

1 **Sensitivity and identifiability of hydraulic and geophysical parameters from**
2 **streaming potential signals in unsaturated porous media**

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12 ***Abstract***

13 Fluid flow in a charged porous medium generates electric potentials called Streaming
14 potential (SP). The SP signal is related to both hydraulic and electrical properties of the soil.
15 In this work, Global Sensitivity Analysis (GSA) and parameter estimation procedures are
16 performed to assess the influence of hydraulic and geophysical parameters on the SP signals
17 and to investigate the identifiability of these parameters from SP measurements. Both
18 procedures are applied to a synthetic column experiment involving a falling head infiltration
19 phase followed by a drainage phase.

20 GSA is used through variance-based sensitivity indices, calculated using sparse Polynomial
21 Chaos Expansion (PCE). To allow high PCE orders, we use an efficient sparse PCE algorithm
22 which selects the best sparse PCE from a given data set using the Kashyap Information
23 Criterion (KIC). Parameter identifiability is performed using two approaches: the Bayesian
24 approach based on the Markov Chain Monte Carlo (MCMC) method and the First-Order
25 Approximation (FOA) approach based on the Levenberg Marquardt algorithm. [The
26 comparison between the two approaches allows to check whether FOA can provide reliable
27 estimation of parameters and associated uncertainties for the investigated highly nonlinear
28 hydrogeophysical problem.](#)

29 GSA results show that at short times, the saturated hydraulic conductivity (K_s) and the
30 voltage coupling coefficient at saturation (C_{sat}) are the most influential parameters, whereas,
31 at long times, the residual water content (θ_r), the Mualem-van Genuchten parameter (n) and
32 the Archies's saturation exponent (n_a) become influential with strong interactions between
33 them. The Mualem-van Genuchten parameter (α) has a very weak influence on the SP
34 signals during the whole experiment.

35 Results of parameter estimation show that, although the studied problem is highly nonlinear,
36 when several SP data collected at different altitudes inside the column are used to calibrate the
37 model, all hydraulic (K_s, θ_r, α and n) and geophysical (n_a and C_{sat}) parameters can be
38 reasonably estimated from the SP measurements. Further, in this case, the FOA approach
39 provides accurate estimations of both mean parameter values and uncertainty regions.
40 Conversely, when the number of SP measurements used for the calibration is strongly
41 reduced, the FOA approach yields accurate mean parameter values (in agreement with
42 MCMC results) but inaccurate and even unphysical confidence intervals for parameters with
43 large uncertainty regions.

44

45 **Keywords**

46 Drainage experiment, Streaming Potential, Global Sensitivity Analysis, Markov chain Monte
47 Carlo, parameter estimation.

48 **1. Introduction**

49 Flow through a charged porous medium can generate an electric potential (Zablocki, 1978;
50 Ishido and Mizutani, 1981; Allegre et al., 2010; Jougnot and Linde, 2013), called Streaming
51 Potential (SP). The SP signals play an important role in several applications related to
52 hydrogeology and geothermal reservoir engineering as they are useful for examining
53 subsurface flow dynamics. During the last decade, surface SP anomalies have been widely
54 used to estimate aquifers hydraulic properties (Darnet et al., 2003). Interest on SP is
55 motivated by its low-cost and high sensitivity to water flow. Either coupled or uncoupled
56 approaches can be used for hydraulic parameter estimation from SP signals (Mboh et al.,
57 2012). In the uncoupled approach, Darcy velocities (e.g., Jardani et al., 2007; Bolève et al.,
58 2009) are obtained from tomographic inversion of SP signals and then used for the calibration
59 of the hydrologic model. In the coupled approach, anomalies related to the tomographic
60 inversion are avoided by inverting the full coupled hydrogeophysical model (Hinnell et al.,
61 2010).

62 The SP signals have been widely studied in saturated porous media (Bogoslovsky and Ogilvy,
63 1973; Patella, 1997; Sailhac and Marquis, 2001; Richards et al., 2010; Bolève et al., 2009,
64 among others). Fewer studies focused on the application of the SP signal in unsaturated flow
65 despite the big interest for such nonlinear problems (Linde et al., 2007; Allegre et al., 2010;
66 Mboh et al., 2012; Jougnot and Linde, 2013). Hence, in this work we are interested in the SP
67 signals in unsaturated porous media. Our main objective is to investigate the usefulness of the
68 SP signals for the characterization of soil parameters. To this aim, we evaluate the impact of
69 uncertain hydraulic and geophysical parameters on the SP signals and assess the identifiability
70 of these parameters from the SP measurements.

71 The impact of soil parameters on SP signals is investigated using Global Sensitivity Analysis
72 (GSA). This is a useful tool for characterizing the influential parameters that contribute the

73 most to the variability of model outputs (Saltelli et al.,1999; Sudret, 2008) and for
74 understanding the behavior of the modeled system. GSA has been applied in several areas, as
75 for risk assessment for groundwater pollution (e.g., Volkova et al., 2008), non-reactive
76 (Fajraoui et al., 2011) and reactive transport experiments (Fajraoui et al., 2012; Younes et al.,
77 2016), for unsaturated flow experiments (Younes et al., 2013), natural convection in porous
78 media (Fajraoui et al., 2017) and seawater intrusion (Rajabi et al., 2015; Riva et al., 2015). To
79 the best of our knowledge, GSA has never been used for SP signals in unsaturated porous
80 media. Hence, in the first part of this study, GSA is performed on a conceptual model inspired
81 from the laboratory experiment of Mboh et al. (2012) where SP signals are measured at
82 different altitudes in a sandy soil column during a falling-head infiltration phase followed by a
83 drainage phase. Four uncertain hydraulic parameters (saturated hydraulic conductivity K_s ,
84 residual water content θ_r and fitting Mualem-van Genuchten parameters α and n) and two
85 geophysical parameters (Archies's saturation exponent n_a and voltage coupling coefficient at
86 saturation C_{sat}) are investigated. GSA of SP signals is performed by computing the variance-
87 based sensitivity indices using Polynomial Chaos Expansion (PCE). To reduce the number of
88 PCE coefficients while maintaining high PCE orders, we use the efficient sparse PCE
89 algorithm developed by Shao et al. (2017) which selects the best sparse PCE from a given
90 data set using the Kashyap Information Criterion (KIC).

91 In the second part of this study, we investigate the identifiability of hydro-geophysical
92 parameters from SP measurements. To this aim, parameter estimation is performed using two
93 different approaches. The first is a Bayesian approach in which model parameters are treated
94 as random variables and characterized by their probability density functions. With this
95 approach, the prior knowledge about the model and the observed data are merged to define
96 the joint posterior probability distribution function of the parameters. In the sequel, Bayesian

97 analysis is conducted using the DREAM_(ZS) software (Laloy and Vrugt, 2012; Vrugt, 2016)
98 based on the Markov Chain Monte Carlo (MCMC) method. MCMC has been successfully
99 used in various inverse problems (e.g., Vrugt et al., 2003, 2008; Arora et al., 2012; Younes et
100 al., 2017). The MCMC method yields an ensemble of possible parameter sets that
101 satisfactorily fit the available data. These sets are then employed to estimate the posterior
102 parameter distributions and hence the optimal parameter values and the associated 95%
103 Confidence Intervals (CIs) in order to quantify parameter's uncertainty. The second inversion
104 approach is the commonly used First-Order Approximation (FOA) approach based on the
105 standard Levenberg-Marquardt algorithm. Two scenarios are considered to check whether
106 FOA can provide reliable estimation of parameters and associated uncertainties for the
107 investigated highly nonlinear hydrogeophysical problem in the case of abundant data (small
108 uncertainty regions) and in the case of scarcity of data (large uncertainty regions). In the first
109 scenario, SP data collected from sensors at five different locations are taken into account for
110 the calibration. In the second scenario; only the SP data from one sensor are used for model
111 calibration.

112 The present study is decomposed as follows. Section 2 presents the hydrogeophysical model
113 and the reference solution. Section 3 reports on the GSA results of SP signals. Then, Section 4
114 discusses results of parameter estimation with both MCMC and FOA approaches for the two
115 investigated scenarios.

116 **2. Test case description and numerical solution**

117 **2.1. Test case description**

118 The test case considered in this work is similar to the laboratory experiment developed in
119 Mboh et al. (2012) involving a falling-head infiltration phase followed by a drainage phase
120 (Figure 1). This experiment is representative of several laboratory SP experiments (Linde et

121 al., 2007; Allegre et al., 2010; Jougnot and Linde, 2013, among others). Quartz sand is evenly
 122 packed in a plastic tube with an internal diameter of 5 cm to a height of $L_s=117.5$ cm. The
 123 column is initially saturated with a ponding of $L_w=48$ cm above the soil surface. Five sensors
 124 allowing SP measurements are installed at respectively 5, 29, 53, 77, and 101 cm from the
 125 surface. The column has a zero pressure head maintained at its bottom. At the top of the
 126 column, the boundary condition corresponds to a Dirichlet condition with a prescribed
 127 pressure head condition during the falling-head phase followed by a Neumann condition with
 128 zero infiltration flux during the drainage phase. During the falling-head phase, the prescribed
 129 pressure head h_{top} has an exponential behavior driven by the saturated conductivity

130 $h_{top} = (L_s + L_w) e^{-\frac{K_s t}{L_s}} - L_s$. The falling-head phase remains until the ponding vanishes at the

131 critical time $t_c = -\frac{L_s}{K_s} \ln\left(\frac{L_s}{L_s + L_w}\right)$.

132 2.2. Mathematical model

133 The total electrical current density \mathbf{j} [$A\ m^{-2}$] is determined from the generalized Ohm's law
 134 as follows:

$$135 \quad \mathbf{j} = -\sigma \nabla \varphi + \mathbf{j}_s \quad (1)$$

136 where φ [V] is the streaming potential, \mathbf{j}_s [$A\ m^{-2}$] is the streaming current density and σ [S
 137 m^{-1}] is the electrical conductivity distribution assumed isotropic.

138 Hence, the conservation equation ($\nabla \cdot \mathbf{j} = 0$) writes

$$139 \quad \nabla \cdot (\sigma \nabla \varphi) = \nabla \cdot \mathbf{j}_s \quad (2)$$

140 Besides, the electrical conductivity distribution can be estimated using the saturation

141 $S_w = \theta/\theta_s$ as follows (Mboh et al., 2012)

$$142 \quad \sigma = \sigma_{sat} S_w^{n_a} \quad (3)$$

143 where σ_{sat} is the electric conductivity at saturation [S m⁻¹] and n_a is the Archie's saturation
 144 exponent (Archie, 1942).

145 The streaming current density j_s can be related to the Darcy velocity \mathbf{q} [cm min⁻¹] by (Linde
 146 et al., 2007 ; Revil et al., 2007)

$$147 \quad \mathbf{j}_s = \left(-\sigma_{sat} \frac{\rho g}{K_s} C_{sat} S_w \right) \mathbf{q} \quad (4)$$

148 where K_s is the saturated hydraulic conductivity [cm min⁻¹], ρ is the water density [kg m⁻³],
 149 g is the gravitational acceleration [m s⁻²] and C_{sat} is the voltage coupling coefficient at
 150 saturation [V Pa⁻¹].

151 Hence, the combination of the previous equations (1-4) leads to the following partial
 152 differential equation governing the SP signals:

$$153 \quad \nabla \cdot (S_w^{n_a} \nabla \varphi) = \nabla \cdot \left(-\frac{\rho g C_{sat} S_w}{K_s} \mathbf{q} \right) \quad (5)$$

154 On the other hand, the flow through an unsaturated soil column can be modelled by the one-
 155 dimensional Richard's equation:

$$156 \quad \frac{\partial \theta}{\partial t} = \left(c(h) + S_s \frac{\theta}{\theta_s} \right) \frac{\partial H}{\partial t} = -\nabla \cdot \mathbf{q} \quad (6)$$

157 where H [cm] and h [cm] are respectively the hydraulic and pressure head such as $H = h - z$;
 158 z [cm] is the depth (downward positive); S_s (-) is the specific storage; θ_s [cm³.cm⁻³] and θ
 159 are the saturated and actual water contents, respectively; $c(h)$ [cm⁻¹] is the specific moisture
 160 capacity; and $K(h)$ [cm min⁻¹] is the hydraulic conductivity.

161 The water velocity \mathbf{q} is given from the Darcy's law:

$$162 \quad \mathbf{q} = -K(h) \nabla H \quad (6)$$

163 In the current study, the standard models of Mualem (1976) and Van Genuchten (1980) are
 164 used to relate pressure head, hydraulic conductivity and water content,

$$165 \quad S_e(h) = \frac{\theta(h) - \theta_r}{\theta_s - \theta_r} = \begin{cases} \frac{1}{(1 + |\alpha h|^n)^m} & h < 0 \\ 1 & h \geq 0 \end{cases} \quad (7)$$

$$K(S_e) = K_s S_e^{1/2} \left[1 - (1 - S_e^{1/m})^m \right]^2$$

166 where S_e (-) is the effective saturation, θ_r [$\text{cm}^3 \text{ cm}^{-3}$] is the residual water content, K_s
 167 [$\text{cm} \cdot \text{min}^{-1}$] is the saturated hydraulic conductivity, $m = 1 - 1/n$, α [cm^{-1}] and n [-] are the
 168 Mualem van-Genuchten shape parameters.

169 **2.3. Numerical model**

170 Although, several numerical techniques have been developed to for the solution of the
 171 multidimensional Richards equation (e.g., Fahs et al., 2009; Belfort et al., 2009, Younes et al.,
 172 2013; Deng and Wang, 2017), the standard finite volume method is used here for the spatial
 173 discretization of the one dimensional Richard's equation (6). The integration of this equation
 174 over the finite volume i between $i-1/2$ and $i+1/2$ gives

$$175 \quad \int_{i-1/2}^{i+1/2} \left(c(h) + S_s \frac{\theta}{\theta_s} \right) \frac{\partial H}{\partial t} dz = q_{i-1/2} - q_{i+1/2} \quad (8)$$

176 using expressions of the Darcy velocity at the element interfaces $q_{i-1/2} = -\frac{K_{i-1/2}}{\Delta z} (H_i - H_{i-1})$

177 and $q_{i+1/2} = -\frac{K_{i+1/2}}{\Delta z} (H_{i+1} - H_i)$ in the case of a uniform spatial discretization with a spatial

178 step Δz , we obtain,

$$179 \quad \Delta z \left(c_i + S_s \frac{\theta_i}{\theta_s} \right) \frac{\partial H_i}{\partial t} = K_{i+1/2} (H_{i+1} - H_i) - K_{i-1/2} (H_i - H_{i-1}) \quad (9)$$

180 Using $\tau = S_w^{n_a}$ and $\delta = \frac{\rho g C_{sat} S_w}{K_s}$, the integration of the equation (5) over the finite volume i

181 yields

$$182 \quad \frac{\tau_{i+1/2}}{\Delta z} (\varphi_{i+1} - \varphi_i) - \frac{\tau_{i-1/2}}{\Delta z} (\varphi_i - \varphi_{i-1}) - \delta_{i+1/2} K_{i+1/2} (H_{i+1} - H_i) + \delta_{i-1/2} K_{i-1/2} (H_i - H_{i-1}) = 0 \quad (10)$$

183 where the values at the interface $\tau_{i\pm 1/2}$, $\delta_{i\pm 1/2}$ and $K_{i\pm 1/2}$ are calculated using the arithmetic

184 mean between adjacent elements (for instance, $\tau_{i+1/2} = \frac{\tau_i + \tau_{i+1}}{2}$).

185 Then, the temporal discretization of the obtained nonlinear ODE/DAE system (9-10) is

186 performed with the method of lines (MOL) using the DASPCK (Brown et al., 1994) time

187 solver. The MOL is suitable for strongly nonlinear systems since it allows high order

188 temporal integration methods with formal error estimation and control (Miller et al., 1998;

189 Younes et al., 2009; Fahs et al., 2009, 2011). In the current study, the relative and absolute

190 local error tolerances are fixed to 10^{-6} .

191 Numerical simulations are performed assuming typical MVG hydraulic parameters for the

192 sandy soil with (according to Carsel and Parrish, 1988) $K_s = 0.495$ cm/min, $\theta_s = 0.43$

193 cm³/cm³, $\theta_r = 0.045$ cm³/cm³, $\alpha = 0.145$ cm⁻¹ and $n = 2.68$. The voltage coupling coefficient

194 at saturation is $C_{sat} = -2.910^{-7}$ V/Pa and the Archie's saturation exponent is $n_a = 1.6$.

195 Based on these hydraulic and geophysical parameters, a reference (mesh independent)

196 solution is obtained using a uniform mesh of 235 cells of 0.5 cm length. Data are generated by

197 sampling the output SP signals every 10 min during 1800 min. Figure 2 shows that the SP

198 signals have an almost linear behavior in the saturated falling-head phase. During the drainage

199 phase, they have a nonlinear behavior and approach the zero voltage for the dry conditions

200 occurring toward the end of the experiment. The SP signals are noised with independent

201 Gaussian random noises with a standard deviation of $2.73 \cdot 10^{-5}$ V. This noise level was

202 obtained by Mboh et al. (2012) from laboratory measurements. The noised data (Figure 2) are
203 used as “observations” in the calibration exercise.

204 **3. Global sensitivity analysis of SP signals**

205 **3.1. GSA method**

206 The aim of GSA is to assess the effect of the variation of parameters on the model output
207 (Mara and Tarantola, 2008). Such knowledge is important for determining the most influential
208 parameters as well as their regions and periods of influence (Fajraoui et al., 2011). The
209 sensitivity of a model to its parameters can be assessed using Variance-based sensitivity
210 indices. These indices evaluate the contribution of each parameter to the variance of the
211 model (Sobol’, 2001). The polynomial chaos theory (Wiener, 1938), has been largely used to
212 perform variance-based sensitivity analysis of computer models (see for instance, Sudret,
213 2008; Blatman and Sudret, 2010; Fajraoui et al., 2012; Younes et al., 2016; Shao et al., 2017;
214 Mara et al., 2017). It can be stated that the PCE method is a surrogate-based approach.
215 However, we argue that this method employs ANOVA (Analysis Of Variance) decomposition
216 and hence can be considered as a spectral method (such as the Fourier amplitude sensitivity
217 test, Cukier et al., 1973; Saltelli et al., 1999). Indeed, with this method, the sensitivity indices
218 are directly obtained from the PCE coefficients without needing to run the surrogate model.

219 Let us consider a mathematical model with a random response $f(\xi)$ which depends on d
220 independent random parameters $\xi = \{\xi_1, \xi_2, \dots, \xi_d\}$. With PCE, $f(\xi)$ is expanded using a set
221 of orthonormal multivariate polynomials (up to a polynomial degree p):

$$222 \quad f(\xi) \approx \sum_{|\alpha| \leq p} s_\alpha \Psi_\alpha(\xi) \quad (8)$$

223 where $\alpha = \alpha_1 \dots \alpha_d \in \square^d$ is a d^{th} -dimensional index. The s_α 's are the polynomial coefficients
 224 and Ψ_α 's are the generalized polynomial chaos of degree $|\alpha| = \sum_{i=1}^d \alpha_i$, such as Hermite,
 225 Legendre and Jacobi polynomials, for instance. In this work, Legendre polynomials are
 226 employed because uniform distributions are considered for the parameters. The non-
 227 informative uniform distributions are used here to express the absence of prior information
 228 which makes all possible values of the parameter equally likely.

229 Equation (8) is similar to an ANOVA representation of the original model (Sobol' 1993),
 230 from which it is straightforward to express $V[f(\xi)]$, the variance of $f(\xi)$ as the sum of the
 231 partial contribution of the inputs,

$$232 \quad V[f(\xi)] = \sum_{\alpha} s_{\alpha}^2, \quad (9)$$

233 The first-order sensitivity index S_i and the total sensitivity index ST_i are defined by

$$234 \quad S_i = \frac{V[E[f(\xi)|\xi_i]]}{V[f(\xi)]} \in [0,1], \quad (10)$$

$$235 \quad ST_i = \frac{E[V[f(\xi)|\xi_{\setminus i}]]}{V[f(\xi)]} \in [0,1], \quad (11)$$

236 where $\xi_{\setminus i} = \xi \setminus \xi_i$, $E[\cdot | \cdot]$ is the conditional expectation operator and $V[\cdot | \cdot]$ the conditional
 237 variance. S_i measures the amount of variance of $f(\xi)$ due to ξ_i alone, while $ST_i \geq S_i$
 238 measures the amount of all contributions of ξ_i to the variance of $f(\xi)$, including its
 239 cooperative non-linear contributions with the other parameters ξ_j . The input/output
 240 relationship is said *additive* when $ST_i = S_i, \forall i = 1, \dots, d$, and in this case $\sum_{i=1}^d S_i = 1$.

241 In the sequel, a PCE is constructed for each SP signal at each observable time. The number of
 242 coefficients for a full PCE representation is $P = (d+p)!/(d!p!)$. The evaluation of the PCE

243 coefficients requires at least P simulations of the nonlinear hydrogeophysical model. Note
244 that P increases quickly with the order of the PCE and the number of parameters. Hence,
245 several sparse PCE representations, where only the significant coefficients are sought, have
246 been proposed in the literature in order to reduce the computational cost of the estimation of
247 the Sobol indices. For instance, Blatman and Sudret (2010) developed a sparse PCE
248 representation using an iterative forward-backward approach based on non-intrusive
249 regression. Fajraoui et al., (2012) developed a technique where only the sensitive coefficients
250 (that affect significantly model variance) are retained in the PCE. Recently, Shao et al.,
251 (2017), developed an algorithm based on Bayesian Model Averaging (BMA) to select the best
252 sparse PCE from a given data set using the Kashyap Information Criterion (KIC) (Kashyap,
253 1982). The main idea of this algorithm is to increase progressively the degree of an initial
254 PCE and compute the KIC until obtaining a satisfactory representation of model responses.
255 This algorithm is used hereafter to compute the sensitivity indices of the SP signals.

256

257 **3.2. GSA results**

258 The SP responses are considered for uniformly distributed parameters over the large intervals
259 shown in Table 1. These intervals include the reference values reported in Mboh et al. (2012).

260 The sensitivity indices of the six input parameters ($K_s, \theta_r, \alpha, n, n_a, C_{sat}$) are estimated using an
261 experimental design formed by $N = 2^{12} = 4096$ parameter sets. The order of the sparse PCE is
262 automatically adapted for each observable time and location. For some observable times, the
263 PCE is highly sparse; it reaches a degree of 31 but contains only 112 nonzero coefficients.

264 Figure 3 depicts the temporal distribution of the streaming potential variance, represented by
265 the blue curve, and the relative contribution of the parameters, represented by the shaded area.

266 This figure corresponds to the temporal ANOVA decomposition for the sensor 1 (at 5 cm

267 from the soil surface) and for the sensor 4 (at 77 cm from the soil surface). Interactions
 268 between parameters are represented by the blank region between the variance curves and the
 269 shaded area. Note that because Dirichlet boundary condition with zero SP is maintained at the
 270 outlet boundary, the variance of the SP signal is zero at the bottom and reaches its maximum
 271 value near the soil surface. Hence, the variance is higher for the first sensor, located at 5 cm
 272 from the soil surface (Figure 3a) than for the sensor 4 located at 77 cm (Figure 3b).

273 The SP signals at different altitudes exhibit similar behavior (Figure 3). In the following, we
 274 comment on the results of sensor 1 (Figure 3a). Because K_s varies between 0.1 cm min^{-1} and
 275 2 cm min^{-1} , the saturated falling-head phase remains until the ponding vanishes at

276
$$t_c = -\frac{L_s}{K_s} \ln\left(\frac{L_s}{L_s + L_w}\right).$$
 Depending on the value of K_s (see Table 1), t_c varies between $t_1 = 20$

277 min and $t_2 = 403$ min. Thus, in Figure 3a, we can see that during a first time period ($t \leq t_1$),
 278 the SP signal is strongly influenced by the value of the parameter C_{sat} . The first order and
 279 total sensitivity indices at $t = 10$ min (Table 2a) confirm that only the saturated parameters K_s
 280 and C_{sat} are influential. C_{sat} is about 17 times more influential than K_s . As expected, the
 281 remaining parameters have no influence during the first period. The total variance is 0.72 mv
 282 and there is no interaction between the two parameters K_s and C_{sat} since $ST_i = S_i$ for both
 283 and $\sum_{i=1}^d S_i = 1$.

284 During the second period ($t_1 \leq t \leq t_2$), the flow is either saturated or unsaturated depending on
 285 the value of K_s . Figure 3a shows that the variance of the SP signal exhibits its maximum
 286 value around 2.4 mv with strong influences of the parameters K_s and C_{sat} and weak
 287 interactions between them (small blank region between the variance curve and the shaded
 288 area). These results are confirmed by the sensitivity indices calculated at $t = 70$ min and

289 reported in Table 2a for the sensor 1. Both first order and total sensitivity indices indicate that
 290 K_s is the most influential parameter. The second influential parameter is C_{sat} which has a
 291 total sensitivity index about 12 times less than K_s . The parameter α is irrelevant since its
 292 total sensitivity index is 109 times less than K_s and its partial variance is
 293 $V_i = S_i \times V_T = 0.01mv$ which is less than the 95% confidence interval associated to the SP
 294 measurement ($\pm 0.055mv$). The total variance at $t = 70$ min is calculated to be $2.17mv$ and
 295 the output/input relationship is close to be additive since $\sum_{i=1}^d S_i = 0.94$ which means that
 296 interactions between parameters exist but are not significant.

297 During the third period ($t \geq t_2$), the variance of the SP signal reduces to 0.3 mv (Figure 3a)
 298 and significant interactions are observed between parameters (large blank region between the
 299 shaded area and the variance curve). Table 2a shows that for $t = 800$ min, which corresponds
 300 to dry conditions, the total variance is 0.22. First-order sensitivity indices are very small,
 301 except for θ_r . The latter is highly influential since it has a significant first-order sensitivity
 302 index ($S_i = 0.27$) and a more significant total- sensitivity index ($ST_i = 0.74$). The parameters
 303 C_{sat} and K_s are irrelevant, they have very small first-order and total sensitivity indices.
 304 Further, strong interactions are observed between the parameters since the sum of the first-
 305 order indices is far from 1 ($\sum_{i=1}^d S_i = 0.47$). The total sensitivity indices are significantly
 306 different from first-order sensitivity indices for almost all parameters. For instance, the ratio
 307 between these two indices is around 4 for α , 5 for n_a and 7 for n . The total sensitivity index
 308 of α remains small (0.065), whereas, significant total sensitivity indices are obtained for n (
 309 $ST_i = 0.27$) and n_a ($ST_i = 0.47$) which indicates that these two parameters are influential

310 (although their first order sensitivity indices are small) because of interaction between
311 parameters.

312 Figure 3b shows similar behavior for the sensor 4 located at 77 cm from the soil surface. The
313 results in Table 2b indicate that the total variance observed at $t = 10, 70$ and 800 min are
314 around 8 times less than for the sensor 1. For the first time period, the first and total
315 sensitivity indices are identical to those observed for the sensor 1 since saturated conditions
316 occur inside the whole column and the same effect of K_s and C_{sat} can be observed whatever
317 the location inside the column. For the second time period, the sensitivity indices for sensor 4
318 (Table 2b) are similar to those observed for the sensor 1. However, the results for the third
319 time period show an improvement of the relevance of the parameter α with an increase of
320 both first and total sensitivity indices. Indeed, compared to the results of the sensor1, both
321 first order and total sensitivity indices have tripled. Moreover, the total sensitivity index for α
322 ($ST_i = 0.22$) becomes close to that of n ($ST_i = 0.24$).

323 In summary, the GSA applied to SP signals identifies the influential parameters and their
324 periods of influence and shows that

- 325 - the parameter C_{sat} is highly influential during the first time period ($t \leq t_1$) where no
326 interactions are observed between parameters;
- 327 - the parameter K_s is highly influential during the second time period ($t_1 \leq t \leq t_2$) where
328 small interactions occur between parameters;
- 329 - the parameters θ_r , n and n_a are influential during the third time period ($t \geq t_2$) where
330 dry conditions occur. During this period, strong interactions take place between
331 parameters;

- 332 - the parameter α has no influence on the SP signals during the two first periods and
333 presents a very small influence ($S_i = 0.015$ and $ST_i = 0.065$) during the third period
334 on the sensor 1 (near the surface of the column);
- 335 - the relevance of the parameter α improves with the distance from the soil surface,
336 although the total variance diminishes with respect to this distance. The influence of
337 α becomes significant ($ST_i = 0.22$) on the sensor 4 (located at 77 cm from the soil
338 surface) during the third period.

339 **4. Parameter estimation**

340 **4.1. MCMC and FOA approaches**

341 Calibration of computer models is an essential task since some parameters (like the Mualem
342 van-Genuchten shape parameters α and n) cannot be directly measured. In such an exercise,
343 the unknown model parameters are investigated by facing the model responses to the
344 observations. Recently, Mboh et al. (2012) showed that inversion of SP signals can yield
345 accurate estimate of the saturated hydraulic conductivity K_s , the MVG fitting parameters α
346 and n and the Archie's saturation exponent (n_a). Moreover, they showed that the quality of
347 the estimation was comparable to that obtained from the calibration of pressure heads. In their
348 study, Mboh et al. (2012) used the FOA approach with the Shuffled Complex Evolution
349 optimization algorithm SCE-UA (Duan et al., 1993).

350 As important as the determination of the optimal parameter sets are the associated 95%
351 Confidence Intervals (CIs) to quantify uncertainty on the estimated values. The determination
352 of CIs is not straightforward if the observed model responses are highly nonlinear functions of
353 model parameters (Christensen and Cooley, 1999). In the sequel, parameter estimation is
354 performed using two approaches: the popular FOA approach and the Bayesian approach
355 based on the Markov chain Monte Carlo (MCMC) sampler. **Contrarily to FOA, the MCMC**

356 method is robust since no assumptions of model linearity or differentiability are required.
 357 Furthermore, prior information available for the parameters can be included. MCMC provides
 358 not only an optimal point estimate of the parameters but also a quantification of the entire
 359 parameter space. Several MCMC strategies have been developed for Bayesian sampling of
 360 the parameter space (Gallagher and Doherty, 2007; Vrugt, 2016). In groundwater and vadose
 361 zone modeling context, the most widely used of these strategies is the Metropolis Hastings
 362 algorithm (Metropolis et al., 1953; Hastings, 1970). It proceeds as follows (Gelman et al.,
 363 1996):

364 i. Choose an initial candidate $\mathbf{x}^0 = (\boldsymbol{\xi}^0, \sigma^0)$ formed by the initial estimate of the
 365 parameter set $\boldsymbol{\xi}^0$ and the hyperparameter σ^0 and a proposal distribution q that
 366 depends on the previous accepted candidate.

367 ii. A new candidate $\mathbf{x}^i = (\boldsymbol{\xi}^i, \sigma^i)$ is generated from the current one \mathbf{x}^{i-1} with the
 368 generator $q(\mathbf{x}^i | \mathbf{x}^{i-1})$ associated with the transition probability $p(\boldsymbol{\xi}^i | \mathbf{y}_{mes}, \sigma)$.

369 iii. Calculate $p(\boldsymbol{\xi}^i | \mathbf{y}_{mes}, \sigma)$ and compute the ratio $\alpha = \frac{p(\boldsymbol{\xi}^i | \mathbf{y}_{mes}, \sigma)q(\mathbf{x}^i | \mathbf{x}^{i-1})}{p(\boldsymbol{\xi}^{i-1} | \mathbf{y}_{mes}, \sigma)q(\mathbf{x}^{i-1} | \mathbf{x}^i)}$.

370 Additionally, draw a random number $u \in [0, 1]$ from a uniform distribution.

371 iv. If $\alpha \geq u$, then accept the new candidate, otherwise it is rejected.

372 v. Resume from (ii) until the chain $\{\mathbf{x}^0, \dots, \mathbf{x}^k\}$ converges or a prescribed number of
 373 iterations i_{\max} is reached.

374 Many improvements have been proposed in the literature to accelerate the MCMC
 375 convergence rate (e.g., Haario et al., 2006; ter Braak and Vrugt, 2008; Dostert et al., 2009,
 376 among others). Vrugt et al. (2009a, 2009b) developed the DREAM MCMC sampler based on
 377 the differential evolution–Markov Chain method of ter Braak (2006) to improve sampling

378 efficiency. DREAM runs multiple Markov chains in parallel and uses subspace sampling and
379 outlier chain correction to speed up MCMC convergence (Vrugt, 2016). Laloy and Vrugt
380 (2012) developed the DREAM_(ZS) MCMC sampler in which a candidate for each chain is
381 drawn from an archive of past states denoted Z which plays the role of the generator q . The
382 interested readers are referred to Vrugt (2016) for more details about properties and
383 implementation of DREAM and DREAM_(ZS). In the current study, the DREAM_(ZS) software is
384 used for the MCMC estimation of the hydrogeophysical parameters. Note that because of the
385 large number of model evaluations required, the MCMC method remains rarely used in
386 practical applications compared to the FOA approach. Indeed, with FOA, the CIs are
387 estimated once by assuming that the Jacobian remains constant within the CIs. This
388 assumption was found to be reasonably accurate in nonlinear problems by Donaldson and
389 Scnabel (1987). However, recently, several authors stated that parameter interdependences
390 and model nonlinearities violate this assumption (see for instance, Vrugt and Bouten, 2002;
391 Vurgin et al. 2007; Gallagher and Doherty, 2007; Mertens et al., 2009; Kahl et al., 2015).
392 In the following, both MCMC and FOA approaches are employed for the inversion of the
393 highly nonlinear hydrogeophysical problem using SP measurements.

394

395 **4.2. Parameter estimation results**

396 Hydrogeophysical parameters are estimated using the DREAM_(ZS) MCMC sampler (Laloy
397 and Vrugt, 2012). Independent uniform distributions are considered for model parameter
398 priors and likelihood hyperparameters (see Table 1). The parameter posterior distribution
399 writes:

$$400 \quad p(\xi / \mathbf{y}_{mes}, \sigma) \propto \sigma^{-N} \exp\left(-\frac{SS(\xi)}{2\sigma^2}\right) \quad (9)$$

401 where $SS(\xi) = \sum_{k=1}^N \left(y_{mes}^{(k)} - y_{mod}^{(k)}(\xi) \right)^2$ is the sum of the squared differences between the
402 observed $y_{mes}^{(k)}$ and modeled $y_{mod}^{(k)}$ SP signals at time t_k for N total number of SP
403 observations.

404 The DREAM_(ZS) software computes multiple sub-chains in parallel to thoroughly explore the
405 parameter space. Taking the last 25% of individuals (when the chains have converged) yields
406 multiple sets used to estimate the updated parameter distributions and therefore the optimal
407 parameter values and their CIs. In the sequel, the DREAM_(ZS) MCMC sampler is used with 3
408 parallel chains.

409 We assume that the saturated water content has been initially measured with a fair degree of
410 accuracy. However, instead of fixing its value (as in Kool et al. (1987), van Dam et al. (1994),
411 Nützmann et al., (1998) among others), we assign to θ_s a Gaussian distribution to take into
412 account associated uncertainty and its effect on the estimation of the rest of parameters. It is
413 assumed here that the saturated water content was accurately measured to be $\theta_s = 0.43$
414 $\text{cm}^3.\text{cm}^{-3}$ by weighing the saturated soil. The corresponding error measurements are
415 independently and normally distributed with a zero mean and a standard deviation $\sigma_\theta = 0.01$
416 $\text{cm}^3.\text{cm}^{-3}$. Hence a Gaussian distribution is assigned to θ_s with a mean value of $0.43 \text{ cm}^3.\text{cm}^{-3}$
417 3 and a 95% CI $[0.41-0.45] \text{ cm}^3.\text{cm}^{-3}$. The rest of hydrogeophysical parameters have non
418 informative uniform distributions over the ranges reported in Table 1. The error
419 (measurement) variance is also considered unknown and is simultaneously estimated with the
420 physical parameters. Two scenarios are considered to check whether the FOA approach can
421 provide reliable estimation of parameters and associated uncertainties for the investigated
422 highly nonlinear hydrogeophysical problem both in the case of abundant data (small
423 uncertainty regions) and in the case of scarcity of data (large uncertainty regions). In the first
424 scenario, SP data collected from the sensors located at the five locations are taken into

425 account for the calibration. In the second scenario; only the SP data from the first sensor
426 located at 5 cm from the soil surface serve as conditioning information for model calibration.
427 Results of the MCMC sampler are compared to those of FOA approach for both scenarios.

428 *Scenario 1: Inversion using all SP measurements*

429 The Figure 4 shows the results obtained with MCMC when the SP data of the five sensors are
430 used for the calibration. The "on-diagonal" plots in this figure display the posterior parameter
431 distributions, whereas the "off-diagonal" plots represent the correlations between parameters
432 in the MCMC sample. Figure 4 shows *nearly* bell-shaped posterior distributions for all
433 parameters. A strong correlation is observed between θ_r and n_a ($r = 0.98$).

434 From the obtained MCMC sample, it is straightforward to estimate the posterior 95%
435 confidence interval of each parameter. The latter as well as the mean estimate value of each
436 parameter obtained with both MCMC and FOA approaches are reported in Table 3.

437 The results this table show that the parameters are well estimated from the SP measurements
438 since (i) identified mean values are very close to the reference solution, (ii) all confidence
439 intervals include the reference solution and (iii) the confidence intervals are rather narrow.

440 The saturated parameters K_s and C_{sat} are very well estimated (with CIs around 2%) because
441 of data collected during the falling-head phase where only these two parameters are
442 influential.

443 The posterior CI of the parameter θ_s is similar to its prior CI. The parameter α is reasonably
444 well estimated with a CI around 35%. Recall that this parameter had very small first-order and
445 total sensitivity indices for sensor 1 but had more significant sensitivity indices for the sensors
446 away from the soil surface (see results for sensor 4 in Table 2b). The parameter θ_r is
447 estimated with a CI around 90% although it was highly influential for all sensors (for
448 instance, a first-order sensitivity index of 0.27 and a total order of 0.74 for sensor 1). The

449 parameters n and n_a had similar GSA behavior with small first-order sensitivities
450 (respectively 0.038 and 0.094 for sensor 1) and large total sensitivities (respectively 0.266 and
451 0.4715 for sensor 1), however, the inversion shows that the parameter n is well estimated
452 with a CI less than 10% whereas the parameter n_a is less well estimated with a CI around
453 35%. These results suggest that GSA outcomes should be interpreted with caution in the
454 context of parameter estimation since (i) a parameter which is not relevant for the model
455 output in one sensor can be influential for another sensor and (ii) GSA does not presume on
456 the quality of the estimation since two parameters with similar sensitivity indices can have
457 different quality of estimation by the inversion procedure.

458 Further, the results of Table 3 show that FOA and MCMC approaches yield similar mean
459 estimated values. Moreover, very good agreement is observed between FOA and MCMC
460 uncertainty bounds. Concerning the efficiency of the two calibration methods for this
461 scenario, the FOA approach is by far the most efficient method since it requires only 95s of
462 CPU time. The MCMC method was terminated after 16,000 model runs which required
463 14,116s. The convergence was reached at around 12,000 model runs. The last 4,000 runs were
464 used to estimate the statistical measures of the posterior distribution. Recall that that
465 contrarily to FOA, MCMC can include prior information available for the parameters and
466 allows a quantification of the entire parameter space.

467

468 *Scenario 2: Inversion using only SP measurements near the surface*

469 In this scenario, the number of measurements used for the calibration is strongly reduced.
470 Only SP measurements from sensor 1 (located at 5 cm below soil surface) are considered.

471 The results of MCMC are plotted in the Figure 5. The correlation observed between θ_r and
472 n_a decreases slightly to $r = 0.95$. Almost bell-shaped posterior distributions are observed for
473 all parameters except for the parameters θ_r and α .

474 The results obtained with MCMC and FOA approaches depicted in Table 4 show that

- 475 - The FOA approach yields accurate mean estimated values similar to MCMC results
476 for all parameters;
- 477 - The MCMC and FOA mean estimated values are close to the reference solution and to
478 the previous scenario. The maximum difference is observed for θ_r for which the
479 mean estimated value with scenario 2 is 15% greater than for scenario 1
- 480 - The MCMC CIs for the parameters K_S , θ_s , n and C_{sat} are close to the previous
481 scenario. The parameters θ_s and n are well estimated (CIs < 10%) and the
482 parameters K_S and C_{sat} are very well estimated (CIs $\leq 5\%$).
- 483 - Due to the reduction of the number of data used for model calibration in the scenario
484 2, the MCMC CIs for the parameters n_a , α and θ_r are much larger than in the
485 previous scenario. Indeed, compared to scenario 1, the CI for n_a and θ_r increases by
486 around 60% whereas the CI of α is 3 times larger than for the scenario 1.
- 487 - The FOA method yields accurate CIs for the parameters θ_s , n , n_a and C_{sat} whereas it
488 overestimates the CIs of θ_r (by 24%), K_S (by 100%) and α (by 427%). Unphysical
489 uncertainty region (including negative values) is obtained for the parameter α

490 These results show that the FOA can fail to provide realistic parameter uncertainties and can
491 yield larger CIs than their corresponding nonlinear MCMC counterpart. Indeed, the
492 linearization in the FOA method assumes that the Jacobian remains constant across the CIs.
493 This assumption was quite fulfilled for the first scenario in which a large number of

494 measurements insured small uncertainty regions. However, the assumption is not fulfilled for
495 some parameters of the current scenario because of the large uncertainty regions induced by
496 the reduction of the number of SP measurements.

497 Concerning the efficiency of the calibration methods, the FOA required approximately 174s
498 of CPU time, the MCMC required much more runs to reach the convergence than in the
499 previous scenario. Indeed, the sampler was used with 50,000 runs (35,000 runs were
500 necessary to reach the convergence).

501 **4. Conclusions**

502 In this work, a synthetic test case dealing with SP signals during drainage experiment has
503 been studied. The test case is similar to the laboratory experiment developed in Mboh et al.
504 (2012) involving a falling-head infiltration phase followed by a drainage phase. GSA and
505 Bayesian parameter inference have been applied to investigate (i) the influence of hydraulic
506 and geophysical parameters on the SP signals and (ii) the identifiability of hydro-geophysical
507 parameters using only SP measurements. The GSA was performed using variance-based
508 sensitivity indices which allow measuring the contribution of each parameter (alone or by
509 interaction with other parameters) to the output variance. The sensitivity indices have been
510 calculated using a PCE representation of the SP signals. To reduce the number of coefficients
511 and explore PCE with high orders, we used the efficient sparse PCE algorithm developed by
512 Shao et al. (2017) which selects the best sparse PCE from a given data set using the Kashyap
513 Information Criterion (KIC).

514 The GSA applied to SP signals showed that the parameters C_{sat} and K_s are highly influential
515 during the first period corresponding to saturated conditions. The parameters θ_r , n and n_a
516 are influential when dry conditions occur. In such conditions, strong interactions take place
517 between these parameters. The parameter α has a very small influence on the SP signals near

518 the soil surface but its sensitivity increases with depth although the total variance decreases
519 with depth.

520 Parameter estimation has been performed using MCMC and FOA approaches to check
521 whether FOA can provide reliable estimation of parameters and associated uncertainties for
522 the investigated highly nonlinear hydrogeophysical problem. All hydraulic (K_s , θ_r , α and
523 n) and geophysical (n_a and C_{sat}) parameters can be reasonably estimated in the first scenario
524 when the whole SP data (measured at five different locations) are used as conditioning
525 information for the model calibration. The confrontation with GSA results shows that the
526 latter should be interpreted with caution when used in the context of parameter estimation
527 since (i) a parameter which is not relevant for the model output in one sensor can be
528 influential for another sensor and (ii) GSA does not presume on the quality of the estimation
529 since two parameters with similar sensitivity indices can have different quality of estimation
530 by the inverse procedure (see for instance, parameters n and n_a). Furthermore, although the
531 studied problem is highly nonlinear, the FOA approach provides accurate estimations of both
532 mean parameter values and CIs in the first scenario. These results are identical to those
533 obtained with MCMC.

534 When the number of SP measurements used for the calibration is considerably reduced
535 (scarcity of data), the MCMC inversion provides larger parameters' uncertainty regions. The
536 FOA approach yields accurate mean parameter values (in agreement with MCMC results) but
537 inaccurate and even unphysical CIs for some parameters with large uncertainty regions.

538

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540 **List of table captions**

541 Table 1. Reference values, lower and upper bounds for hydraulic and geophysical parameters.

542 Table 2. The first-order sensitivity index S_i and the total sensitivity index ST_i for the SP
543 signal at 5 cm and 77 cm below the soil surface at different times.

544 Table 3: Estimated mean values (underlined), confidence intervals (CIs) and size of the
545 posterior CIs (*italic*) with MCMC and FOA approaches for scenario 1.

546 Table 4: Estimated mean values (underlined), confidence intervals (CIs) and size of the
547 posterior CIs (*italic*) with MCMC and FOA approaches for scenario 2.

Parameters	Lower bounds	Upper bounds	Reference values
K_s [cm min ⁻¹]	0.1	2	0.495
θ_r [cm ³ min ⁻³]	0	0.2	0.045
α [cm ⁻¹]	0.01	0.2	0.145
n	1.5	7	2.68
n_a [-]	1	3	1.6
$C_{sat} \times (-10^{-7})$ [V/Pa]	2	4	2.9

Table 1. Reference values, lower and upper bounds for hydraulic and geophysical parameters.

	K_s [cm min ⁻¹]	θ_r [cm ³ min ⁻³]	α [cm ⁻¹]	n	n_a	C_{sat} [V/Pa]
a- sensor 1 (5 cm from the soil surface)						
t=10 min (total variance = 0.72)						
S_i	0.055	0	0	0	0	0.942
ST_i	0.057	0	0	0	0	0.945
t=70 min (total variance = 2.17)						
S_i	0.841	0.217	0.005	0.014	0.008	0.045
ST_i	0.894	0.043	0.008	0.028	0.021	0.078
t=800 min (total variance = 0.224)						
S_i	0.053	0.266	0.015	0.038	0.094	0.008
ST_i	0.085	0.738	0.065	0.266	0.472	0.041
b- sensor 4 (77 cm from the soil surface)						
t=10 min (total variance = 0.094)						
S_i	0.055	0	0	0	0	0.942
ST_i	0.057	0	0	0	0	0.945
t=70 min (total variance = 0.2744)						
S_i	0.839	0.015	0.014	0.013	0.005	0.053
ST_i	0.891	0.028	0.024	0.025	0.011	0.086
t=800 min (total variance = 0.224)						
S_i	0.099	0.225	0.054	0.043	0.085	0.01
ST_i	0.138	0.621	0.218	0.238	0.379	0.043

Table 2. The first-order sensitivity index S_i and the total sensitivity index ST_i for the SP signal at 5 cm and 77 cm below the soil surface at different times.

	MCMC	FOA
K_s [cm min ⁻¹]	<u>0.49</u> (0.487-0.498) <i>0.01</i>	<u>0.49</u> (0.487-0.497) <i>0.01</i>
θ_s [cm ³ min ⁻³]	<u>0.43</u> (0.41-0.45) <i>0.04</i>	<u>0.43</u> (0.41-0.45) <i>0.04</i>
θ_r [cm ³ min ⁻³]	<u>0.046</u> (0.025-0.068) <i>0.04</i>	<u>0.046</u> (0.026-0.066) <i>0.04</i>
α [cm ⁻¹]	<u>0.14</u> (0.12-0.17) <i>0.05</i>	<u>0.14</u> (0.12-0.16) <i>0.04</i>
n	<u>2.64</u> (2.54-2.77) <i>0.23</i>	<u>2.64</u> (2.54-2.76) <i>0.22</i>
n_a	<u>1.64</u> (1.37-1.98) <i>0.6</i>	<u>1.64</u> (1.38-1.90) <i>0.5</i>
C_{sat} [V/Pa]	<u>2.90</u> (2.89-2.91) <i>0.02</i>	<u>2.90</u> (2.89-2.91) <i>0.02</i>

Table 3: Estimated mean values (underlined), confidence intervals (CIs) and size of the posterior CIs (italic) with MCMC and FOA approaches for scenario 1.

	MCMC	FOA
K_s [cm min ⁻¹]	<u>0.49</u> (0.481-0.495) <i>0.014</i>	<u>0.49</u> (0.474-0.503) <i>0.029</i>
θ_s [cm ³ min ⁻³]	<u>0.43</u> (0.41-0.45) <i>0.04</i>	<u>0.43</u> (0.41-0.45) <i>0.04</i>
θ_r [cm ³ min ⁻³]	<u>0.053</u> (0.011-0.093) <i>0.08</i>	<u>0.053</u> (0.002-0.103) <i>0.1</i>
α [cm ⁻¹]	<u>0.13</u> (0.07-0.20) <i>0.13</i>	<u>0.13</u> (-0.15-0.43) <i>0.58</i>
n	<u>2.54</u> (2.44-2.68) <i>0.24</i>	<u>2.56</u> (2.44-2.68) <i>0.24</i>
n_a	<u>1.82</u> (1.36-2.41) <i>1.05</i>	<u>1.78</u> (1.29-2.27) <i>0.98</i>
C_{sat} [V/Pa]	<u>2.89</u> (2.88-2.91) <i>0.03</i>	<u>2.89</u> (2.88-2.91) <i>0.03</i>

Table 4: Estimated mean values (underlined), confidence intervals (CIs) and size of the posterior CIs (italic) with MCMC and FOA approaches for scenario 2.

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553 Figure 1. Illustration of the experimental device.

554

555 Figure2. Reference SP signals. Solid lines represent the reference SP solution and dots
556 represent the sets of perturbed data serving as conditioning information for model calibration.

557

558 Figure 3. Time distribution of the SP variance at 5cm (a) and 77cm (b) depth. The shaded area
559 under the variance curve represents the partial marginal contributions of the random input
560 parameters; the contribution of interactions between parameters is represented by the blank
561 region between the shaded area and the variance curve.

562

563 Figure 4. MCMC solutions when all SP data are considered for the calibration. The diagonal
564 plots represent the inferred posterior probability distribution of the model parameters. The
565 off-diagonal scatterplots represent the pairwise correlations in the MCMC drawing.

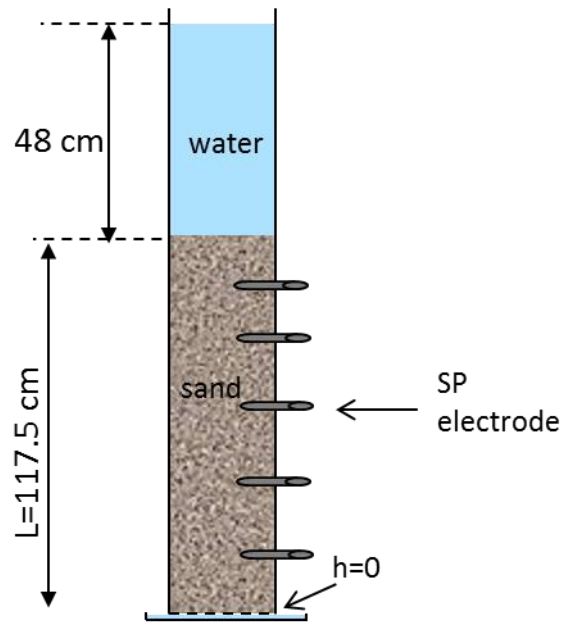
566

567 Figure 5. MCMC solutions when calibration is performed using only SP data located at 5 cm
568 from the surface. The diagonal plots represent the posterior probability distribution of the
569 parameters. The off-diagonal scatterplots represent the pairwise correlations in the MCMC
570 drawing.

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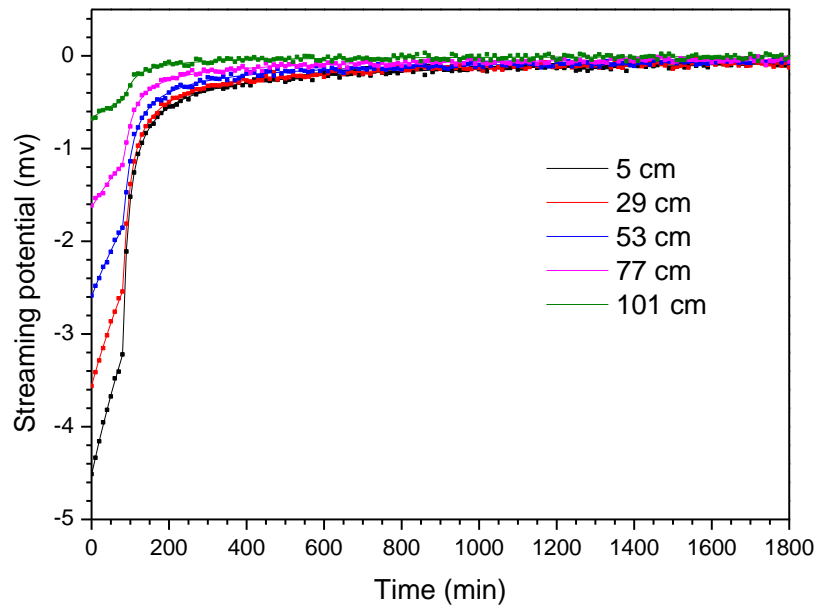


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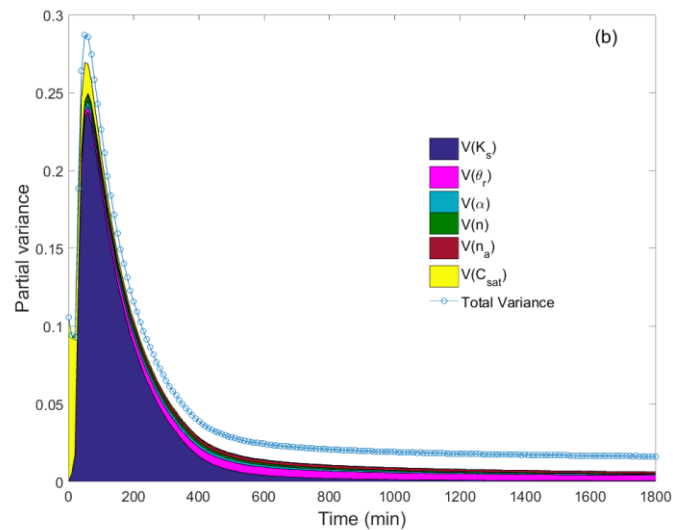
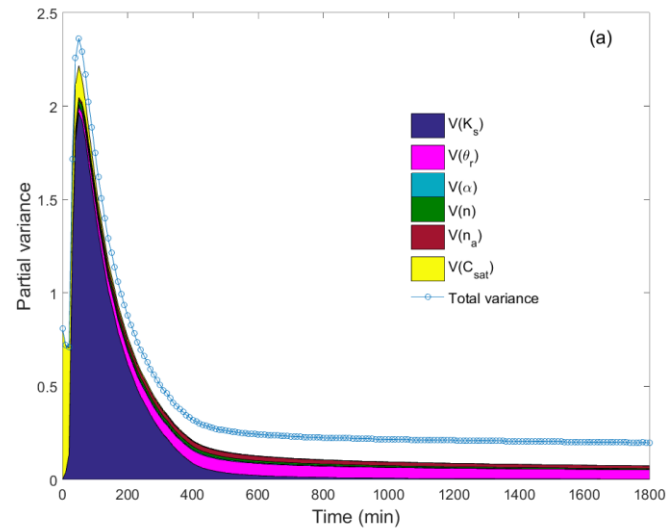


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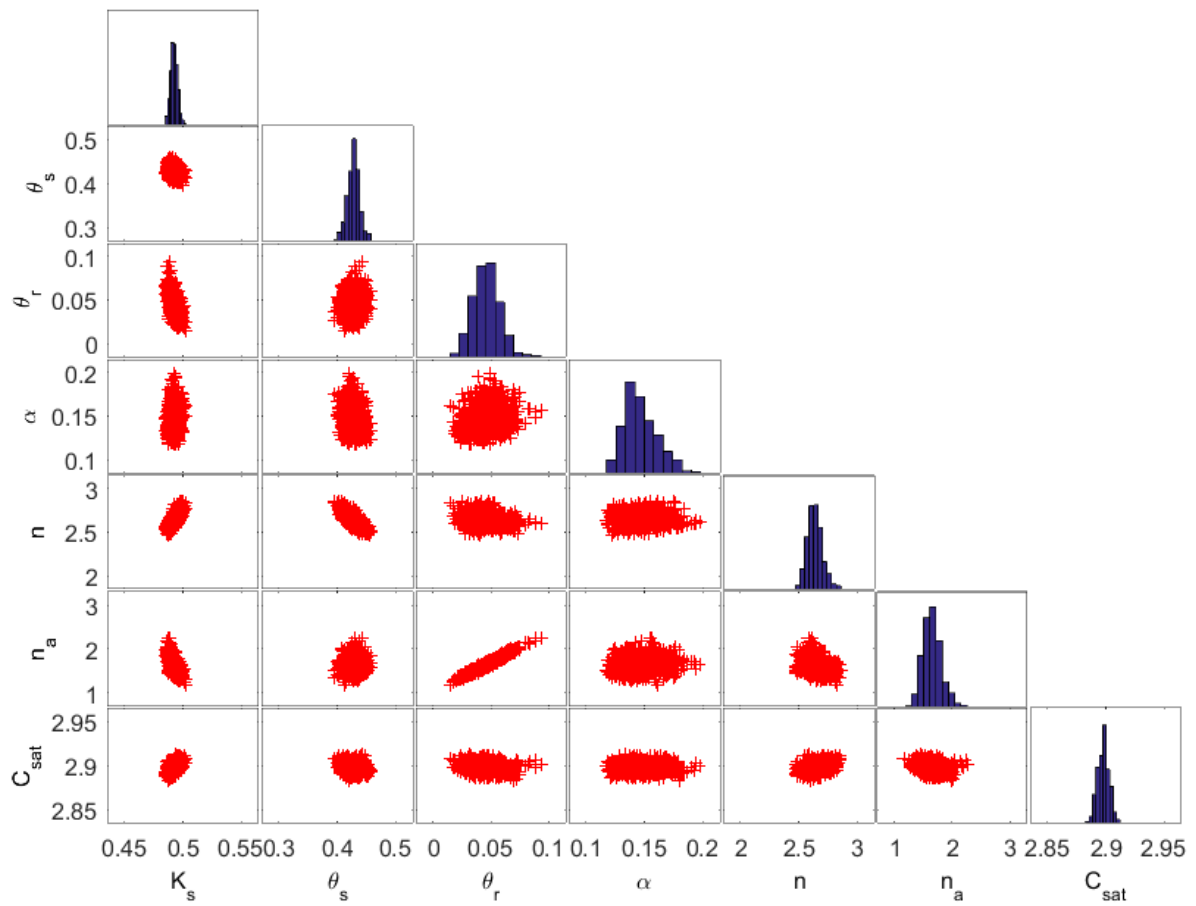


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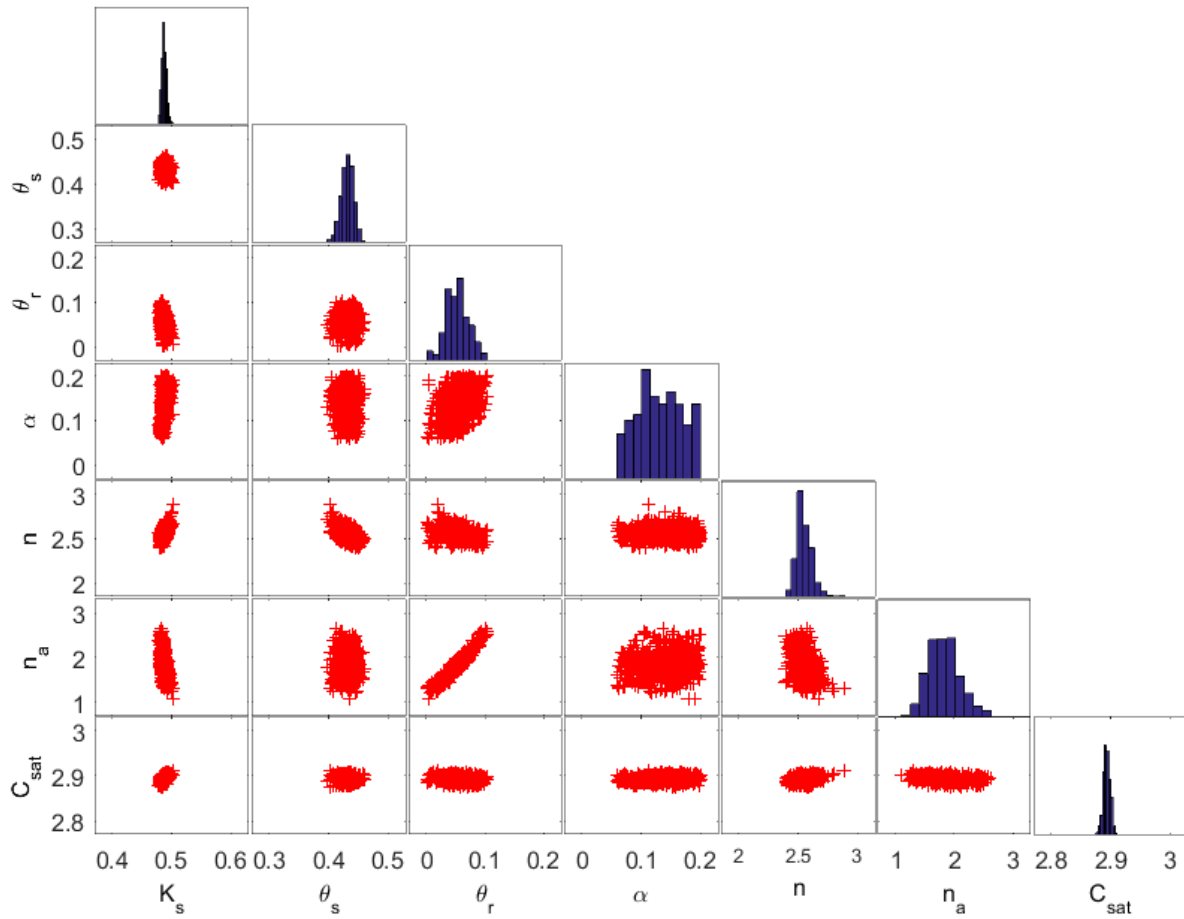


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