Using a multi-hypothesis framework to improve the understanding of flow dynamics during flash floods

Audrey Douinot[1], Hélène Roux[1], Pierre-André Garambois[2], and Denis Dartus[1]

[1]Institut de Mécanique des Fluides de Toulouse (IMFT), Université de Toulouse, CNRS - Toulouse FRANCE
[2]Laboratoire des Sciences de l'ingénieur, de l'informatique et de l'imagerie (ICUBE) - INSA Strasbourg, Strasbourg, France
Correspondence to: Audrey Douinot (audreydouinot@gmail.com)

Abstract. A method of multiple working hypotheses was applied to a range of catchments in the Mediterranean area to analyse different types of possible flow dynamics in soils during flash flood events. The distributed, process-oriented model, MARINE, was used to test several representations of subsurface flows, including flows at depth in fractured bedrock, and flows through preferential pathways in macropores. Results showed contrasted performances of the submitted models, revealing different hydrological behaviours along the catchment set. The benchmark study offered a characterization of the catchments reactivity through the description of the hydrographs formation. The quantification of the different flow processes (surface, intra-soil flows) was consistent with the scarce in-situ observations but remains uncertain, as a result of equifinality issue. The spatial description of the simulated flows over the catchments, made available by the model, enabled to spot counterbalancing effects between internal flow processes, including the compensation for the water transit time in the hillslopes and in the drainage network. New insights are finally proposed into strategical monitoring and calibration constraints setting up.

1 Introduction

1.1 Flash flood events: an issue for forecasters

Flash floods are "sudden floods with high peak discharges, produced by severe thunderstorms that are generally of limited areal extent". (IAHS-UNESCO-WMO (1974); Garambois (2012); Braud et al. (2014)). They are often linked to localised and major forcings (greater than 100 mm, Gaume et al. (2009)) at the heads of steep-sided, meso-scale catchments (with surface areas of 10-250 km^2).

The large specific discharges, and intensities of precipitation, makes the flash floods being classified as extreme. Nevertheless, those events are not scarce nor unusual since on average, there were no fewer than five flash floods a year on the Mediterranean Arc between 1958 and 1994 (Jacq, 1994), and they tend to be amplified against a background of climate change (Llasat et al., 2014; Colmet Daage et al., 2016). Flash floods constitute a significant hazard and, therefore, a considerable risk for populations (UNISDR 2009, Llasat et al. (2014)). They are particularly dangerous due to their characteristics: (i) the sud-

denness of events makes it difficult to warn populations in time, and can lead to panic, thus increasing risk, when a population is unprepared (Ruin et al., 2008); ii) the traditional connected monitoring systems are not adapted to the temporal and spatial scales of the flash floods (Borga et al., 2008; Braud et al., 2014); iii) the magnitude of floods implies significant amounts of kinetic energy, which can transform transitory rivers into torrents, resulting in the transport of debris ranging from fine sediments to tree trunks, as well as the scouring of river beds and the erosion of banks (Borga et al., 2014).

A major area of interest for flash floods is, therefore, better risk assessment, to enable them to be forecast and the relevant populations to be pre-warned. Greater knowledge and understanding is required to better identify the determining factors that result in flash floods. In particular, in order to implement a regional forecasting system, the properties of the catchments, and the climatic forcing and linkages between them which lead to flash flood events need to be characterised.

1.2 Flash flood events: understanding flow processes

Due to the challenges involved in forecasting flash floods, there has been considerable research done on the subject over the last ten years. Examples include the HYDRATE project (2006-2010, Gaume and Borga (2013)), which enabled the setting up of a comprehensive European database of flash flood flash events, as well as the development of a reference methodology for the observation of post-flood events; the EXTRAFLO project (2009-2013, Lang et al. (2014)) to estimate extreme precipitation and floods for French catchments; the HYMEX project (2010-2020, Drobinski et al. (2014)) focusing on the meteorological cycle at the Mediterranean scale, and, in particular, on the conditions that allow extreme events to develop; the FLASH project (2012 - 2017, Gourley et al. (2017)) assessing the ability and the improvement of a flash flood forecasting framework in USA on the basis of real-time hydrological modelling with high resolution forcing; or the FLOODSCALE project (2012-2016, Braud et al. (2014)), based on a multi-scale experimental approach to improve observation of the hydrological processes that lead to flash floods.

In the North-Western Mediterranean context - specially concerned by specific autumnal convective meteorological events - the European cited research demonstrates, in particular, the importance of cumulative rainfall (Arnaud et al., 1999; Sangati et al., 2009; Camarasa-Belmonte, 2016), previous soil moisture state (Cassardo et al., 2002; Marchandise and Viel, 2009; Hegedüs et al., 2013; Mateo Lázaro et al., 2014; Raynaud et al., 2015) and the storage capacity of the area affected by the precipitation (Viglione et al., 2010; Zoccatelli et al., 2010; Lobligeois, 2014; Garambois et al., 2015a; Douinot et al., 2016). The combined influence of the spatial distribution of precipitation and event-related storage capacities, reported in the study of a number of particular events (Anquetin et al., 2010; Le Lay and Saulnier, 2007; Laganier et al., 2014; Garambois et al., 2014; Faccini et al., 2016), suggests a hydrological reaction, in some areas of the catchments, that arises from localised soil saturation. This statement surmises that there is little direct Hortonian flow, but rather the production of runoff through excess soil saturation, or lateral fluxes in the soil resulting from the activation of preferential pathways.

The geochemical monitoring of eight intense precipitation events, over a 3.9 km^2 catchment area (Braud et al., 2014), underlined the dominance of the intra-soil dynamic. First, analysis of the water from the first 40 cm of the soil layer revealed a "flushing" phenomenon, the water present at the start being replaced by so-called "new" rainwater (Braud et al., 2016a; Bouvier et al., 2017). In addition, even if the peaks of the floods mainly consisted of new water, with a proportion varying

between 50% and 80%, it appears that, over the entire period of the events, old water accounts for between 70% and 80% of the total volume of water discharged, which supports the dominance of water pathways in the soil.

Finally the geological properties themselves appear to be markers of the storage capacities available over the time scales involved in flash floods (which are of the order of a day). From simple flow balances of flash flood events (Douinot, 2016),

5 studies of the diverse hydrological responses of several catchments over the same precipitation episode (Payrastre et al., 2012), or the application of regional hydrological models dedicated to flash flood simulation (Garambois et al., 2015b), the literature tends to demonstrate the low storage capacity of non-karst sedimentary and marl-type catchments, and, conversely, the potential for storing large volumes of water in the altered rocks of granitic or schist formations.

1.3 Applying a multi-hypothesis framework for improving hydrological understanding of the flash flood events

10 The knowledge gained about the development of the flow processes (for example, the tracing of events carried out during the FLOODSCALE project, Braud et al. (2014)), relates to studies on a number of specific sites where flash floods could be observed while they were taking place. However, being able to generalise the knowledge gained is limited by the specific nature of each study (McDonnell et al., 2007) and by the gap between the spatial scale of forecasts (meso-scale), compared with that of the in-situ observations ($<10\,\mathrm{km}^2$) (Sivapalan, 2003). Hydrological modelling work can be considered as a means of extrap-

15 olating knowledge to an extended geographical area, possibly covering catchments with differing physiographic properties.

Moreover, hydrological models viewed as "tentative hypotheses about catchment dynamics" are interesting tools for testing hypotheses about hydrological functioning using a systematic methodology. A considerable amount of recently published work has involved comparative studies, using numerical models to develop or validate the hypotheses about the type of hydrological functioning that is most likely to reproduce hydrological responses accurately (Buytaert and Beven, 2011; Clark et al., 2011;

20 Fenicia et al., 2014; Coxon et al., 2014; Ley et al., 2016; Fenicia et al., 2016). Using a same model's structure but differing solely in terms of the hypotheses tested, in the form of modules, the comparison is then focused and restricted to the hydrological assumptions tested. Doing this avoids the limitations on interpretation that are often encountered in comparative studies of models (Perrin et al., 2013), where numerical choices can influence results independently of the underlying assumptions.

The multiple working hypotheses framework is usually applied using a flexible conceptual and lumped model framework,

25 such as the FUSE (Clark et al., 2008) or SUPERFLEX (Fenicia et al., 2011). But also, Clark et al. (2015a) and Clark et al. (2015b) have proposed a unified structure to test multiple working hypotheses within a distributed modeling framework. To our knowledge, the case studies using the aforementioned frameworks are related to continuous hydrological studies in order to assess hydrological hypotheses through the overall hydrological signature of the catchments. In this work, we extend the method of multiple working hypotheses to the assessment of an event-based hydrological model framework.

30 The objective is to test a number of proposed hydrological functioning that occur during flash flood events on a set of contrasting catchments in the French Mediterranean area. While the proportion of flows passing through the soil appears to be significant, questions arise about how they form:

– Are they subsurface flows that take place in a restricted area of the root layer, as a result of preferential path activation? Or, are they lateral flows taking place at greater depth comparable to those seen in some aquifer?

– Does the geological bedrock or an altered substratum play a role limited to that of mere storage reservoir, or is it actively involved in flood flows formation?

5 – Which are the flow processes proportions, according to the events and the catchments?

The aim of this article is to attempt to answer these questions using a multi-model approach that tests different types of hydrological dynamics. The study was based on MARINE, a physically based, distributed hydrological model (Roux et al., 2011; Garambois et al., 2015a), which was developed specifically to model flash floods in the catchments of the French Mediterranean Arc. Several new representations for the soil column and underground flows were proposed (Douinot, 2016) and included in the

10 MARINE model, in the form of modules that can be used to test different hydrological functions (Section 3). Those different hydrological dynamics were applied to a set of catchments - presented in Section 2 - with physiographic properties representative of the whole of the French Mediterranean Arc. The performance of each model was then examined and subjected to a comparative study (Section 4 and 5). The contributions of the results for improving the hydrological functioning understanding are lastly discussed in Section 6 before concluding.

15 2   Catchments and data used in the study

2.1   Study catchment set

We studied the behaviour of four catchments and eight nested catchments in the French Mediterranean Arc (Figure 1). The catchments (in the order they are numbered in Figure 1) were those of the Ardèche, Gardon, Hérault and Salz rivers; these were selected for the following reasons: (i) they are representative of the physiographic variability found in areas where flash

20 floods occur; (ii) numerous studies of flash floods have already been carried out on the Gardon and Ardèche (Ruin et al., 2008; Anquetin et al., 2010; Delrieu et al., 2005; Maréchal et al., 2009; Braud et al., 2014), which could guide the interpretation of the modelling results (Fenicia et al., 2014); and (iii) a considerable number of observations of flash flood events are available for these catchments.

The main physiographical and hydrological properties of the catchments are presented in Table 1. Figure 2 shows the con-

25 trasted geological properties of the studied area : the catchments are marked by a clear upstream / downstream difference. The Ardèche catchment upstream of Ucel sits essentially on a granite bedrock with some sandstone on its edges, while downstream, the geology changes to a predominantly schist and limestone formations. Similarly, the upstream part of the Gardon catchment consists of schistose bedrock while, downstream, the bedrock is impermeable marl-type and granite formation. The Herault catchment is splitted into mostly schist and granitic head watersheds (the Valleraugue and la Terrisse sub-catchments) and a

30 predominantly limestone plateau (Saint Laurent le Minier sub-catchment). Finally, the Salz is characterised by sedimentary bedrock comprising sandstone and limestone (Figure 2).
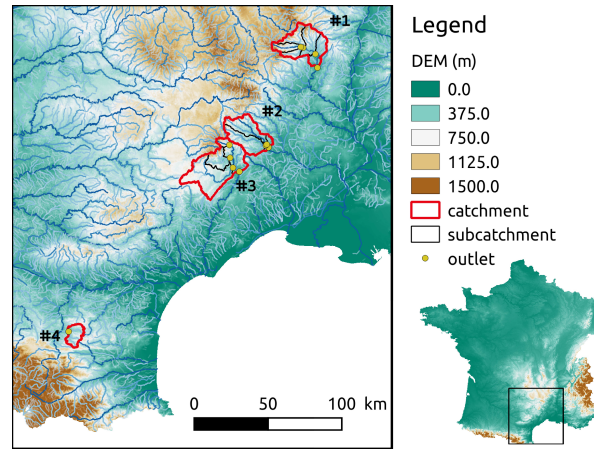
Figure 1. Locations of the catchments studied, with a topographic visualisation at $25\ m$ resolution (Source: IGN, MNT BDALTI)

The Ardèche and the Gardon catchments have been subject to intensive monitoring and studies (see lter reference, https://data.lter-europe.net/deims/site/rbv_fr_13), leading to prior knowledge on hydrological understanding. Both the local in-situ experiments (Ribolzi et al., 1997; Braud and Vandervaere, 2015; Braud et al., 2016a, b) and the modelling studies focused on this area (Garambois et al., 2013; Vannier et al., 2013) tend to a hydrological classification according to those contrasted geological

5   properties and, in agreement with the usual hydrogeological signature found in the litterature (Sayama et al., 2011; Pfister et al., 2017a). Marls, sandstone and limestones without karst are characterized by limited storage capacities, resulting in higher runoff coefficients, and high sensitivity to the initial soil moisture (Ribolzi et al., 1997; Braud et al., 2016a). In contrast, in granite and schist transects located on hillslope of the Ardèche catchment, infiltration tests and analysis of electrical resistivity signals show high permeability of the geological substratum in depth (measured up to 2.5 m in depth); and high storage capacities reach-

10 ing up to 600 mm in 7 out of 10 assessments with artificial forcing, the 3 remaining test suggesting local unaltered bedrock (Braud et al., 2016a, b). The natural resistivity profile suggests a regular soil bedrock interface when the latter consist in schist, while the granite one presents a more chaotic structure. Finally, the continuous comparative study of two experimental sites over surface areas of the order of one km^2 - one located on the schist upstream part of the Gardon catchment, the other one on it granite downstream part - suggests rapid subsurface flow processing on the schist area, while flow formation appears to

15 be controlled by the extension of the saturated zone related to the river on the granitic site (Ayral et al., 2005; Maréchal et al., 2009, 2013).

Table 1. Physiographic properties and hydrological statistics of the 12 catchments *ID*: coding name of the catchments used at figure 1 and table 2; area [km^2]; mean slope [-]; soil properties: mean soil depth [m] and main soil texture (Tx) : Ls = sandy loam texture, L = loam texture; Lsi = silty loam texture; Geology: percentage of bedrock geology [%] including sandstone (Sa), limestone (Li), granite and gneiss (GG), marls (Ma) and schists (Sc) subcategories - [i] bold values are the dominant geology; mean annual precipitation ($P[mm]$) ; Hydrometry: discharge time-series availability (Period); mean inter-annual discharge ($Q[m^3.km^{-2}.s^{-1}]$); 2 year return period of maximum daily discharge ($Q_{D2}[m^3.km^{-2}.s^{-1}]$); 10 year return period of maximum hourly discharge ($Q_{H10}[m^3.km^{-2}.s^{-1}]$). Hydrometric statistics are calculated from HydroFrance databank, (http://www.hydro.eaufrance.fr/) and the pluviometric ones using rainfall data from the raingauge network of the French flood forecasting services.

ID	River	Outlet	Area	Slope	Depth	Tx	Sa	Li	GG	Ma	Sc	P	Q	Q_{D2}	Q_{H10}	Period
			[km^2]	[−]	[m]	[-]	[%]	[%]	[%]	[%]	[%]	[mm]	[m^3.km^{-2}.s^{-1}]			Period
♯1a	L'Ardèche	Vogüé	622	0.17	0.47	Ls	10.5	5.7	**71.9**	0.0	11.9	1587	0.041	0.62	2.25	00 - 15
♯1b		Ucel	477	0.20	0.45	Ls	13.7	0.0	**84.5**	0.0	1.8	1577	0.046	0.79	2.30	05 - 15
♯1c		Pont de la Beaume	292	0.22	0.39	Ls	14.0	0.0	**86.0**	0.0	0.0	1690	0.056	0.75	2.53	00 - 15
♯1d		Meyras	99	0.24	0.32	Ls	5.4	0.0	**94.6**	0.0	0.0	1720	0.036	0.72	2.92	00 - 15
♯2a	Le Gardon	Anduze	543	0.16	0.25	L	7.2	1.5	18.0	12.1	**61.2**	1370	0.026	0.48	1.82	94 - 15
♯2b		Corbès	220	0.16	0.27	L	9.3	0.0	34.2	9.0	**47.5**	1460	0.022	0.57	2.28	94 - 15
♯2c		Mialet Roucan	240	0.17	0.22	L	2.0	0.6	2.9	9.4	**85.1**	1407	0.023	0.62	2.54	02 - 15
♯3a	L'Hérault	Laroque	912	0.14	0.26	Lsi	6.7	**54.5**	11.7	3.2	24.0	1160	0.019	0.39	1.21	00 - 15
♯3b	La Vis	St Laurent le Minier	499	0.10	0.26	Lsi	4.0	**83.0**	1.0	3.2	8.8	930	0.018	0.42	1.10	00 - 15
♯3c	L'Arre	La Terrisse	155	0.19	0.25	L	19.5	12.3	27.2	6.2	**34.8**	1130	0.027	0.61	2.0	00 - 15
♯3d	L'Hérault	Valleraugue	46	0.27	0.25	L	0.0	0.0	0.0	0.0	**100.0**	1920	0.049	1.13	4.0	08 - 15
♯4	La Salz	Cassaigne	144	0.13	0.37	Lsi	33.5	**56.5**	0.0	5.1	4.9	700	0.008	0.20	1.31	01 - 15
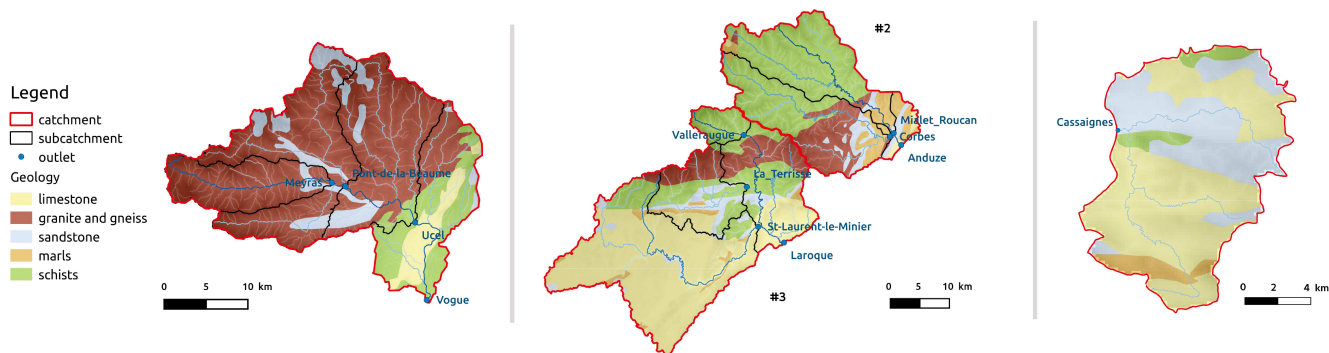
Figure 2. The geology of the Ardèche catchment (left), the Gardon and Hérault catchments (center), and the Salz catchment (sources : BD Million-Géol, BRGM)

2.2 Forcing inputs and hydrometric data

The hydrometric data were derived from the network of operational measurements (HydroFrance databank, http://www.hydro. eaufrance.fr/). Eight to twenty years of hourly discharge observations were available, according to the dates when the hydrometric stations were installed (Table 1).

Flood events with peak discharges that had exceeded the 2-year return period daily discharge (Q_{D2}, in Table 1, corresponds to the alert threshold for flood forecasting centres in France) were selected as events to be included in the study. Thus, only one criterion for hydrological response was considered. This led to a selection of precipitation events of varying origins (for instance: rainfall induced by mountains, stagnant convective cells; and rainfall occurring in different seasons - mainly in autumn and early spring). Such a selection risked complicating the study because flow processes can vary from one season to another. Nevertheless, it allowed us to test the ability of the model to deal with different (non linear) flow physics regimes. Note also that, moderate or intense rainfall events without respective hydrological response might be abducted from the analysis. Nevertheless the first alert threshold used here is small enough to have a selection of flood events with contrasted runoff coefficient (see Table 2).

Precipitation measurements were taken from Météo France's ARAMIS radar network (Tabary, 2007), which provides precipitation measurements, at a resolution of $1\ km \times 1\ km$, every five minutes. The French flood forecasting service (SCHAPI: Service central d'hydrométéorologie et d'appui à la prévision des inondations) used then the CALAMAR patented software (Badoche-Jacquet et al., 1992) to produce rainfall depth data by combining these radar measurements with raingauge data. This processed dataset is here used as inputs of the model. Each rainfall product is firstly assessed through an individual sensitivity analysis of the standard MARINE model (DWF model, see section 3.1). When presenting an atypical sensivity to the soil depth parameter, the rainfall event is discarded of the study, as suggesting questionable measurements. Depending on the availability of the results of rainfall and hydrometric measurements, 7 to 14 intense events were selected for each catchment (Table 2).

Each set is finally splitted into a calibration and validation subsets as follow: the extreme events were kept for validation, and a minimum number of 3 calibration events is chosen in order to cover the wide range of soil moisture initial condition.

As the MARINE model is event-based, it must be initialised to take into account the previous moisture state of the catchment, which is linked to the history of the hydrological cycle. This was done using spatial model outputs from Météo-France's SIM operational chain (Habets et al., 2008), including a meteorological analysis system (SAFRAN, Vidal et al. (2010)), a soil - vegetation - atmosphere model (ISBA, Mahfouf et al. (1995)) and a hydrogeological model (MODCOU, Ledoux et al. (1989)). Based on the work of Marchandise and Viel (2009), the spatial daily root-zone humidity outputs (resolution = $8\ km \times 8\ km$) simulated by the SIM conceptual model were used for the systematic initialisation of MARINE.

Table 2. Properties of the flash flood events: average on the event set ($\pm$ standard deviation). *ID*: coding name of the concerned catchments (Figure 1 : ♯1 for the Ardèche; ♯2 for the Gardon; ♯3 for the Hérault and ♯4 for the Salz); N_{evt}: number of observed flash flood events; P [mm] mean precipitation ; $I_{max}[mm.h^{-1}]$: maximal intensity rainfall per event; Q_{peak}: specific flood peak $[m^3.km^{-2}.s^{-1}]$; Hum: initial soil moil moisture according to SIM output (Habets et al., 2008); CR: runoff coeficient [%]

ID	Outlet	N_{evt}	P [mm]	$I_{max}[mm.h^{-1}]$	$Q_{peak}[m^3.km^{-2}.s^{-1}]$	Hum [%]	CR [−]
♯1a	Vogüé	10	192 ($\pm$93)	17.3 ($\pm$6.2)	1.33 ($\pm$0.57)	58 ($\pm$6)	0.50 ($\pm$0.16)
♯1b	Ucel	10	208 ($\pm$105)	19.1 ($\pm$7.1)	1.41 ($\pm$0.70)	56 ($\pm$5)	0.47 ($\pm$0.17)
♯1c	Pont de la Beaume	10	222 ($\pm$122)	20.5 ($\pm$6.2)	1.79 ($\pm$0.82)	56 ($\pm$5)	0.51 ($\pm$0.22)
♯1c	Meyras	10	235 ($\pm$141)	25.6 ($\pm$10.6)	2.15 ($\pm$1.15)	56 ($\pm$4)	0.51 ($\pm$0.20)
♯2a	Anduze	13	182 ($\pm$69)	26.9 ($\pm$12.6)	2.10($\pm$1.67)	53 ($\pm$7)	0.31 ($\pm$0.13)
♯2b	Corbès	14	196 ($\pm$73)	31.4 ($\pm$11.6)	1.90 ($\pm$0.93)	55 ($\pm$7)	0.32 ($\pm$0.15)
♯2c	Mialet Roucan	14	177 ($\pm$72)	30.9 ($\pm$13.2)	1.85 ($\pm$0.85)	51 ($\pm$7)	0.33 ($\pm$0.15)
♯3a	Laroque	7	188 ($\pm$95)	16.0 ($\pm$8.1)	0.82 ($\pm$0.43)	59 ($\pm$8)	0.45 ($\pm$0.16)
♯3b	St Laurent le Minier	7	153 ($\pm$95)	18.4 ($\pm$8.9)	1.14 ($\pm$0.31)	56 ($\pm$9)	0.47 ($\pm$0.16)
♯3c	La Terrisse	7	193 ($\pm$103)	22.1 ($\pm$12.1)	1.63 ($\pm$0.87)	52 ($\pm$8)	0.60 ($\pm$0.23)
♯3d	Valleraugue	7	156 ($\pm$110)	16.4 ($\pm$8.7)	2.14 ($\pm$1.33)	48 ($\pm$6)	0.62 ($\pm$0.22)
♯4	Cassaigne	8	136 ($\pm$47)	17.8 ($\pm$6.2)	1.48 ($\pm$0.64)	57 ($\pm$7)	0.55 ($\pm$0.24)

3 The multi-hypothesis hydrological modelling framework

3.1 The MARINE model

The MARINE model is a distributed mecanistic hydrological model specially developed for flash flood simulations. It models the main physical processes in flash floods: infiltration, overland flow, lateral flows in soil and channel routing. Conversely, it does not incorporate low-rate flow processes such as evapotranspiration or base flow.

MARINE is structured into three main modules that are run for each catchment grid cell (see Figure 3). The first module allows the separation of surface runoff and infiltration using the Green-Ampt model. The second module represents subsurface downhill flow. It was initially based on the generalised Darcy Law used in the TOPMODEL hydrological model (Roux et al., 2011), but was developed in greater detail as part of this study (see Section 3.2). Lastly, the third module represents overland

5   and channel flows. Rainfall excess is transferred to the catchment outlet using the Saint-Venant equations simplified with kinematic wave assumptions. The model distinguishes grid cells with a drainage network - where channel flow is calculated on a triangular channel section (Maubourguet et al., 2007), from grid cells on hillslopes - where overland flow is calculated for the entire surface area of the cell.
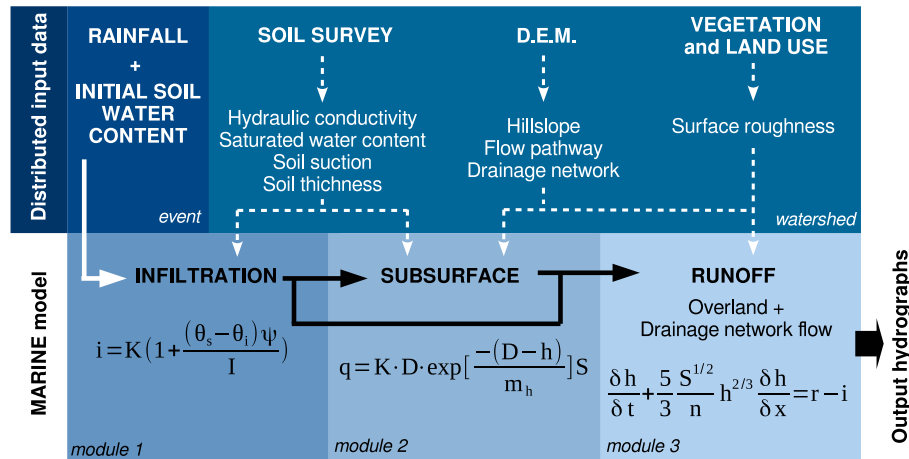
Figure 3. The MARINE model structure, parameters and variables. The Green and Ampt infiltration equation contains the following parameters: infiltration rate i [m.s^{-1}], cumulative infiltration I [mm], saturated hydraulic conductivity K [m.s^{-1}], soil suction at the wetting front Ψ [m], and, saturated and initial water contents, θ_s and θ_i [m^3.m^{-3}], respectively. Subsurface flow contains the following parameters: soil thickness [m], lateral saturated hydraulic conductivity K [m.s^{-1}], local water depth h [m], transmissivity decay with depth m_h (m), and bed slope S [m.m^{-1}]. The kinematic wave contains the following parameters: surface water depth h [m], time t [s], space variable x [m], rainfall rate r [m.s^{-1}], infiltration rate i [m.s^{-1}], bed slope S [m.m^{-1}], Manning roughness coefficient n [m$^{-1/3}$.s]. The module 2 described in this figure corresponds to the standard definition applied in the MARINE model.

The MARINE model works with distributed input data such as: i) a digital elevation model (DEM) of the catchment to
10 shape the flow pathway and distinguish hillslope cells from drainage network cells, according to a drained area threshold; ii) soil survey data to initialize the hydraulic and storage properties of the soil, which are used as parameters in the infiltration and lateral flow models; iii) vegetation and land-use data to configure the surface roughness parameters used in the overland flow model.

The MARINE model requires parameters to be calibrated in order to be able to reproduce hydrological behaviours accurately.
15 Based on sensitivity analyses of the Garambois et al. (2013) model, five parameters are calibrated: soil depth - C_z, the saturation hydraulic conductivity used in lateral flow modelling - C_{kss}, hydraulic conductivity at saturation, used in infiltration modelling

- C_k, and friction coefficients for low and high-water channels - n_r and n_p, respectively, with n_r and n_p uniform throughout the drainage network. C_{kss}, C_k and C_z are the multiplier coefficients for spatialised, saturated hydraulic conductivities and soil depths. In this study, modifications of the module 2 (i.e. subsurface downhill flow) were tested in order to determine the possible ways that a number of proposals for intra-soil hydrological functioning could be modelled. Consequently, instead of C_z and C_{kss}, new parameters of calibration were introduced, as described in the following section.

3.2 Modelling lateral flows in the soil: the development of a multi-hypothesis framework

We proposed several modifications to Module 2 - the subsurface dowhill flow submodel - covering three hypotheses about hydrological functioning:

- Deep Water Flow model (DWF): it assumed deep infiltration and the formation of an aquifer flow in highly altered rocks. In hydrological terms the pedology-geology boundary was transparent. The soil column could be modelled as a single entity of depth D_{tot} (m), which is at least equal to the soil depth D_{BDsol} (m) (see Figure 4). Given the lack of knowledge and available observations, a uniform calibration was applied to the depth of altered rocks - D_{WB} (m) - which is rapidly accessible on the scale of a rain event. Groundwater flow was described using the generalised Darcy Law (q_{dw}, Equation 1). The exponential growth of the hydraulic conductivity at saturation, as the water table (h_{dw}) rises, assumed an altered-rock structure where hydraulic conductivity at saturation decreases with depth (the TOPMODEL approach).

$$q_{dw} = K_{dw} \cdot D_{tot} \, exp\left(\frac{h_{dw} - D_{tot}}{m_h}\right) \cdot S \tag{1}$$

with $h_{dw}[m]$, the water depth of the unique water table; $m_h[m]$, the decay factor of the hydraulic conductivity at saturation with soil depth; $S[-]$, the bed slope; $K_{dw} = C_{kdw} \cdot K_{BDsol}[m.s^{-1}]$, the simulated hydraulic conductivity at saturation; and $D_{tot} = D_{BDsol} + D_{WB}$, the soil column depth. Calibrated parameters are in red color.

- Subsurface Flow model (SSF): it assumed that the formation of subsurface lateral flows was due to the activation of preferential paths, like the in-situ observations of Katsura et al. (2014) and Katsuyama et al. (2005). The altered soil-rock interface acts as a hydrological barrier. The rapid saturation of shallow soils results in the development of rapid flows due to the steep slopes of the catchments and the existence of rapid water flows circulating through the macropores as the soil becomes saturated. The soil column was thus represented by a two-layers model (see Figure 5): an upper layer of depth equal to the soil depth D_{BDsol} (m) and a lower layer of uniform depth D_{WB} (m). The lateral flows in the upper layer were described by the generalised Darcy Law. However, variations in hydraulic conductivity were expressed as a function of the mean water content of the layer (θ_{soil}) and not of the height of water (h_{soil}) that would form a perched water table (Equation 2). Expressing the variability in hydraulic conductivity as a function of the saturation rate indeed appears to be a more appropriate choice for representing the activation of preferential paths in the soil by the increase in the degree to which the soil is filled. The decay factor of the hydraulic conductivity as a function of the saturation rate - m_θ - was set according to the linearized empirical relations, developed by Van Genuchten (1980), between hydraulic

conductivity and soil water content for the different classes of soil textures. Flows in the lower soil layer (q_{dw}, Equation 3), in the form of a deep aquifer, were limited by setting the hydraulic conductivity of the substratum as being equivalent to that of the soil divided by 50 (this choice being guided by the orders of magnitude generally observed in the literature (Le Bourgeois et al., 2016; Katsura et al., 2014)). The altered rocks were thus assumed to mainly play a storage role. Infiltration occurring between the two layers was initially restricted by the Richards equations which were incorporated using the set hydraulic properties of the substratum (Equation 4). When the upper layer is saturated, filling by a piston effect is allowed. The depth of the soil layer, D_{BDsol}, was set according to the soil data, while the depth of the substratum - D_{WB} - was calibrated in the same way as in the DWF model.

$$q_{ss} = K_{ss} \cdot D_{BDsol} \, exp \left(\frac{\theta_{soil} - 1}{m_\theta} \right) \cdot S \tag{2}$$

$$q_{dw} = K_{dw} \cdot D_{WB} \, exp \left(\frac{h_{WB} - D_{WB}}{m_h} \right) \cdot S \tag{3}$$

$$q_{inf} = -K_{dw} \frac{\delta H(\theta_{soil}, \theta_{WB})}{\delta z} \tag{4}$$

with: h_{soil} and $h_{WB}[m]$, the soil water depth in the upper and lower layer respectively; θ_{soil} and $\theta_{WB}[-]$, the soil water content of the upper and lower layer respectively; $m_\theta[-]$, the decay factor of the hydraulic conductivity with soil water content θ_{soil}; and $K_{ss} = C_{kss} \cdot K_{BDsol}$ and $K_{dw} = 0.02 \cdot K_{ss} \, [m.s^{-1}]$, the simulated hydraulic conductivity at saturation of the upper and lower layer in the SSF model respectively.

- The Subsurface and Deep Water Flow model (SSF-DWF): it assumed that the presence of subsurface flow was due to both local saturation of the top of the soil column, but also the development of a flow at depth, as a result of significant volumes of water introduced by infiltration and a very altered substratum whose apparent hydraulic conductivity was already relatively high. This hypothesis of the process led to a modelling approach analogous to the SSF model (Figure 5), where the hydraulic conductivity at substrate saturation - K_{dw} - was no longer simply imposed, but, instead, calibrated using an additional coefficient, C_{kdw}.

$$K_{dw} = C_{kdw} \cdot K_{BDsol} \qquad \text{in SSF-DWF model} \tag{5}$$

The soil water content prior to simulation was, similarly, initialised for each model, in order to ensure, for a fixed depth of altered rock, that the same volume of water was allocated for all models. The SIM humidity indices (Section 2.2) were used to set an overall water content for all groundwater flow models for a given flood.

4 Methodology for calibrating and evaluating the models

4.1 Calibration method

The three hydrological models studied - DWF, SSF and SSF-DWF - were calibrated for each catchment by weighting 5,000 randomly drawn samples from the parameter space for each model (the Monte Carlo Method). The weighting was done using
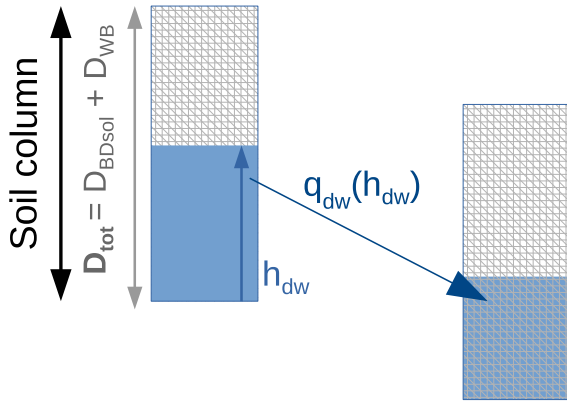
Figure 4. DWF model: flow generation by infiltration at depth and support of a deep aquifer ($q_{dw}(h_{dw})$, equation 1).
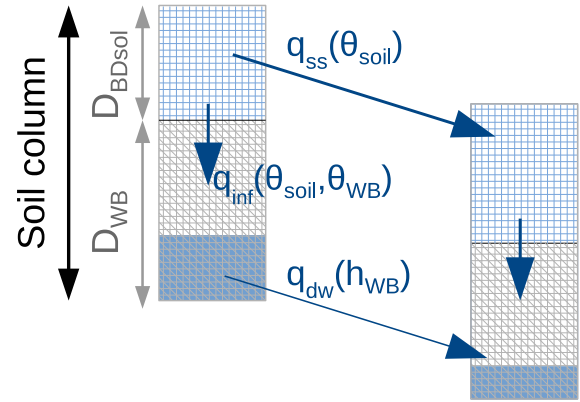
Figure 5. SSF and SSF-DWF models: flow generation by the saturation of the upper part of soil column and activation of preferential paths (q_{ss}), with support flow at depth (q_{dw}), and water exchanges from the upper layer to the lower one according to both soil water content ($q_{inf}(\theta_{soil}, \theta_{WB})$). See equations 2, 3 and 4, for the definition of the flows.

the DEC (Discharge Envelope Catching) score (equation 6), discussed by Douinot et al. (2017), in order to integrate the a priori uncertainties of modelling $\left((\sigma_{mod,i}), \; i = 1...n\right)$ (equation 7) and those related to the flow measurements $\left((\sigma_{\hat{y}_i}), \; i = 1...n\right)$ (equation 8). The choice of DEC is justified by the desire to adapt the evaluation criterion to the modelling objectives (for example, by focusing calibration on reproduction of the rise and peaks of floods in order to be able to forecast flash floods) while always being aware of the uncertainties in the reference flow measurements.

Given the lack of information, these uncertainties $\left((\sigma_{\hat{y}_i}), \; i = 1...n\right)$ were set at 20 % of the measured discharge, which is in line with the literature on discharge measurements from operational stations (Le Coz et al., 2014), and increased linearly with the 10-year hourly discharge, beyond which, as a general rule, the observed flow is no longer measured, but derived by extrapolation from a discharge curve, making it less accurate (equation 8). The envelop $\left((\hat{y}_i \pm 2\sigma_{\hat{y}_i}), \; i = 1...n\right)$ consequently defines the 95 % confidence interval of the observed flows.

The modelling uncertainties $\left((\sigma_{mod,i}), \; i = 1...n\right)$ were set at a minimum value - as a function of the basic catchment module, thus ensuring that the evaluation of the hydrographs would not be unduly affected by the reproduction of relatively low flows which were strongly dependent on initialisation using previous moisture data that were not the subject of this study. In addition, it was assumed that a modelling uncertainty of 10 % around the confidence interval of observed flows was acceptable (equation 7). Finally, the overall overarching envelop $\left((\hat{y}_i \pm 2\sigma_{\hat{y}_i} \pm 2\sigma_{mod,i}), \; i = 1...n\right)$ defines hereafter the acceptability zone, that is to say the interval into which any simulated flow would be considered as acceptable, according to the modelling and measurement uncertainty definitions.

$$DEC = \frac{1}{n}\sum_{i=1}^{n}\epsilon_i^{DEC} \quad = \quad \frac{1}{n}\sum_{i=1}^{n}\frac{d_i}{\sigma_{mod,i}} \tag{6}$$

$$\sigma_{mod,i} \quad = \quad 0.5 * Q + 0.025 * \hat{y}_i \tag{7}$$

$$\sigma_{\hat{y}_i} \quad = \quad 0.05 * \hat{y}_i * \left(1 + \frac{\hat{y}_i}{Q_{H10}}\right) \tag{8}$$

with ϵ_i^{DEC} the DEC modelling error at time i; $\hat{y}_i$ and $\sigma_{\hat{y}_i}$ the observed discharge and the uncertainty of measurement at time i; d_i the discharge distance between the model prediction at time i (y_i) and the confidence interval of observed flows $\left(\hat{y}_i \pm 2\sigma_{\hat{y}_i}\right)$; $\sigma_{mod,i}$ the simulated uncertainty at time i; Q and Q_{H10} respectively the mean inter-annual discharge and the 10-year maximum hourly discharge of the related catchment.

4.2 Metrics and key points in model evaluation and comparison

Results of the models were firstly assessed and benchmarked using performance scores (section 5.1). The evaluation focused on the performance of the models in reproducing the hydrographs in overall terms, but also, more specifically, on their ability to reproduce the characteristic stages of floods: rising flood waters, high discharges, and flood recession. These stages were defined as follows:

- Rising flood waters: the period between the moment when the observed flow rate exceeded the mean inter-annual discharge of the catchment and the date of the first flood peak.

- High discharges: this stage includes the points for which the observed flow was greater than 0.25 times the maximum flow during the event.

- Flood recession: this stage begins after a period of t_c (the catchment concentration time according to Bransby's formula (Pilgrim and Cordery, 1992): $t_c = 21.3 \cdot L/(A^{0.1} \cdot S^{0.2})$) after the peak of the flood, and ends when discharge is rising again (or, where appropriate, at the end of the event - the time of peak flooding + 48h).

The DEC score has provided a standard assessment of the modelling errors enabling a reasonable weighting of the simulations. However, for a sake of easy understanding, the percentage of acceptable points of the simulated median time series - Qmed_INT [%] (Douinot et al., 2017) - was chosen to evaluate the ability of the models to reproduce overall flows, rising flood waters and high discharges. A point is defined as acceptable when the median simulated value stands within the modelling acceptability zone $\left((\hat{y}_i \pm 2\sigma_{\hat{y}_i} \pm 2\sigma_{mod,i}), \ i = 1...n\right)$.

Conversely, Qmed_INT was not relevant for the evaluation of the capacity to reproduce recessions, because the calculation of this score during the recession interval strongly depends on performance at high discharges. Instead, we used the A_{slope} score defined in the equation 9. It calculates the average standard error in simulating the decreasing rate of the discharge during

the flood recession interval. Through the consideration of the A_{slope} score here, it was assumed that the recession rate is a relevant feature of the catchment's hydrologic properties (Troch et al., 2013; Kirchner, 2009).

$$A_{slope} = \frac{\sum_{i=k}^{l} \left| \frac{dy_i}{dt} - \frac{d\hat{y}_i}{dt} \right|}{\sum_{i=k}^{l} \frac{d\hat{y}_i}{dt}} \tag{9}$$

where $\frac{d\hat{y}_i}{dt}$ and $\frac{dy_i}{dt}$ are respectively the observed and the simulated recession rates at a time step i which belongs to the flood
5   recession interval $(i = k...l)$.

The evaluation was completed through the description of the modelling errors (section 5.2), in order to identify those that were inherent in the choice of model structure, regardless of the calibration methodology adopted (Douinot et al., 2017). Attention was paid on the a priori and a posteriori confidence interval of the model simulations respectively defined by $\left([y_i^{prior-5th}, y_i^{prior-95th}], \; i = 1...n \right)$ and $\left([y_i^{DEC-5th}, y_i^{DEC-95th}], \; i = 1...n \right)$ where $y_i^{prior-5th}$ and $y_i^{prior-95th}$ are the
10   5th and the 95th percentile of the 5000 model simulation values at time i, and where $y_i^{DEC-5th}$ and $y_i^{DEC-95th}$ are the 5th and the 95th percentile of the same but weighted series according to the DEC calibration criterion.

Those confidence intervals were standardized according to the DEC modelling error definition (equation 6), respectively defining the a priori and a posteriori confidence intervals of the modelling errors:

$$\epsilon_i^{\alpha-xth} = \begin{cases} 0 & \text{if} \quad | y_i^{\alpha-xth} | \leq 2 \cdot \sigma_{\hat{y}_i} \\ \frac{y_i^{\alpha-xth} \pm 2 \cdot \sigma_{\hat{y}_i}}{2 \cdot \sigma_{mod_i}} & \text{otherwise} \quad (- \text{ if } y_i^{\alpha-xth} > 0 ; + \text{ if } y_i^{\alpha-xth} \leq 0) \end{cases} \tag{10}$$

15   *with $\epsilon_i^{\alpha-xth}$ is the xth percentile of the α modelling errors distribution at time i.*

The latter definition allows for an informative translation of the prior and posterior confidence intervals (Douinot et al., 2017): a value of $\epsilon_i^{\alpha-xth}$ equal to 0 indicates that the $y_i^{\alpha-xth}$ bound lies within the discharge confidence interval; if $0 < \epsilon_i^{\alpha-xth} \leq 1$, the $y_i^{\alpha-xth}$ bound lies within the acceptability zone; and if $\epsilon_i^{\alpha-xth}$ is larger than 1 then errors of modelling is detected or remained. In addition, the benchmark of both a priori and a posteriori confidence intervals allows for highlighting
20   which were the remaining modelling errors that were induced by the model's assumptions, and those that were induced by the calibration.

5   Results

5.1   Performance of the models

5.1.1   Overall performances of the models

25   **Assessment of the performances by catchment :** Figure 6 shows the average and standard deviations of the Qmed_INT scores obtained after calibration of the DWF, SSF and SSF-DWF models for each catchment studied. The DWF model assuming deep infiltration and the formation of an aquifer flow in altered bedrocks showed better performance in the Ardèche catchment (♯1),

while in the Gardon (♯2) and the Salz (♯4) catchments, the SSF and SSF-DWF models, assuming the formation of subsurface flows due to the activation of preferential flowpaths by local saturation (SSF), with development of flow at depth (SSF-DWF), produced the most accurate results. On the Hérault catchment (♯3), the modelling results obtained with each model, in terms of Qmed_INT, were less obvious, although the SSF-DWF model seemed to stand out to some extent. The differences in model

5   performance were more pronounced for the validation events. The better-performing models tended to be more consistent, with equivalent Qmed_INT scores on calibration and validation events (for example, the DWF model on the Ardèche (♯1) or the SSF and SSF-DWF models on the Gardon (♯2)). There was also a deterioration in performance in several models that had already been judged less effective (for example, the SSF and SSF-DWF models on the Ardèche (♯1), or the DWF model on the two catchments of the Hérault, ♯3c and ♯3d).
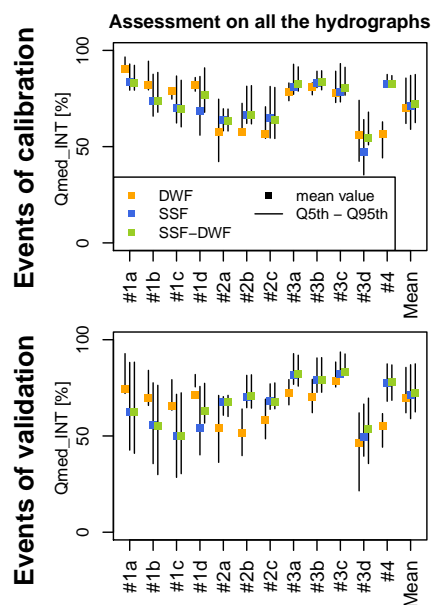
Figure 6. Qmed_INT scores: mean Qmed_INT scores obtained for the calibration (top) and validation (bottom) events, by model and catchment. The Qmed_INT scores were calculated for the whole hydrograph. The x axis refers to the ID number of each catchment (Figure 1). Finally, *Mean* attribute refers to the average results over all the catchments obtained with each model.

10   **SSF model versus SSF-DWF model:** As a reminder, the difference between the SSF and SSF-DWF models is that the latter has an extra calibration parameter - C_{kdw} - to be able to initialise a significant lateral flow in the subsoil horizons of the soil column (see Equation 3). The lateral hydraulic conductivity in the deep layer is configured using the hydraulic conductivity from BD-sol: $K_{dw} = C_{kdw} \cdot K_{BDsol}$, with C_{kdw} set to $0.02 \cdot C_{kss}$ in the SSF model and calibrated in the SSF-DWF model. The small differences between the SSF and SSF-DWF models showed that this flexibility does not produce any significant

15   improvement, with the exceptions of the Ardèche catchment at Meyras and the Hérault catchment at Valleraugue. These two areas have a number of common features that could explain the similar modelling results: they are at the heads of high elevation catchments with steep slopes (Table 1), and are subject to considerable annual meteorological forcing. The calibration of C_{kdw}
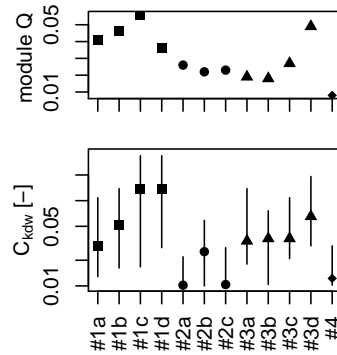
Figure 7. Top: Mean inter-annual discharge ($m^3.km^{-2}.s^{-1}$) for the catchments. Bottom: a posteriori distribution of the calibration of the subsoil horizon hydraulic conductivity in the SSF-DWF model (the C_{kdw} parameter, Equation 3)

consistently tended to simulate significant flow at depth for these two catchments, exclusively higher values from the prior confidence interval having been selected (Figure 7). In general, the calibration of the C_{kdw} parameter of the SSF-DWF model correlates with the more or less sustained, annual hydrological activity of the catchments: the confidence interval of the C_{kdw} coefficient is restricted to low values for the catchments with low mean inter-annual discharges (♯2a, ♯2b, ♯2c, ♯3a, ♯3b, ♯4) and inversely for the catchments with high mean inter-annual discharges (♯1, ♯3c and ♯3d).

5.1.2 Detailed performances: assessment of the models to simulate the different stages of an hydrograph

Figure 8 shows the detailed assessments according to the specific stages of the hydrographs. It highlights whether the overall performances (Figure 6) reflect uniform results along the hydrographs, or if they actually hide contrasted likelihood of the simulations over the different hydrographs's stages.

Uniform results are observed on the Gardon catchment at Corbes and Anduze (♯2a and ♯2b) and on the Salz catchment (♯4): the SSF and SSF-DWF models demonstrated clearly superior performances for all stage-specific assessment on those catchments. For the Gardon catchment at Mialet (♯2c), the detailed assessment (Figure 8) shows that the overall superiority of the SSF and SSF-DWF models is mainly due to a better simulation of the rising limb. Nevertheless, for any score, the SSF and SSF-DWF models present either similar of the best modelling results compared to the DWF model.

On the Ardèche catchments (♯1a, ♯1b, ♯1c, ♯1d), the overall performances reflect the simulation of the high discharges and of the flood recessions. There, the DWF model gives the best results to simulate those hydrograph's stages. Conversely, it deals slightly less well with the simulation of the rising flood waters. As it would be shown in the section 5.2, all the models tend to underestimate initial flows prior to the event and during the onset of a flood. The DWF model, in particular, exhibits this modelling weakness (see, for example, the onset of floods in the hydrographs for the 18/10/2006 and 01/11/2014 events in Ucel (♯1b), Figure 10), which explains the poorer performance. It can be noticed that the SSF-DWF model clearly better simulated the rising flood waters of the Ardèche head watershed (♯1d), explaining the overall good performance as well of this model on this catchment (Figure 6).
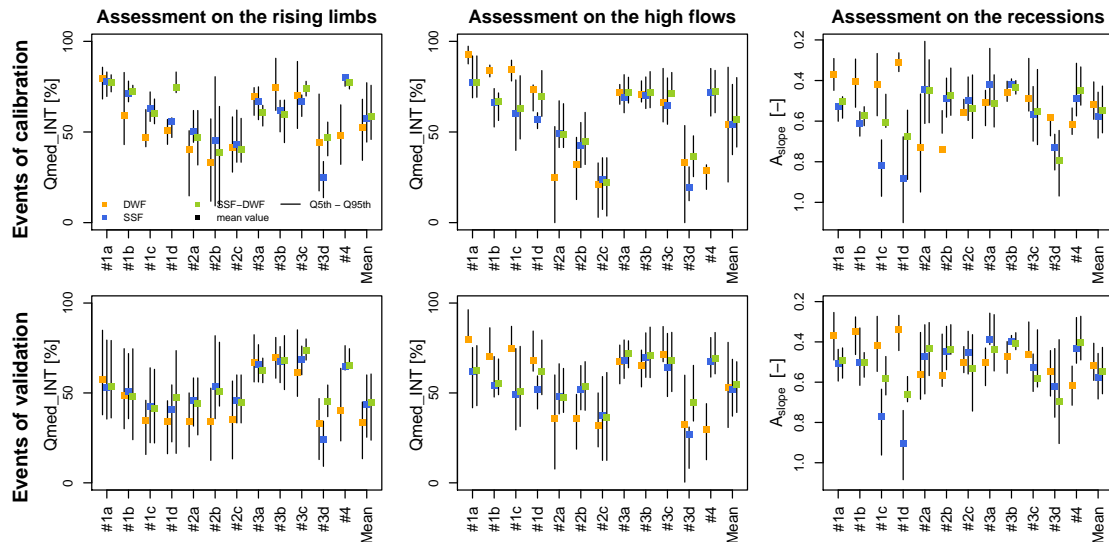
Figure 8. Assessment of the models by catchment over the different stages of the hydrographs. Left : Qmed_INT scores calculated over the rising flood waters stage; center: Qmed_INT scores calculated over the high discharges stage; right: A_{slope} scores. High Qmed_INT scores and conversely low A_{slope} values indicate good performances of the model.

On the Hérault, the detailed evaluation enabled us to distinguish the performance of the different models. On the one hand, on the 2 larger catchments ($\sharp 1a$ and $\sharp 1b$), the DWF model get slightly better performances for rising flood waters simulations, while the SSF model gave more clearly better simulations of the flood recessions. On the other hand the SSF-DWF model generated the best simulations of the rising flood waters and of the high flows on the upstream catchments of La Terrisse ($\sharp 3c$) and Valleraugue ($\sharp 3d$), while the DWF model simulated better flood recession. These constrated results explained why there is not a specific model that stands out on this catchment. In addition, it suggests a marked influence of the physiographic properties on the development of flow processes because they are correlated with the differences in the geological and topographical properties of the Hérault ($\sharp 3$; see Figure 2 and Table 1). The hydrological behaviours simulated for the Valleraugue and La Terrisse sub-catchments, which are predominantly granitic and schistose, and where slopes are very steep, can be distinguished from those of Laroque and Saint-Laurent-le-Minier, which are mainly sedimentary and in the form of large plateaus.

5.1.3 Summary of the assessment

The figure 9 sums up the highlighted models according to the assessed hydrograph's stage. It shows when one's model has a clearly higher performance according to the following definition: a model is assessed as clearly superior when the lower bound of the confidence interval of it score is higher than the median values of the scores obtained with the other models. It reveals that the catchments set might be divided in 4 groups:

- a first group of catchments where the SSF and DWF-SSF models uniformly give either similar or better performances than the DWF models. This is the case for the Gardon ($\sharp 2$) and the Salz ($\sharp 4$) catchments;
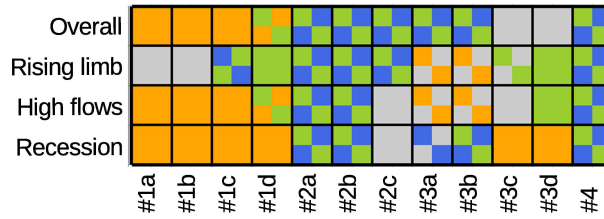
Figure 9. Summary of the models's benchmark. A (2) color(s) is (are) attributed for each score and each catchment when one (or two) models give(s) clearly superior performance: the score of a model is defined as clearly superior when the lower bound of it confidence interval is higher than the median values obtained with the other models. The superiority of a model might be half attributed whether the criteria is only respected for the calibration processes. Color attribution: orange for the DWF model; blue for the SSF model; green for the DWF-SSF model; and grey when the superiority of one's model is undetermined.

– a second group of catchments where the DWF model gives the best results according to all the scores besides the rising flood waters assessment. This is the case for the downstream Ardèche catchments ($\sharp 1a$, $\sharp 1b$, $\sharp 1c$);

– a third group where the models's results are not really discernible. For those catchment, the DWF model appears to slightly simulate better the rising flood and the high discharge, while the recession is better represented by SSF model. This is the case for the downstream Hérault catchments ($\sharp 3a$, $\sharp 3b$);

– a last group where the SSF-DWF model slighty generated better the rising flood and the high discharge, while the recession is better represented by DWF model. In this group are the head watersheds of the Hérault ($\sharp 3c$, $\sharp 3d$) and of the Ardèche ($\sharp 1d$) catchments.

5.2 Modelling errors inherent in the models' structures

For the sake of conciseness, only the simulation over one catchment is presented. Figure 10 shows the simulation results of the three models over the Ardèche catchment at Ucel ($\sharp 1b$). It shows the simulated hydrographs, and their confidence intervals, compared with observed flows, as well as the inherent errors in the simulations. This highlights the modelling errors due to the choice of model structure (DWF, SSF or SSF-DWF models). When - at a time i - the a priori confidence interval (grey color) does not cross the acceptability region (green color), it means that no parameter set gives an acceptable simulation, and consequently modelling errors due to the structure - assumptions - of the model is detected. When the posterior confidence interval (salmon color) is outside the acceptability zone, modelling error is remaining. Finally whether the prior (posterior) interval is large or small, the model's structure allows for reaching a more or less large range of simulated values (the model prediction is more or less uncertain respectively).
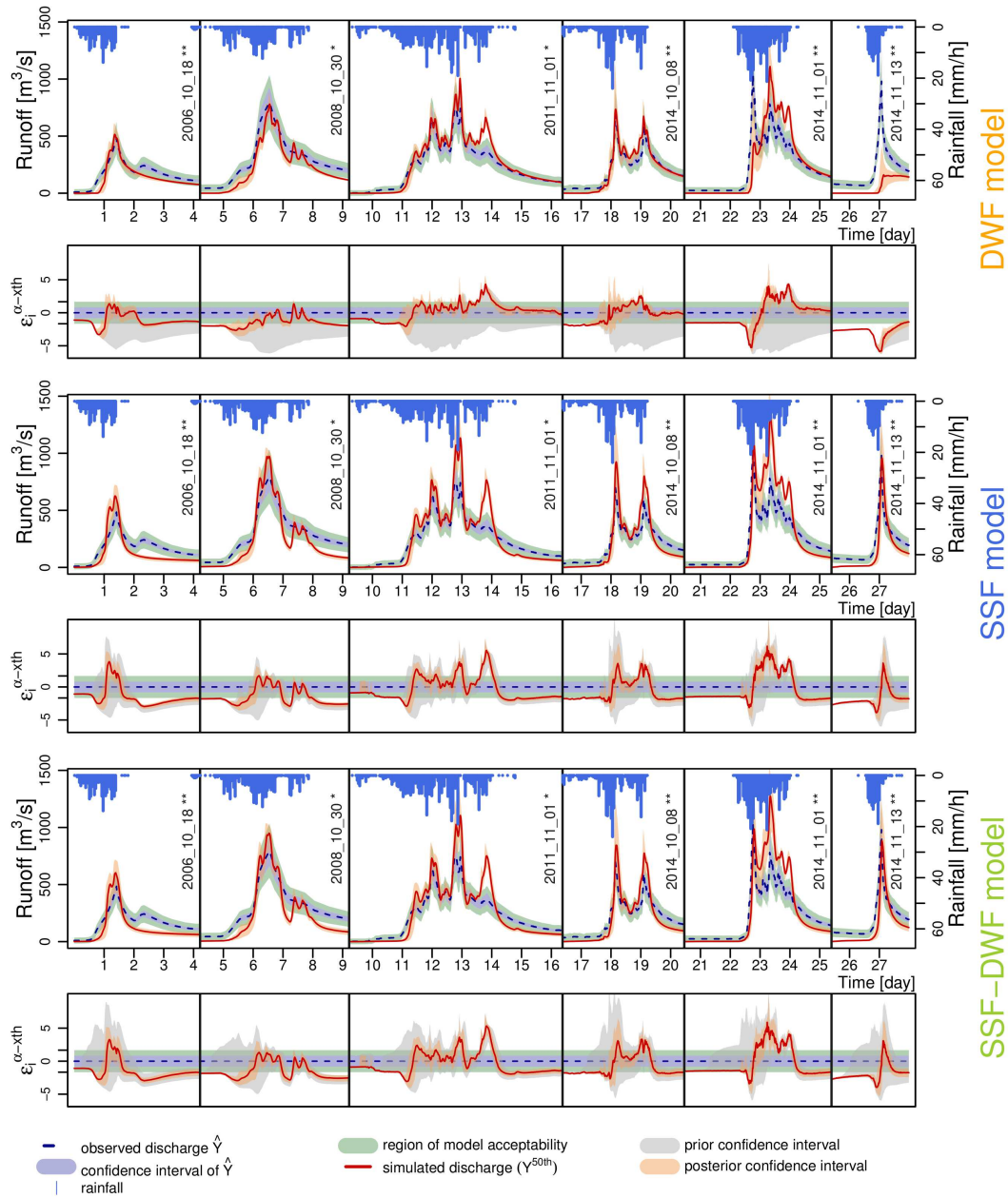
Figure 10. Calibration of the three models for the Ardèche catchment at Ucel, ♯1b). The results of the simulation of five flood hydrographs, and the inherent modelling errors (equation 10) for each model (top: DWF; centre: SSF; bottom: SSF-DWF). The median simulation and the posterior confidence interval are shown, respectively, in red and salmon. The confidence intervals of the measured flows and the acceptability zone are shown, respectively, in green and blue. The a priori confidence interval for each model (i.e. with no calibration) are shown in grey. (∗): event of calibration; (∗∗): event of validation.

Representing the soil column with either one compartment (the DWF model) or two compartments (SSF or SSF-DWF models) leads to distinct a priori confidence interval of modelling errors (grey). The DWF model constrains the simulated flows at the beginning of the event, before the onset of precipitation, because the width of the confidence interval of the modelling errors is low at that point. More specifically, it tends to underestimate the initialisation discharges because the variation interval of the errors over this period is predominantly negative. This may explain this model's relative difficulty in reproducing the onset of floods, since the calibration of the parameters did not allow the acceptability zone on this part of the hydrograph to be reached. A resulting interpretation applicable to the catchment sets is that good results in modelling the rising flood waters with the DWF model means that the observed rising flow is relatively slow and could be reached in spite of the restrictive modelling structure (as example ♯3a, ♯3b).

Likewise, it can be noted that the one-compartment structure (i.e. the DWF model) allows flexibility in the modelling of high discharges and flood recessions, because the confidence interval of the modelling errors is quite large over these periods in the hydrograph. However, it also led to the underestimation of high discharges and flood recessions. In fact, the prior modelling error interval (in grey color) has a negative bias with respect to the acceptability zone. The calibration finally allows the simulations to be selected, at the intersection of the acceptability zones and a priori confidence in modelling errors. This generally corresponds to the calibration of a low-depth altered rock D_{WB}, in order to make the model more sensitive to soil saturation and more responsive, via the generation of early runoff. From that resulted low D_{WB}, the simulated water storage capacity is limited, which might explains the inadequacy of the DWF model for catchment with small runoff coefficients (♯2, table 2).

Conversely, the two-compartment structure (the SSF and SSF-DWF models) offers flexibility in modelling the beginning of events, flood warnings and high discharges, but the ability to model flood recessions is more constrained. SSF and SSF-DWF models simulate fast flood recession in comparison to the DWF model, suggesting that good results in modelling the flood recession with the SSF model might be interpreted as fast return to normal or low discharge are observed on the related catchments (as example, ♯2, ♯4).

In the SSF and SSF-DWF models, the addition of a flux calibration parameter in the subsoil horizons, not surprisingly, leads to wider variations in the a priori modelling errors. A surprising finding, however, is that the calibration of the lateral conductivity of the deep layer, C_{kdw}, seems to affect only the simulation at the beginning of the hydrographs (see the events of 01/11/2011 and 13/11/2014, figure 10), and has very little effect on flood recessions. The high similarities of the prior modelling intervals of the SSF and SSF-DWF models explain the similar performances of those models. In the same way, when there is improvement of the performance through the SSF-DWF, it concerns the early rising of the flood, as the detailed performances have already shown it, the SSF-DWF enabling high and early start of the flood events.

5.3 Analysis of relevance of the internal hydrological processes simulated

5.3.1 Characterisation of the hydrological processes simulated

The proportional volumes of the water making up the hydrographs, that arise from the three main simulated paths - on the surface, through the top or through the deep layer of the soil - were calculated. Figure 11 shows the simulated runoff contribution, i.e. the water which has not passed through the soil at any point. The contributions of these surface flows on the whole of the hydrograph (Figure 11, left) and those that support high discharges (Figure 11, right) are distinguished. Note that the other contributions are not detailed, being correlated to the runoff assessment, and therefore leading to a similar analysis.

The runoff contribution simulated by the DWF model discredits even more that model for representing the hydrological behaviour of the Gardon (♯2) and Salz (♯4) catchments. Really high proportion of runoff contribution over the entire hydrograph were simulated, ranging from 40 to 98 %. In contrast, the few experimental measurements made on the Gardon (Bouvier et al., 2017; Braud et al., 2016a) provide evidence on the proportions of new water - which might be seen as an upper bound for runoff contribution volume - ranging from 20 to 40 % of the volumes in the hydrograph. The SSF and SSF-DWF model conversely gave more reasonable runoff contribution, although remaining high, ranging from 19 % and 62 %.
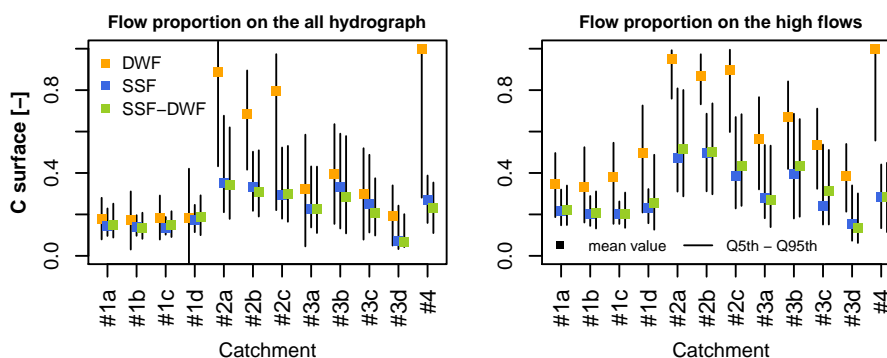
Figure 11. Proportion of surface runoff in the flows at the outlet. Left: The proportion over the whole hydrograph; right: the proportion at high discharges (Observed flow greater than 0.25 times the maximum flow during the event).

The assessment of the flow contributions through the most suitable model's simulations for each catchment, revealed in section 5.1 are consistant with the catchment set's diversity. Considering the DWF model for the Ardèche catchment and the SSF and SSF-DWF models for the Gardons catchment, the runoff contributions to the high flows of the hydrographs were slightly lower on the three downstream Ardèche catchments (♯1a, ♯1b, ♯1c, with runoff contributions included between 17 and 57 %) compared to the runoff contributions on the Gardon catchment (♯2a, ♯2b, ♯2c) and on the upstream part of the Ardèche (♯1d, with runoff contributions between 20 and 78 %). It correlates the properties of the catchments and the rainfall forcing, the first catchment subset (♯1a, ♯1b, ♯1c) having deeper soil cover, with more permeable soil texture (see table 1), and being forced by rainfall with lower maximal intensities (see table 2) than the second one (♯2a, ♯2b, ♯2c).

On the downstream catchments of the Hérault (♯3a, ♯3b), the variation intervals of the surface flows estimated by the three models overlap. It may explain why the three models can achieve good reproductions of the hydrological signal : the calibration step makes possible, from that integrated point of view, to obtain an analogous distribution of the flows processes.

Notwithstanding the uncertainty related to the model's choice when any model has been identified most suitable through the performances, the largest uncertainties are related to the parameterization of the models, a consequence of the equifinality of the solutions when calibrating a hydrological model against the solely criterion of the reproduction of the hydrological signal. While, in terms of plausibility, several sets of parameters may be equivalent, even for the same model, these sets of parameters are likely to lead to different hydrological functioning.

5.3.2 Detailed study of four plausible simulations on the Hérault watershed at Saint Laurent-le-Minier

Spatialised and integrated changes in moisture levels and flow velocities generated within the catchments have been considered in order to give new details on the different impacts of the models's structure, but also to explain the resulting uncertainty when assessing the flow processes distribution. Next, are described the results of four simulations, equally considered to be plausible according to the DEC criterion, obtained from the DWF and SSF models (2 simulations by model, see Table 3). The Hérault catchment at Saint-Laurent-le-Minier (♯3b) has been considered because of the equivalence of the models to represent that catchment. Figure 12 compares the changes over time in the state of soil saturation and the different simulated flow velocities of the four "model + parameter set" configurations (Table 3). Figure 13 compares the spatial distributions of these variables, at a given moment.

Table 3. Realistic models and parameter sets for the Hérault catchment at Saint-Laurent-le-Minier (♯3b). C_{soil}: the contribution to the hydrograph of flows passing through the soil ; C_{kdw}/C^*_{kss} : the value of the parameter C_{kdw} for model DWF (Equation 1) or the value of the parameter C^*_{kss} for the model SSF (Equation 2).

ID	NSE	$D_{WB}[m]$	$C_k[-]$	$C_{kdw}/C^*_{kss}[-]$	$n_r[-]$	$n_p[-]$	$C_{soil}[\%]$
DWF1	0.82	0.15	17.3	8711	19.6	19.11	61
DWF2	0.84	0.11	2.34	4416	19.16	7.63	39
SSF1	0.89	0.40	15.81	45284	15.96	5.86	68
SSF2	0.89	0.34	2.08	22543	14.06	6.42	53

In terms of hydrographs, quite logically given the similar likelihood scores, the simulations differed very little. The notable difference in the generation of hydrographs is the contribution of the different simulated flowpaths. The proportions of water passing through the soil column (via sub or surface-soil horizons) were highly variable: with an average of 39 % for the DWF2 model, 53 % for the SSF2 model, 61 % for the DWF1 model and 68 % for the SSF1 model (Table 3). This is both due: i) to the structural choices (DWF and SSF) which involved a different saturation dynamics and the incorporation of different types of flow, and ii) to the choice of the parameters which involved flow velocities of different orders of magnitude.

The choice of a model's structure (DWF and SSF) implied differences in soil moisture spatial distribution and dynamic, which in turn impacted the timing of the flow processes. With the DWF structure, the soil moisture distribution is sensitive to the soil depth spatial distribution, as a result of the decrease in the simulated intra-soil flows as a function of water table height (cf. Section 3.2, Equation 1). Consequently, the DWF model produced a greater contrast in saturation levels between different

5 areas of the catchment (Figure 13, a, d). With the SSF model, the overall catchment saturation level were more related to the topography: saturated cells were observed close to the drainage network, and, conversely, lower water content in the upper reaches of the catchments. In fact, for the SSF model, rainfall forcing is mainly involved in saturation of the upper soil layer (the dashed lines in Figure 12-b), which reacts very rapidly to precipitation.

As a result of the contrasted soil moisture dynamic, the flow velocities simulated in the soil showed consecutive differences.

10 At the start of flooding, the SSF structure resulted in an early increase in flow velocities due to a higher and more homogeneous saturation level of the upper soil layer (Figure 12-c). Conversely, with the DWF model that simulated a more heterogeneous spatial saturation of the catchment, the simulated velocities increase was delayed, and the maximum values reached was two to four times lower.

The dynamic in the drainage network as well were impacted by the choice of the structure. The runoff velocities average

15 reflected the earlier inlet of the subsurface flow processes through the fast saturation of the upper compartment with the SSF model (Figure 12- e). The DWF model yields a more contrasting variation in the runoff velocities in the drainage network, mirroring variations in soil saturation levels.

The choice of parameters mainly implied different ranges of value for the velocities simulated, in the soil, on the surface of the hillslope and in the drainage network. The calibration of the C_{kss} and C_{kdw} parameters controlled the order of magnitude

20 in the subsurface velocities (Table 3, and Figure 13, b, e, h, k). As well, the calibrated C_k (infiltration capacity control) and D_{WB} (depth of the subsoil horizon) parameters controlled the infiltration, leading to more or less high number of cells with saturation excess or infiltration capacity reached (Figure 13, c, f, i, l), and consequently to more or less high proportion of runoff over the hillslope (Figure 12, d).

Several order of magnitude were actually allowed while respecting the calibration objective because transit time of the

25 different water pathways compensate each other. As foreshadowed by those four configurations, the selection of plausible parameter sets for any model in any catchment shows: i) a positive correlation between the parameters C_k, and n_r and n_p, suggesting the necessity of slowing down flows in the drainage network when a larger proportion of runoff from the catchments is simulated (i.e low C_k would imply low n_r and n_p) and vice versa; ii) a positive correlation between C_k and C_{kss}, C_{kdw} parameters, suggesting the necessity of accelerating the intra-soil flows when high infiltration rate is allowed and consequently,

30 when larger proportion of subsurface flow is simulated. Thus, a degree of compensation occurs in the simulated transfer times between the various water paths, from the hillslopes to the drainage network, and from the drainage network towards the outlet.
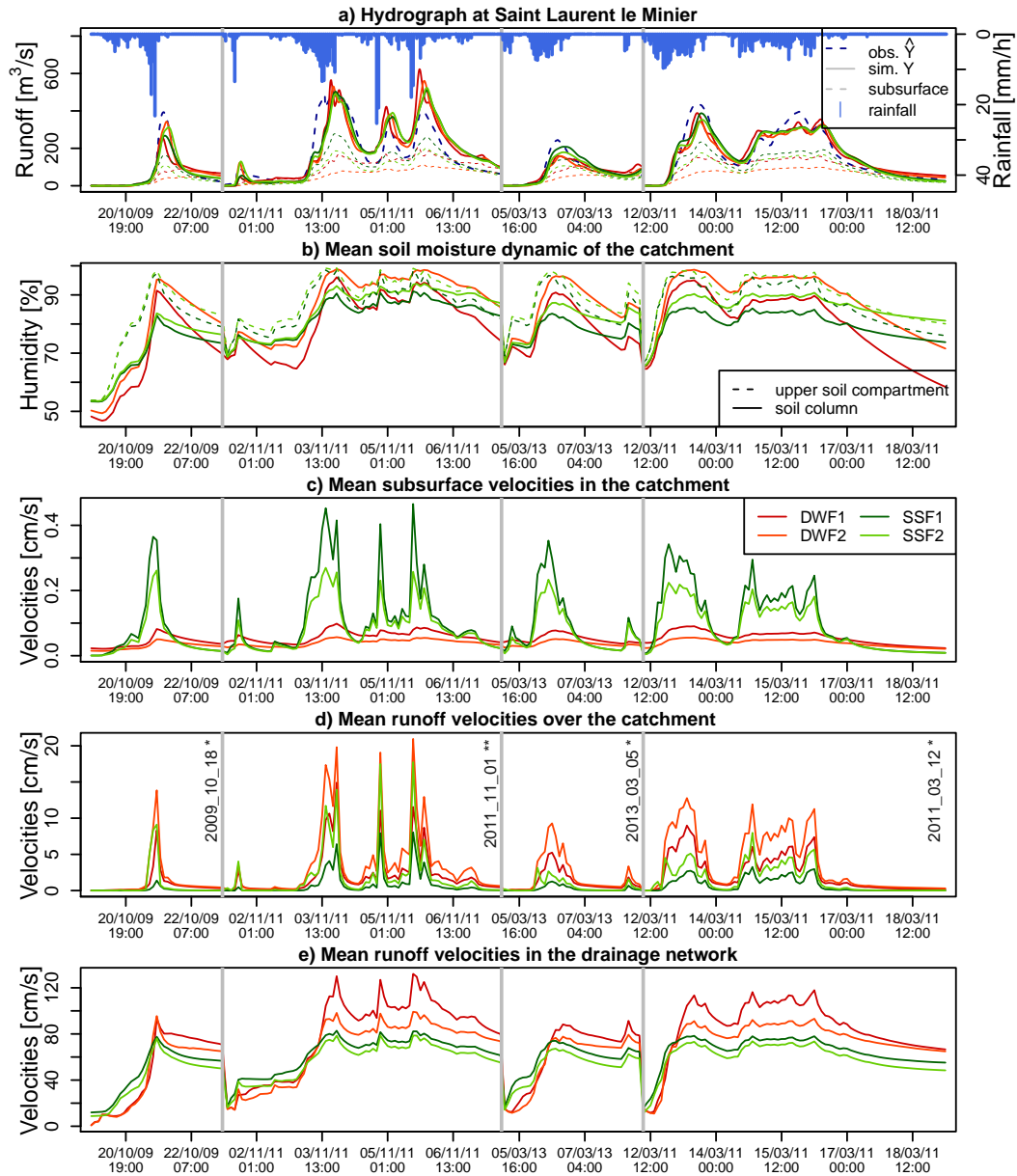
Figure 12. Comparison of the results of four equally plausible simulations on the Hérault at Saint Laurent le Minier (Table 3). a) Flood hydrographs (solid lines) and outlet flows transiting via the soil (dashed lines). b) Evolution in the overall moisture content of the soil column. c) Evolution in simulated mean velocities in the subsoil horizon (DWF model) and in the upper part of the soil column (SSF model). d) Average runoff velocities on the hillslopes. e) Average runoff velocities in the drainage network. (∗): event of calibration; (∗∗): event of validation.
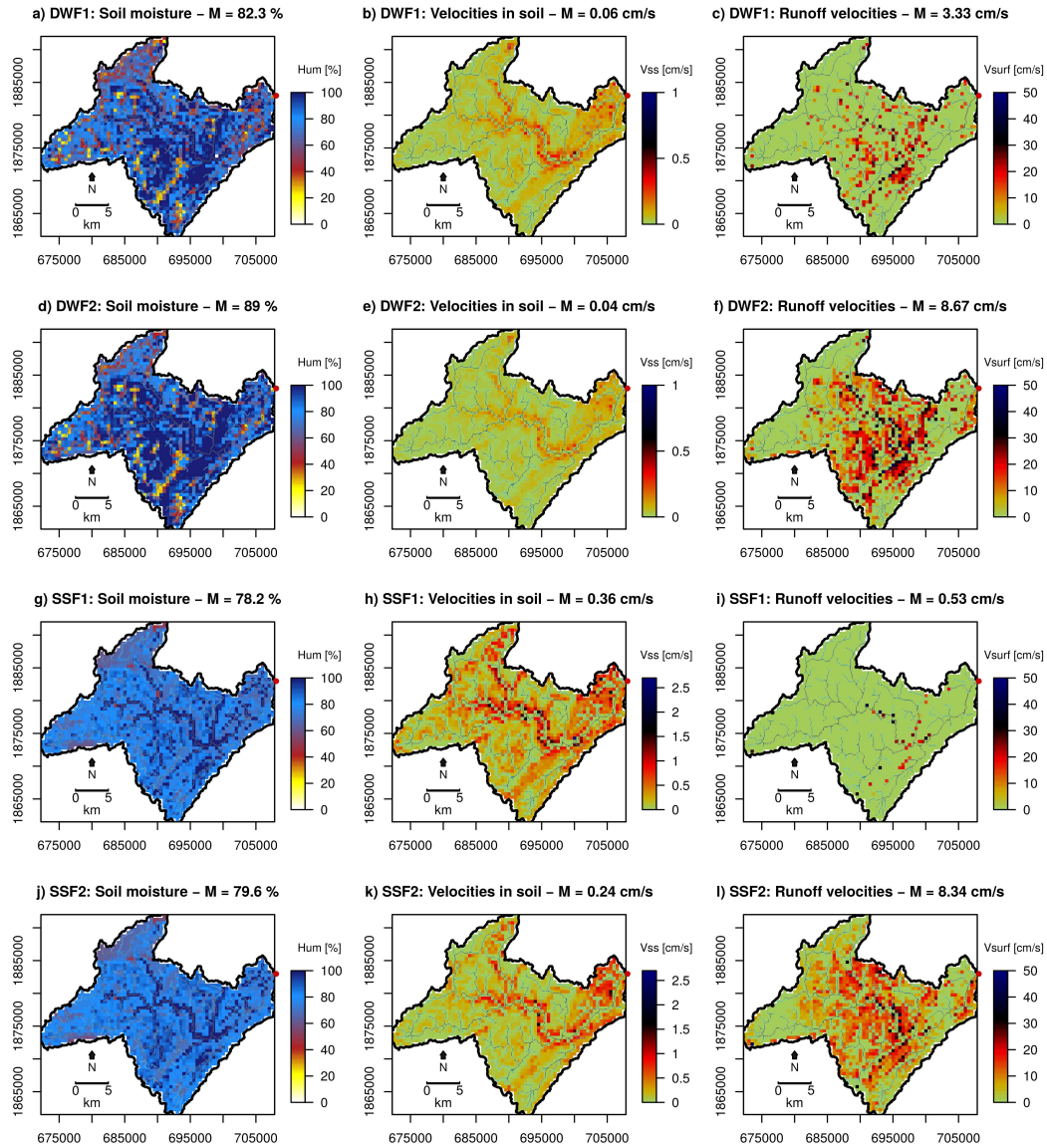
Figure 13. Spatialised outputs for a given moment during the event of 18/10/2009 (during the development of the flood, where $Q = 74\ m^3.s^{-1}$): a-d-g-j) soil moisture conditions simulated, respectively, by the configurations DWF1, DWF2, SSF1, SSF2; b-e-h-k) discharges in the soil simulated, respectively, by the configurations DWF1, DWF2, SSF1, SSF2 (N.B: different colour scheme); c-f-i-l) surface flow velocities simulated, respectively, by the configurations DWF1, DWF2, SSF1, SSF2.

6 Discussion

6.1 On the hydrological functioning of the catchments studied

The benchmark of the models's performance on the catchment set leads to reveal 4 subsets, suggesting 4 distinct hydrological behaviours. According to the modelling assumptions (Section 5.1), the resulting errors in simulating the different stages of the hydrographs (Section 5.2) and the catchment properties (Section 2.1), the hydrological behaviour of the catchment can be interpreted subset by subset as follow:

– The SSF and SSF-DWF models showed better overall performance (with no particular pattern) in the first subset : the Gardon (♯2) and Salz (♯4) catchments. This suggests, on the one hand, rapid catchment reactivity with fast rising flood waters as well as fast flood recession, and on the other, formation of the flows in the soil through local saturation tied to the climate forcing. Although the models exhibited similar performances, the contrasting physiographic characteristics of these catchments suggest that there are different explanations for this better fit of the SSF-DWF model. On the Gardon, the very high intensities of the observed events (Table 2) and/or the low soil depth (Table 1) may explain the limitations on vertical infiltration due to the properties of the soil and/or geological bedrock. As a result, the rapid formation of a saturated zone at the top of the soil column, favours runoff and subsurface flux by activating preferential paths in the soil. This interpretation is in agreement with the field studies achieved on a shist upstream sub-catchment of the Gardon, the shist substratum being the predominantly geology of the Gardon catchment (see section 2.1, Ayral et al. (2005); Maréchal et al. (2009, 2013)). On the Salz (♯4), the soil is deeper and the precipitation intensities lower. On the other hand, the geological bedrock composed of marls, sandstone and limestone is assumed to have low permeability and the soil is less conductive due to its predominantly silt-loam texture. As a result, despite the lower forcing intensities, the surface soil can reach saturation, which might explain why the SSF model offers the best fit.

– The considerable hydrological responses, in terms of volumes, on the Ardèche second subset, appear to be linked to hydrological activity at depth, including that taking place during intense floods, as suggested by the better fit of the DWF model. Here, in particular, the model gave a better representation of the relatively slow and uniform hydrological recessions from one event to the next, reflecting an aquifer-type flow whose discharge properties are governed by the properties of the catchment bedrock only. This interpretation is enforced by the field studies achieved this time in a granite experimental sub-catchment localised in the downstream part of the Gardon (Section 2.1, Ayral et al. (2005); Maréchal et al. (2009, 2013)), the Ardèche catchment being granitic. The somewhat delayed flood timing that the structure of the one-compartment model imposed seems to indicate that there are more rapid flows at the beginning of an event, which this model structure is not able to represent. A plausible explanation is the default calibration, which uses a uniform depth of active subsoil horizons, D_{WB}, during a flood. This might mask the appearance of local saturation zones, and the subsequent runoff due to shallow soil and discontinuities in the permeable base layer (for example, in the downstream sedimentary layers, where infiltration tests have shown the appearance of runoff, see Section 2.1). In contrast, the SSF and SSF-DWF models did not display this weakness because the varying nature of soil depths (D_{BDsol},

which determines the depth of the upper compartment) allowed the rapid development of flows via preferential paths in the soil blocks, thus enabling the simulation of such local dynamics.

- The third subset consists in the downstream part of the Hérault (♯3a, ♯3b). The models's performances contrasted with the Hérault catchment heads (♯3c, ♯3d), suggesting a hydrological behaviours related to the contrasted geological properties. An interpretation of hydrological functioning is nevertheless not possible, given the similar overall results offered by the models and that no distinctions can be drawn according to other criteria.

- The last subset consists in the catchment heads (♯1d, ♯3c, and ♯3d). We observed superior performances from the DWF and SSF-DWF models, with a particular improvement in the forecasting of rising flood waters when using the SSF-DWF model. This suggests the presence of several types of flow in the soil with strong support from flows at depth, which corroborates the high mean inter-annual discharges associated with these catchments, and additionally the presence of rapidly formed flows, providing a good simulation of the rising flood waters. The fact that the model SSF-DWF, that precisely alleged to represent the simultaneous setting up of shallow and deep subsurface flows, did not completely outperformed the 2 others models is interesting. From our point of view, it points out the limit of their artificial implementation, using a threshold infiltration from the top layer to the deep one. In the reality, the simultaneous setup of the two fluxes more probably refers to the spatial heterogeneity of the soil properties, specially in the head watersheds - within a catchment cell (2.5 km^2), that either might allow deep infiltration or fast top soil saturation.

6.2 Overcoming the remaining uncertainty

The submitted multi-hypothesis test classically faced to the equifinality issue related to the parameter uncertainty, and highlighted the one related to the model's structure. The comparative and detailed description of the simulation revealed the model's structure controls, and thus gyving almost direct guidelines to overcome the equifinality issue.

One of the objectives of the study, the assessment of the flow contributions to the hydrographs, is not completely reached, mainly because of the parameter uncertainty (Section 5.3.1). The benchmark of modeling configurations, scanning the different simulated processes (Section 5.3.2), showed how the calibration lead to that uncertainty. The wide range of values, that has been allowed through the parameter setup, enabled counterbalancing effects between the internal velocities simulated. As a direct consequence, variable flow contributions could be simulated, while finally producing similarly likely hydrographs. This points out direct further objectives for improving and better restraining the calibration of the models. While several ranges of value for the internal flow velocities have been simulated, a reasonable restriction based on the velocity likelihood could be foreseen. This further perspective should also shift experimental studies toward a better assessment of the water transit time along the different pathways at hillslope scale, either using direct methods such the water isotope tracing (Tetzlaff et al., 2018), or developing imaginative indirect ones such as the diatom tracing (Pfister et al., 2017b), or even taking advantage of suspended particles and water turbidity measurements.

The equifinality of the models on several catchments mostly points out the limit of the assessment of hydrological model through the solely use of the hydrological discharge time series at the outlet. Leading up toward a multi-criteria calibration,

the detailed comparative description detailed discrepancies of the simulations and thus guidelines for integrating judicious information to differentiate the models's adequacy. The distinguished saturation spatial patterns generated by the DWF and SSF structures suggest the relevancy of the soil moisture distribution assessment along hillslopes and soil heterogeneities, as the first structure implied a soil moisture dynamic related to local soil properties, while the latter implied a soil moisture pattern

5 related to the distance to the drainage network. In addition, the description of the a priori modelling errors (Section 5.3.2) points the way towards an optimal consideration of the early rising limb and the flood recession, when calibrating the models over the discharge time series. Indeed, the model's structure appeared to mostly control these particular stages, specially the simulated timing of the first stage and the simulated dynamic of the latter one. A consequently need of accurate discharge benchmarks, particularly during these stages, should direct further the river monitoring toward high temporal resolution of the river level,

10 the rising and receding flood stages being short periods during flash floods, and efforts for reducing the uncertainty of the rating curve at low and moderate flow, rather than getting extreme discharge measurements, including as example hysteresis of the discharge curves (Le Coz et al., 2014).

7 Conclusions

7.1 Summary of the study's objectives and methodology

15 The objective of the study was to improve our understanding of flash flooding on the French Mediterranean Arc. In particular, attention was paid to the dynamics of soil saturation in catchments during these events, and their possible relationship with the physiographic diversity encountered. The method used consisted in considering hydrological models as a diagnostic tool to test hypotheses about the functioning of catchments.

Based on the structure of the MARINE model - a hydrological model with a physical and distributed basis - three types of

20 dynamic of soil saturation were postulated and tested. In the first case (the DWF model), we assumed an aquifer dynamic, with infiltration at depth, and the generation of strong base support, according to the volume of infiltrated water; in the second case (the SSF model), it was the activation of preferential paths at the soil/altered rock interface that generated the majority of the flows passing through the soil, with the lower part of the soil column serving only as a storage reservoir; and in the third case (the SSF-DWF model), there was flow generation via both the activation of preferential pathways, initially by saturation of

25 the top of the soil column, and a significant increase in the base flux via the subsequent infiltration of water present at deeper levels.

The same calibration strategy was used for the three models on a set of 12 catchments which are representative of the diverse characteristics of the Mediterranean Arc. Whether a model offers a good fit was evaluated on the basis of: scores representing overall, or partial model performance in terms of simulating the hydrographs; the proportions of the processes simulated; and

30 the timing and form of flood recession.

7.2   Conclusions on our understanding of the processes involved

The specific use of a multi-hypothesis framework supports a clear comparison of the hydrological behaviours, which in turn has provided the main basis of the insights of this study. From the application and validation of the three hydrological models, the 12 catchments of the study could be classified into four categories: i) the Gardon and Salz catchments, for which the SSF
5   model is better suited to reproducing the hydrological signal. For these catchments, this highlights the importance of local and surface soil dynamics in the generation of flows, especially at the beginning of a flood; (ii) the Ardèche catchments, for which the DWF model most accurately reproduce the observed flows. This indicates more regular and integrated hydrological functioning at the catchment level, with the flows generated being directly related to the moisture history and rainfall volumes; (iii) the Hérault catchments at Valleraugue and La Terrisse, and the Ardèche catchment at Meyras, which have steep-sloped
10   catchment heads, where the SSF-DWF model stands out, suggesting both sustained and significant hydrological activity at depth during flash floods, and surface activity in the establishment of early flows at the beginning of events; (iv) the Hérault catchments at Laroque and Saint-Laurent-le-Minier, for which no model shows any significant difference.

The modelling results help to draw consistent assumptions on hydrological behaviours, which corroborate when available, the knowledge and observations on the overall hydrological functioning of the catchments, or the experimental estimations of
15   flow processes. The results suggest that the behaviour of catchments under extreme forcing is a continuation of the hydrological functioning normally encountered.

The assessment of the flow processes in the catchments remains uncertain, owing to the equifinality issue. The analysis of the internal processes enabled to explain the compensation effects between the simulated flow pathways, and the resulting uncertainty of the calibrated parameter sets, on the only basis of the discharge time series. In addition, other detailed descriptions
20   of the simulations, such as the spatial dynamic of the soil moisture distribution or the modelling errors, highlighted the actual impacts of the model's assumptions on the simulations. The revealed discrepancies between models - namely the range of values of the flow velocities, the spatial pattern of the soil moisture, the early rising limb timing and the recession rate of the hydrographs, finally defined pertinent milestones for improving the assessment of the model's adequacy.

Competing interests.   The authors declare that they have no conflict of interest.

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