

1    **Hydro-stochastic interpolation coupling with Budyko approach for prediction of**  
2    **mean annual runoff**

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## Abstract

Hydro-stochastic interpolation method based on traditional block-kriging has often been used to predict mean annual runoff in river basins. A caveat in such method is that the statistic technique provides little physical insight on relationships between the runoff and its external forcings, such as climate and landscape. In this study, the spatial runoff is decomposed into a deterministic trend and stochastic fluctuations describing deviations from it. The former is described by the Budyko method (Fu's equation) and the latter by stochastic interpolation. The coupled method of stochastic interpolation and the Budyko method is applied to spatially interpolate runoff in the Huaihe River Basin of China, after dividing it into 40 sub-basins. Results show that the coupled method significantly improves the accuracy of predicted mean annual runoff. The error of the predicted runoff from the coupled method is much smaller than that from the Budyko method and the hydro-stochastic interpolation method. The determination coefficient for cross-validation,  $R_{cv}^2$ , from the coupled method is 0.87, larger than 0.81 from the Budyko method and 0.71 from the hydro-stochastic interpolation. Further comparisons indicate that the coupled method also has reduced error in overestimating low runoff and underestimating high runoff suffered by the other two methods. These results support that the coupled method offers an effective and accurate way to predict mean annual runoff in river basins.

**Keywords:** Coupled Budyko and hydro-stochastic interpolation method; mean annual runoff; prediction accuracy; Huaihe River Basin

## 1. Introduction

Runoff observed at the outlet of a basin is a crucial element for investigating the hydrological cycle of the basin. Because runoff is influenced by both deterministic and stochastic processes, estimating the spatial patterns of runoff and associated distribution of water resources in ungauged basins has been one of the key problems in hydrology (Sivapalan et al., 2003), and a thorny issue in water management and planning (Imbach, 2010; Greenwood et al., 2011).

In estimating and predicting runoff and regional water resources availability, we have often used regional or global runoff mapping and geostatistical interpolation method. In these methods, the value of a regional variable at a given location is estimated as a weighted average of observed values at neighboring locations. This interpolation of runoff, which is assumed as an auto-correlated generalized stochastic field (Jones, 2009), uses secondary information often referring to more than one variable (Li and Heap, 2008). Spatial autocorrelation of the runoff values is measured by the covariance or semi-variance between pairs of locations as a function of their Euclidian distance (such as in the ordinary kriging). The values obtained by the interpolation methods are the best linear unbiased estimate in the sense that the expected bias is zero and the mean squared error is minimized (Skøien et al., 2006). The ordinary kriging (OK) estimates the local mean as a constant; corresponding residuals are considered as random. Because the spatial mean could also be used as a trend or nonstationary variation in space, OK has been developed into various geostatistical interpolation methods, such as kriging with a trend by incorporating local trend within a confined neighborhood as a smoothly varying

function of the coordinates. Block kriging (BK) is an extension of OK for estimating a block value instead of a point value by replacing the point-to-point covariance with the point-to-block covariance (Wackernagel, 1995).

Unlike precipitation or evaporation which we often interpolate to find its values at specific points in space, runoff is an integrated spatially continuous process in basins (Lenton and RodriguezIturbe, 1977; Creutin and Obled, 1982; Tabios and Salas, 1985; Dingman et al., 1988; Barancourt et al., 1992; Blöschl, 2005). Streamflows are naturally organized in basins (Dooge, 1986; Sivapalan, 2005), e.g., rivers flow through sub-basins. The river network constraints water balance between upstream and downstream in a basin. The hierarchically organized river network requires that the sum of the interpolated discharge from sub-basins equals to the observed runoff at the outlet of the entire basin. Previous studies have indicated that runoff interpolation may overestimate the actual runoff without adequate information of spatial variation of runoff (Arnell, 1995), e.g., neglecting the river network in connecting sub-basins or processing basin runoff behavior as points in space (Villeneuve et al, 1979; Hisdal and Tveito, 1993). In nested basins, Gottschalk (1993a and b) developed a hydro-stochastic approach for interpolating runoff. It takes the concept that runoff is an integrated course in the hierarchical structure of river network. Distance between a pair of basins is measured by geostatistical distance instead of the Euclidian distance. The covariogram among points in conventional spatial interpolation is replaced by covariogram between basins. In this concept, runoff is assumed spatially homogeneous over basins, i.e., the expected value of runoff is a constant in space (Sauquet, 2006).

The observed patterns of runoff reveal systematic deviations from homogeneity assumption because of influences heterogeneous rainfall among others. Meanwhile, we can describe the hydrological variables of interest in deterministic forms of functions, curves or distributions, and construct conceptual and mathematical models to predict hydro-climate variability (Wagener et al, 2007). For example, Qiao (1982), Arnell (1992) and Gao et al. (2017) used such approach and derived empirical relationships between runoff and its controlling factors of climate, land-cover and topography in various basins. However, the deterministic method in describing complex runoff patterns suffers inevitable loss of information (Wagener et al, 2007) because of existence of uncertainty in many hydrological processes and especially in observations. Thus, hydrological variables also contain information of stochastic nature and should be treated as outcomes from deterministic and stochastic processes. Recently, the method along this line is introduced: kriging with an external drift (KED) (Goovaerts, 1997; Li and Heap, 2008; Laaha et al., 2013). It accounts for deterministic patterns of spatial variables and incorporates the local trend within the neighborhood search window as a linear function of a smoothly varying secondary variable, instead of a function of spatial coordinates.

The inclusion of deterministic terms in the geostatistical methods has been shown to increase interpolation accuracy of basin variables, such as mean annual runoff (Sauquet, 2006), stream temperature (Laaha et al., 2013), and groundwater table (Holman et al., 2009). Those deterministic terms are often described by empirical formulae linking spatial features, e.g., variability of mean annual runoff in elevation (Sauquet, 2006), and relationship between the mean annual stream temperature and the

altitude of gauges (Laaha et al., 2013). As a semi-empirical approach to model the deterministic process of runoff, the Budyko framework has been popularly used to analyze relationship between mean annual runoff and climatic factors, e.g., aridity index (Milly, 1994; Koster and Suarez, 1999; Zhang et al., 2001; Donohue et al., 2007; Li et al., 2013; Greve et al., 2014). Many efforts have been devoted to improving the Budyko method by including effects of other external forcing factors, such as land-use and land-cover (Donohue et al., 2007; Li et al., 2013; Han et al., 2011; Yang et al., 2007), soil properties (Porporato et al., 2004; Donohue et al., 2012), topography (Shao et al., 2012; Xu et al., 2013; Gao et al., 2017), hydro-climatic variations of seasonality (Milly, 1994; Gentine et al., 2012; Berghuijs et al., 2014), and groundwater levels (Istanbulluoglu et al., 2012). However, it has been found that use of the deterministic equation in the Budyko method alone still comes with large errors in prediction of runoff in many basins (e.g., Potter and Zhang, 2009; Jiang et al., 2015).

The aim of this study is to combine the stochastic interpolation with semi-empirical Budyko method to further improve spatial interpolation/prediction of mean annual runoff in the Huaihe River Basin (HRB), China. In this study, the spatial runoff from sub-basins in the HRB is separated into the deterministic trend and its residuals both of which are estimated by the Budyko method and interpolation method. The residuals that are calculated as difference between the observed and the estimated runoff from the Budyko method, are used in the stochastic interpolation as described in Gottschalk (1993a, 1993b, and 2000). After that, the runoff of any sub-basin is predicted as the sum of the interpolated residuals and the Budyko estimated value. The improved method is

tested in the HRB. For comparison, the leave-one-out cross-validation approach was applied to evaluate performance of the three interpolation methods: the Budyko method, hydro-stochastic interpolation, and our coupled Budyko and stochastic interpolation method.

## 2. Methodologies

### 2.1 Spatial estimation of mean annual runoff by Budyko method

The Budyko method explains the variability of mean annual water balance on a regional or global scale. It describes dependence of actual evapotranspiration ( $E$ ) on precipitation ( $P$ ) and potential evapotranspiration ( $E_0$ ) (Williams et al., 2012). Their original relationship ( $E/P \sim E_0/P$ ) derived by Budyko (1974) is deterministic and nonparametric. It was later developed into parametric forms (Fu, 1981; Choudhury, 1999; Yang et al., 2008; Gerrits et al., 2009; Wang and Tang, 2014). Among them, the one-parameter equation derived by Fu (Fu, 1981, Zhang et al. 2004) has been used frequently. This relationship is written

$$\frac{E}{P} = 1 + \frac{E_0}{P} - \left(1 + \left(\frac{E_0}{P}\right)^\omega\right)^{\frac{1}{\omega}} \quad (1)$$

or

$$R = P \cdot \left(1 + \left(\frac{E_0}{P}\right)^\omega\right)^{\frac{1}{\omega}} - E_0 \quad (2)$$

where,  $P$ ,  $E$ ,  $E_0$ , and  $R$  are mean annual precipitation, actual evapotranspiration, potential evapotranspiration, and runoff (units: mm), respectively, and  $\omega$  is a dimensionless model parameter in the range of  $(1, \infty)$ . In these formulae, the larger the  $\omega$  is, the smaller the partition of precipitation into runoff.

The parameter  $\omega$  can be calculated using observed  $P$ ,  $E_0$ , and  $R$  in gauged sub-basins. The mean value of  $\omega$  of a basin can be obtained by averaging  $\omega$  of sub-basins, or by minimizing the mean absolute error ( $MAE$ ) in fitting the curve in Eq. (1) with  $E/P \sim E_0/P$  ( $E = P - R$ ) (Legates and McCabe, 1999). Using the mean value of  $\omega$ , Eq. (2) can be used to predict ungauged basin runoff or to interpolate spatial variation of runoff, using meteorological data in targeted sub-basins (Parajka and Szolgay, 1998).

## 2.2 Hydro-stochastic interpolation method

Gottschalk (1993a) described the hydro-stochastic interpolation method based on the kriging method for prediction of spatial runoff. Gottschalk's method redefines a relevant distance between basins, and identifies the river network and supplemental water balance constraints as follows.

As a spatially integrated continuous process, the predicted runoff of a specific unit of an area  $A_0$  in a basin,  $r^*(A_0)$ , can be expressed as

$$r^*(A_0) = \sum_{i=1}^n \lambda_i r(A_i) = \Lambda^T R \quad (3)$$

where,  $r(A_i)$  is the observed runoff in a gauged basin  $i$  with an area  $A_i$  ( $i = 1, \dots, n$ ,  $n$  is the total number of gauged basins),  $\lambda_i$  is the weight of basin  $i$ ,  $\Lambda$  is the transposed column vector of the weights, and  $R$  is the column vector of runoff  $r(A_i)$ .

Because  $r^*(A_0)$  is an estimate of the true value  $r(A_0)$ , the best linear unbiased estimate should satisfy  $E[r^*(A_0) - r(A_0)] = 0$ , where  $E$  is the expected value. If the runoff is taken as a point process at the location of interest  $u_0$ , the following set of equations has been developed to solve for the optimal weights under the second order



stationary assumption for hydrologic variables (Ripley, 1976),

$$\begin{cases} \sum_{j=1}^n \lambda_i \text{Cov}(u_i, u_j) + \mu = \text{Cov}(u_i, u_0), & i, j = 1, 2, \dots, n \\ \sum_{i=1}^n \lambda_i = 1. \end{cases} \quad (4)$$

In (4),  $\text{Cov}(u_i, u_j)$  is the theoretical covariance function between each pair of gauged stations ( $i=1, \dots, n, j=1, 2, \dots, n$ ),  $\text{Cov}(u_i, u_0)$  is the theoretical covariance of runoff between the location of interest  $u_0$  and each of the gauged stations  $u_i$ , and  $\mu$  is the Lagrange multiplier. After calculating the weights,  $\lambda_i$ , and substituting them into Eq. (3), we can solve for  $r^*(A_0)$ .

According to Sauquet et al. (2000), a basin consisting of  $n$  sub-basins with areas  $A_j$  ( $j = 1, \dots, n$ ) and observations of runoff can be further divided into  $M$  non-overlapping sub-basins with areas  $\Delta A_i$ . Those  $M$  sub-basins can be used as the fundamental units in hydro-stochastic interpolation. The sum of the interpolated runoff for each non-overlapping sub-basin should be equal to the observed runoff at the river outlet.

This constraint can be written as

$$R_T = \sum_{i=1}^M \Delta A_i r(\Delta A_i) \quad (5)$$

where,  $R_T$  is the streamflow observed at the outlet of the basin,  $\Delta A_i$  is the non-overlapping area of sub-basin  $i$ ,  $r(\Delta A_i)$  is the runoff depth for sub-basin  $i$  ( $i = 1, \dots, M$ ). The runoff prediction for each  $\Delta A_i$  is a linear combination of weights and runoff observations in the  $n$  sub-basins, i.e.,  $r(\Delta A_i) = \sum_{j=1}^n \lambda_j^i r(A_j)$ . Substituting it in Eq. (5) we can get

$$R_T = \sum_{i=1}^M \Delta A_i \left( \sum_{j=1}^n \lambda_j^i r(A_j) \right) = \sum_{i=1}^M n_i a \left( \sum_{j=1}^n \lambda_j^i r(A_j) \right). \quad (6)$$

In (6),  $r(A_j)$  is the runoff depth for sub-basin  $j$  ( $j = 1, \dots, n$ ) with discharge observations, and  $\lambda_j^i$  is the weight ( $i = 1, \dots, M; j = 1, \dots, n$ ).

Sauquet et al. (2000) divided a basin into  $n_T$  grids with equal area,  $a$ . The discharge data are converted into runoff depth under the assumption that the runoff distribution across each basin is uniform. Thus,  $R_T = n_T ar_T$  for the runoff of depth  $r_T$  at the outlet of the basin. Further considering basin areas in the river network, Sauquet et al. (2000) derived the weight matrices and described a hydro-stochastic method to optimize the weights  $\lambda_j^i$  ( $i= 1, \dots, M; j= 1, \dots, n$ ) in Eq. (6). Their interpolation is calculated simultaneously on multiple  $M$  non-overlapping sub-basins.

To develop the theoretical covariance function and weight matrices, the key step is to define the distance between pairs of sub-basins from the identified runoff hierarchical structure in the river network. Gottschalk (1993b) defined an appropriate geostatistical distance between sub-basins  $A$  and  $B$ ,

$$d(A, B) = \frac{1}{AB} \int \int_{AB} ||u_1 - u_2|| du_1 du_2 \quad (7)$$

where,  $A$  and  $B$  are the areas of sub-basins A and B, respectively,  $u_1$  and  $u_2$  are the locations of pairs of points inside basins A and B, and  $du_1$  and  $du_2$  are the differential symbol of  $u_1$  and  $u_2$ , respectively.

The theoretical covariogram,  $Cov(A, B)$ , is derived in a similar way as geostatistical distance by averaging the point process covariance function  $Cov_p$

$$Cov(A, B) = \frac{1}{AB} \int \int_{AB} Cov_p(||u_1 - u_2||) du_1 du_2 \quad (8)$$

where,  $Cov_p(||u_1 - u_2||)$  is the theoretical covariance function value of pairs of points in basins A and B with distance  $||u_1 - u_2||$ .

In Eq. (8),  $d(A, B)$  is calculated based on grid division in each of the sub-basins (Sauquet et al., 2000). We can obtain the mean distance  $d$  between all possible pairs of

points (point at the center of a grid) in sub-basins  $A$  and  $B$ . For  $n$  sub-basins with observations, there are  $n(n+1)/2$  pairs of the sub-basins with mean distance  $d_i$  ( $i=1, \dots, n(n+1)/2$ ).

Corresponding to  $d_i$ , the empirical covariogram  $Cov_e(d_i)$  can be calculated using the runoff depth of pairs of the sub-basins. The geostatistical distances  $d_i$  are then divided into fixed intervals (50 km in this study) to calculate the mean of  $Cov_e(d_i)$  within each of the distance interval. The mean of  $Cov_e(d_i)$  vs. the geostatistical distances  $d_i$  is used to draw a scatter diagram of  $Cov_e(d_i)$ .

The trial-and-error fitting method is used to calibrate  $Cov_p(d)$  in Eq. (8) to best fit  $Cov_e(d)$ . Only independent sub-basins are used to calculate the covariance function to avoid spatial correlation of nested sub-basins.

### 2.3 Coupling stochastic interpolation with Budyko method

The above stochastic interpolation procedure assumes a stationary stochastic variation of runoff among sub-basins or spatial homogeneity in runoff (Sauquet, 2006), despite variations in river network. For nonstationary variation of runoff resulting from spatial heterogeneity in a river network, the spatial runoff can be decomposed into nonstationary deterministic and stochastic components:

$$R(x) = R_d(x) + R_s(x). \quad (9)$$

In (9),  $R(x)$  is runoff at location  $x$ ,  $R_d(x)$  is the deterministic component of the spatial trend or the external drift (Wackernagel, 1995) that results in nonstationary variability in space.  $R_s(x)$  is the stochastic component considered to be stationary.

In this study,  $R$  in Eq. (2) is used as an external drift function in estimating the deterministic component  $R_d(x)$  in all sub-basins, i.e.,  $R_d(x)$  in Eq. (9) is substituted in Eq. (2) by setting  $R_d(x) = R$ . The residuals between  $R_d(x)$  and observed runoff are calculated for all gauged sub-basins. Furthermore, these residuals are interpolated for all ungauged sub-basins and set as the stochastic component  $R_s(x)$  in Eq. (9) using the "residual kriging" method (Sauquet, 2006). In particular,  $R_s(x)$  in Eq. (9) is replaced by  $r^*(A_0)$  in Eq. (3) after setting  $r^*(A_0) = R_s(x)$  for the stochastic interpolation scheme described in section 2.2. The superposition of these estimates of both components on the right-hand side in Eq. (9) yields the prediction of  $R(x)$ .

## 2.4 Cross validation

To validate this prediction procedure, we use the leave-one-out cross-validation method (Kearns, 1999). In addition, we examine and compare quantitatively the performances of our coupled model with the Budyko and the hydro-stochastic interpolation methods. The performance of each method is evaluated by the following metrics (Laaha and Blöschl, 2006):

$$MAE = \frac{1}{n} \sum_{j=1}^n [R(x_i) - R^*(x_i)] \quad (10)$$

$$MSE = \frac{1}{n} \sum_{j=1}^n [R(x_i) - R^*(x_i)]^2 \quad (11)$$

$$RMSE = \sqrt{\frac{1}{n} \sum_{j=1}^n [R(x_i) - R^*(x_i)]^2} \quad (12)$$

where,  $R^*(x)$  and  $R(x)$  are the predicted and observed runoff, respectively,  $MAE$  is mean absolute error,  $MSE$  is mean square error, and  $RMSE$  is the root-mean-square error.

The determination coefficient for cross-validation is

$$R_{cv}^2 = 1 - \frac{V_{cv}}{V_{NK}} \quad (13)$$

where,  $V_{cv}$  is the mean square error ( $MSE$ ), and  $V_{NK}$  is the spatial variance ( $V_{NK} = \frac{\sum_{j=1}^n [R(x_i) - \bar{R}]^2}{n-1}$ ), in which  $\bar{R}$  is the mean  $R(x)$  of the runoff over all the tested sub-basins. In addition to these evaluation metrics, the prediction result is evaluated by regression analysis of the observation vs. prediction.

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### 261 3. Study catchment and data

262 The Huaihe River Basin (HRB) – the sixth largest river basin in China, is used in  
 263 evaluation of our coupled model and in comparison of it to the other two methods. HRB  
 264 has a strong precipitation gradient from humid climate in the east and semi-humid in the  
 265 west (Hu, 2008), and it is one of the major agricultural areas in China with the highest  
 266 human population density in the country. Each year, millions of tons of water are  
 267 consumed to meet the domestic and agriculture needs. Water resources per capita and  
 268 per unit area is less than one-fifth of the national average. Moreover, more than 50% of  
 269 the water resources is exploited, much higher than the recommended 30% for  
 270 international inland river basins (Yan et al., 2011). Intense precipitation in a few very  
 271 rainy months makes the region highly vulnerable to severe floods as well as droughts  
 272 (Zhang et al., 2015). Thus, having the knowledge of spatial distribution of runoff is vital  
 273 for water resources planning and management for the region.

274 Our study area is located upstream of the Bengbu Sluice in HRB and has a size of  
 275 121,000 km<sup>2</sup> (Fig. 1). The river network in the area is derived from data packages of the  
 276 National Fundamental Geographic Information System, developed by National

Geomatics Center of China. HRB is divided into 40 sub-basins, according to available hydrological stations with records from 1961-2000 (Fig. 2). Areas of the sub-basins vary from the smallest of 17.9 km<sup>2</sup> to the largest of 30630 km<sup>2</sup>. Among the 40 sub-basins, 27 are independent sub-basins and 13 are nested sub-basins.

Annual precipitation data used in this study are from 1961-2000 and are obtained from a monthly mean climatological dataset at 0.5-degree spatial resolution. Those data were developed at China Meteorological Administration and are accessible at: [http://data.cma.cn/data/detail/dataCode/SURF\\_CLI\\_CHN\\_PRE\\_MON\\_GRID\\_0.5.htm](http://data.cma.cn/data/detail/dataCode/SURF_CLI_CHN_PRE_MON_GRID_0.5.htm). The dataset was derived from observations at 2472 stations in China, using Thin Plate Spline (TPS) interpolation method and the ANUSPLIN software. Pan evaporation data at 21 meteorological stations in HRB are used to interpolate  $E_0$  using the ordinary kriging interpolation method and ArcGIS. The interpolated  $E_0$  are used to derive the annual potential evapotranspiration in the sub-basins of HRB. The statistical features of mean annual precipitation ( $P$ ),  $E_0$  and runoff depth ( $R$ ) from 1961-2000 are summarized in Table 1. They show that  $P$  varied between 638-1629 mm, annual temperature between 11-16°C, and the mean annual  $E_0$  between 900-1200 mm. Sub-basins in the north, e.g., ZM, ZQ, XY and ZK in Fig. 2, are relatively dry with the dryness index ( $E_0/P$ ) above 1.3. Sub-basins in the south, e.g., MS, HBT and HC, are wetter with dryness index below 0.8. The average mean annual  $R$  is about 400 mm, fluctuating from a minimum of 90 mm in the north to a maximum of 1000 mm in the south. The temporal and spatial variation in runoff is relatively small in the south and large in the north.

## 4 Results

#### 4.1 Prediction of runoff by Budyko method

Actual evapotranspiration  $E$  is estimated using long-term mean annual water balance ( $E=P-R$ ) from 1961–2000 at the 40 sub-basins, and the results are shown in Table 1. Also in Table 1 are the calculated  $\omega$  values for the sub-basins. They vary from 1.43 of the sub-basin HWH to 3.16 of JJJ. The average of  $\omega$  is 2.32 for the 40 sub-basins. The comparison  $E/P$  vs.  $E_0/P$  is shown in Fig. 3. The best fit (curve) for  $E/P$  vs.  $E_0/P$ , or  $R$  vs.  $E_0/P$ , is also shown in Fig. 3; it gives an alternative for average  $\omega$  of the sub-basins. The fitted value of  $\omega$  for the 40 sub-basins determined from this process is 2.213, very close to that calculated directly from the 40 individual sub-basins.

Using  $\omega=2.213$  in HRB, Fu's equation in Eq. (2) can be written as

$$R = P \cdot \left( 1 + \left( \frac{E_0}{P} \right)^{2.213} \right)^{\frac{1}{2.213}} - E_0. \quad (14)$$

Eq. (14) and Fig. 3 clearly show the deterministic trend of runoff in the study basin. According to the water limit criterion,  $E = P$ , and the energy limit line criterion,  $E = E_0$ , in Fig. 3a, the smaller the index  $\frac{E_0}{P}$  is the smaller the  $\frac{E}{P}$  will be (Fig. 3a) or the larger the runoff will be (Fig. 3b) from the sub-basins in HRB. In Figs. 3b and 3c, the lower  $R$  in the northern sub-basins indicates drier conditions ( $E_0/P > 1.4$ ), while the higher  $R$  in the southern sub-basins assures wetter conditions ( $E_0/P < 0.8$ ).

Using  $P$  and  $E_0$  given in Table 1 for the 40 sub-basins, we predicted runoff  $R$  by Eq. (14), the Budyko method, and the deviation of the prediction from the observation. The results are summarized in Tables 1 and 2. The  $MAE$  of predicted  $R$  is 94 mm, and  $RMSE$  is 112 mm. The largest absolute error is in sub-basin HWH (328 mm), and the smallest in sub-basin XX (24 mm). The largest relative error is 81.6% of the observed runoff in

sub-basin XZ, and the smallest is 5.0% of the observed runoff in XHD. They represent absolute errors of 91 and 37 mm in those two sub-basins, respectively.

## 4.2 Runoff from the hydro-stochastic interpolation method

For comparison, the observed runoff was used in the hydro-stochastic interpolation following the procedure detailed in section 2.2. In order to obtain the distance  $d$  between the pairs of sub-basins, the study area is divided into 40 row  $\times$  50 column. According to Eq. (7), the geostatistical distance between any two sub-basins, A and B, is calculated by averaging the distances between all pairs of grid points in sub-basins A and B (all the possible pairs of sub-basins are  $40 \times 41/2$  for the 40 sub-basins in this study). According to the estimated distance for pairs of sub-basins and the observed runoff at the 40 sub-basins (Table 1), the empirical covariance  $Cov_e(d)$  is estimated for each pair of the sub-basins. From plots of the mean  $Cov_e(d)$  of the independent sub-basin pairs vs. the corresponding distance  $d$  with an interval of 50 km, we get an empirical covariogram that is shown in Fig. 4. The theoretical covariance function  $Cov_p(d)$  fitting to the empirical covariogram is determined

$$Cov_p(d) = 6 \times 10^5 \exp(-d/28.62). \quad (15)$$

This function is further used to calculate the average theoretical covariances  $Cov(A,B)$  in Eq. (8). Finally, the weight matrices are determined using our program in MatLab.

The interpolated runoff depth ( $R$ ) over the 40 sub-basins along with the deviations from the observation are shown in Table 1. The  $MAE$  and  $RMSE$  of  $R$  are 103 and 140 mm, respectively. The largest absolute and relative error is in the sub-basin JZ (401 mm



and 68.8%), and the smallest is in DPL (1 mm and 0.3%) (Table 2). These results indicate that the errors from this interpolation method are in general larger than those from the Budyko method, suggesting that the observed runoff is more influenced by the deterministic trend in the basin.

### 4.3 Hydro-stochastic interpolation with Fu's equation (our coupled method)

We use Fu's equation, Eq. (2), to evaluate the deterministic trend or the external drift function  $R_d^*(x)$ , and deviation of the trend from the observation,  $R_s^*(x)$ , assuming a spatially auto-correlated process. The  $R_s^*(x)$  is then used in the stochastic interpolation.

The empirical residual covariogram of  $R_s^*(x)$  for each pair of sub-basins vs. sub-basin distance is shown in Fig. 5. From the result in Fig. 5a, we obtain the exponential function for  $Cov_p(d)$

$$Cov_p(d) = 13030 \exp(-d/23.9). \quad (16)$$

From Eq. (16), weight matrices of runoff deviation are determined by Eq. (4) using our program in MatLab. They are then used to predict runoff deviation. The scatterplot of the predicted residuals vs. the observed residuals shown in Fig. 5b delineates a positive correlation between the predicted and the observed residuals. However, the large scatter indicates limited performance by the residual model alone. Because this interpolation scheme represents the spatial runoff deviation, the sum of the interpolated runoff deviation and the simulated runoff by Fu's equation is the total interpolated runoff in the sub-basins.

The predicted runoff using this procedure is given in Table 1, with the *MAE* at 71

mm and *RMSE* at 93 mm over the 40 sub-basins. The largest absolute error is in the sub-basin QL (220 mm) and the smallest in ZM (4 mm) (Table 2). The largest relative error is 47.2% of the observed runoff in XZ, and the smallest is 1% of the observed runoff in sub-basin BLY. They represent absolute error of 52 and 8 mm, respectively.

#### 4.4 Comparisons of predicted runoff by the three methods

Comparing the results summarized in Table 2, we find that our coupled method of the deterministic and stochastic processes substantially reduces the runoff prediction error in HRB. The *MAE* and *RMSE* of the runoff from our coupled method are much smaller than those from the Budyko and the hydro-stochastic interpolation methods. In cross-validation (Table 2), our coupled method has  $R_{cv}^2=0.87$ , larger than 0.81 and 0.71 from the Budyko method and the hydro-stochastic interpolation, respectively. The errors in runoff at the sub-basins are significantly reduced. The error in HWH is 216 mm from the coupled method, compared to 328 mm from the Budyko method and 300 mm from the hydro-stochastic interpolation. The error in JZ is 120 mm from the coupled method, compared to 179 mm from the Budyko method and 401 mm from the hydro-stochastic interpolation.

Our correlation analysis between predicted and observed  $R$  is shown in Fig. 6. The predicted runoff from our coupled method is highly correlated with the observed ( $R^2=0.87$ ). In contrast,  $R^2=0.82$  and 0.79 for the Budyko method and the hydro-stochastic interpolation, respectively. Our analysis indicates that the latter two methods overestimate low runoff and underestimate high runoff, as indicated by large departures

from 1:1 line in Fig. 6. Similar large deviation of the runoff predicted by the hydro-stochastic interpolation has also been reported by Sauquet et al. (2000), Laaha and Blöschl (2006), and Yan et al. (2011).

Spatial distributions of runoff in the HRB calculated from the three methods are shown in Fig. 7. They again show significant differences. Compared to the result from our coupled method (Fig. 7c), the Budyko method overestimates runoff in most of northern sub-basins (Fig. 7a), where climate is relatively dry and runoff is small (ranging from 140-280 mm). The hydro-stochastic interpolation method underestimates runoff in some southern sub-basins (Fig. 7b), where wet climate has fostered extremely high runoff (800~1100mm) in HWH, BLY, and ZC (Table 1). The results from our coupled method are closest to the observed distribution of runoff among the three methods (Fig. 7d). Compared to the errors in predicted runoff by the Budyko method and hydro-stochastic interpolation (Fig. 7 and Table 1), our coupled method reduces the error in 70% of all the sub-basins (28 of the 40 sub-basins).

## 5. Discussions and conclusions

In this study, we use the Budyko's deterministic method to describe mean annual runoff, which is an integrated spatially continuous process and determined by both the hydro-climatic elements and the hierarchical river network. A deviation from the Budyko estimated runoff is used by the stochastic interpolation that assumes spatially auto-correlated error. The deterministic aspects of runoff described in Budyko method are reflected in regional trends at locations (sub-basins), and deviations from the trends

caused by stochastic processes are taking into account by the weights as a function of autocorrelation and distance. Weights are larger for near points/basins and smaller for distant points/basins. Information from both the Budyko method and the stochastic interpolation are integrated in our coupled method to predict the runoff.

We have tested this coupled method and compared its results to the Budyko method and the hydro-stochastic interpolation in the Huaihe River basin (HRB) in China. Our results show that the deterministic process strongly affects spatial variations in runoff over the 40 sub-basins in HRB. By coupling the deterministic and the stochastic interpolation, our coupled method outperforms the Budyko method and the stochastic interpolation and significantly increases the runoff prediction accuracy. The *MAE* of predicted runoff over the 40 sub-basins in HRB by our coupled method is reduced to 71 mm from 94 mm in the Budyko method and 103 mm in the stochastic method; *RMSE* is reduced to 93 mm from 112 mm in the Budyko method and 140 mm in the stochastic method. The largest error in predicted runoff in the HRB, in the sub-basins HWH and JZ, is also substantially reduced. The runoff error in HWH is reduced to be 216 mm in our coupled method from 328 mm in the Budyko method and 300 mm in the stochastic method. Similarly, the runoff error in sub-basin JZ is reduced to be 120 mm in the coupled method from being 179 mm in the Budyko method and 401 mm in the hydro-stochastic method. The cross-validation results show that the deterministic coefficient  $R_{cv}^2$  in our coupled method is 0.87, larger than 0.81 and 0.71 in the Budyko and the hydro-stochastic interpolation method, respectively. Furthermore, for high and low runoff in sub-basins of HRB our coupled method gives more accurate predictions.

While substantial progress has been made by our coupled method, its results show that more effort is needed to further improve the accuracy of runoff prediction. For example, runoff prediction errors remain large from our coupled method in some sub-basins. In sub-basins MS, QL, HWH, and HNZ, the absolute error of predicted runoff is larger than 150mm and relative error of predicted runoff is larger than 20% of the observed runoff. In sub-basins BGS and XZ, the relative error of predicted runoff is larger than 40% of the observed runoff. Such large errors are largely attributed to large prediction errors intrinsic to the Budyko method (e.g., MS, QL, HWH, and XZ in Table 1). Possible causes to the errors could be from additional external factors influencing the runoff, such as land-cover, soil properties, hydro-climatic variations and groundwater. Including some or all these effects to improve the Budyko method or incorporating these effects as secondary information (e.g., multi-located co-kriging) will aid our understanding of the deterministic processes and help increase runoff prediction accuracy by our coupled method.

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**Captions of figures:**

1. Figure 1 Topography and river network of study area
2. Figure 2 Sub-basins and hydrological stations of study area.
3. Figure 3  $E/P \sim E_0/P$ , (b)  $R \sim E_0/P$  for the 40 sub-basins (the solid line is the best fit function), and (c) sub-basins in the north and south of the study basin. Note: in (b) and (c), blue color indicates wetter climate in the south and yellow color indicates drier climate in the north.
4. Figure 4 Empirical covariogram ( $Cov_e(d)$ ) from sub-basin runoff data and theoretical covariogram by fitted covariance function  $Cov_p(d)$  of study area.
5. Figure 5 (a) Empirical covariogram ( $Cov_e(d)$ ) from the residual  $R_s(x)$  and theoretical covariogram by fitted covariance function  $Cov_p(d)$  of study area, and (b) the scatterplot of the predicted vs. the observed residuals.
6. Figure 6 Cross validation of predicted runoff vs. observation by (a) Budyko method, (b) hydro-stochastic interpolation, (c) our coupled method, and (d) the scatterplot of the predicted vs. the observed residuals for (c). The dashed-line is 1:1.
7. Figure 7 Spatial distribution of mean annul runoff estimated from (a) Budyko method, (b) hydro-stochastic interpolation, (c) our coupled method, and (d) observation.

666 Table 1: Summary of hydro-meteorological data and predicted runoff of sub-basins in HRB.

No.	Station s	Basin area (km <sup>2</sup> )	P (mm)	R (mm)	E <sub>0</sub> (mm)	E <sub>0</sub> /P	E (mm)	Budyko method			Hydro-stochastic interpolation		Coupled method	
								$\omega$	Predicted R (mm)	Error (mm)	Predicted R (mm)	Error (mm)	Predicted R (mm)	Error (mm)
1	CTG	3090	1012	366	932	0.92	646	2.41	399	32.85	357	8.29	442	75.89
2	XHD	1431	1517	740	974	0.64	776	2.41	777	36.94	819	78.85	785	44.21
3	SQ	3094	822	168	1024	1.25	653	2.83	248	79.29	154	14.34	189	20.40
4	MS	1970	1517	672	957	0.63	845	3.06	786	114.28	705	33.18	833	161.55
5	BGS	2730	877	225	1029	1.17	651	2.57	279	53.93	331	105.51	321	95.80
6	XC	4110	945	225	997	1.06	720	3.02	332	106.82	197	27.83	261	35.87
7	BT	11280	910	223	993	1.09	687	2.85	310	86.94	205	18.10	220	3.73
8	ZK	25800	678	123	1061	1.56	555	2.54	163	39.96	101	21.54	101	21.60
9	JJJ	5930	1347	513	969	0.72	834	3.16	640	127.27	369	143.29	555	42.76
10	HB	16005	1092	335	937	0.86	757	3.15	455	120.48	197	137.61	383	48.20
11	ZQ	3410	739	118	1083	1.47	621	2.83	190	71.71	101	17.02	125	7.56
12	HPT	4370	1629	764	984	0.60	865	2.92	868	103.53	729	34.69	896	131.58
13	XX	10190	987	367	1053	1.07	620	2.10	343	23.77	297	70.54	325	41.95
14	BB	121330	850	215	1024	1.20	635	2.54	264	49.48	71	143.43	175	39.74
15	WJB	30630	1003	294	957	0.95	709	2.85	384	90.29	225	68.43	280	14.17
16	LZ	390	963	345	1078	1.12	618	2.09	320	24.96	335	10.87	337	8.57
17	NLD	1500	1019	439	1101	1.08	581	1.86	351	88.30	350	88.75	388	50.60
18	ZMD	109	690	212	1093	1.58	478	1.94	163	48.65	265	52.90	157	54.73
19	BLY	737	1504	868	1126	0.75	635	1.69	695	173.27	783	85.32	861	7.54
20	HWH	292	1560	1068	1127	0.72	492	1.43	740	328.03	768	299.97	852	216.14
21	ZC	493	1512	838	1112	0.74	674	1.79	708	130.23	700	137.94	790	48.34
22	BQY	284	1268	693	1094	0.86	575	1.68	527	166.21	543	150.04	568	125.47
23	QL	178	1559	970	1090	0.70	589	1.60	756	214.17	749	221.28	749	220.34
24	HNZ	805	1480	640	1114	0.75	840	2.41	681	41.37	576	63.94	816	175.57
25	TJH	152	1305	699	1090	0.84	605	1.74	556	143.66	309	390.52	556	143.05
26	LX	77.8	1025	484	1079	1.05	540	1.75	361	123.77	302	182.46	368	116.82
27	ZLS	1880	755	253	1104	1.46	502	1.91	194	58.45	197	55.37	223	29.21
28	ZT	501	1021	437	1101	1.08	583	1.87	351	85.87	212	225.14	452	14.74

29	XGS	375	830	302	1088	1.31	528	1.91	238	63.74	99	202.58	317	15.33
30	JZ	46	1103	583	1107	1.00	520	1.63	404	178.81	182	401.32	463	120.48
31	GC	620	638	111	1055	1.65	528	2.51	145	34.18	53	57.92	125	14.85
32	ZM	2106	645	97	1039	1.61	548	2.72	150	53.48	72	24.71	100	3.62
33	YZ	814	979	235	1083	1.11	743	2.85	329	94.07	271	35.66	321	85.76
34	XZ	1120	746	111	1040	1.39	636	3.06	202	90.66	84	27.12	163	52.32
35	GZ	1030	855	342	1098	1.28	513	1.81	250	92.10	230	111.80	260	81.82
36	DPL	1770	1067	331	1066	1.00	736	2.57	393	61.62	330	1.02	437	105.29
37	XX2	256	1301	606	1092	0.84	695	2.00	552	53.68	708	101.78	732	126.63
38	PH	17.9	1248	708	1094	0.88	540	1.61	512	196.04	605	102.78	564	144.41
39	HC	2050	1255	454	1095	0.87	802	2.54	517	63.36	328	125.79	537	83.61
40	HK	2141	871	227	1077	1.24	644	2.44	264	37.28	273	46.15	243	16.02

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671 Table 2: Interpolation cross-validation errors between the predicted and observed runoff in 40 sub-  
 672 basins from the three methods.

Evaluation indicators	Budyko method	Hydro-stochastic interpolation	Coupling method
<i>MAE</i> (mm)	94	103	71
<i>MSE</i> (mm <sup>2</sup> )	12561	19828	8557
<i>RMSE</i> (mm)	112	140	93
Max absolute error (mm)	328	401	220
Min absolute error (mm)	24	1	4
Max relative error (%)	82	69	47
Min relative error (%)	5	0.3	1
$R_{cv}^2$	0.81	0.71	0.87

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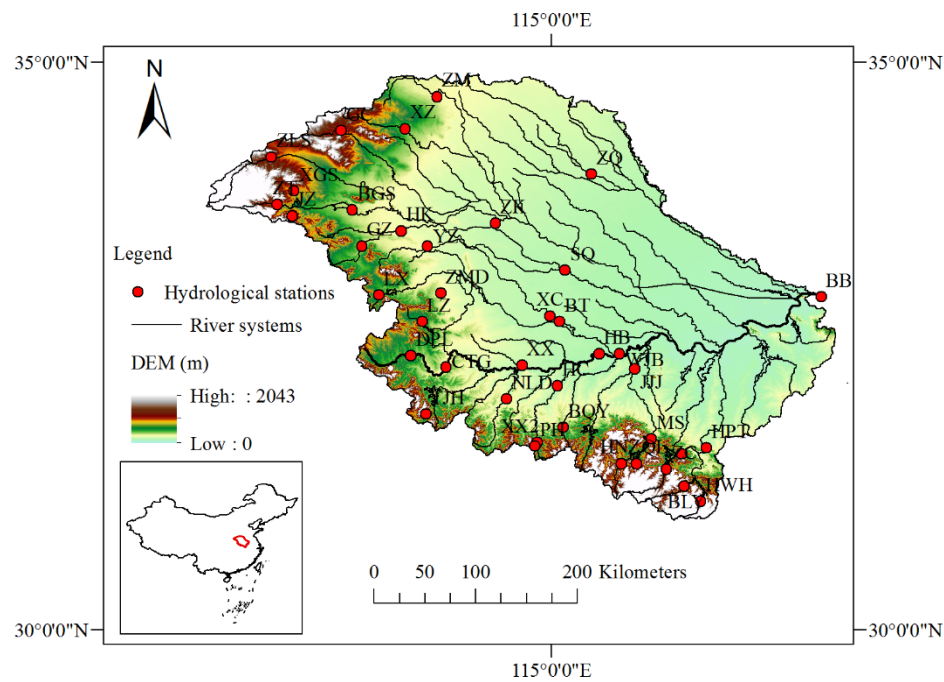


Figure 1: Topography and river network of study area.

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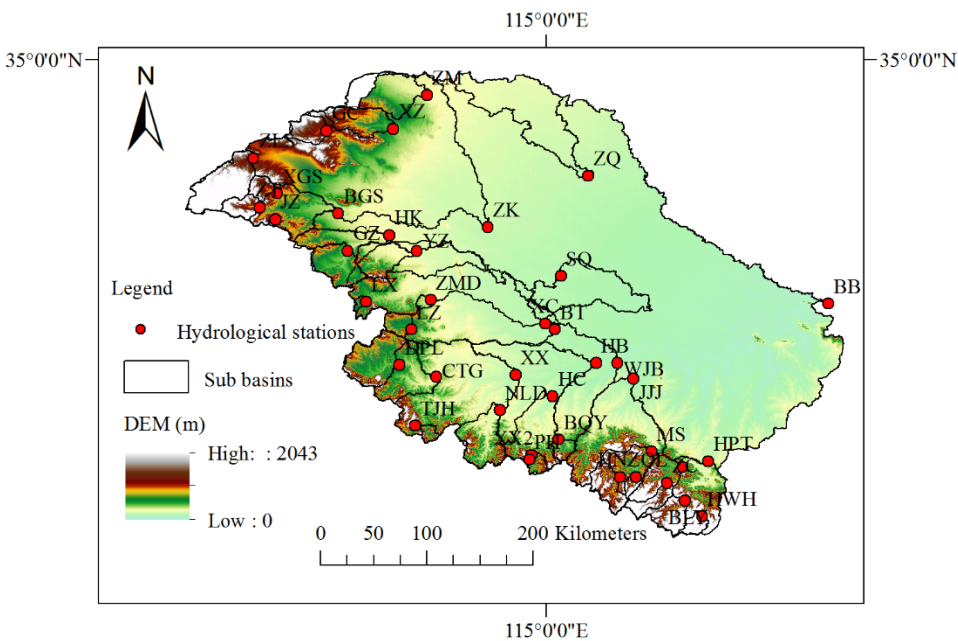


Figure 2: Sub-basins and hydrological stations of study area.

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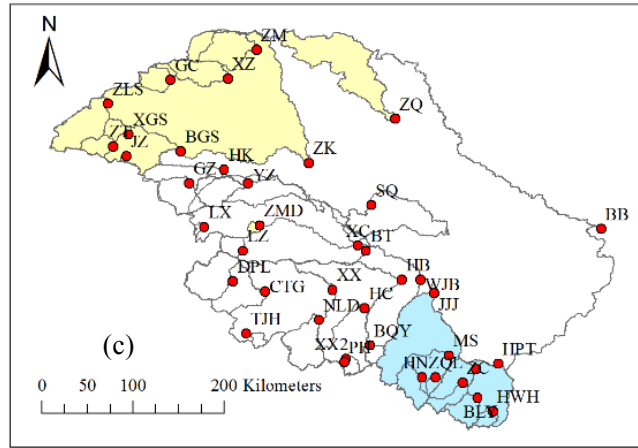
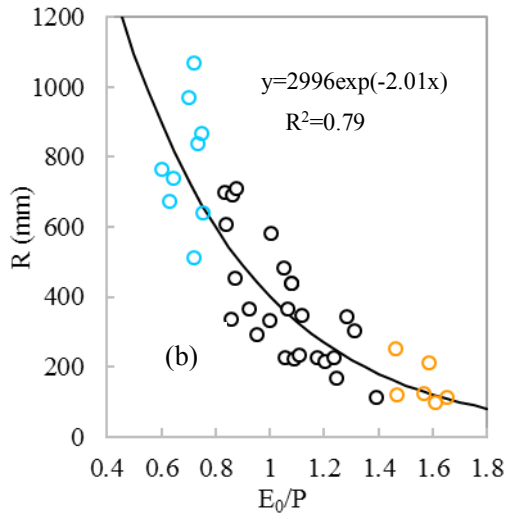
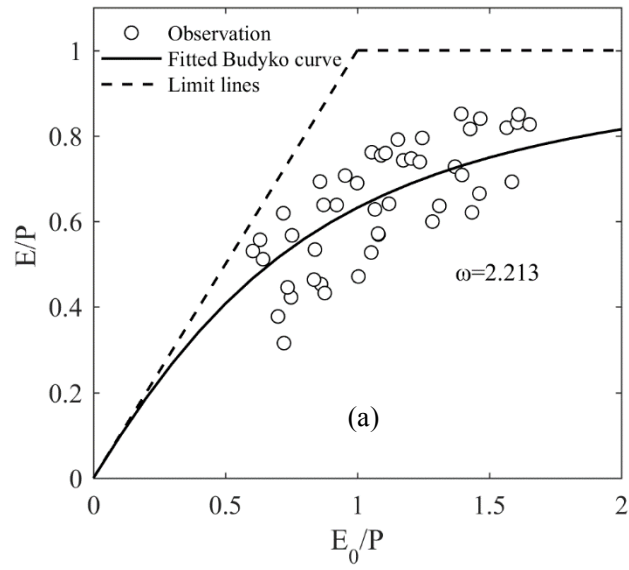
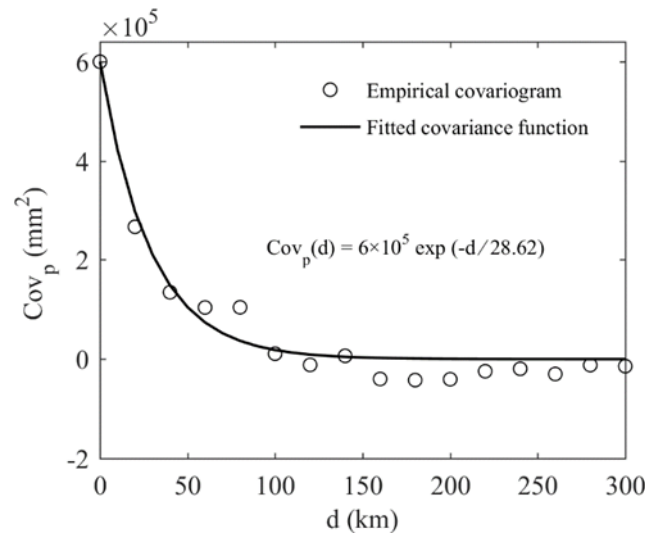


Figure 3: (a)  $E/P \sim E_0/P$ , (b)  $R \sim E_0/P$  for the 40 sub-basins (the solid line is the best fit function), and (c) sub-basins in the north and south of the study basin. Note: in (b) and (c), blue color indicates wetter climate in the south and yellow color indicates drier climate in the north.



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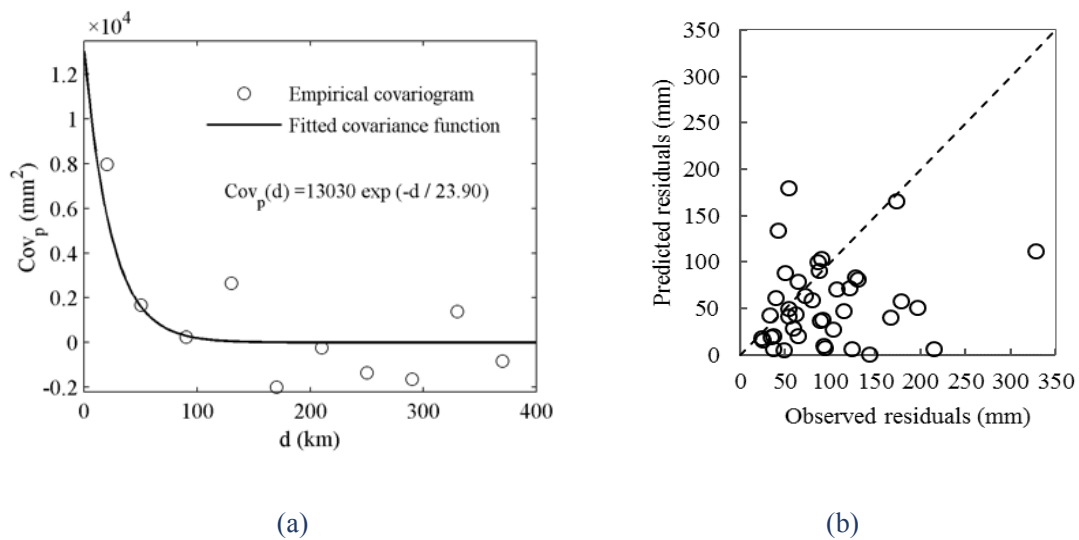


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697 Figure 4: Empirical covariogram ( $Cov_e(d)$ ) from sub-basin runoff data and theoretical covariogram

698 by fitted covariance function  $Cov_p(d)$  of study area.

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703 Figure 5: (a) Empirical covariogram ( $Cov_e(d)$ ) from the residual  $R_s(x)$  and theoretical covariogram

704 by fitted covariance function  $Cov_p(d)$  of study area, and (b) the scatterplot of the predicted vs. the

705 observed residuals.

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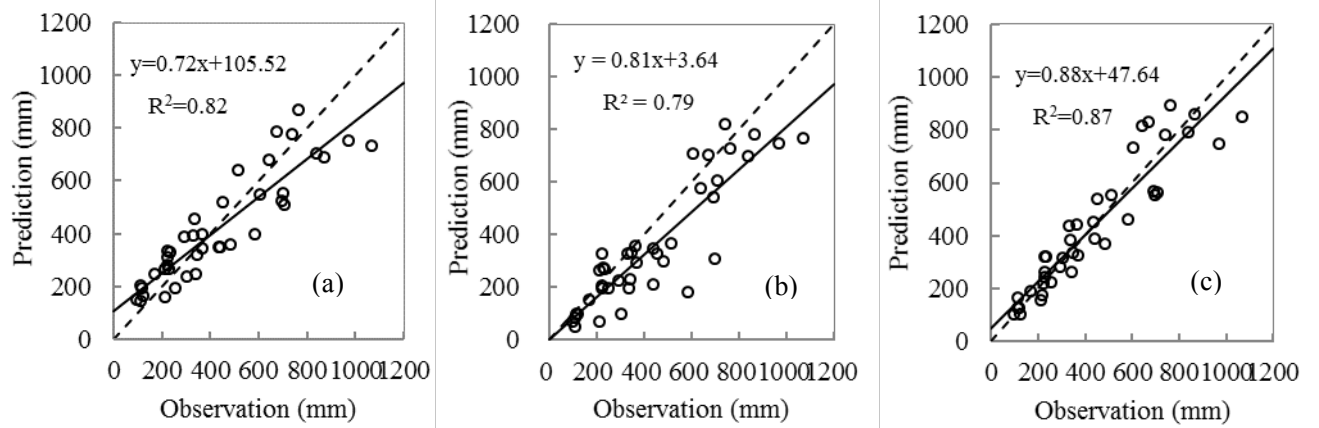


Figure 6: Cross validation of predicted runoff vs. observation by (a) Budyko method, (b) hydro-stochastic interpolation, and (c) our coupled method. The dashed-line is 1:1.

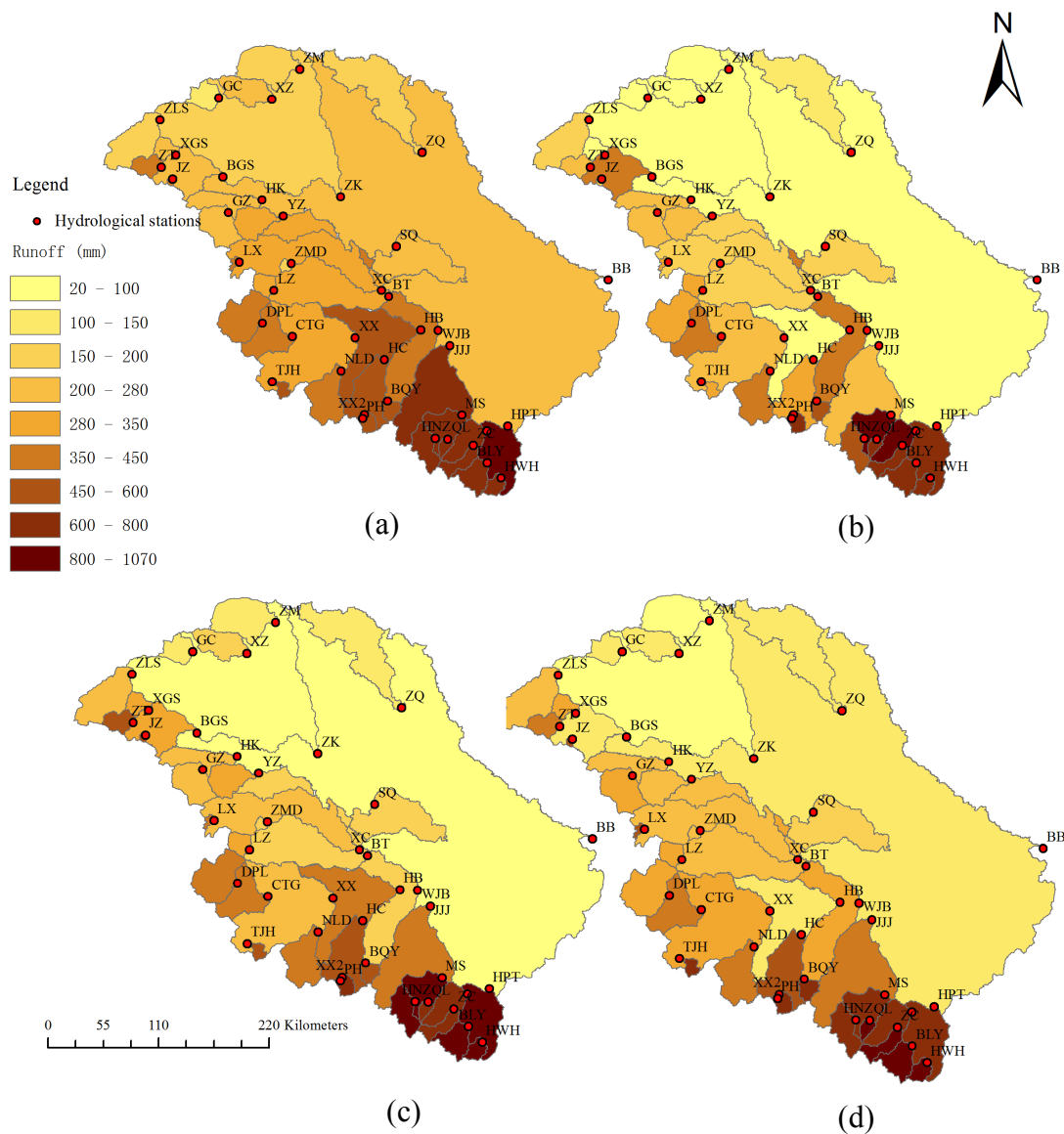


Figure 7: Spatial distribution of mean annual runoff estimated from (a) Budyko method, (b) hydro-stochastic interpolation, (c) our coupled method, and (d) observation.