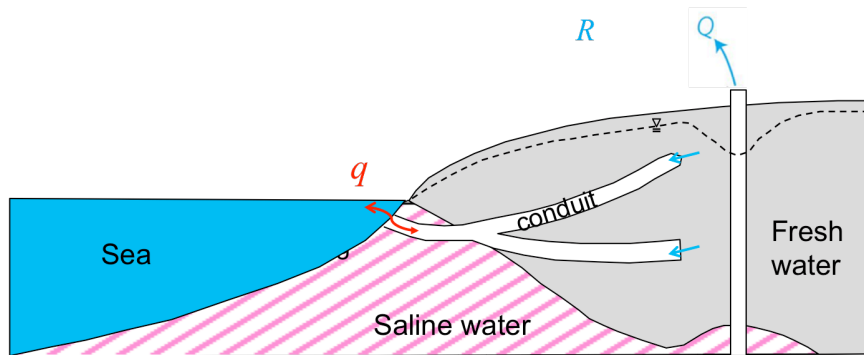
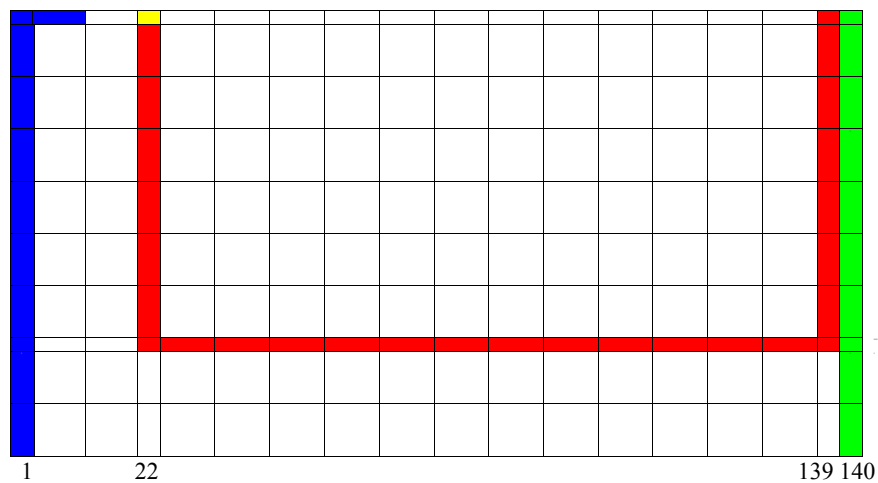


Figure 2. Schematic figure of a coastal karst aquifer with conduit networks and a submarine spring opening to the sea in a cross section. Flow direction q would be seaward when sea level drops, pumping rate Q is low and precipitation recharge R is large; however, reversal flow occurs when sea level rises, pumping rate Q is high or precipitation recharge R is small.



Explanations:

- Constant head and constant concentration of the submarine spring and outlet of karst conduit system, however, various in different cases of numerical models
- Sea-edge boundary: constant head (0.0 ft in normal sea level case) and constant concentration (35 PSU)
- Inland boundary: constant head (5.0 ft) and constant concentration (0 PSU)
- Conduit: high hydraulic conductivity, porosity and specific storage
- Porous medium: low hydraulic conductivity, porosity and specific storage

Figure 3. Schematic figure of finite difference grid discretization and boundary conditions applied in the SEAWAT model. Every cell represents 10 horizontal cells and 4 vertical cells, except the boundary and conduit layer in color with smaller width. The submarine spring is located at column #22, layer #1, and the inland spring is located at column #139, layer #1. The conduit system starts from the top of column #22, descends downward to layer #29, horizontally extends to column #139, and then rises upward to the top through column #139.

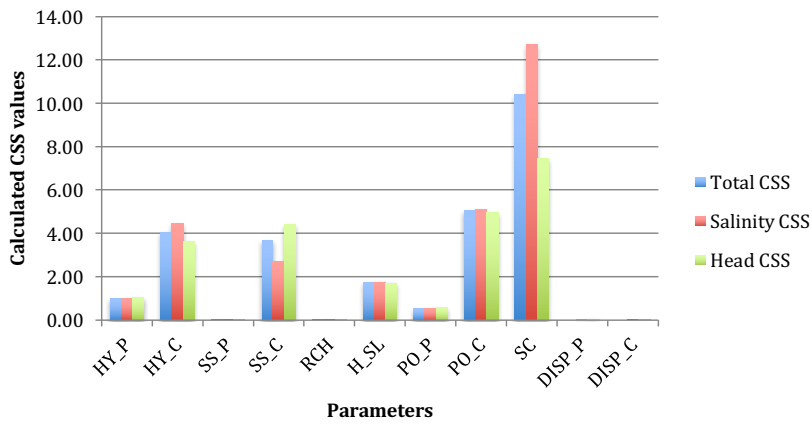


Figure 4. The CSSs (Composite Scaled Sensivities) of all parameters with respect to simulations in the conduit (layer #29) in the local sensitivity analysis.

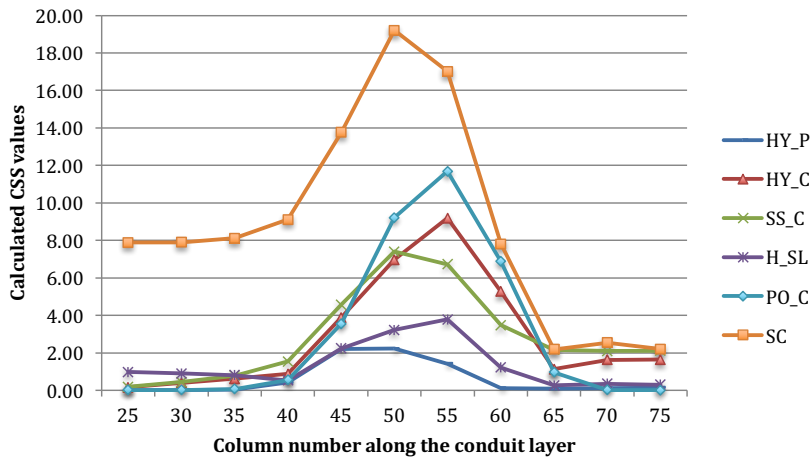


Figure 5. The CSSs (Composite Scaled Sensivities) of selected parameters at different locations along the conduit layer (from column #25 to column #75) in the local sensitivity analysis.

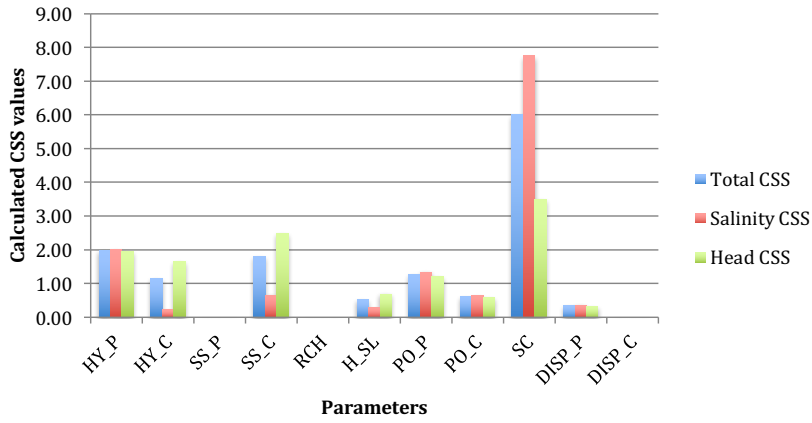


Figure 6. The CSSs (Composite Scaled Sensitivities) of all parameters with respect to simulations in the porous medium (layer #24) in the local sensitivity analysis.

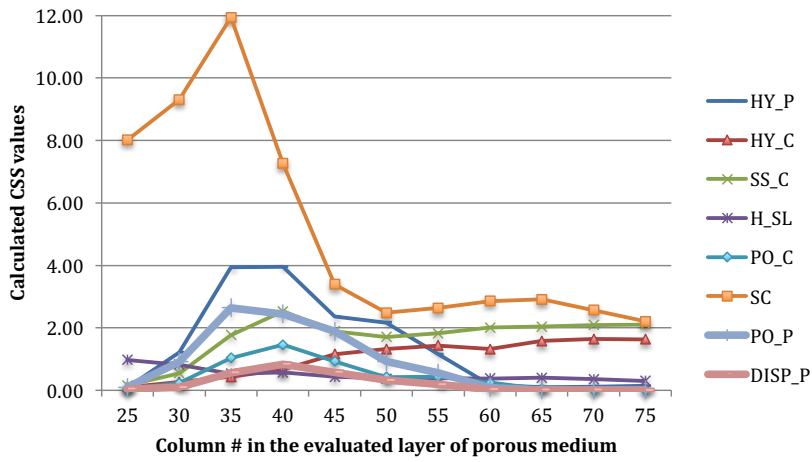
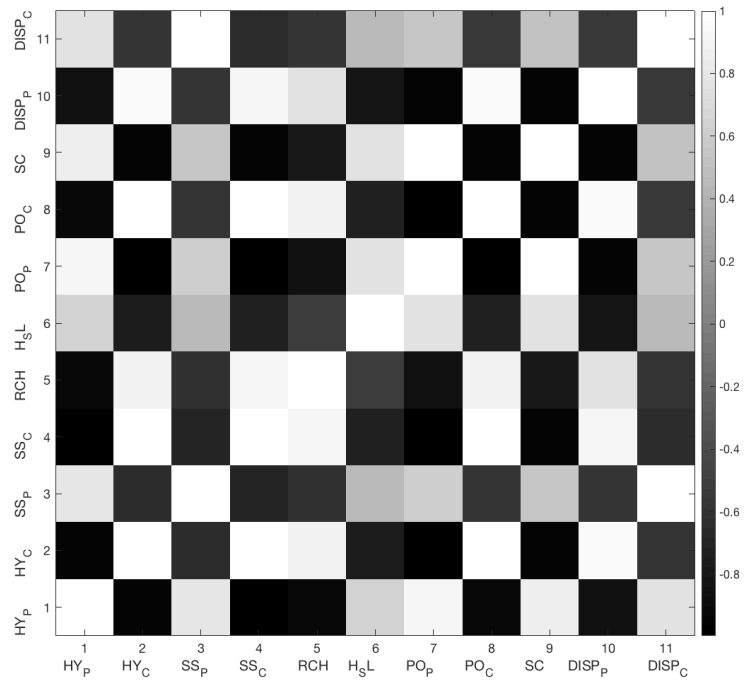


Figure 7. The CSSs (Composite Scaled Sensitivities) at different locations in the porous medium (from column #25 to column #75 at layer #24) in the local sensitivity analysis.

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Figure 8. The Pearson-pattern correlation coefficient matrix for all eleven parameters.

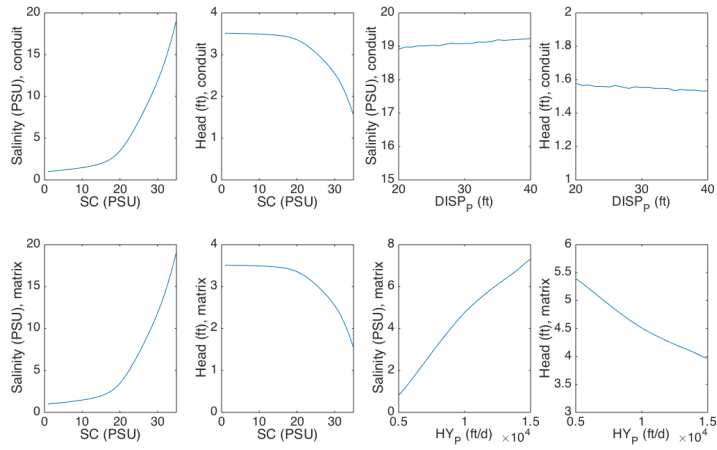


Figure 9. The non-linear relationship between head and salinity simulations with respect to parameters SC, DISP_P and HY_P. (Note that the scale for each plot is different).

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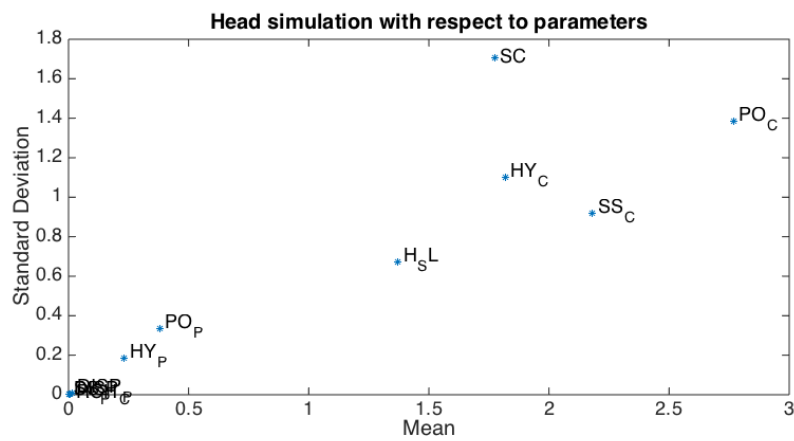
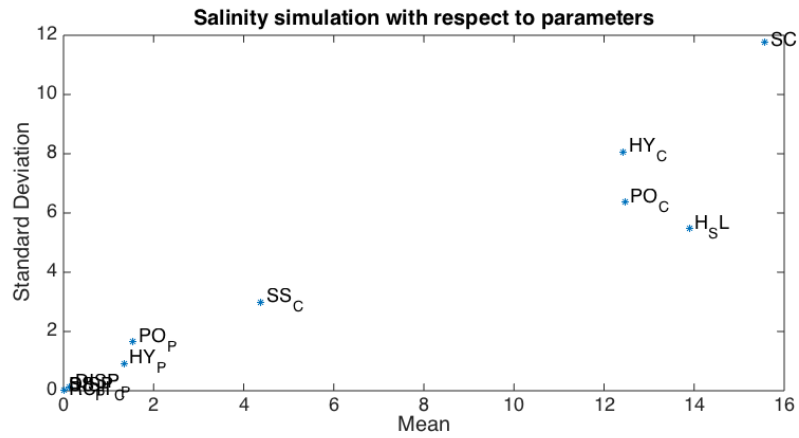


Figure 10. Mean and standard deviation of the EEs (elementary effects) of parameters with respect to simulations in the conduit (column #50, layer #29) in the global sensitivity analysis by Morris method: a) salinity simulation (top); b) head simulation (bottom).

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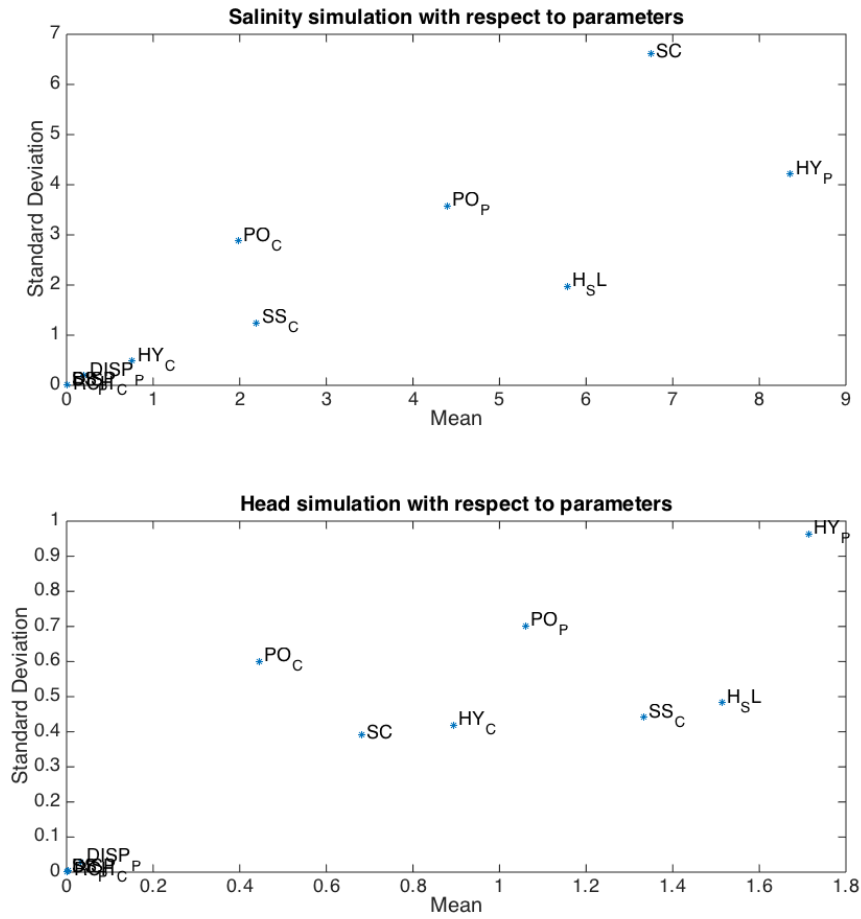


Figure 11. Mean and standard deviation of the EEs (elementary effects) of parameters with respect to simulations in the porous medium (column #35, layer #24) in the global sensitivity analysis by Morris method: a) salinity simulation (top); b) head simulation (bottom).

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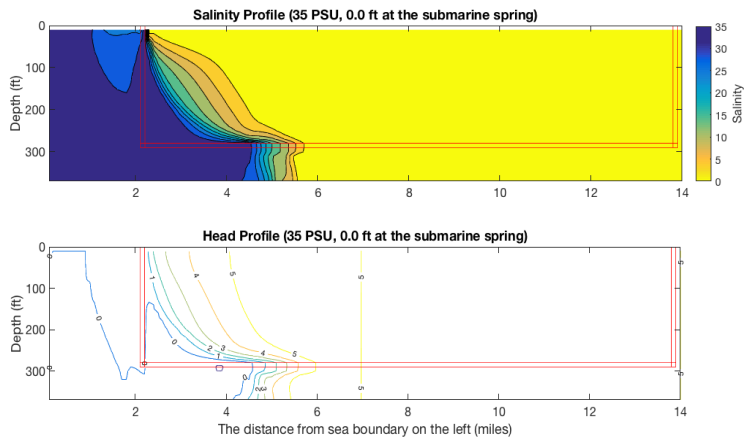


Figure 12. Salinity (top) and head (bottom) simulations of the maximum seawater intrusion benchmark case (35 PSU, 0.0 ft at the submarine spring).

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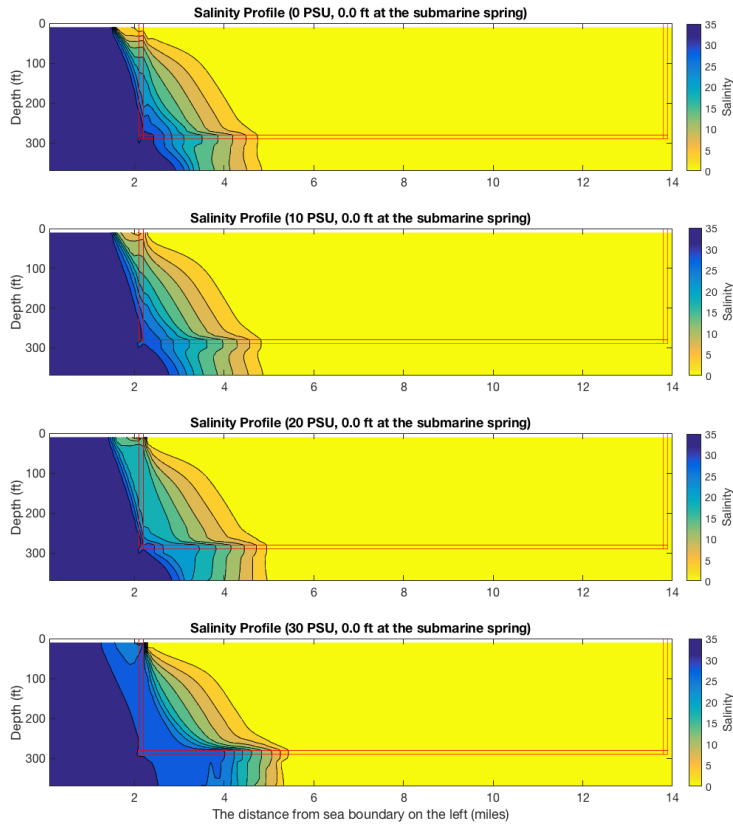


Figure 13. Salinity simulation of seawater intrusion with various salinity at the submarine spring, indicating different rainfall recharge and freshwater discharge conditions: A) 0.0 PSU, 0.0 ft at the submarine spring; B) 10.0 PSU, 0.0 ft at the submarine spring; C) 20.0 PSU, 0.0 ft at the submarine spring; D) 30.0 PSU, 0.0 ft at the submarine spring (from top to bottom).

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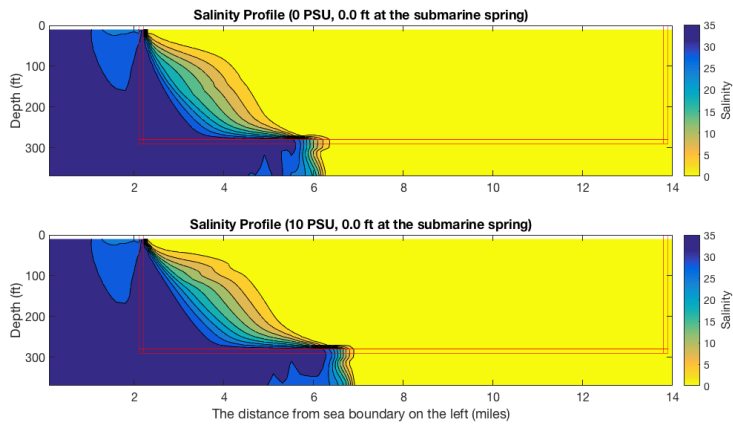


Figure 14. Salinity simulation of seawater intrusion with various sea level conditions: A) 35.0 PSU, 3.0 ft at the submarine spring; B) 35.0 PSU, 6.0 ft at the submarine spring (from top to bottom).

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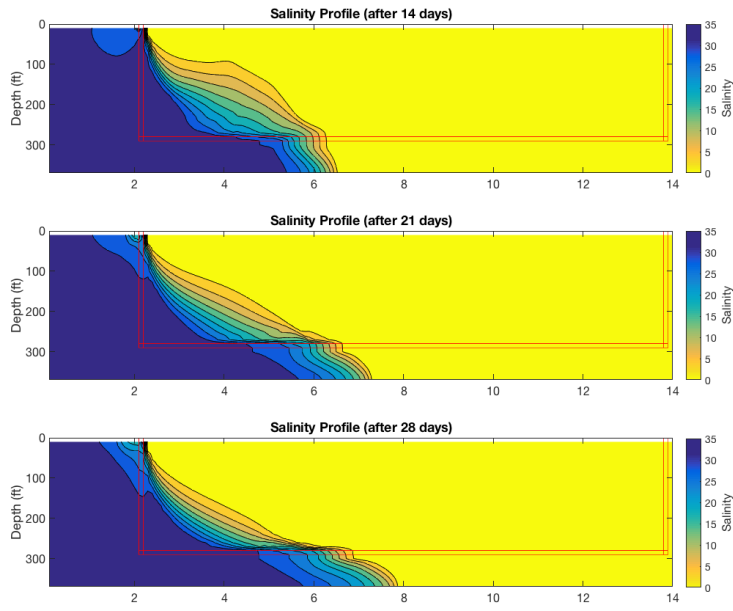


Figure 15. Salinity simulation of the maximum seawater intrusion benchmark case (35 PSU, 0.0 ft at the submarine spring) with extend simulation time during a low rainfall period: A) 14-day simulation period; B) 21-day simulation period; C) 28-day simulation period (from top to bottom).

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Different from the conclusion of homogeneous model in Shoemaker (2004), dispersivity is no longer an important parameter in such the dual-permeability aquifer model, conflicted with Shoemaker (2004) result that dispersivity is an important parameter in a homogeneous porous medium domain without the preferential advective flow. In a dual-permeability karst aquifer system, advection transport is dominated not only in the preferential conduit flow and but also in the surrounding porous medium as well, while dispersion becomes less unimportant. In this study, the uncertainty of dispersivity sensitivities can be significant when the Peclet number in the conduit is large beyond its criteria for solving transport equation by finite difference method, which requires the Peclet number to be smaller than 4 (Zheng and Bennett, 2002). An experiment of deactivating the DSP (dispersion) package in SEAWAT confirms that dispersion is negligible within the conduit network in this study, and the mixing is mostly due to the numerical dispersion instead of the solution of dispersion equation in this study.