- 1 Economic impacts of drought risks for water utilities through
- 2 Severity-Duration-Frequency assessment under climate change
- **scenarios**
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- Abstract
- Climate change and increasing water demands show the need for implementing planning
- strategies for financial sustainability of sectors, which are highly dependent on water resources,
- such as water utility companies. The financial vulnerability of these companies increases
- considering water supply growth and low availability scenarios, resulting in less profits or
- economic bankrupt. Generally, methods to estimate financial impacts caused by drought are not
- as numerous and clear as those for floods due to the complex characteristics of the phenomenon.
- Therefore, we propose a new assessment to estimate the business interruption cost considering
- the uncertainties in the climate and urban demand projections in the medium and long term.
- The methodology integrates the semi-distributed hydrological simulation procedures linked to
- the Water Evaluation and Planning system (WEAP) under radiative climate forcing scenarios
- 23 RCP 4.5 and 8.5 from the regional climate model outputs Eta-INPE/MIROC5 and HadGEM-
- 24 ES (RCM). The approach continues with the hydrological drought assessment "Severity-
- 25 Duration-Frequency" (SDF), based on stationary and non-stationary water demand assumptions
- to establish the method's threshold levels. Likewise, the methodology defines a water tariff
- 27 price delimited by the drought duration and the system's robustness analysis to determine
- revenue loss scenarios in the water utility, through planning periods: 2007-2040, 2041-2070,
- and 2071-2099. As a case study, the approach is applied to the Cantareira Water Supply System
- in the São Paulo Metropolitan Region (SPMR), the main water supply source for about 11
- million people. The results show that the water-cost outputs based on Eta-MIROC5 present
- higher revenue losses in the company than those based on HadGEM-ES. Meanwhile, the
- relationship between RCP scenarios 4.5 and 8.5 showed lower variability compared to the
- analyzed climate-with-water demand scenarios. However, the Non-stationary demand (NSD)

- trend imposed larger differences in the drought resilience financial gap, suggesting that the
- demand-related uncertainty would be far greater than that associated with climate sensitivity.
- 37 **Key Words:** Climate change, Severity-Duration-Frequency assessment, Water utility revenue
- 38 losses, Hydrological droughts

1. Introduction

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- 40 Climate change, population growth and uncontrolled urban/industrial development make
- society more dependent on water (Montanari et al., 2013). The complex interaction between
- 42 meteorological, terrestrial and socio-economic water distribution schemes are the main factors
- that define droughts (Lloyd-hughes, 2013; Van Loon et al., 2016b, 2016a; Wada et al., 2013).
- Therefore, in order to address a potential drought scenario in the future with demand as a
- determinant anthropogenic factor, society is required to rethink the way forward, mainly to
- 46 reduce its vulnerability by regulating its demand (Falkenmark and Lannerstad, 2004;
- Kunreuther et al., 2013; Wanders and Wada, 2015).
- Apparently, droughts may not be as apparent as floods, but have proven to be one of the most
- complex risks due to their slow development, strong and long lasting impacts as well as broad
- 50 geographic coverage (Bressers and Bressers, 2016; Frick et al., 1990a; Smakhtin and Schipper,
- 51 2008; Van Lanen et al., 2013). Furthermore, various studies have shown that more severe and
- 52 prolonged droughts are expected for the future, leading to greater economic consequences,
- environmental degradation and loss of human lives (Asadieh and Krakauer, 2017; Balbus,
- 2017; Berman et al., 2013; Freire-González et al., 2017; Prudhomme et al., 2014; Shi et al.,
- 55 2015; Stahl et al., 2016; Touma et al., 2015). Therefore, it is essential to create adequate risk
- 56 perception, aiming to reduce the risks, mitigate the impacts and build a more resilient-drought
- community (Bachmair et al., 2016; Mishra and Singh, 2010; Nam et al., 2015).
- 58 The most visible impacts on the urban water supply are strongly related to hydrological
- droughts and not directly to meteorological droughts (Bachmair et al., 2016; Van Lanen et al.,
- 60 2016). A hydrological drought is defined as a negative anomaly in surface and subsurface water
- levels (Mishra and Singh, 2010; Van Loon, 2015; Wanders et al., 2017). These negative
- anomalies on the surface, related to an excessive level of water demand can cause water systems
- to collapse (Mehran et al., 2015; Van Loon et al., 2016b; Wanders and Wada, 2015). Therefore,
- 64 in this study we address hydrological droughts as the main driver of business interruption in the
- water utility company, specifically when urban water demand exceeds the supply system offer
- 66 (Bressers and Bressers, 2016; Frick et al., 1990a, 1990b).

The definitions of drought losses (or drought costs) are not as clear as those regarding floods or 67 68 methods for estimating drought costs, although diverse, not as numerous as floods. (Freire-González et al., 2017; Logar and van den Bergh, 2013; Meyer et al., 2013). In a comprehensive 69 70 review by Logar and van der Bergh (2012), the authors suggest a division of drought costs as direct, indirect and non-market costs. Furthermore, Meyer et al. (2013) suggest extra categories, 71 differentiating Business interruption costs as primary tangible costs, although not configured as 72 "due to direct physical contact". Despite a diversified range of methods presented by Meyer et 73 al. (2013) and Logar and van der Bergh (2012), several are either: for non-tangible or indirect 74 75 methods, specific for the agricultural sector or economy wide oriented (i.e. fit to a broader scale application) which in our case would likely incur in less precise results. Regarding the 76 77 allocation of water companies by reduced water availability, several approaches seem to be adequate, such as market valuation techniques or ex-post evaluations, that is, comparing 78 79 changes in GDP or changes in price between affected and unaffected years In Brazil, from 2013 to 2015, the population of the Sao Paulo Metropolitan Region (SPMR) 80 81 experienced the most acute water crisis in its history (Coutinho et al., 2015; Nobre and Marengo, 2016; Taffarello et al., 2016). According to the Federation of Industries of the State 82 of Sao Paulo (FIESP), it was estimated that 60,000 households, business and industrial sectors, 83 which represent almost 60% of the state's industrial GDP, were affected by a lack of water 84 (Marengo et al., 2015). Likewise, during 2014 and 2015, the Sao Paulo State Water Utility 85 Company (SABESP) recorded an average annual liquid net income reduction of approximately 86 63% compared to 2013, leading to a major financial crisis in the company (GESP, 2016; 87

SABESP, 2017a). To analyze the water utility drought impacts, several control strategies are usually implemented as price-based policies that seek to change the user's consumption pattern 89

based on economic penalties or incentives (Buurman et al., 2017; Millerd, 1984; Rossi and

Cancelliere, 2013). However, the implementation of these strategies entails a great complexity

in their planning and high risks of economic impacts for the water company (SABESP, 2015;

Watts et al., 2012).

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To deal with global change, understanding the interplay between multiple drivers of risks and socioeconomic development is increasingly required to inform effective actions to manage new drought risks and pursue sustainable development. However, as long as there are no systematic and detailed studies on the assessment drought impacts on the regional economy, shaping financial planning policies is a complex and uncertain task that must be reinforced. Therefore, based on the drought Severity-Duration-Frequency characterization, we explore the water

- utility company business interruption cost assessment by integrating an analysis framework 100 driven by climate change, water-demand scenarios and the supply system robustness. This 101 paper describes an academic exercise to manage drought financial planning, running the Eta-102 INPE (RCM) outputs, through a semi-distributed hydrological model of the water supply 103
- system developed using WEAP. 104
- The sections of this article outline interconnected methods and criteria, explained as follows. 105
- 106 In Section 2, the text describes the study area and water crisis contextualization. Section 3
- 107 outlines the methodological approach starting with the hydrological modeling, characterization
- 108 of the droughts using the threshold level method, the formulation of the SDF curves of the
- system and subsequently, the climatic, hydrological and economic aspects of the methodology. 109
- 110 In Section 4, the results and discussions are shown as financial drought planning scenarios.
- Finally, in Section 5, the conclusions and recommendations are presented regarding the 111
- 112 proposed approach.

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2. Study area and water crisis contextualization.

- The Cantareira Water Supply System, hereafter referred to as the Cantareira System, is located 114 in South-East Brazil between the states of Sao Paulo and Minas Gerais. The rainy season in the 115 Cantareira System generally begins at the end of September and ends in March. In this period, 116 on average 72% of the rainfall in the region is accumulated (Marengo et al., 2015). In 117 hydrological terms, 2265 km² of drainage area into the system historically generates an annual 118 mean tributary discharge of 38.74 m³/s. Structurally, the system consists of the damming and 119 interconnection of five basins with a useful total storage volume of 988.8 hm³, arranged to 120 121 transfer water from the Piracicaba River Basin to the Upper Tietê Basin (Fig. 1). As a result, the system had been configured to supply water to about 11 million people in the SPMR before 122
- 123 the last acute water crisis in 2013-15 (De Andrade, 2016; Marengo et al., 2015; Nobre et al., 2016; Nobre and Marengo, 2016; PCJ/Comitês, 2016, 2006).

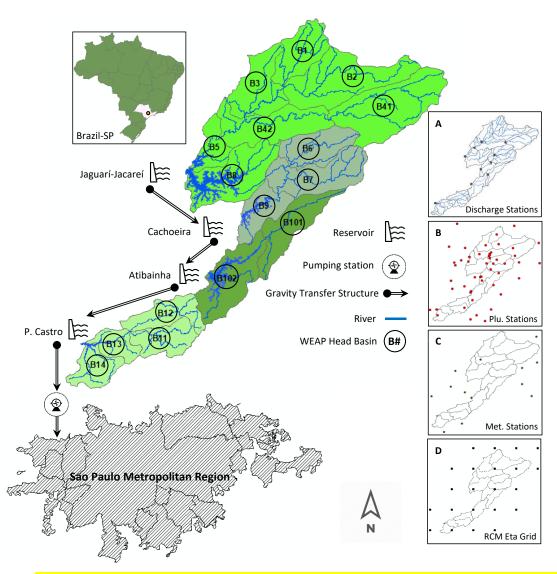


Figure 1. System structure composition and catchment areas "Cantareira System": Jaguarí-Jacareí, Cachoeira, Atibainha and Paiva Castro watersheds. Panel A: Discharge gauge stations; Panel B: rainfall gauge stations; Panel C: Meteorological gauge stations and Panel D: Centroid of the Eta-INPE grid.

Previously in the SPMR, some severe water shortages were recorded. The first one was during 1953-1954, then from 1962-1963 (Nobre et al., 2016), which apparently motivated the construction of the Cantareira System and the latest one was from 2000-2001 (Cavalcanti and Kousky, 2001). Thus, the system, designed to supply the increasing demand for water in the SPMR, began its partial operation in 1974 and its construction was completed in 1981 with a 30-year permit to transfer up to 35 m³/s according to a periodic technical assessment (Mohor and Mendiondo, 2017). The Cantareira System is currently administered by SABESP, which mainly operates the water network in the SPRM, and the Government of Sao Paulo state is its main shareholder.

However, various studies have identified changes in rainfall trends and temperature extremes, showing an increase in the intensity and frequency of days with heavy rainfall and longer duration of hot dry periods between rainfall events in South America and southeast Brazil (Chou et al., 2014b; Dufek and Ambrizzi, 2008; Haylock et al., 2006; J. A. Marengo et al., 2009; Jose A. Marengo et al., 2009b, 2009a; Nobre et al., 2011; Zuffo, 2015). Although historically, the SPRM study area is not affected by droughts of the same order of Northeast Brazil, the SPRM is progressively becoming vulnerable to water shortages. Therefore, during the recent period of the acute crisis 2013-2015, SABESP undertook reactive measures to control the consumption in the SPMR, such as (Marengo et al., 2015): programmed water cut-offs; bonuses and penalties to reduce and increase consumption, respectively; extraordinary increases of water tariff prices; network pressure reduction; water use from the reservoirs' dead volume; social awareness campaigns to inform people about shortages; and water distributed by tankers in the most critical areas of the city to provide the Basic Water Requirement (BWR) for human needs. Nevertheless, according to SABESP, there is a slow system recovery, which enables the reestablishment of pre-crisis supply levels (SABESP, 2018a).

3. Methodology

The methodology was structured in three modules that are summarized in Figure 2. In the first module, the hydrological simulation was approached by the Water Evaluation and Planning tool (WEAP) (Yates et al., 2005a). The model was calibrated and validated based on the available historical hydrometeorological information (2004-2015) for the study area. Then, from the calibrated hydrological model and the RCM Eta-INPE historical period datasets, the base discharge scenarios were estimated.

In the second module, following the Threshold Level Method (TLM), the "threshold" had to be defined according to stationary and non-stationary assumptions of water demand in the SPMR. Afterwards by analyzing the duration series and extreme deficits through GEV (Generalized Extreme Value) distribution, the Severity-Duration-Frequency curves (SDF) were developed to calculated the intra-annual deficit (J. H. Sung and Chung, 2014). To complete the second module, the average water price and the Cantareira system robustness analysis is defined per each cubic meter of deficit (Mens et al., 2015), as a function of the supply warranty time during the hydrological drought events, to configure the baseline analysis scenarios.

The final module evaluates the Water Utility Company economic profit losses through the baselines scenarios, under the hydrological scenarios developed with the model WEAP, driven

by the Eta-INPE RCPs scenarios (2007-2040, 2041-2070, 2071-2099), previously processed by the TLM approach. It should be clarified that, for the analysis under the non-stationary assumption, the growth of water consumption is represented in each projection time step, that is, 2005-2040 corresponds to 31 m³/s, 2041-2070 corresponds to 38 m³/s and 2071-2099 corresponds to 43m³/s.

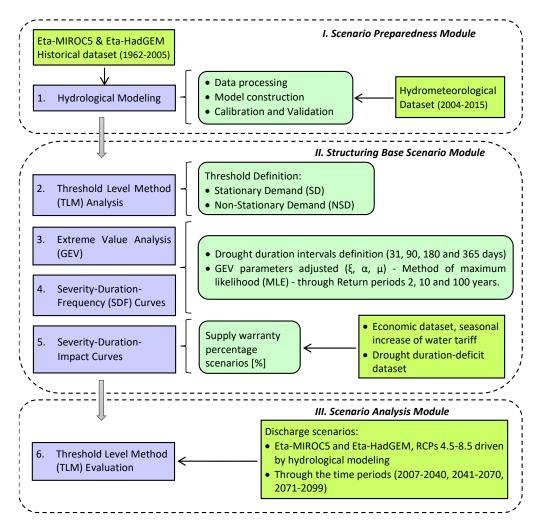


Figure 2. Methodology flowchart and main inputs.

The results, presented as water utility company revenue losses were developed from a set of potential scenarios involving climate uncertainty, human triggering factors and the prediction under extreme theory (Baumgärtner and Strunz, 2014; Wanders and Wada, 2015). The methodology sought to provide a planning water-security support analysis in areas highly dependent on surface water resources.

3.1. Hydrological projections

Currently the RCM Eta-INPE (Brazilian National Institute for Space Research) plays an important role in providing information for local impact studies in Brazil and other areas in

South America (Chou et al., 2014b). In order to assess the uncertainties of climate change 186 impacts, the simulation results of the Eta-INPE model were used in this paper. The model is 187 nested within the GCMs MIROC5 and HADGEM2-ES, forced by two greenhouse gas 188 concentration scenarios (RCPs) 8.5 and 4.5 [W/m²] used in AR5 (IPCC 5th Assessment 189 Report); with a horizontal grid size resolution of 20 km x 20 km and up to 38 vertical levels 190 through 30 years of time slices (periods) distributed as follows: 1961-2005 (as the baseline 191 period), 2007-2040, 2041-2070 and 2071-2099 (Chou et al., 2014a, 2014b; Prudhomme et al., 192 2014). The climate projections of the Eta-INPE model were used to drive the WEAP Rainfall 193 194 Runoff Model-soil moisture method (World Bank, 2017; Yates et al., 2005a). The WEAP, is 195 an integrated water resource planning tool used to develop and assess scenarios that explore 196 physical changes (natural or anthropogenic) and has been widely used in various basins throughout the world (Angarita et al., 2018; Bhave et al., 2014; Esteve et al., 2015; Foster and 197 198 Brozovic, 2018; Groves et al., 2008; Howells et al., 2013; Hund et al., 2018; Mousavi and Anzab, 2017; Psomas et al., 2016; Purkey et al., 2008; Vicuña et al., 2011; Vicuna and Dracup, 199 200 2007; Yates et al., 2005b). Climate-driven models, such as WEAP provide dynamic tools by incorporating hydroclimatological variables to analyze, in this case, a one-dimensional, quasi 201 202 physical water balance model, which depicts the semi-distributed hydrologic response through 203 the surface runoff, infiltration, evapotranspiration (Penman-Monteith equation), interflow, percolation and base flow processes (Forni et al., 2016). 204 The hydrological model comprises 16 sub-basins with a spatial resolution ranging from 67 to 205 272 km² (see Figure 1), which defines the natural discharge produced by the Cantareira System. 206 207 The observed hydrologic data (discharge and rainfall) were taken from HIDROWEB (the National Water Agency database [ANA]), SABESP and the São Paulo state Water and 208 209 Electricity Department [DAEE]. A network of 52 rain gauge stations and 11 discharge gauge stations were configured, with inputs and outputs by a monthly time-step. On the other hand, 210 211 the meteorological data from 14 gauging stations (temperature, relative humidity, wind speed and cloudiness fraction) were taken from the National Institute of Meteorology and Center for 212 213 Weather Forecasting and Climate Research (CPTEC) databases (see Figure 1: panels A, B, C 214 and D). For the basin characterization, we adopted the soil map from (De Oliveira et al., 1999) 215 (1:500,000) and the land use map of 2010 from (Molin et al., 2015) (1:60,000).

The hydrological model was calibrated using a mixed calibration process. A first approximation

of the calibration parameters was made by the Model-Independent Parameter Estimation &

Uncertainty Analysis software (PEST), automatic calibration tool in WEAP (Doherty and

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Skahill, 2006; Seong et al., 2015; Skahill et al., 2009; Stockholm Environment Institute (SEI),

2016), and then the calibration parameters were refined using a manual adjustment technique.

In the modeling process, a two-year warm-up period from 2004 to 2005 was established, for

the calibration period from January 2006 to December 2010 and from January 2011 to August

223 2015 as the validation period. Although more extensive periods of calibration and validation

are suggested to better represent hydrological dynamics (Gibbs et al., 2018), the absence of

observed data restricted the extension of assessment periods. During this process, the following

variables were calibrated: Kc (Crop Coefficient), SWC (Soil Water Capacity), DWC (Deep

Water Capacity), RZC (Root Zone Conductivity), DC (Deep Conductivity) and PFD

228 (Preferential Flow Direction). The objective functions to measure model performance, widely

used in hydrologic applications, were the Volumetric Error Percent Bias (PBIAS), Standard

Deviation Ratio (SDR), Nash-Sutcliffe Efficiency (NSE), NSE of the logarithmic of discharges

(NSE_{Log}) which is more sensitive to low-flows, Coefficient of determination (R²) and

Volumetric Efficiency (VE), where the joint maximization of the NSE_{Log} and PBIAS criteria

was the objective function to measure model performance (Muleta, 2012).

The WEAP model was calibrated based on eleven discharge gauge stations (see Figure 1, panel

A) from the ANA-HIDROWEB dataset (www.ana.gov.br), four of these located at the reservoir

entrance of each sub-system (Jaguarí-Jacareí, Cachoeira, Atibainha and Paiva Castro).

Cantareira's reservoirs (sub-systems) were set up as a single Equivalent System (ES), where

238 the specific water demands are considered (ANA and DAEE, 2004; PCJ/Comitês, 2006). This

ES can be expressed as follows:

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$$ES_{Cantareira} = \sum_{i}^{n} QN_{i} - \sum_{i}^{n} WD_{i}$$

Equation 1.

Where $ES_{Cantareira}$ is the available water for withdrawal from the system, QN is the natural

242 discharge from the reservoir i (sub-system) and WD is the specific water demand in each

reservoir (such as the reservoir downstream urban supply).

244 It is worth noting the sub-basins areas in this case are smaller than each cell of the adopted

climate model (400 km²) and although RCMs are an alternative to downscale the coarse

resolution GCM, often RCM outputs deviate from the observed climatological data (Kim et al.,

247 2015; Liersch et al., 2016; Smitha et al., 2018a). Therefore, in order to adjust the RCM output

dataset, the projections of the Eta-INPE scenarios had to be spatially relocated and bias

corrected from observed historical climate conditions (rain and temperature). For this, the

"Additive Corrections and Scaling" method was used, a simple approach that assumes the

- relative mean biases between observed data and model projections (Maraun and Widmann,
- 252 2018; Smitha et al., 2018b). The hydrological discharge projections 2007-2099 forced by
- 253 GCMs and RCPs scenarios can be seen in Appendix B (Fig. B-1).

3.2. SDF curve development

Following the flowchart of Figure 2, the Threshold Level Method (TLM) is traditionally used to estimate hydrological drought events from continuous discharge time series (Wanders et al., 2017). TLM was originally called 'Crossing Theory Techniques" and it is also referred to as run-sum analysis (Hisdal et al., 2004; Nordin and Rosbjerg, 1970; Şen, 2015). Usually, different criteria may be used to define the threshold in hydrological drought analysis by the TLM approach (Rivera et al., 2017; Sen, 2015; Tosunoglu and Kisi, 2016). In this study, two monthly desired-yield thresholds were implemented. They were defined from the pre-established water demand in the system (Hisdal et al., 2004; J. H. Sung and Chung, 2014). Initially, a stationary demand (SD) of 31 m³/s was defined as the historical average demand and another nonstationary demand (NSD) of 31 to 42 m³/s over time was defined as a hypothesis representative of the population growth in the SPRM (see Figure 3). These water demand values are consistent with the ANA/DAEE, 2004 study, according to the record and projection scenarios of the population growth of the IBGE¹. On the other hand, the continuous discharge series were defined from the hydrological modelling result based on Eta-INPE historical dataset (baseline period 1962-2005). From the results of the TLM approach in the Cantareira System, the baseline (historical) scenario, based on Eta-MIROC5 model simulations, showed the greatest hydric deficit under the two water demand scenarios analyzed (SD and NSD), see Figure 3.

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¹ Brazilian Institute of Geography and Statistics: http://www.ibge.gov.br/home/

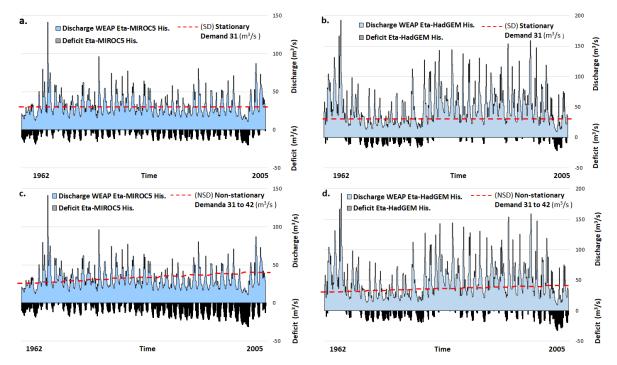


Figure 3. TLM Evaluation from historical discharge WEAP-Eta (base line scenarios), under Stationary (SD) and Non-Stationary Demand (NSD) assumptions as the "threshold level": a. 31 m³/s and Eta-MIROC5. b. 31 m³/s and Eta-HadGEM. c. 31 to 42 m³/s and Eta-MIROC5. d. 31 to 42 m³/s and Eta-HadGEM. Total River basin 2265 km².

Based on the time series of "severity" (or deficit, in m³) and duration (days) in the Cantareira System, obtained from TLM evaluation of the Eta-INPE historical scenarios (see Figure 3), the SDF curves were constructed. To estimate the return periods of drought events of a particular severity and duration, the block maxima GEV frequency analysis distribution was used. In this case, the GEV distribution is useful because it provides an expression that includes all three types of extreme value distributions (Tung et al., 2006).

In various studies addressing SDF curve development, the GEV distribution was consistent with the data sets of extremes, where distributions that use three parameters were required to express the upper tail data (J H Sung and Chung, 2014; Svensson et al., 2016; Todisco et al., 2013; Zaidman et al., 2003). On the other hand, it is suggested that for other durations of drought, other probability distribution functions can be explored (Dalezios et al., 2000; Razmkhah, 2016). However, in this study we took advantage of the versatility of the GEV distribution, considering its flexibility to fit a set of data through the expressions:

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$$F(x) = exp\left[-\left\{1 + \xi\left(\frac{x-\mu}{\sigma}\right)\right\}^{1/\xi}\right] \quad \xi \neq 0$$
 Equation 2.

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$$F(x) = exp\left[-exp\left(-\frac{x-\mu}{\alpha}\right)\right]$$
 $\xi = 0$ Equation 3.

- Where the cumulative distribution function F(x) depends on μ as a location parameter, α as a
- scale parameter and ξ as a shape parameter. Therefore, if, $\mu + \alpha / \xi \le x \le \infty$ for $\xi < 0$ is a Type III
- 294 (Weibull), $-\infty \le x \le \infty$ for $\xi = 0$ is a Type I (Gumbel), and $-\infty \le x \le \mu + \alpha/\xi$ for $\xi > 0$ is a Type
- 295 II (Frechét) distribution (Stedinger et al., 1993).
- In order to fill a considerable number of events per interval, droughts were classified into four
- 297 time intervals from 0 to 31, 0 to 90, 0 to 180 and 0 to 365 days. Thus, considering the adoption
- of the GEV distribution, the model parameters ξ , α and μ for cumulative durations defined and
- return periods of 2, 10 and 100 years were estimated using the Method of Maximum Likelihood
- 300 Estimator (MLE).

3.3. Water price and Hydrological drought relationship

- According to the flowchart in Figure 2, drought can be addressed as a somewhat unusual
- economic phenomenon in that it affects both supply (the source) and demand (users), especially
- in systems dependent on water from a single source (Moncur, 1987). As expected, episodes of
- water scarcity pose technical, legal, social and economic problems for managers of urban water
- 306 systems. Traditionally to overcome these episodes, reservoirs play a key role in water supply
- and demand management, providing security against hydrological extremes (Mehran et al.,
- 308 2015). However, when the water deficit intensifies, the structural measures are not enough and
- they must be accompanied by contingency measures, for example, water price regulation
- instruments, implemented as an incentive for more efficient use (Mechler et al., 2017).
- In Brazil, each state-owned sanitation company has its own water charging policy, where the
- vast majority use block tariffs as a pricing policy, including SABESP (De Andrade Filho et al.,
- 2015; Mesquita and Ruiz, 2013; Ruijs et al., 2008). In Sao Paulo State, the tariff policy system
- is regulated by Decree 41.446/96, also for services provided by SABESP. For the water tariff
- setting, several factors are taken into account, such as service costs, debtors forecast, expenses
- amortization, environmental and climatic conditions, quantity consumed, sectors and economic
- 317 condition of the user (SABESP, 1996). These sectors are divided into residential, industrial,
- 318 commercial or public, and the value that is charged for the service is always progressive. In
- other words, there is a standard minimum consumption with a fixed value and, based on that,
- such factors vary the consumption ranges (SABESP, 2018b). From the total water withdrawn
- from the Cantareira System, urban use is predominant in SPRM, where approximately 49% of
- 322 the total is for household needs, 31% for industrial needs and 20% for irrigation
- 323 (Consórcio/PCJ, 2013). In this study, we consider the water-withdrawal for domestic and
- industrial use in the SPMR, due to the direct dependence of these sectors on the SABESP water

supply network, as well as the supply priority that the domestic sector have according to Brazilian law during drought periods (Lei N° 9.433 do GOBERNO DO BRASIL, 1997). Figure 4 shows the TLM analysis with a constant threshold under the same discharge scenario (SABESP 2000-2016), differentiated by the monthly and annual accumulation of the variable. The monthly step represents the system's natural discharge without regulation ("a" in Figure 5), while the regulated discharge is represented by the annual aggregation of monthly natural discharges ("b" in Figure 5). Assuming this, without the reservoir system ("a" case), with direct water withdrawals (Threshold = 31 m³/s), the average accumulated deficit over these 17 years would be 225% greater than with the reservoir system implemented ("b" case).

The TLM analysis (Figure 5) showed two hydrological drought periods in 2000-2003 and 2010-2015: one with a lower and another with a higher deficit, respectively. While for the period from 2004 to 2009, a series of smaller droughts in both magnitude and frequency could be overcome by the reservoir system. On the other hand, in 2010-2015, the accumulated deficit, under the regulated scenario, would exceed the useful storage in 70% while for the period 2000-2003, the accumulated deficit only reached 43% of the system's useful storage capacity. Therefore, it is clear that over a long period of deficit or strong multi-year droughts, the storage system could be accompanied by other contingency complementary measures.

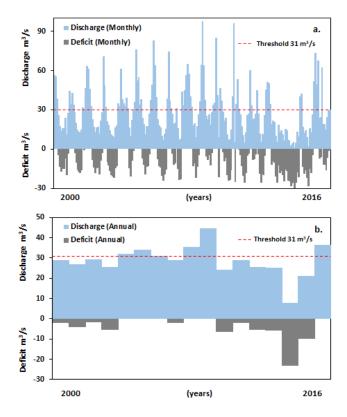


Figure 4. TLM analysis under two time step assumptions during, 2000-2016 period and Threshold equal to 31 m^3 /s. a) Monthly discharge and b) Annual discharge.

Urban drought management programs incur costs that must be assumed to overcome the water 345 crisis with equity (Molinos-Senante and Donoso, 2016). SABESP in the SPMR, for example, 346 through price-based policies² controlled the consumption rates of water users when the 347 hydrological deficit scenarios were presented in the Cantareira System (Iglesias and Blanco, 348 2008; Millerd, 1984; SABESP, 2018c, 1996). Therefore, during the 2014/2015 drought in 349 SPRM, reactive economic contingencies were implemented, such as increased water tariffs, 350 extra fees and price incentives, which had a detrimental effect on the company's profit margin, 351 which provides the water resource (GESP, 2016; SABESP, 2017a, 2016a). 352 353 However, the financial exposure does not always exhibit a strong correlation with weather indices (Zeff and Characklis, 2013). We established a drought revenue loss cost estimation 354 based on the Market Price method (Meyer et al., 2013). To do this, we developed an empirical 355 relationship between the water price (impacts) and drought duration (severity) (Bachmair et al., 356 357 2016; Grafton and Ward, 2008; Hou et al., 2018; Mens et al., 2015). Based on the TLM approach, the monthly discharge time series and a constant threshold (31 m3/s) from 2000 to 358 359 2018 (Figure 5) was analyzed; aiming to associate the drought characteristics with the adjustment rates of the SABESP database. On one hand, the upper part of Figure 5 shows the 360 drought duration and the annual tariff adjustment with a Pearson correlation coefficient " r_{xy} " of 361 0.481 between them. On the other hand, the lower part represents the deficit volume for each 362 drought duration. In this case, the Pearson correlation coefficient between drought duration and 363 tariff adjustment showed an " r_{xy} " value of 0.453. 364 From the calculated correlation coefficients, a T-student significance test with an alpha of 5% 365 was implemented. Based on the test, it was found that the adjustment rate and the water deficit 366 present a high to medium significance, despite having a lower correlation coefficient. However, 367 in this study the drought duration was assumed as the feature to relate with water price, due to 368 the frequency analysis of the series of annual maxima. Even though the correlation coefficient 369 values showed relatively low values, the use of these drought characteristics may be useful 370 given the lack of information regarding drought and its economic impacts on the study area. 371

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² Database "percentage rate increase" 2001-2018 SABESP: http://www.sabesp.com.br/CalandraWeb/CalandraRedirect/?temp=4&proj=investidoresnovo&pub=T&db=&doc id=9AA0FF2088FBF0A8832570DF006DE413&docidPai=AB82F8DBCD12AE488325768C0052105E&pai=fil ho10

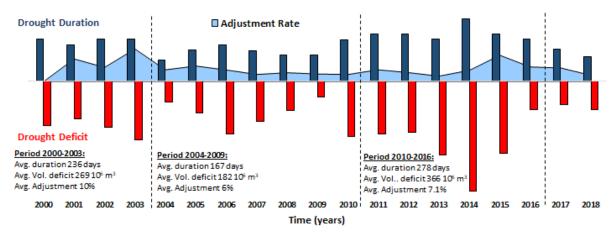


Figure 5. Empirical relationship between Cantareira System drought duration "blue-bar in days" [derived from monthly average discharge analysis], Cantareira System drought deficit "red-bar in 10⁶-m³" and annual price adjustment rates under variate hydrological conditions in percentage.

From the recent drought events in SPMR, which significantly affected the water supply i.e in 2000/2001 (Cavalcanti and Kousky, 2001) - 2013/2015 (Nobre et al., 2016) and the TLM analysis that showed some degree of correlation between annual events and priced adjustment rates, an empirical system analysis robustness against the drought duration was proposed (Mens et al., 2015). Our robustness analysis is based on the assumption of the main impacts derived from water supply problems in the SPRM, which appear to be related with medium to prolonged duration events and medium to high severity (up to 365 days or two consecutive annual cycles). Therefore, three priced-adjustment vs. drought duration scenarios were established (see Figure E-2 in the supplementary material). First, 100% water availability. In this scenario, the reservoir network is not essential to ensure water supply (drought duration between 0 and 90 days). Second, the water availability with supply warranty and dependence on the storage system. In this scenario, the reservoir network provides resilience during droughts of smaller magnitudes and duration (drought duration between 90 and 180 days). Third, stored water shortage and forced interruption of supply. In this scenario, the water deficit prevails with extra fees and other savings measures (drought duration between 90 and 365 days).

Since the water price formation study is not part of this work as it entails a complex microeconomic analysis (Garrido, 2005), we adopted the average prices of water (Bulk Water Tariff, 2016) in the SPMR, for the Domestic and Industrial sectors (SABESP, 2016b). Therefore, based on the previous analysis (Figure 6), the following was adopted: First, during the most severe droughts, an increase in the water tariff for the following period is expected. Second, on the contrary, when the smaller deficits are overcome with the water stored in the system, the increase in tariffs is a consequence of the annual Consumer Price Index (CPI) and

other tariff updates according to the law (SABESP, 2016b). Thus, the approach requires some additional assumptions explained as follows:

- i. Based on the current average prices for the domestic and industrial sectors, a base-water-price was established to analyze US\$ 3.38 per m³, assuming that this value is given considering normal supply conditions or 100% water availability,
- ii. From the SDF curve construction intervals (cumulative drought duration) and three class intervals of the annual tariff adjustment (min. 6% to max. 17%, see Figure E-1 in appendix E), the water prices were established (see Table 1).

Table 1. Main assumptions for establishing the tariff water price according to the drought duration.

Drought Duration	Water Tariff Adjustment	Average price	Cantareira System robustness
Interval (days*)	adopted (%)	$(US\$/m^3)$	characteristics scenarios
From 0 to 31	0	3.38	100% water availability base scenario
From 0 to 90	6	3.58	100% water availability
From 0 to 180	10	3.71	Water availability with storage dependency
From 0 to 365	17	3.95	Water deficit (multi-year droughts)

* Cumulative drought duration

Table 1 represents the inelastic behavior of the Price Elasticity of Demand (PED) showing closer intervals as water supplies are reduced due to drought and higher prices imposed to try to reduce demands. Hence a successful price-based rationing policy requires a progressive increase if the demand becomes predominantly inelastic (Mays and Tung, 2002), as the proposed hypothesis establishes in this case. More studies of price elasticity and water scarcity can be found in (Freire-González et al., 2017; Mansur and Olmstead, 2012; Ruijs et al., 2008).

The final step of the methodology (see Figure 2) defines the calculation of the drought impacts through the management horizons (2007-2040, 2041-2070 and 2071-2099). This calculation was carried out for the cumulative drought periods greater than 180 days, considering that from this duration, the supply begins to show an important dependence of the Cantareira reservoir System.

4. Results and discussions

4.1. Hydrological modeling

The hydrological model structure performed in monthly time steps and was calibrated-validated following the described procedure in Section 3.1. To improve the calibration procedure, multiple statistical evaluation criteria were used (Gibbs et al., 2018; Kumarasamy and Belmont, 2017). This is important because analyzing multiple statistics can provide an overall view of the model based on a comprehensive set of indexes on the parameters representing the statistics of the mean and extreme values of the hydrograph (Moriasi et al., 2007).

The equivalent system hydrograph for calibration and validation periods are shown in Figure 6. The colors in Figure 6 represent the classifications suggested by (Moriasi et al., 2007) and are as follows: green for "very good" (NSE > 0.75; PBIAS < $\pm 10\%$; RSR < 0.50), yellow for "good or satisfactory" (0.75 > NSE > 0.5; $\pm 10\%$ < PBIAS < $\pm 25\%$; 0.50 < RSR < 0.60), red for "unsatisfactory" (NSE < 0.5; PBIAS > $\pm 25\%$; RSR > 0.70). Moreover, the correlation coefficient (R²) and the VE criterion values close to 1.0 mean that the prediction dispersion is equal to that of the observation (Krause and Boyle, 2005; Muleta, 2012). It is important to note that in the validation period (2011-2015), most of the recent drought event were simulated with an acceptable performance, although there is a tendency to overestimate periods of low flow.

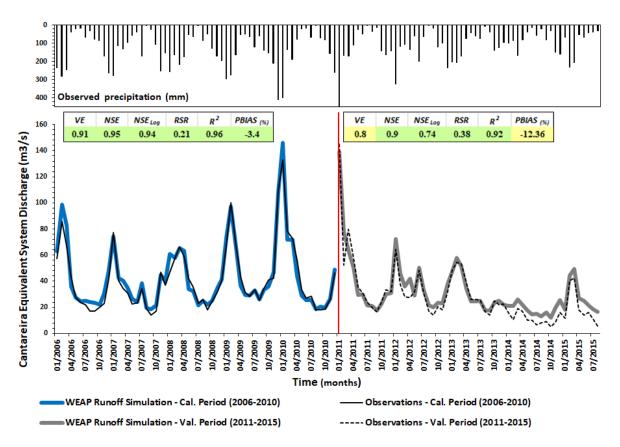
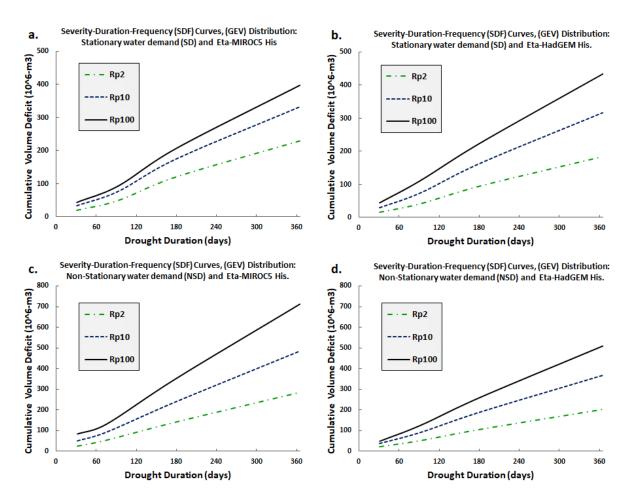


Figure 6. WEAP Hydrographs Cantareira Equivalent System (ES) performance criteria for Calibration (2006 - 2010) - Validation (2011 – 2015) periods. The calibration and validation performance criteria for each sub-basin in the system can be found in the "Complementary Material" - Appendix A. – Table A-1.

Individual watershed hydrological modelling performance ratings are presented in Appendix-A, Table A-1. Moreover, several statistical criteria were considered in the evaluation of the calibration process, where each criterion covers a different aspect of the resulting hydrograph. Five sub-basins were modeled within the Jaguarí-Jacareí sub-system (Sub B-F28, B-F23, B-F25, Jaguarí and Jacareí). This five sub-system represents approximately 46% of the total available water and showed the best modelling performance statistics, compared to the other subsystems.

4.2. SDF curves

Using the traditional frequency analysis, the severity-duration-frequency curves for two threshold levels and two discharges (from Historical_RCMs WEAP outputs) were developed as shown in Fig. 7. For the SDF curves configuration, the Generalized Extreme Values (GEV) function was used. It can, therefore be observed from the SDF results that according to the fit data set (Appendix C), the shape parameter (ξ) varies with the drought duration, therefore for a drought interval of more than 180 days, the Probability Distribution Function (PDF) Type I presents a better fit, even for the two proposed demand scenarios. On the other hand, droughts with duration intervals of less than 90 days, under stationary and non-stationary demand scenarios, had a better fit to FDP Type III (see Tables D-1 to D-4 in Appendix D). Moreover, the fit diagnostic plots "Empirical quantile vs Model quantile" (QQ-plot) and "Return level vs Return period" (RR-plot) show the relationship between the model, the data fit and prediction capacity (Appendix C). Therefore, in terms of the quantiles, the QQ-plot shows the data trend to follow the model line in most cases. While the predictive capacity of the model, represented by the RR-plot, shows a decrease as the return period increases.



Based on the relationship between the Cantareira System Drought-Cost-Robustness curve (see details in Figure E-2 supplementary material) and the SDF curves (see Figure 7), the base functions of Severity-Duration-Impact of drought were built to estimate the base-line scenarios of damage cost in the water utility company. These scenarios are shown in Figure 8, under different recurrence events (Rp scenarios), climate projections (RCPs – RCMs) and demand variability scenarios (SD – NSD). Each pair of lines in Figure 8 (continuous and dashed), show a range of uncertainty associated with the considered change drivers.

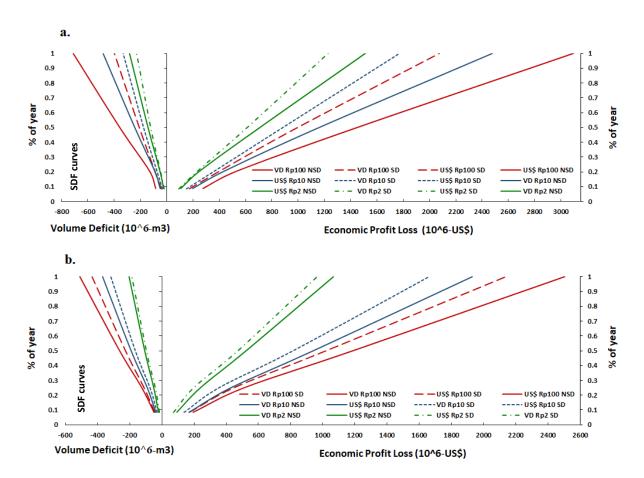


Figure 8. Severity-Duration-Impact curves. Sector **a.** Severity-Duration-Frequency-Profit Loss under the historical *Eta-MIROC5* scenario. Sector **b.** Severity-Duration-Frequency-Profit Loss under the historical *Eta-HadGEM* scenario. Note: *SD* and *NSD* are the stationary or non-stationary demands, respectively; "*VD*" is the volume deficit, under return period of 2, 10 and 100 years; % of year is the drought event duration in relation to one year.

4.3. Economic impacts under climate change

The results describe the net present value (NPV) of the potential economic impacts produced by hydrological drought durations greater than 180 days. These impacts are presented considering the climate, demand, severity and recurrence scenarios during the analysis periods:

2007-2040, 2041-2070 and 2071-2099. The evaluation of the drought's economic impact in the water company showed in general, revenue losses per analysis period between 0.003% and 0.021% related to the GDP in the SPMR in 2017 (SEADE, 2018). This relatively low range of percentage revenue losses is, in fact, significant for the regional economy since SPMR accounts for approximately 18% of the Brazilian GPD.

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Figure 9 shows the economic impacts on the water utility company under an analysis that independently discriminates radiation scenarios (RCP), GCM and water demand. Figure 9 also compares the relative difference between scenarios and time periods, using the median statistic and standard deviation, the latter as a measure of dispersion. In general, the results in Figure 9 reveal that under the driver water demand, the most propitious scenarios are configured for the generation of greater economic impacts (on average), followed by radiation and GCM drivers, respectively. Likewise, in Sector "a", the impacts analyzed under RCP scenarios 4.5 and 8.5 showed a low difference percentage in variability and median. This can be explained from the study by Chou et al. 2014, where the Eta-INPE results establishes that, in the future, there is no clear trend in the average precipitation and during the summer, the time series show a trend for a reduction in precipitation in both emission scenarios, RCP 8.5 and 4.5. While for sector "b" (RCM), the outputs nested in Eta-MIROC5 presented higher revenue losses in the company than those based on HadGEM-ES. This difference can be attributed to the annual cycle of precipitation, which shows that the ETA-INPE simulations driven by MIROC5 produces generally less precipitations during the dry season, therefore the water deficit during this period will be more critical (Chou et al 2014a). Finally, sector "c", where the Non-stationary demand (NSD) trend imposed the larger differences in the magnitude and variability percentage impacts (human influences), suggesting that the demand-related (population growth) uncertainty would be far greater than that associated with climate sensitivity.

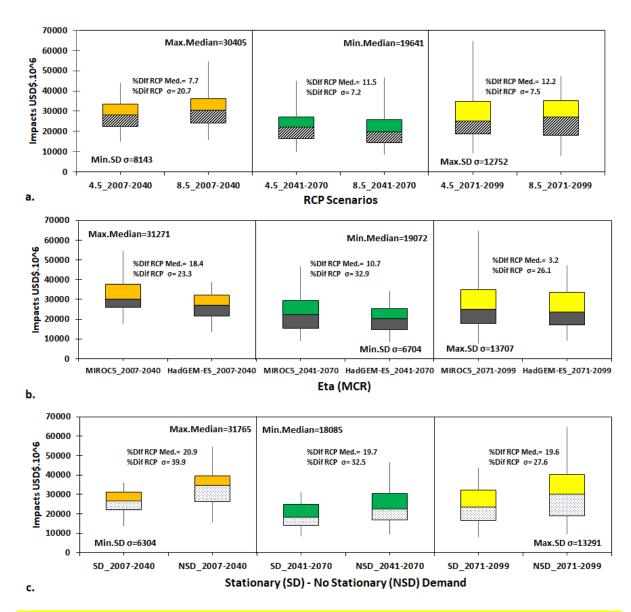


Figure 9. Impacts and relative differences between scenarios in median 50^{th} percentile (Med.) and standard deviation (σ). Sector "a": Impacts based on RCP scenarios. Sector "b": Impacts based on RCM scenarios. Sector "c": Impacts based on demand scenarios. Through analysis periods, Orange (2007-2040), Green (2041-2070) and Yellow (2071-2099).

Under a different grouping configuration for the analysis of the results (see Figure 10), the impacts assessment was conditioned by the scenarios joint study of climate forcing (Eta-GCM) and radiation (RCP). Based on this scheme, it was found that the largest economic impact was represented by the Eta-MIROC5_4.5 climate-forcing scenario, while smaller impacts (on average) were observed in the Eta-HadGEM-ES_4.5 scenario. In addition, the Eta-MIROC5 scenario showed the maximum values of the median 50th percentile (Max.Med.) and standard deviation (Max.SD) between the set of time period panels, which concludes that the climate

forcing based on the MIROC5 model is the main driver of the impacts and variability between analyzed climate drivers (GCM).

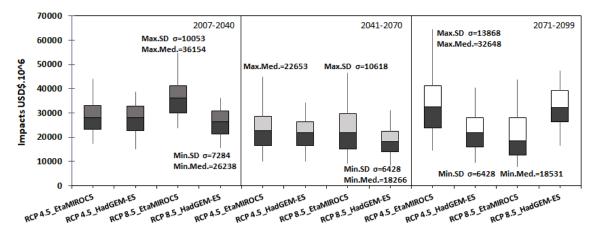


Figure 10. Economic impacts comparison between Eta-INPE_RCP_GCM based scenarios throughout the projection time period: first panel 2007-2040, second panel 2041-2070 and third panel 2071-2099.

Figure 11 describes a third results analysis scheme. In this case, the impacts are evaluated based on the return periods. In the box plot of Figure 13, an increasing tendency of the dispersion is observed in the measure in which the projected time horizon is more distant, probably due to the greater uncertainty in future climate projections (Cubasch et al., 2001). On the other hand, the higher periods of return reflect impacts of greater magnitude, as expected.

In all cases, the average economic impacts projected for the period 2040-2071 presented lower values compared with the other two periods analyzed. According to a study by Lyra et al. (2017) in which the most recent Eta-INPE model simulations were performed at more detailed scales, the annual total precipitation (PRCPTOT) and maximum number of consecutive days with precipitation (CDD - CWD) indexes for the Sao Paulo region showed better results in terms of favoring water availability during this period. On the contrary, the period 2007-2040 presented the greatest economic impacts (evidence of the recent water crisis) with the lowest dispersion (less uncertainty) in relation to the other projected time periods. While the projection of the 2071-2099 period showed an impact magnitude close to the 2007-2040 period, given that both Eta-INPE simulations intensify the reduction of precipitation toward the end of the century in Southeast Brazil, with an annual rainfall reduction above 40% and a reduction of precipitation extremes (Chou et al., 2014a; Lyra et al., 2017).

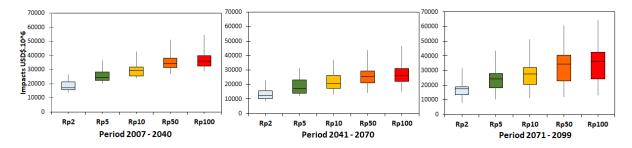


Figure 11. Drought impact variability between return period scenarios during the projection periods: first panel 2007-2040, second panel 2041-2070 and third panel 2071-2099.

4.4. Considerations on Uncertainties

The methodology adopted here includes a model chain, typical in exercises of hydrological regime projection through hydrologic modelling under climate change projections (Fowler et al., 2007; Jones, 2000; Wilby and Harris, 2006). This model chain incorporates several sources of uncertainty such as those listed by Honti et al. (2014) and Jobst et al., (2018): 1) the climate model; 2) the downscaling method or an RCM application, the latter as in our work; 3) the hydrological model, and (4) the inherent modeling uncertainty of coupling different climate-hydrology spatiotemporal scales.

In this case, the systematic analysis of change drivers (uncertainty sources) offers a set of results around potential scenarios to frame uncertainty (Refsgaard et al., 2007; Rodrigues et al., 2015) while the drivers sensitivity analysis is proposed as a part of the results in this study. Montanari (2007), however, advocates that some methods commonly used for uncertainty assessment do not address uncertainty, but only model sensitivity. Moreover, although some studies indicate that the climate projections surpass the hydrological uncertainties (Bates et al., 2008; Nóbrega et al., 2011), Honti et al. (2014) reinforces that different methods of uncertainty assessment may lead to different conclusions.

Our methodology also included a drought indicator development, through the TLM approach, demand scenarios and a drought cost estimation based on the Market Price method. (Hou et al., 2018; Mens et al., 2015). Results showed that drought deficits are influenced not only by the modeled inflows at a lumped scale, throughout the period of 2007-2099, but also in our case study by the reservoir operation. In fact, the spatially-combined operation of existing reservoirs may be different from our considerations, adopting an "equivalent system" (ES) without a future layout change. On the one hand, the system demand scenarios are based on current (historical period 2004-2016, (SABESP, 2017b)) best knowledge information, and the adoption of two scenarios aimed at giving a broader, realistic view of the different possible outcomes

due to expected population growth (ANA and DAEE, 2004; IBGE, 2018). On the other hand, the economic loss estimation, based on the aforementioned drought event measures, does not incorporate eventual market changes, currency changes or even subsidies. Conversely, our loss estimation assumes that those economic measures, i.e. water tariff adjustments were and would continue being adopted by the water utility, as a trigger determinant once the drought hazard happened. Because this triggering factor would temporarily occur either promptly or slowly, when structural measures were not enough to secure water supply under eventual hydrometeorological conditions and water demand, uncertainties in cost analysis could increase.

5. Conclusions and recommendations

This paper developed a methodology with application to assess economic impacts of drought risks for water utilities through an assessment under climate and water demand scenarios. In this example, the SDF framework has linked climate, hydrology and economy factors, using Sao Paulo Metropolitan Region dependence on the Cantareira Water Supply System, Brazil. In this paper, we consider these results preliminary, but with valuable information for a water utility interested in the drought risk losses.

Methodologically, first we characterized the hydrological droughts through the SDF curves, from the hydrological modeling by the baseline period of the RCM. Second, the SDF was coupled with a local water demand development based on the supply warranty time percentage during the drought events. Under these assumptions, an empirical drought economic impact curve was setup, representing the Water Utility Company profit losses due to the impossibility of supplying demand during hydrological drought periods. Additionally, our results could elicit further implications for drought risk reduction and management.

The main results of the methodology implemented were: the great financial vulnerability of the water utility company of the SPMR against the hydrological drought. Possibly the maximum supply capacity of the system is reaching its limit due to the growing demand and the new challenges represented by climate change. The main driver of economic impacts under the analysis scheme turned out to be the water demand dynamics. The Eta-MIROC5 scenario proved to be primarily responsible for the economic impacts compared to other climate drivers. Comparatively, the RCP 4.5 and 8.5 radiation scenarios showed no major difference between them. The scenario of projected impacts for the period 2071-2040 showed the greatest dispersion among time scenarios, while the closest scenario 2007-2041 showed less dispersion and greater impacts on average. The WEAP model proved to be a versatile tool for the

construction-calibration-validation process of the model, when it is implemented in climate impact studies. The approach for the characterization of the drought "TLM" showed to be a tool easily applicable to describe quantitative changes in hydrological drought during long periods with change in water demands (Thresholds).

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On one hand, this SDF framework could help analyze the impacts from key drivers, such as climate, land use and water withdrawal rates in complex or recurrent drought patterns. In addition, this SDF framework could link interdisciplinary studies, with broader relationships in relation to water security, energy security and food security. Thus, we recommend future research of the SDF framework linked to: Palmer's drought indices (Rossato et al., 2017); a model-based framework to disaster management (Horita et al., 2017); an ecosystem-based assessment for Eco-hydrological modelling (Taffarello et al., 2018); effectiveness of drought securitization under climate change scenarios (Mohor and Mendiondo, 2017). Moreover, the SDF framework is capable of integrating actions towards: dynamic price incentive programs related to wise human-water co-evolution patterns; water-sensitive programs under deep cultural features; socio-hydrological observatories for water security; feasibility analysis of the economic impacts of implementing new technologies for water economy and flow measurement; leakage control; detecting and legalizing illegal connections and water reuse, among others. Furthermore, dissimilarities pointed out from climate scenarios (see Figure 11) would suggest a set of possibilities to face the uncertainty. For instance, the SDF framework would guide the decision-making of water utility profits to cope with economic impacts of drought risks in the long and medium term. In addition, the expected profit loss over the longterm would serve as the initial estimate for financial contingency arrangements as insurance schemes or community contingency funds. In general, the SDF framework developed here can be proposed as a planning tool to mitigating drought-related revenue losses, as well as being useful for the development of water resource securitization strategy in sectors that depend on water to sustain their economies.

The following should be considered for further studies to strengthen decision-making based on results of the tool: despite having achieved an acceptable performance, the inclusion of more gauge stations could not only improve calibration performance but also cover a larger sample space of events, increasing the confidence of projections. Introduce a direct measure of the economic impacts resulting from multi-year deficits of annual duration not entire, although, the methodology can assimilate multiple consecutive years and entire deficits, the cumulative impacts would be underestimated. On the other hand, in order to have a methodological

- comparative standard, more regional studies of SDF curves need to be implemented,
- considering the spatialized analysis and broader statistics methods. Finally, it is a fact that the
- reliability of SDF curve estimates depend on the quality and extent of the records used, or in
- this case, the capacity of regional climate models to reproduce the observed distribution of
- 639 extreme events.

Acknowledgments

- The authors would like to thank the support from several agencies in Brazil and Colombia: the
- Administrative Department of Science, Technology and Innovation (COLCIENCIAS) Doctoral
- Program Abroad No 728-2015, CAPES-PROEX-1650/2017/23038.013525/2017-30, CAPES
- 644 Pró-Alertas #88887.091743/2014-01, CNPq #307637/2012-3, CNPq #312056/2016-8 PQ and
- 645 CNPq #465501/2014-1 and FAPESP 2014/50848-9 Water Security of the INCT-Climate
- 646 Change II. The Sao Paulo State Water Utility Company, SABESP, kindly provided relevant
- information for this study. All co-authors declare no conflict of interest. The third author thank
- 648 to Coordination of Superior Level Staff Improvement (CAPES) and to the Programme of
- Postgraduate in Hydraulics and Sanitation (PPG-SHS) for the postdoctoral fellowship.

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Detailed information is available in the Supplementary Material.

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1078								

1 Complementary Material.

- 2 A. Appendix.
- 3 The performance criteria described below have been used in this study to test how well the
- 4 calibrated model fits the observed data. These evaluation statistics have been selected based on
- 5 recommendations in the literature (Moriasi et al., 2007; Muleta, 2012).

6

- 7 Nash-Sutcliffe Efficiency (NSE)
- 8 $NSE = 1 \frac{\sum_{i=1}^{N} (S_i O_i)^2}{\sum_{i=1}^{N} (S_i O_{mean})^2}$ Equation A-1.
- 9 Where, " S_i " is the model simulated output and " O_i " observed hydrologic variable.

10

11 Volumetric Efficiency (VE)

12
$$VE = 1 - \frac{\sum_{i=1}^{N} |S_i - O_i|}{\sum_{i=1}^{N} O_i}$$
 Equation A-2.

13 Ratio of Standard Deviation of Observations to RMS (RSR)

14
$$RSR = \frac{\sqrt{\sum_{i=1}^{N} (S_i - O_i)^2}}{\sqrt{\sum_{i=1}^{N} (S_i - O_{mean})^2}}$$
 Equation A-3.

15 Percent bias (PBIAS)

16
$$PBIAS = \frac{\sum_{i=1}^{N} (O_i - S_i)}{\sum_{i=1}^{N} O_i} \cdot 100$$
 Equation A-4.

17 Coefficient of Determination (R²)

18
$$R^2 = \left(\frac{\sum_{i=1}^{N} [o_i - o_{mean}] \cdot [S_i - S_{mean}]}{\{\sum_{i=1}^{N} [o_i - o_{mean}]^2\}^{0.5} \cdot \{\sum_{i=1}^{N} [S_i - S_{mean}]^2\}^{0.5}}\right)^2$$
 Equation A-5.

- 20 Cantareira basins performance criteria for Calibration and Validation periods. *Cal. =
- 21 Calibration period and Val. = Validation period, are shown in the Table A-1. The classification
- of colors are as follows: green for "very good" (NSE > 0.75; PBIAS $< \pm 10\%$; RSR < 0.50),
- 23 yellow for "good or satisfactory" (0.75 > NSE > 0.5; $\pm 10\%$ < PBIAS < $\pm 25\%$; 0.50 < RSR <
- 24 0.60), red for "unsatisfactory" (NSE < 0.5; PBIAS > $\pm 25\%$; RSR > 0.70). Moreover, the

correlation coefficient (R²) and the VE criterion values close to 1.0 mean that the prediction dispersion is equal to that of the observation.

Table A-1. Performance criteria results on the Cantareira modeled basins.

Watersheds	Area	V	'E	N	SE	RS	SR	PBIA	S (%)	NSE	Log	K	2
watersneas	(km^2)	Cal.	Val.	Cal.	Val.	Cal.	Val.	Cal.	Val.	Cal.	Val.	Cal.	Val.
Sub B-F28	269.0	0.79	0.72	0.74	0.52	0.49	0.57	1.64	4.89	0.69	0.69	0.74	0.53
Sub B-F23	508.4	0.83	0.8	0.87	0.86	0.38	0.38	9.52	5.58	0.78	0.85	0.9	0.88
Sub B-F25	179.5	0.87	0.77	0.93	0.84	0.27	0.42	5.45	-9.54	0.91	0.78	0.94	0.86
Jaguarí	67.8	0.88	0.72	0.93	0.84	0.27	0.48	-3.3	-21.1	0.89	0.61	0.93	0.9
Watersheds	Area	V	E	N.	SE	RS	'R	PBIA	S (%)	NS	E_{Log}		\mathbb{R}^2
watersneas	(km^2)	Cal.	Val.	Cal.	Val.	Cal.	Val.	Cal.	Val.	Cal.	Val.	Cal.	Val.
Jacareí	201.0	0.8	0.75	0.71	0.87	0.44	0.42	2.08	-1.54	0.49	0.75	0.79	0.87
Watersheds	Area (km²)	V	E	NS	SE	RS	SR .	PBL	AS (%)	N	SE_{Log}		R^2
		Cal.	Val.	Cal.	Val.	Cal.	Val.	Cal.	Val.	Cal.	Val.	Cal.	Val.
Sub B-F24	172.8	0.83	0.78	0.85	0.76	0.41	0.47	-9.91	10.5	0.83	0.79	0.89	0.82
Sub B-F30	119.7	0.92	0.85	0.85	0.73	0.36	0.5	1.54	2.56	0.84	0.79	0.86	0.73
Cachoeira	97.1	0.71	0.70	0.78	0.81	0.5	0.53	-20.3	-13.5	0.58	0.53	0.87	0.85
Watanahada	Area	Area VE		NSE		RS	'R	PBIA	S (%)	N.	SE_{Log}		R^2
Watersheds	(km^2)	Cal.	Val.	Cal.	Val.	Cal.	Val.	Cal.	Val.	Cal.	Val.	Cal.	Val.
Sub B-F34	135.5	0.85	0.84	0.53	0.35	0.7	0.68	14.6	2.59	0.42	0.38	0.84	0.69
Atibainha	176.2	0.80	0.72	0.75	0.74	0.44	0.53	9.41	-12.2	0.77	0.66	0.83	0.85
													_ 2
Watersheds	Area		/E		'SE		RSR		<i>IAS</i> (%)		SE_{Log}		R^2
D C	(km ²)	Cal.	Val.	Cal.	Val.	Cal.	Cal.						Val.
P. Castro	333.7	0.81	0.78	0.73	0.72	0.58	0.53	-2.81	8.54	0.67	7 0.63	0.9	0.74

B. Appendix

Fig. 3.5-1 shows that in the future there is no clear trend in the average discharge, since in some periods the curve exhibits an increase and in other periods a decrease. In addition, the average discharge per time period showed higher values during the 2041-2070 scenarios. On the other hand, the average discharge per model showed higher values in the Eta/HadGEM model results compared to the Eta/MIROC5 model.

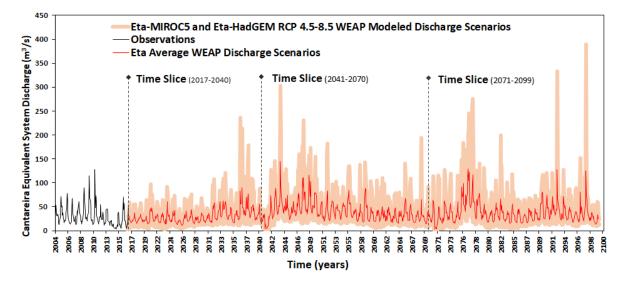


Figure B-1. Discharge projection scenarios modeled in WEAP, driven by RCM Eta-MIROC5 and Eta-HadGEM under RCP 4.5 - 8.5 scenarios.

50 C. Appendix.

51 Fit diagnostic plot of Generalized Extreme Value (GEV) distribution.

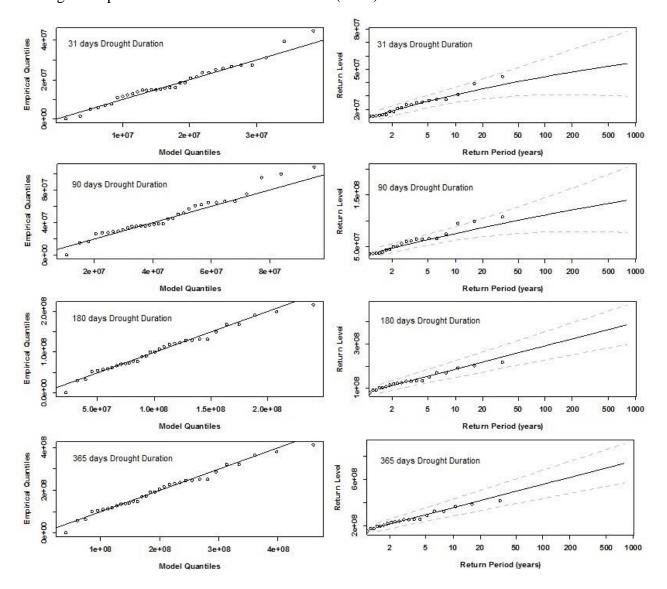


Figure C-1. Diagnostic plots for stationary GEV model under historical Eta-HadGEM scenario and stationarity demand (monthly drought duration intervals): Left panel QQ-plot in [m³]; Right panel, return level [m³] vs return period plot.

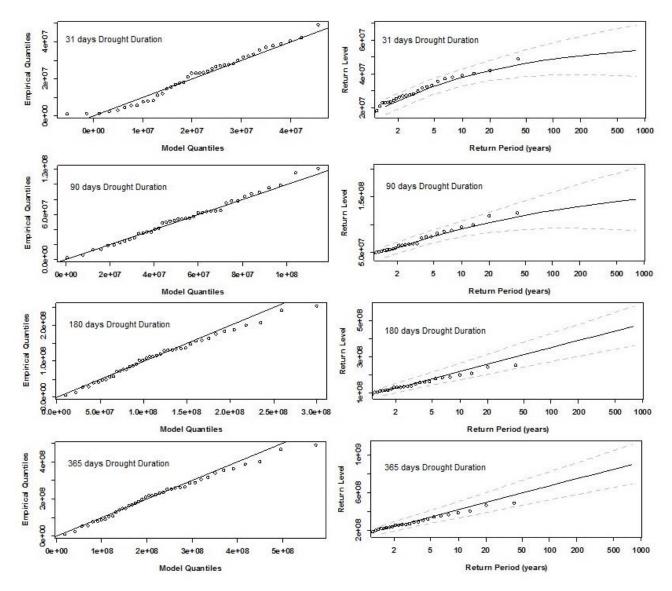


Figure C-2. Diagnostic plots for stationary GEV model under historical Eta-HadGEM scenario and non-stationarity demand (monthly drought duration intervals): Left panel QQ-plot [m³]; Right panel [m³], return level vs return period plot.

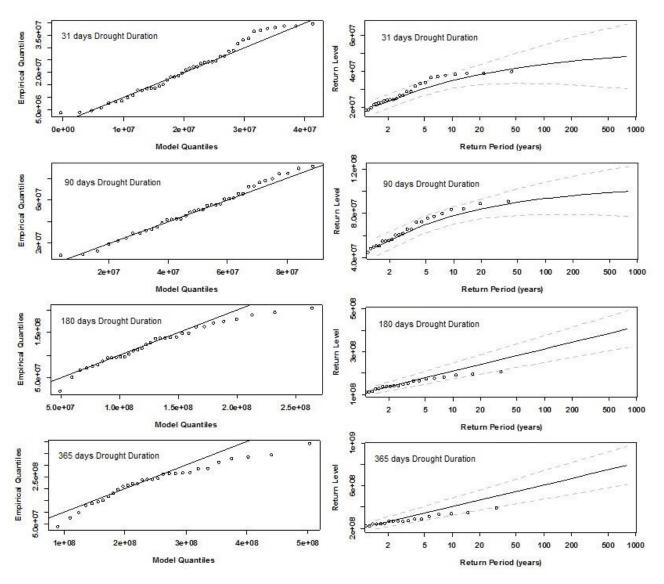


Figure C-3. Diagnostic plots for stationary GEV model under historical Eta-MIROC5 scenario and stationarity demand (monthly drought duration intervals): Left panel QQ-plot [m³]; Right panel, return level [m³] vs return period plot.

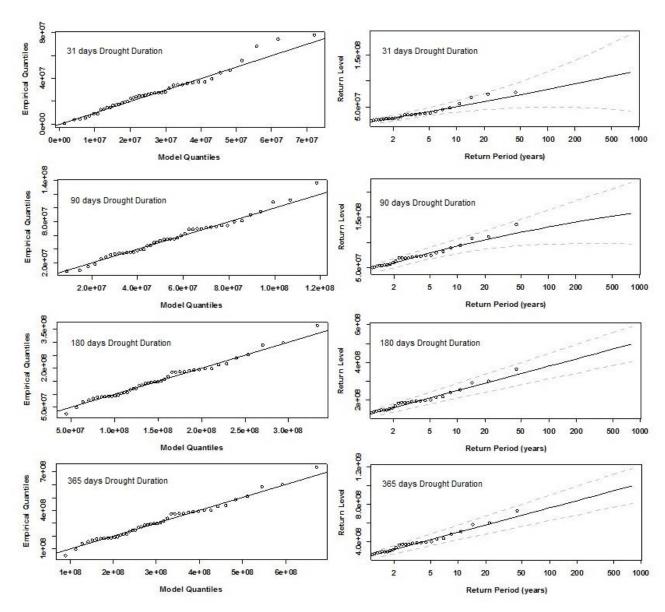


Figure C-4. Diagnostic plots for stationary GEV model under historical Eta-MIROC5 scenario and non-stationarity demand (monthly drought duration intervals): Left panel QQ-plot [m³]; Right panel, return level [m³] vs return period plot.

94 D. Appendix.

95

Table D-1. Adjusted parameters GEV distribution Adjusted for SDF curve under Eta-MIROC5. Hist.-Stationary

97 Demand scenario.

Drought	EtaMIROC5	Negative		
Duration	Location (µ)	Scale (o)	Shape (ξ)	Log- Likelihood
31 days	1.69E+07	1.06E+07	-2.88E-01	773.50
90 days	4.25E+07	2.29E+07	-3.67E-01	714.02
180 days	1.06E+08	4.48E+07	0.00E+00	629.90
365 days	2.00E+08	8.86E+07	0.00E+00	592.68

98 99

Table D-2. Adjusted parameters GEV distribution Adjusted for SDF curve under Eta-MIROC5. Hist.-Non-

100 Stationary Demand scenario.

Drought	EtaMIROC5 H	Negative		
Duration	Location (µ)	Scale (o)	Shape (ξ)	Log- Likelihood
31 days	1.90E+07	1.33E+07	2.74E-02	791.25
90 days	4.36E+07	2.40E+07	-1.07E-01	813.71
180 days	1.20E+08	5.62E+07	0.00E+00	853.50
365 days	2.42E+08	1.13E+08	0.00E+00	884.40

101

102

Table D-3. Adjusted parameters GEV distribution Adjusted for SDF curve under Eta-HadGEM. Hist.-Stationary

103 Demand scenario.

Drought	EtaHADGEM-	Negative		
Duration	Location (µ)	Scale (σ)	Shape (ξ)	Log- Likelihood
31 days	1.33E+07	8.64E+06	-1.10E-01	576.79
90 days	3.53E+07	1.88E+07	-5.54E-02	605.61
180 days	8.00E+07	4.56E+07	0.00E+00	631.75
365 days	1.53E+08	8.73E+07	0.00E+00	653.16

104105

Table D-4. Adjusted parameters GEV distribution Adjusted for SDF curve under Eta-HadGEM. Hist.-Non-

106 Stationary Demand scenario.

Drought	EtaHADGEN	Negative Log-		
Duration	Location (µ)	Scale (σ)	Shape (ξ)	Likelihood
31 days	1.62E+07	1.32E+07	-3.04E-01	728.85
90 days	4.13E+07	2.71E+07	-1.84E-01	761.14
180 days	8.63E+07	5.73E+07	0.00E+00	792.70
365 days	1.65E+08	1.10E+08	0.00E+00	819.49

107

108

E. Appendix

Histogram for the SABESP tariff adjustment data series during the period 2000-2016 (Figure

E-1) and Cantareira System Drought-Cost-Robustness curve (Figure E-2).

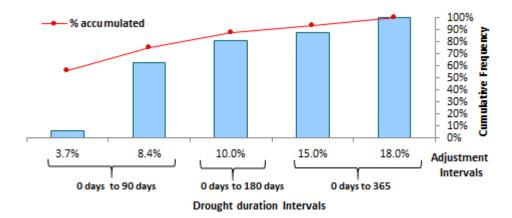


Figure E-1. Relationship assumptions between Drought duration intervals and water tariff adjustments. Series structure: 16 data in total; first interval 1 frequency, second interval 9 frequencies, third interval 3 frequencies, fourth interval 1 frequency and fifth interval 2 frequencies; average 7.85%, minimum 3.7% and maximum 18.9%.

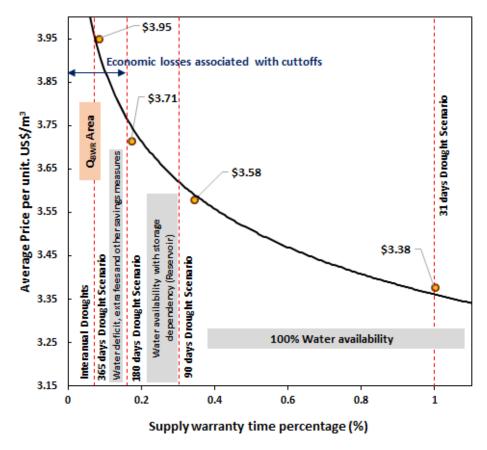


Figure E-2. Cantareira System Drought-Cost-Robustness curve, based on the water price and drought duration. The supply warranty time is a defined index for the construction of the drought impact curve. In this case, the draught impact curve describe the relationship between the duration of the drought (supply guarantee time), the water price adjustment rate and the system robustness. Supply warranty time is the ration between 100% Supply warranty time during 31 days and the Analysis Scenario of Supply warranty time (days). For example, 31 days/31days=1; 31days/90days=0.34; and 31days/180days=0.17 and 31 days/365 days=0.084.