

A dimensionless approach for the runoff peak assessment: effects of the rainfall event structure

Ilaria Gnecco¹, Anna Palla¹, Paolo La Barbera¹

¹Department of Civil, Chemical and Environmental Engineering, University of Genova, Genoa, 16145, Italy

5 *Correspondence to:* Anna Palla (anna.palla@unige.it)

Abstract. The present paper proposes a dimensionless analytical framework to investigate the impact of the rainfall event structure on the hydrograph peak. At this aim a methodology to describe the rainfall event structure is proposed based on the similarity with the Depth-Duration-Frequency (DDF) curves. The rainfall input consists in a constant hyetograph where all the possible outcomes in the sample space of the rainfall structures can be condensed. Soil abstractions are modelled using the Soil Conservation Service method and the Instantaneous Unit Hydrograph theory is undertaken to determine the dimensionless form of the hydrograph; the two-parameter gamma-distribution is selected to test the proposed methodology. The dimensionless approach is introduced in order to implement the analytical framework to any study case (i.e. natural catchment) for which the model assumptions are valid (i.e. linear causative and time invariant system). A set of analytical expressions are derived in case of constant-intensity hyetograph to assess the maximum runoff peak with respect to a given rainfall event structure irrespective of the specific catchment (such as the return period associated to the reference rainfall event). Looking at the results, the curve of the maximum values of the runoff peak reveals a local minimum point corresponding to the design hyetograph derived according to the statistical DDF curve. A specific catchment application is discussed in order to point out the dimensionless procedure implications and to provide some numerical examples of the rainfall structures with respect to observed rainfall events; finally their effects on the hydrograph peak are examined.

20 **1 Introduction**

The ability to predict the hydrologic response of a river basin is a central feature in hydrology. For a given rainfall event, estimating rainfall excess and transforming it to runoff hydrograph is an important task for planning, design and operation of water resources systems. For these purposes, design storm based on the statistical analysis of the annual maximum series of rainfall depth are used in practice as input data to evaluate the corresponding hydrograph for a given catchment. Several models are documented in the literature to describe the hydrologic response (e.g. Chow et al., 1988, Beven, 2012): the simplest and most successful is the unit hydrograph concept proposed firstly by Sherman (1932). Due to a limited availability of observed streamflow data mainly in small catchment, the attempts in improving the peak flow predictions are documented in the literature since the last century (e.g. Henderson, 1963; Meynink and Cordery, 1976) to date. Recently, Rigon et al. (2011) investigated the dependence of peak flows on the geomorphic properties of river basins. In the

framework of flood frequency analysis, Robinson and Sivapalan (1997) presented an analytical description of the peak discharge irrespective of the functional form assumed to describe the hydrologic response. Goel et al. (2000) combine a stochastic rainfall model with a deterministic rainfall-runoff model to obtain a physically based probability distribution of flood discharges; results demonstrate that the positive correlation between rainfall intensity and duration impacts the flood flow quantiles. Vogel et al. (2011) developed a simple statistical model in order to simulate observed flood trends as well as the frequency of floods in a nonstationary world including changes in land use, climate and water uses. Iacobellis and Fiorentino (2000) proposed a derived distribution of flood frequency identifying the combined role played by climatic and physical factors at the catchment scale. Bocchiola and Rosso (2009) developed a derived distribution approach for flood prediction in poorly gauged catchments to shift the statistical variability of rainfall process into its counterpart in terms of statistical flood distribution. Baiamonte et al. (2017) investigated the role of the antecedent soil moisture condition in the probability distribution of peak discharge and proposed a modification of the rational method in terms of a-priori modification of the rational runoff coefficients.

In this framework, the present research study takes a different approach by exploring the role of the rainfall event features on the peak flow rate values. Therefore the main objective is to implement a dimensionless analytical framework that can be applied to any study case (i.e. natural catchment) in order to investigate the impact of the rainfall event structure on hydrograph peak. Since the catchment hydrologic response and in particular the hydrograph peak is subjected to a very broad range of climatic, physical, geomorphic and anthropogenic factors, the focus is posed on catchments where lumped rainfall-runoff model are suitable for deterministic event-based analysis. In the proposed approach, the rainfall event structure is described by investigating the maximum rainfall depths for a given duration d in the range of durations $[d/2; 2d]$ within that specific rainfall event, differently from the statistical analysis of the extreme rainfall events. Other authors (e.g. Alfieri et al., 2008) have previously discussed the accuracy of literature design hyetographs (such as the Chicago hyetograph) for the evaluation of peak discharges during flood event on the contrary the proposed methodology allows to investigate the impact of the above mentioned rainfall event structure on the magnification of the runoff peak neglecting the expected rainfall event features condensed in the Depth-Duration-Frequency (DDF) curves.

The first specific objective is to define a structure relationship of the rainfall event able to describe the sample space of the rainfall event structures by means of a simple power function. The second specific objective is to implement a dimensionless approach that allows to generalize the assessment of the hydrograph peak irrespective of the specific catchment characteristic (such as the hydrologic response time, the variability of the infiltration process, etc.) thus focusing on the impact of the rainfall event structure.

Finally a specific catchment application is discussed in order to point out the dimensionless procedure implications and to provide some numerical examples of the rainfall structures with respect to observed rainfall events; furthermore their effects on the hydrograph peak are examined.

2 Methodology

A dimensionless approach is proposed in order to define an analytical framework that can be applied to any study case (i.e. natural catchment). It follows that both the rainfall depth and the rainfall-runoff relationship that are strongly related to the climatic and morphologic characteristics of the catchment, are expressed through dimensionless forms.

- 5 The rainfall event is then described as simple hyetographs of a given durations; this simplification is consistent with the use of deterministic lumped models based on the linear system theory (e.g. Bras, 1990). The proposed approach is therefore valid within a framework that assumes that the watershed is a linear causative and time invariant system, where only the rainfall excess produces runoff. In detail, the rainfall-runoff processes are modelled using the Soil Conservation Service (SCS) method for soil abstractions and the Instantaneous Unit Hydrograph (IUH) theory. Consistently with the assumptions of the
- 10 UH theory, the proposed approach is strictly valid when the following conditions are maintained: the known excess rainfall and the uniform distribution of the rainfall over the whole catchment area.

2.1 The dimensionless form of the rainfall event structure function

- Rainfall DDF curves are commonly used to describe the maximum rainfall depth as a function of duration for given return periods. In particular for short durations, rainfall intensity has often been considered rather than rainfall depth, leading to
- 15 intensity-duration-frequency (IDF) curves (Borga et al., 2005). Power laws are commonly used to describe DDF curves in Italy (e.g. Burlando and Rosso, 1996) and elsewhere (e.g. Koutsoyiannis et al., 1998). The proposed approach describes the internal structure of rainfall events based on the similarity with the DDF curves. Referring to a rainfall event, the maximum rainfall depth observed for a given duration is described in terms of a power function similarly to the DDF curve, as follows:

$$h(d) = a' d^n \quad (1)$$

- 20 where h [L] is the maximum rainfall depth, a' [LT^{-n}] and n [-] are respectively the coefficient and the structure exponent of the power function for a given duration, d [T]. For each duration d_i , the corresponding power function exponent, n , is estimated based on the maximum rainfall depth values observed in the range of duration $[d_i/2; 2d_i]$ by means of a simple linear regression analysis. Based on such assumptions, the structure exponent n allows describing the rainfall event based on a simple rectangular hyetograph thus representing the rainfall event structure at a given duration. In other words, a rainfall
- 25 event that is characterized by a specific n structure exponent at a given duration is only one of the possible outcomes in the sample space of the rainfall structures. The n structure exponent mathematically ranges between 0 and 1: the two extreme values represent un-realistic events characterized by opposite internal structure; when the structure exponent n tends to zero the internal structure of the rainfall event is comparable to a Dirac impulse while it is comparable to a constant intensity rainfall for n close to one. As an example, Figure 1 describes the rainfall event structure according to the above illustrated
- 30 approach. In Fig. 1, the observed rainfall depth (at the top), the observed maximum rainfall depths (at the centre), and the corresponding rainfall structure exponent (at the bottom) are reported on hourly basis.

In order to correlate the rainfall event structure function to the DDF curve, a reference rainfall event has to be defined in terms of the maximum rainfall depth, h_r , occurred for the reference duration, t_r . Focusing on a given catchment, the reference duration, t_r is assumed equal to the hydrologic response time of the catchment; thus, assuming a specific return period T_r [T], the reference value of the maximum rainfall depth, h_r [L], is defined according to the corresponding DDF curves, as follows:

$$h_r(T_r, t_r) = a(T_r)t_r^b \quad (2)$$

where $a(T)$ [LT^{-b}] and b [-] are respectively the coefficient and the scaling exponent of the DDF curve.

Referring to a rainfall duration corresponding to t_r the rainfall depth is assumed equal to the reference value of the maximum rainfall depth. Based on this assumption a relationship between the parameters of the DDF curve and the rainfall event structure function can be derived as follows:

$$h(t_r) = h_r(T_r, t_r) \rightarrow a't_r^n = a(T_r)t_r^b \rightarrow \frac{a'}{a(T_r)} = \frac{t_r^b}{t_r^n} \quad (3)$$

From Eq. (3) it is possible to derive the coefficient of the rainfall event structure function, a' for a given reference duration, t_r . Note that the a' coefficient is assumed valid in the range $[t_r/2; 2t_r]$ similarly to the n structure exponent.

The dimensionless approach is then introduced since it allows defining an analytical framework that can be applied to any study case (i.e. natural catchment) for which the model assumptions are valid (i.e. linear causative and time invariant system). The reference values h_r and t_r are directly linked to the climatic and morphologic characteristics of the specific catchment, therefore the dimensionless approach based on the h_r and t_r values allows to generalize the results irrespective of the specific catchment characteristic (such as the return period associated to the reference rainfall event).

Based on the proposed approach, the dimensionless form of the rainfall depth, h_* , is defined by the ratio of the rainfall depth, h , to the reference value of the maximum rainfall depth, h_r ; similarly the dimensionless duration, d_* , is expressed by the ratio of the duration, d , to the reference time, t_r . Therefore, the dimensionless form of the rainfall structure relationship may be expressed utilizing Eqs. (1), (2) and (3):

$$h_*(d_*) = \frac{h}{h_r} = \frac{a' d^n}{a(T_r)t_r^b} = \frac{d^n}{t_r^n} = d_*^n \quad (4)$$

2.2 The dimensionless form of the Unit Hydrograph

The hydrologic response of a river basin is here predicted through a deterministic lumped model: the interaction between rainfall and runoff is analysed by viewing the catchment as a lumped linear system (Bras, 1990). The response of a linear system is uniquely characterized by its impulse response function, called the Instantaneous Unit Hydrograph (IUH). For the IUH, the excess rainfall of unit amount is applied to the drainage area in zero time (Chow et al., 1988).

To determine the dimensionless form of the unit hydrograph a functional form for the IUH and thus the S-hydrograph has to be assumed. In this paper the IUH shape is described with the two-parameter gamma distribution (Nash, 1957):

$$f(t) = \frac{1}{k\Gamma(\alpha)} \left(\frac{t}{k}\right)^{\alpha-1} e^{-\left(\frac{t}{k}\right)} \quad (5)$$

where $f(t)$ [T^{-1}] is the IUH, $\Gamma[-]$ is the gamma function, α [-] is the shape parameter while k [T] is the scale parameter. In the well-known two-parameter Nash model, the parameters α and k represent the number of linear reservoirs added in series and the time constant of each reservoir, respectively. The product αk is the first order moment thus corresponding to the mean lag time of the IUH. Note that the IUH parameters can be related to watershed geomorphology; in these terms the Geomorphologic Unit Hydrograph (GIUH) theory attempts to relate the IUH of a catchment to the geometry of the stream network (e.g. Rodriguez-Iturbe and Valdes, 1979; Rosso, 1984). The use of the Nash IUH allows defining an analytical framework to assess the relationship between the maximum dimensionless peak and the n structure exponent for a given dimensionless duration and similar analytical derivation can be carried out for simple synthetic IUHs. The dimensionless form of the IUH is obtained by using the dimensionless time, t_* , defined as follows:

$$t_* = \frac{t}{\alpha k} \quad (6)$$

The proposed dimensionless approach is based on the use of the IUH scale parameter as the reference time of the hydrologic response (i.e. $t_r = \alpha k$). Using the first order moment in the dimensionless procedure, the proposed approach can be applied to any IUH form even if, for experimentally derived IUH, the analytical solution of the problem is not feasible.

By applying the change of variable $t = \alpha k t_*$, the IUH may be expressed as follows:

$$f(t) = \frac{1}{k\Gamma(\alpha)} \left(\frac{\alpha k t_*}{k}\right)^{\alpha-1} e^{-\left(\frac{\alpha k t_*}{k}\right)} \quad (7)$$

The dimensionless form of the IUH, $f(t_*)$, is defined and derived from Eq. (7) as follows:

$$f(t_*) = f(t) \cdot \alpha k = \frac{\alpha}{\Gamma(\alpha)} (\alpha t_*)^{\alpha-1} e^{-(\alpha t_*)} \quad (8)$$

Note that for the dimensionless IUH the first order moment is equal to one and the time-to-peak, t_{I*} , can be expressed as follows:

$$\frac{df(t_*)}{dt_*} = 0 \rightarrow t_{I*} = \frac{\alpha-1}{\alpha} \quad (9)$$

The dimensionless Unit Hydrograph (UH) is derived by integrating the dimensionless IUH:

$$S(t_*) = \int_0^{t_*} f(\tau_*) d\tau_* \quad (10)$$

where $S(t_*)$ is the dimensionless S-curve (e.g. Henderson, 1963).

For a unit dimensionless rainfall of a given dimensionless duration, d_* , the dimensionless UH is obtained by subtracting the two consecutive S curves that are lagged d_* :

$$U(t_*) = \begin{cases} S(t_*) & \text{for } t_* < d_* \\ S(t_*) - S(t_* - d_*) & \text{for } t_* \geq d_* \end{cases} \quad (11)$$

where $U(t_*)$ is the dimensionless UH. The time-to-peak of the dimensionless UH, t_{p*} , is derived by solving $dU(t_*)/dt_* = 0$. Using (8) and (11) and recognizing that $t_{p*} \geq d_*$ gives the following equation for t_{p*} :

$$f(t_{p*}) = f(t_{p*} - d_*) \rightarrow t_{p*} = d_* \frac{\frac{\alpha d_*}{e^{\alpha-1}}}{\frac{\alpha d_*}{e^{\alpha-1}} - 1} = d_* \frac{1}{1 - e^{-\frac{\alpha d_*}{\alpha-1}}} \quad (12)$$

5 Similar expressions for the time-to-peak are available in the literature (e.g. Rigon et al., 2011; Robinson and Sivapalan, 1997). Consequently the peak value of the dimensionless UH may be expressed as a function of d_* by:

$$U_{max}(d_*) = S(t_{p*}) - S(t_{p*} - d_*) \quad (13)$$

2.3 The dimensionless runoff peak analysis

Based on the unit hydrograph theory and assuming a rectangular hyetograph of duration d_* , the dimensionless convolution
10 equation for a given catchment becomes:

$$Q(t_*) = i_e(d_*)U(t_*) \quad (14)$$

where $Q(t_*)$ is the dimensionless hydrograph and $i_e(d_*)$ is the dimensionless excess rainfall intensity.

Note that the hypothesis of rectangular hyetograph is not motivated in order to simplify the methodology but in order to describe the rainfall event structure. Based on such approach, the rainfall event structure at a given duration is represented
15 throughout the n structure exponent, it follows that the rainfall event is described by a simple rectangular hyetograph. It has to be noticed that the constant hyetograph derived by a given n structure is assumed valid in the same range of duration from which it is derived $[d_i/2; 2d_i]$.

In the following sections the dimensionless hydrograph and the corresponding peak are examined in case of constant and variable runoff coefficients.

20 2.3.1 The analysis in case of constant runoff coefficient

By considering a constant runoff coefficient, φ_0 [-], similarly to the dimensionless rainfall depth h_* the dimensionless excess rainfall depth h_{e*} is defined by:

$$h_{e*} = \frac{\varphi_0 h}{\varphi_0 h_r} = d_*^n \quad (15)$$

The corresponding dimensionless excess rainfall intensity becomes:

$$25 \quad i_{e*} = d_*^{n-1} \quad (16)$$

From Eqs. (13), (14) and (16), the dimensionless hydrograph and the corresponding peak may be expressed by:

$$Q(t_*) = d_*^{n-1} U(t_*) \quad (17)$$

$$Q_{max}(d_*) = d_*^{n-1} U_{max}(d_*) = d_*^{n-1} [S(t_{p*}) - S(t_{p*} - d_*)] \quad (18)$$

In order to investigate the critical condition for a given catchment which maximizes the runoff peak, the partial derivative of the Eq. (18) with respect to the variable d_* is calculated.

$$\frac{\partial Q_{max}(d_*)}{\partial d_*} = 0 \rightarrow \frac{f(t_{p*})d_*}{1-n} = S(t_{p*}) - S(t_{p*} - d_*) = U_{max}(d_*) \quad (19)$$

The analytical expression for estimating the critical duration of rainfall that maximizes the peak flow was firstly derived by Meynink and Cordery (1976). Similarly, from Eq. (19) it is possible to analytically derive the n structure value that maximizes the dimensionless runoff peak for a specific duration d_* referring to a given catchment:

$$n = 1 - \frac{f(t_{p*})d_*}{U_{max}(d_*)} \quad (20)$$

2.3.2 The analysis in case of variable runoff coefficient

The variability of the infiltration process across the rainfall event as well as the initial soil moisture conditions significantly affect the hydrological response of the catchment. In order to take into account these elements a variable runoff coefficient, φ , is introduced. The variable runoff coefficient is estimated based on the SCS method for computing soil abstractions (SCS, 1985). Since the analysis deals with high rainfall intensity events it would be reasonable to force the SCS-method in order to always produce runoff (Boni et al., 2007). The assumption that the rainfall depth always exceeds the initial abstraction is implemented in the model by supposing that a previous rainfall depth at least equal to the initial abstraction occurred; therefore, the excess rainfall depth h_e is evaluated as follows:

$$h_e = \varphi h = \frac{h^2}{h+S} \rightarrow \varphi = \frac{h}{h+S} \quad (21)$$

where S is the soil abstraction [L]. The variable runoff coefficient is therefore described as a monotonic increasing function of the rainfall depth. It follows that the runoff component is affected by the variability of the infiltration process: the runoff is reduced in case of small rainfall events and is enhanced in case of heavy events.

The dimensionless excess rainfall depth, h_{e*} , is defined by:

$$h_{e*} = \frac{h_e}{h_{e_r}} = \frac{\varphi h}{\varphi_r h_r} = \frac{\varphi}{\varphi_r} h_* = \frac{\varphi}{\varphi_r} d_*^n \quad (22)$$

where h_{e_r} [L] is the reference excess rainfall depth and φ_r [-] is the corresponding reference runoff coefficient.

The corresponding dimensionless excess rainfall intensity becomes:

$$i_{e*} = \frac{\varphi}{\varphi_r} d_*^{n-1} \quad (23)$$

From Eq. (21) the ratio $\frac{\varphi}{\varphi_r}$ may be determined in terms of h_* :

$$\frac{\varphi}{\varphi_r} = \frac{h/(h+S)}{h_r/(h_r+S)} = h_* \left(\frac{h_r+S}{h+S} \right) = h_* \left(\frac{1+S_*}{h_*+S_*} \right) \quad (24)$$

where S_* is the dimensionless soil abstraction defined by the ratio of S to h_r .

- 5 According to the dimensionless approach proposed in the present paper, different initial moisture conditions can be analyzed by considering different S_* associated with different CN conditions (i.e. CN_I or CN_{III} or different soil characteristics) for the same reference rainfall depth.

The ratio $\frac{\varphi}{\varphi_r}$ is lower than one when the dimensionless rainfall depth is lower than one and vice versa. In the domain of $h_* < 1$ (i.e. $d_* < 1$), the variable runoff coefficient implies that the runoff component is reduced with respect to the reference case

- 10 and vice versa. The impact of the ratio $\frac{\varphi}{\varphi_r}$ on the runoff production is enhanced if S_* increases thus causing a wider range of runoff coefficients.

From Eqs. (13), (14) and (23), the dimensionless hydrograph and the corresponding peak may be expressed by:

$$Q(t_*) = \frac{\varphi}{\varphi_r} d_*^{n-1} U(t_*) \quad (25)$$

$$Q_{max}(d_*) = \frac{\varphi}{\varphi_r} d_*^{n-1} U_{max}(d_*) = \frac{\varphi}{\varphi_r} d_*^{n-1} [S(t_{p*}) - S(t_{p*} - d_*)] \quad (26)$$

- 15 Similarly to the runoff peak analysis carried out in case of the constant runoff coefficient, the partial derivative of the Eq. (26) with respect to the variable d_* is calculated:

$$\frac{\partial Q_{max}(d_*)}{\partial d_*} = 0 \rightarrow f(t_{p*}) d_* = [S(t_{p*}) - S(t_{p*} - d_*)] \left[1 - 2n + \frac{nd_*^n}{d_*^n + S_*} \right] \quad (27)$$

From Eq. (27) it is possible to implicitly derive the n structure value that maximizes the dimensionless runoff peak for a specific duration d_* referring to a given catchment.

20 3 Results and discussion

The proposed dimensionless approach is derived using the two-parameter gamma distribution for the shape parameter equal to 3. Such assumption is derived by using the Nash model relation proposed by Rosso (1984) to estimate the shape parameter based on Horton order ratios according to which the α parameter is generally in the neighbourhood of 3 (La Barbera and Rosso, 1989; Rosso et al., 1991). In Figure 2, the dimensionless rainfall duration is plotted vs. the dimensionless time-to-

peak together with the dimensionless IUH and the corresponding dimensionless UH for $d_*=1.0$. Note that the dotted grey line indicates the UH peak while the dashed grey lines show t_{p*} , $f(t_{p*})$ and $f(t_{p*} - d_*)$, respectively.

The dimensionless UH is evaluated varying the dimensionless rainfall duration in the range between 0.5 and 2 according with the n structure definition in the range of durations $[d_i/2; 2d_i]$; then the runoff peak analysis is carried out in case of
5 constant and variable runoff coefficients. The achieved results are presented with respect to the above mentioned dimensionless duration range $[0.5; 2]$ that is wide enough to include the duration of the rainfall able to generate the maximum peak flow for a given catchment (Robinson and Sivapalan, 1997).

Finally the dimensionless procedure is referred to a small Mediterranean catchment. In the catchment application the dimensionless procedure is fully specified as from the evaluation of the rainfall structures associated with three observed
10 rainfall events as far as the determination of the reference peak flow and consequently of the dimensionless hydrograph peaks for the three observed rainfall structures.

3.1 Maximum dimensionless runoff peak with constant runoff coefficient

The dimensionless form of the hydrograph is shown in Figure 3 with varying the rainfall structure exponents, n , for the selected dimensionless rainfall duration. The hydrographs are obtained for excess rainfall intensities characterized by
15 constant runoff coefficient and rainfall structure exponents of 0.2, 0.3, 0.5 and 0.8.

The impact of the rainfall structure exponents on the hydrograph form depends on the rainfall duration: for d_* lower than one, the higher n the lower is the peak flow rate and vice versa. Figure 4 illustrates the 3D mesh plot and the contour plot of the dimensionless runoff peak as a function of the rainfall structure exponent and the dimensionless rainfall duration. In the 3D mesh plot as well as in the contour plot, it is possible to observe a saddle point located in the neighbourhood of d_* and n
20 values equal to 1 and 0.3, respectively. Note that the intersection line (reported as bold line in Fig. 4) between the saddle surface and the plane of the principal curvatures where the saddle point is a minimum indicates the highest values of the runoff peak for a given n structure exponent.

In Figure 5, the maximum dimensionless hydrograph peak and the corresponding rainfall structure exponent are plotted vs. the dimensionless time-to-peak. Further, the dimensionless IUH and the corresponding dimensionless UH for $d_*=1.0$ are
25 reported as an example. The reference line (indicated as short-short-short dashed grey line in Fig. 5) illustrates the lower control line corresponding to the rainfall duration infinitesimally small. Note that the rainfall structure exponent that maximizes the runoff peak for a given duration can be simply derived as a function of the dimensionless time-to-peak (see Eq. 20). The maximum dimensionless runoff peak curve tends to one for long dimensionless rainfall duration ($d_* > 3$) when the corresponding n structure exponent tends to one (see Eq. 18): for high-values of n structure, the critical conditions occur
30 for long durations that correspond to paroxysmal events for which the rainfall intensity remains fairly constant. The local minimum of the maximum dimensionless runoff peak curve (see Fig.5) occurs at t_{p*} of 1.29 corresponding to n structure value of 0.31 and d_* of 1, thus pointing out that the less critical runoff peak occurs at n structure exponent values

corresponding to the ones typically derived by the statistical analysis of the annual maximum rainfall depth series in Mediterranean climate. Furthermore, it can be observed that different rainfall event conditions (i.e. rainfall structure exponent n and duration d_*) in the neighborhood of the local minimum point could determine comparable effects in term of the runoff peak value.

5 3.2 Maximum dimensionless runoff peak with variable runoff coefficient

The excess rainfall depth, in the case of variable runoff coefficient, is evaluated by assigning a value to the reference runoff coefficient. In particular, the reference runoff coefficient is defined as follows utilizing Eq. (21):

$$\varphi_r = \frac{h_r}{h_r + S} \rightarrow \varphi_r = \frac{1}{1 + S_*} \quad (28)$$

In order to provide an example of the proposed approach, the presented results are obtained assuming a dimensionless soil abstraction S_* of 0.25. It follows that the reference runoff coefficient φ_r is equal to 0.8.

Similarly to the results presented for the case of constant runoff coefficient, Figure 6 illustrates the dimensionless hydrographs obtained for excess rainfall intensities characterized by variable runoff coefficient and n structure exponents of 0.2, 0.3, 0.5 and 0.8. at assigned dimensionless rainfall duration ($d_*=0.5, 1.0, 1.5,$ and 2.0). The dimensionless hydrographs, obtained for the variable runoff coefficient, show the same behaviours of the ones derived for the constant runoff coefficient (see Figs. 3 and 6), even if they differ in magnitude, thus confirming the role of the variable runoff coefficient on the runoff peak. In particular, due to the variability of the infiltration process, the runoff peaks slightly decrease for rainfall duration lower than one (i.e. $d_*=0.5$) when compared with the ones observed in case of constant runoff coefficient while they rise up for duration larger than one (i.e. $d_*=1.5$ and 2).

Figure 7 shows the 3D mesh plot and the contour plot of the dimensionless runoff peak as a function of the rainfall structure exponent and the dimensionless rainfall duration in case of variable runoff coefficient. By comparing Figs. 7 and 4, it emerges that the contour lines observed in case of variable runoff coefficient reveal a steeper trend with respect to constant runoff coefficient ones indeed the impact of the n structure exponent on the runoff peak is enhanced when the runoff coefficient is assumed variable. The saddle point is again located in the neighbourhood of d_* and n values equal to 1 and 0.3, respectively while the curve of the maximum values of the runoff peak (reported as bold line in Fig. 7) is moved on the left.

In Figure 8, the maximum dimensionless hydrograph peak and the corresponding rainfall structure exponent are plotted vs. the dimensionless time-to-peak in case of variable runoff coefficient. Results plotted in Fig. 8 confirm that the maximum runoff peak curve reveals the local minimum point at t_{p*} of 1.29 corresponding to n of 0.26 and d_* of 1. Referring to S_* of 0.25, the maximum dimensionless runoff peak tends to 1.25 for long dimensionless rainfall duration ($d_* > 3$) when consequently the n structure exponent tends to one (see Eqs. 24 and 26). Figure 9 illustrates the influence of different variable runoff coefficients (i.e. for instance different initial moisture conditions or different soil characteristics) on the maximum dimensionless runoff peak. Similarly to Fig. 8, the maximum dimensionless hydrograph peak (see the top graph)

and the corresponding rainfall structure exponent (see the centre graph) are plotted vs. the dimensionless time-to peak in case of variable runoff coefficient (for S_* values of 0.25 and 0.67) together with the comparison to the case of constant runoff coefficient. The maximum dimensionless runoff peak are similar for short rainfall duration (i.e. t_{p*} lower than 1.5) when the variable runoff coefficient reduces the runoff component with respect to the reference runoff case (that is also the constant runoff case i.e. $S_*=0$). On the contrary, the maximum dimensionless runoff peak increases with increasing the dimensionless soil abstraction for long rainfall duration. Such behaviour is due to the rate of change in the runoff production with respect to the rainfall duration: with increasing the rainfall volume the relevance of runoff with respect to the soil abstraction rises. In other words, the n structure exponent that maximizes the runoff peak, decreases for increasing the dimensionless soil abstractions (see Eq. 27).

10 3.3 Catchment application

In order to point out the dimensionless procedure implications and to provide some numerical examples of the rainfall event structures, the proposed methodology has been implemented for the Bisagno catchment at La Presa station, located at the centre of Liguria Region, (Genoa, Italy).

The Bisagno – La Presa catchment has a drainage area of 34 km² with an index flood of about 95 m³/s. The upstream river network is characterized by main channel length of 8.36 km and mean streamflow velocity of 2.4 m/s. Regarding the geomorphology of the catchment, the area (R_A), bifurcation (R_B) and length (R_L) ratios that are evaluated according to the Horton-Strahler ordering scheme, are respectively equal to 5.9, 5.6 and 2.5. By considering the altimetry, vegetation and limited anthropogenic exploitation of the territory, the Bisagno – La Presa is a mountain catchment characterized by an average slope of 33%. The soil abstraction, S_{II} is assumed equal to 41 mm; its evaluation is based on the land use analysis provided in the framework of the EU Project CORINE (EEA, 2009). The mean value of the annual maximum rainfall depth for unit duration (hourly) and the scaling exponent of the DDF curves are respectively equal to 41.31 mm/h and 0.39. Detailed hydrologic characterization of the Bisagno catchment can be found elsewhere (Bocchiola and Rosso, 2009; Rulli and Rosso, 2002; Rosso and Rulli, 2002). Focusing on the rainfall-runoff process the two parameters of the gamma distribution are evaluated based on the Horton order ratio relationship (Rosso, 1984). The shape and scale parameters are estimated equal to 3.4 and 0.25 h respectively, thus corresponding to the lag time of 0.85 h.

In this application three rainfall events observed in the catchment area have been selected in order to analyse the different runoff peaks occurred for the three rainfall event structures. For comparison purposes, the selected events are characterized by analogous magnitude of the maximum rainfall depth observed for the duration equal the reference time (i.e. $h_r = 80$ mm, $t_r = 0.85$ h).

30 Figure 10 illustrates the rainfall event structure curves derived for the three selected rainfall events. The graphs at the top report the observed rainfall depths while the central graphs show the estimated rainfall structure exponents. At the bottom of Fig. 10, by considering the three structure exponents corresponding to the Bisagno – La Presa reference time (i.e. $n = 0.55, 0.62, 0.71$), the rainfall event structure curves are derived for a rainfall durations ranging between $0.5 \cdot t_r$ and $2 \cdot t_r$; for

comparison purpose, the DDF curve is also reported. Based on each rainfall structure curve, four rectangular hyetographs with duration of 0.425, 0.85, 1.275, and 1.7 h in the range $[t_r/2; 2t_r]$ are derived to evaluate the impact on the hydrograph peak of the Bisagno – La Presa catchment. Note that the analysis is performed in case of variable runoff coefficient whose reference value is equal to 0.66 (i.e. $S_* = 0.5$; $S = 41$ mm). In Figure 11, the net hyetographs, the corresponding hydrographs and the reference value of the runoff peak flow are plotted for the three investigated rainfall structure exponents. The reference value of the runoff peak flow (dash-dot line) is evaluated by assuming a constant-intensity hyetograph of infinite duration and having excess rainfall intensity equal to the one estimated for the reference time. The role of the rainfall structure exponent emerges in the different decreasing rate of the excess rainfall intensity with the duration, thus resulting in the corresponding increasing rate of the peak flow values.

Figure 12 shows the contour plot of the dimensionless hydrograph peak in case of variable runoff coefficient ($S_* = 0.5$). The maximum runoff peak curve is also reported (bold line) together with the dimensionless hydrograph peaks (grey-filled stars) for the selected rainfall structure exponents ($n = 0.55, 0.62, 0.71$) and durations ($d_* = 0.5, 1.0, 1.5, \text{ and } 2.0$). Note that, these selected rainfall structures represent only three of the possible outcomes in the sample space of the rainfall structures that are described in the contour plot. Similarly to Fig. 7, the Bisagno – La Presa catchment application shows a curve of the highest values of the runoff peak characterized by a local minimum (saddle point) in the neighbourhood of d_* and n values equal to 1 and 0.3, respectively.

4 Conclusions

The proposed analytical dimensionless approach allows investigating the impact of the rainfall event structure on the hydrograph peak. At this aim a methodology to describe the rainfall event structure is proposed based on the similarity with the Depth-Duration-Frequency (DDF) curves. The rainfall input consists in a constant hyetograph where all the possible outcomes in the sample space of the rainfall structures can be condensed through the n structure exponent. The rainfall-runoff processes are modelled using the Soil Conservation Service (SCS) method for soil abstractions and the Instantaneous Unit Hydrograph (IUH) theory. In the present paper the two-parameter gamma distribution is adopted as IUH form; however the analysis can be repeated using other synthetic IUH forms obtaining similar results.

The proposed dimensionless approach allows defining an analytical framework that can be applied to any study case for which the model assumptions are valid; the site-specific characteristics (such as the morphologic and climatic characteristics of the catchment) are no more relevant being included within the parameters of the dimensionless procedure (i.e. $h_r(T_r)$ and t_r) thus allowing to figure out the implication on the hydrograph peak irrespective of the absolute value of the rainfall depth (i.e. the corresponding return period). A set of analytical expressions has been derived to provide the estimation of the maximum peak with respect to a given n structure exponent. Results reveal the impact of the rainfall event structure on the runoff peak thus pointing out the following features:

- the curve of the maximum values of the runoff peak reveals a local minimum point (saddle point);

- different combinations of n structure exponent and rainfall duration may determine similar conditions in terms of runoff peak;
 - analogous behaviour of the maximum dimensionless runoff peak curve is observed for different runoff coefficients although wider range of variation are observed with increasing soil abstraction values.
- 5 Referring to the Bisagno – La Presa catchment application ($h_r= 80\text{mm}$; $t_r= 0.85$ h and $S_*= 0.5$), the saddle point of the runoff peaks is located in the neighbourhood of n value equal to 0.3 and rainfall duration corresponding to the reference time ($d_*=1$). Further, it emerge that the maximum runoff peak value, corresponding to the scaling exponent of the DDF curve, is comparable to the less critical one (saddle point). Findings of the present research suggest reviewing the derived flood distribution approaches that coupled the information on precipitation via DDF curves and on the catchment response based
- 10 on the iso-frequency hypothesis. Future research with regard to the structure of the extreme rainfall event is needed; in particular the analysis of several rainfall data series belonging to a homogeneous climatic region is required in order to investigate the frequency distribution of specific rainfall structures.
- The developed approach, besides suggesting remarkable issues for further researches and unlike the merely analytical exercise succeeds in highlighting once more the complexity in the assessment of the maximum runoff peak.

15 References

- Alfieri, L., Laio F. Claps P.: A simulation experiment for optimal design hyetograph selection, *Hydrol. Process.*, 22, 813–820, doi: 10.1002/hyp.6646, 2008.
- Baiamonte, G., Singh, V.P.: Modelling the probability distribution of peak discharge for infiltrating hillslopes, *Water Resour Res*, 53(7), 6018–6032, doi: 10.1002/2016WR020109, 2017.
- 20 Beven, K.: *Rainfall-Runoff Modelling: The Primer: Second Edition*, Wiley-Blackwell, Chichester UK, 2012.
- Bocchiola, D. and Rosso, R.: Use of a derived distribution approach for flood prediction in poorly gauged basins: A case study in Italy, *Adv. Water Resour.*, 32, 1284–1296, doi:10.1016/j.advwatres.2009.05.005, 2009.
- Boni, G., Ferraris, L., Giannoni, F., Roth, G. and Rudari, R.: Flood probability analysis for un-gauged watersheds by means of a simple distributed hydrologic model, *Adv. Water Resour.*, 30(10), 2135–2144, doi:10.1016/j.advwatres.2006.08.009,
- 25 2007.
- Borga, M., Vezzani, C. and Dalla Fontana, G.: Regional rainfall depth-duration-frequency equations for an alpine region, *Nat. Hazards*, 36, 221–235, doi:10.1007/s11069-004-4550-y, 2005.
- Bras, R.L.: *Hydrology: An Introduction to Hydrological Science*, Addison-Wesley Publishing Company, New York, 1990.
- Burlando, P. and Rosso, R.: Scaling and multiscaling models of depth–duration–frequency curves for storm precipitation, *J.*
- 30 *Hydrol.*, 187, 45–64, doi:10.1016/S0022-1694(96)03086-7, 1996.
- Chow, V.T., Maidment, D.R. and Mays, L.W.: *Applied Hydrology*, McGraw-Hill, New York, 1988.

- European Environment Agency (EEA): Corine Land Cover (CLC2006) 100 m-version 12/2009, Copenhagen, Denmark, 2009.
- Goel, N., Kurothe, R., Mathur, B. and Vogel, R.: A derived flood frequency distribution for correlated rainfall intensity and duration, *J. Hydrol.*, 228, 56–67, doi:10.1016/S0022-1694(00)00145-1, 2000.
- 5 Henderson, F.M.: Some Properties of the Unit Hydrograph, *J. Geo- phys. Res.*, 68(16), 4785-4794, 1963.
- Jacobellis, V. and Fiorentino, M.: Derived distribution of floods based on the concept of partial area coverage with a climatic appeal, *Water Resour. Res.*, 36, 469–482, doi:10.1029/1999WR900287, 2000.
- Koutsoyiannis, D., Kozonis, D. and Manetas, A.: A mathematical framework for studying intensity–duration–frequency relationships, *J. Hydrol.*, 206, 118–135, doi:10.1016/S0022-1694(98)00097-3, 1998.
- 10 La Barbera, P. and Rosso, R.: On the fractal dimension of stream networks, *Water Resour. Res.*, 25(4), 735-741, doi:10.1029/WR025i004p00735, 1989.
- Meynink, W.J.C. and Cordery, I.: Critical duration of rainfall for flood estimation, *Water Resour. Res.*, 12(6), 1209-1214, doi:10.1029/WR012i006p01209, 1976.
- Nash, J.E.: The form of the instantaneous unit hydrograph, *International Union of Geology and Geophysics Assembly of*
- 15 *Toronto 3*, 114-120, 1957.
- Rigon, R., D'Odorico, P. and Bertoldi, G.: The geomorphic structure of the runoff peak, *Hydrol. Earth Syst. Sci.*, 15, 1853-1863, doi:10.5194/hess-15-1853-2011, 2011.
- Robinson, J.S. and Sivapalan, M.: An investigation into the physical causes of scaling and heterogeneity of regional flood frequency, *Water Resour. Res.*, 33(5), 1045-1059, doi:10.1029/97WR00044, 1997.
- 20 Rodriguez-Iturbe, I. and Valdes, J.B.: The geomorphic structure of hydrologic response, *Water Resour. Res.*, 18(4), 877–886, 1979.
- Rosso, R., Bacchi, B. and La Barbera, P.: Fractal relation of mainstream length to catchment area in river networks, *Water Resour. Res.*, 27(3), 381-388, doi:10.1029/90WR02404, 1991.
- Rosso, R.: Nash model relation to Horton order ratios, *Water Resour. Res.*, 20(7), 914-920, doi:10.1029/WR020i007p00914,
- 25 1984.
- Rosso, R. and Rulli, M.C. An integrated simulation method for flash-flood risk assessment: 2. Effects of changes in land-use under a historical perspective, *Hydrol. Earth Syst. Sci.*, 6(2), 285-294, 2002.
- Rulli, M.C. and Rosso, R.: An integrated simulation method for flash-flood risk assessment: 1. Frequency predictions in the Bisagno River by combining stochastic and deterministic methods, *Hydrol. Earth Syst. Sci.*, 6(2), 267-283, 2002.
- 30 Sherman, L.K.: Streamflow from rainfall by the unit-graph method, *Engineering News-Record*, 108, 501–505, 1932.
- Soil Conservation Service (SCS): *Hydrology, National Engineering Handbook, Section 4 – Hydrology*, U.S.D.A., Washington D.C, 1985.
- Vogel, R.M., Yaindl, C. and Walter, M.: Nonstationarity: Flood magnification and recurrence reduction factors in the United States, *J. Am. Water Resour. Assoc.*, 47(3), 464-474, doi:10.1111/j.1752-1688.2011.00541, 2011.

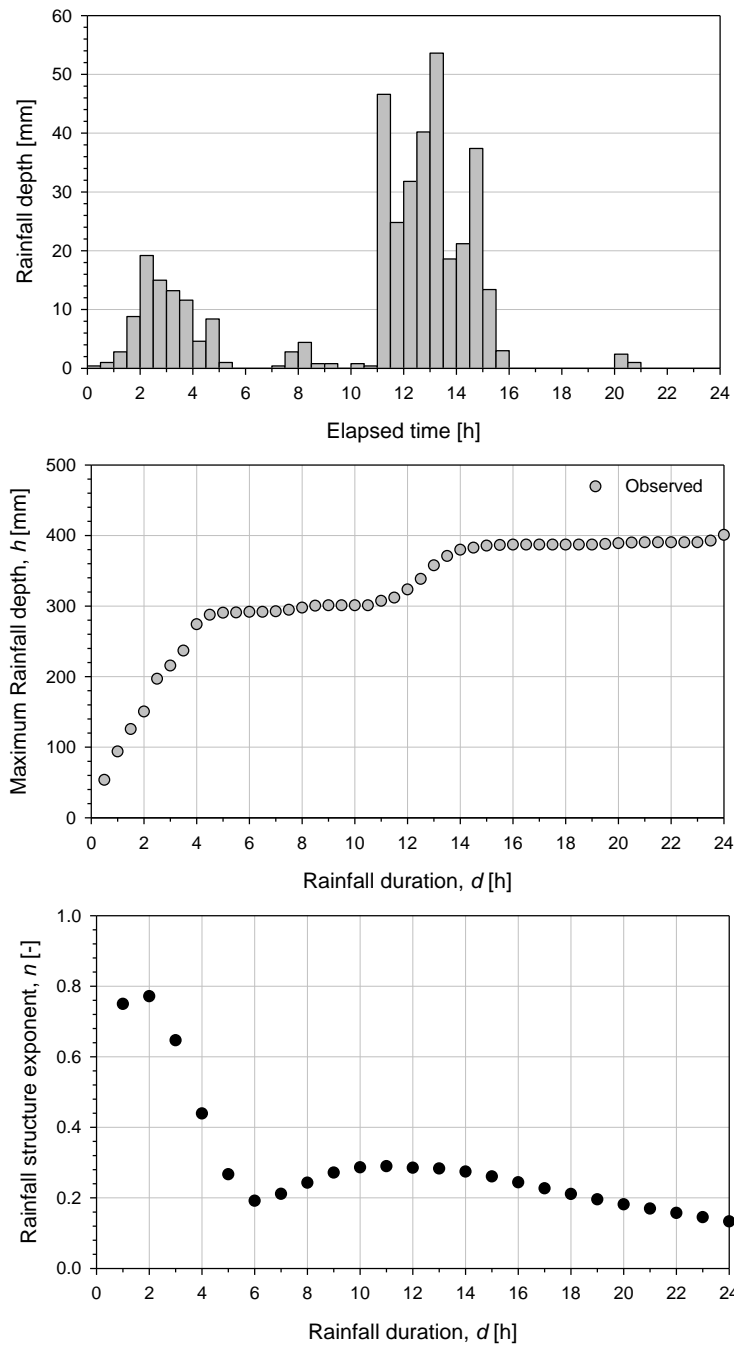


Figure 1: Rainfall event structure: the observed rainfall depth (at the top), the observed maximum rainfall depths (at the centre), and the corresponding rainfall structure exponent (at the bottom) are reported.

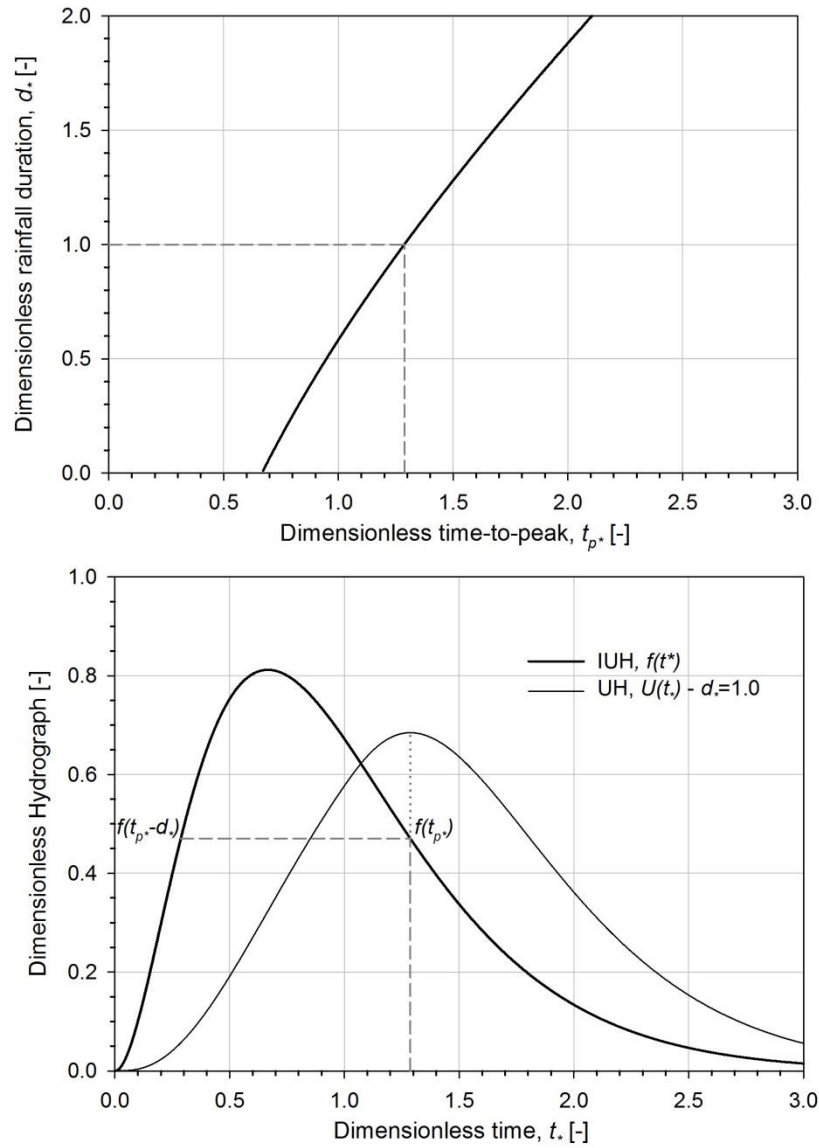


Figure 2: Dimensionless rainfall duration vs. dimensionless time-to-peak; Dimensionless Instantaneous Unit Hydrograph and the corresponding dimensionless Unit Hydrographs for $d_*=1.0$. Note that the shape parameter α is equal to 3.

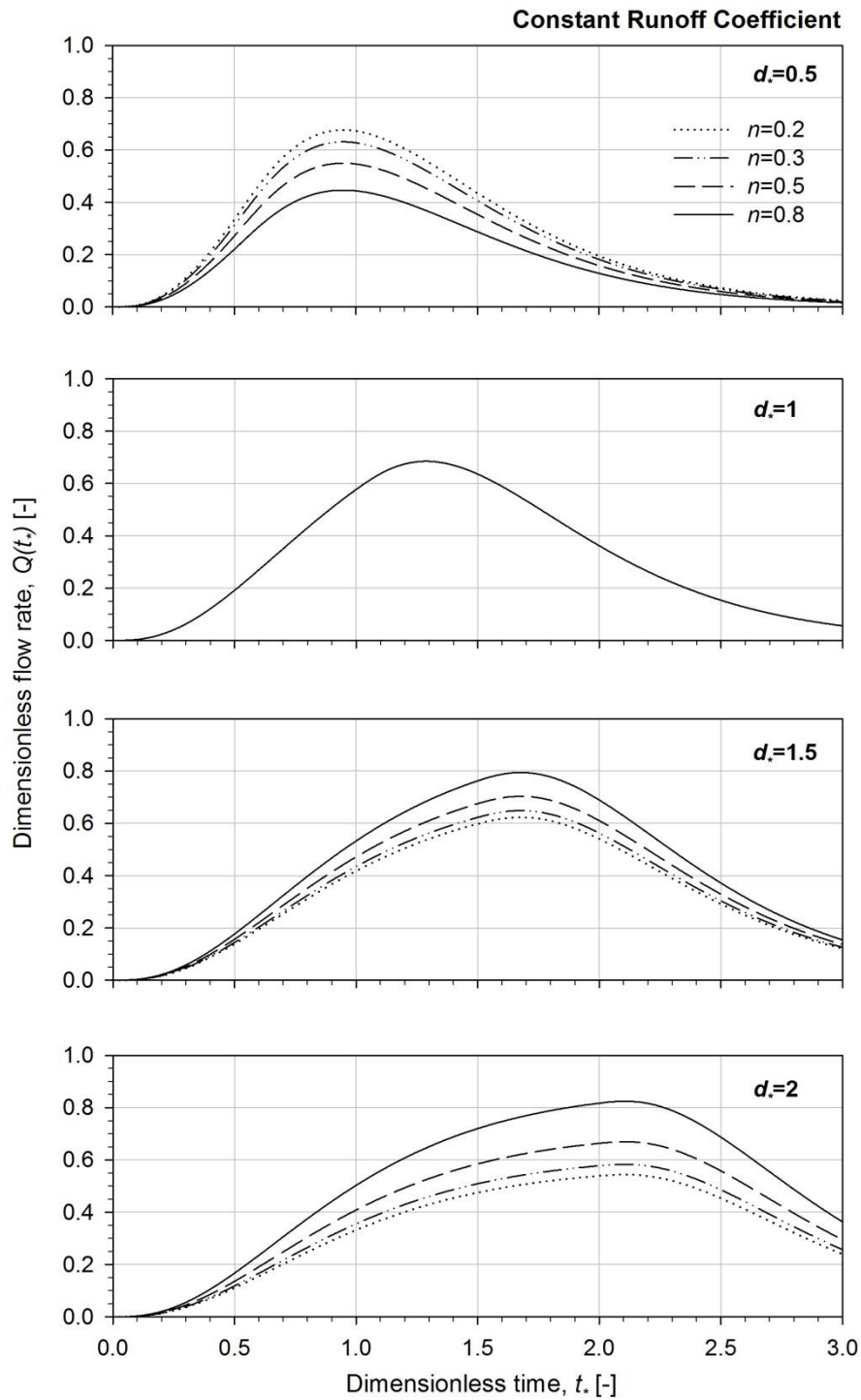


Figure 3: Dimensionless hydrographs obtained for excess rainfall intensities characterized by constant runoff coefficient and different rainfall structure exponents, n ($n = 0.2, 0.3, 0.4$ and 0.5) at assigned dimensionless rainfall duration, d_* ($d_*=0.5, 1.0, 1.5,$ and 2.0). Note that the shape parameter α is equal to 3.

Constant Runoff coefficient

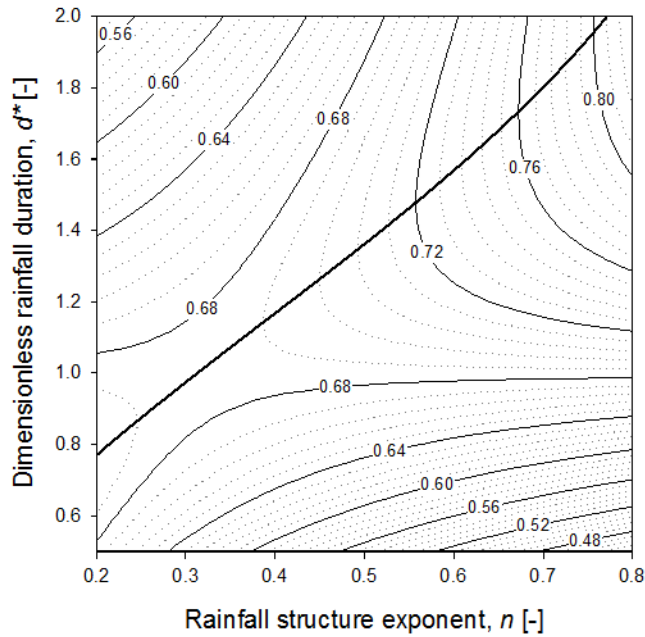
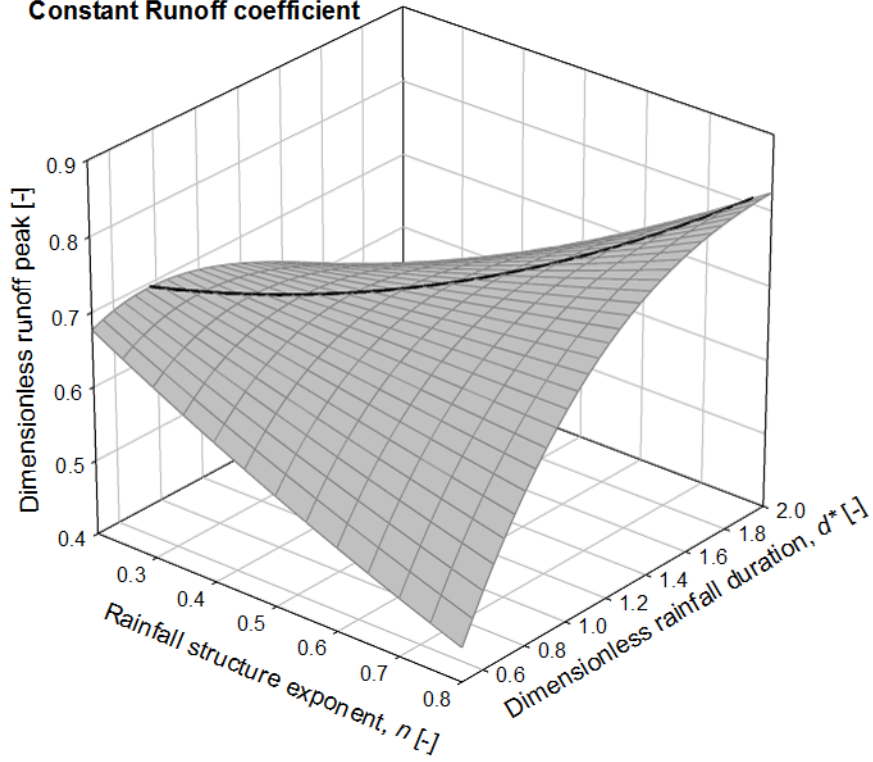


Figure 4: 3D mesh plot (at the top) and contour plot (at the bottom) of the dimensionless hydrograph peak as a function of the rainfall structure exponent and the dimensionless rainfall duration in case of constant runoff coefficient. The maximum dimensionless hydrograph peak curve is also reported (bold line).

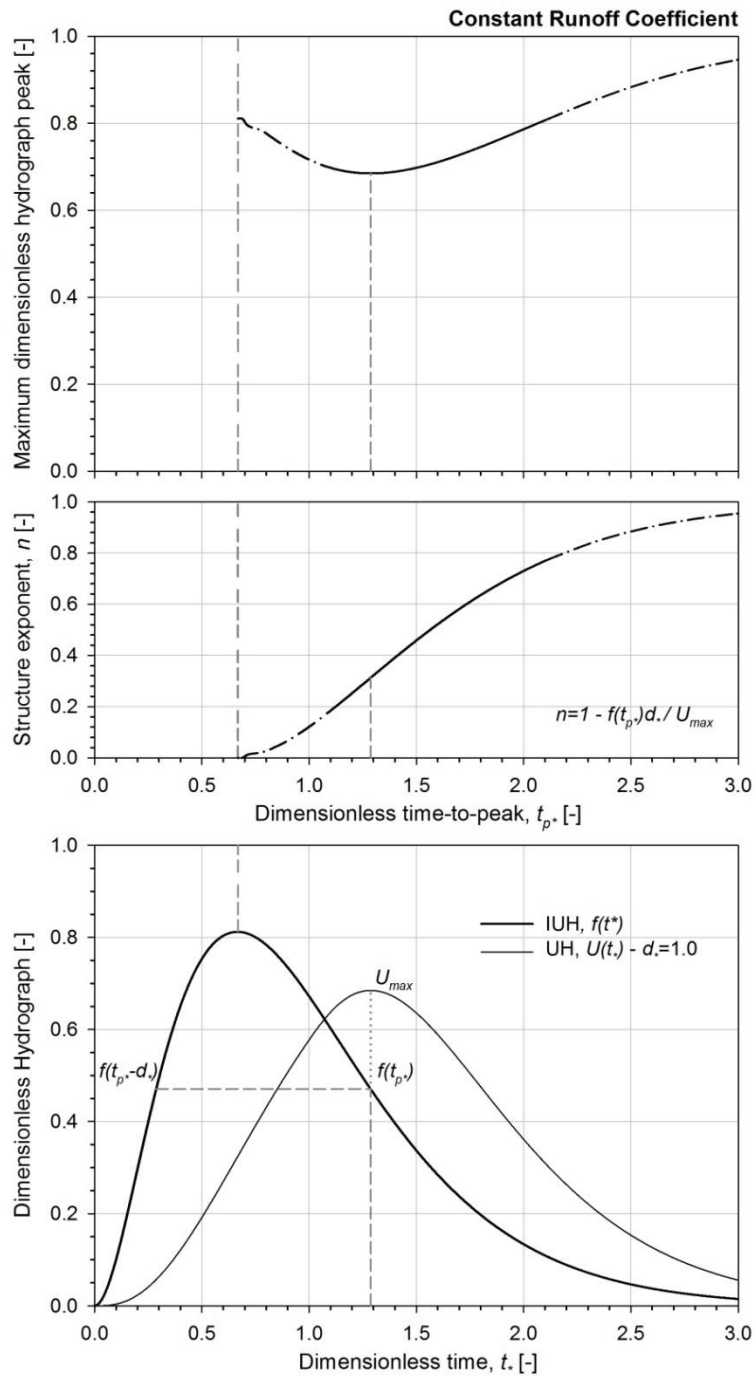


Figure 5: Maximum dimensionless hydrograph peak and the corresponding rainfall structure exponent vs. dimensionless time-to-peak in case of constant runoff coefficient; Dimensionless Instantaneous Unit Hydrograph and the corresponding dimensionless Unit Hydrographs for $d_* = 1.0$. Note that the shape parameter α is equal to 3.

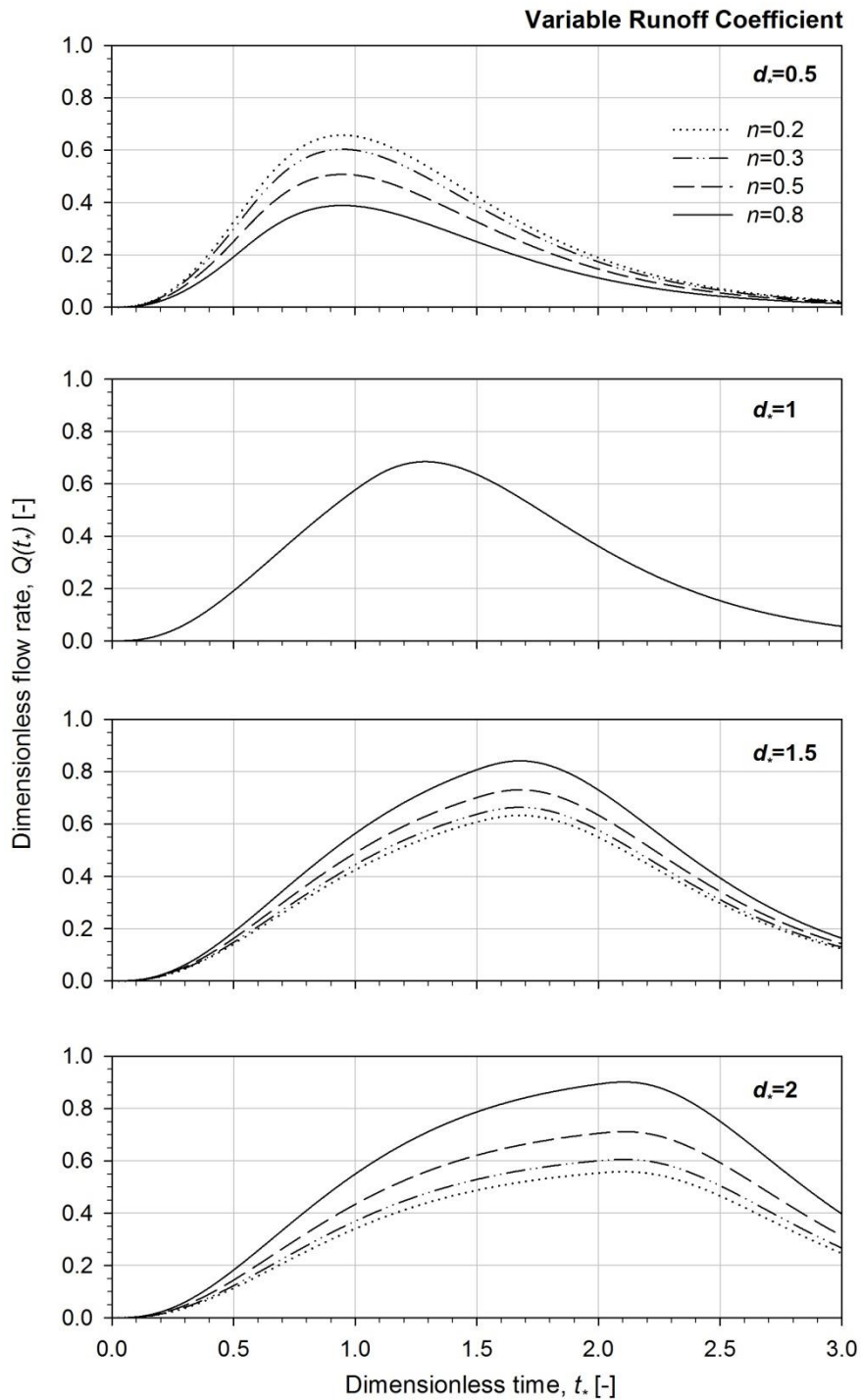


Figure 6: Dimensionless hydrographs obtained for excess rainfall intensities characterized by variable runoff coefficient and different rainfall structure exponent, n ($n = 0.2, 0.3, 0.4$ and 0.5) at assigned dimensionless rainfall duration, d_* ($d_*=0.5, 1.0, 1.5,$ and 2.0). Note that the shape parameter α is equal to 3.

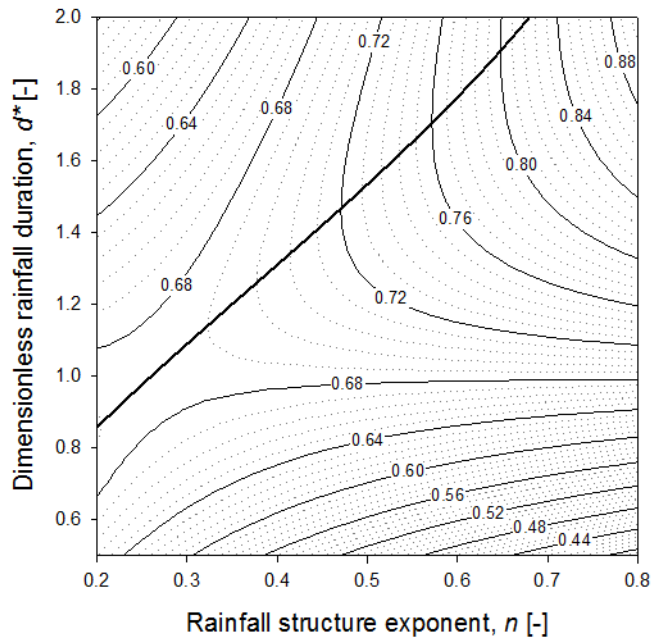
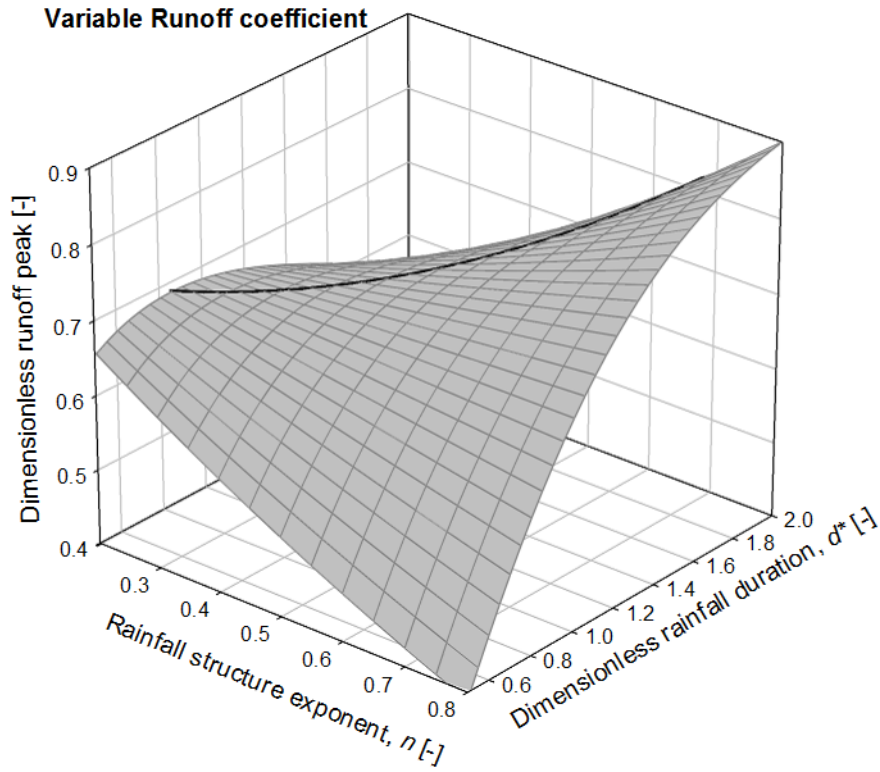
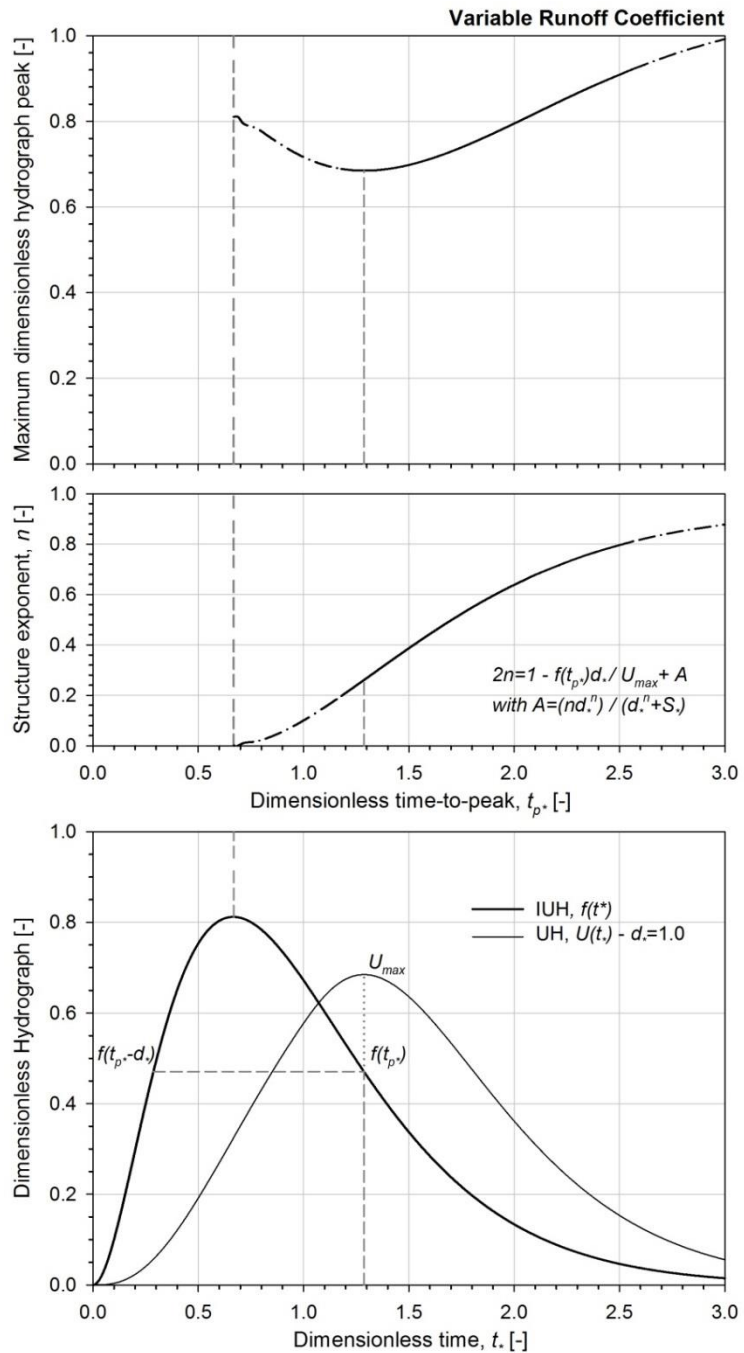


Figure 7: 3D mesh plot (at the top) and contour plot (at the bottom) of the dimensionless hydrograph peak as a function of the rainfall structure exponent and the dimensionless rainfall duration in case of variable runoff coefficient. The maximum dimensionless hydrograph peak curve is also reported (bold line).



5 **Figure 8: Maximum dimensionless hydrograph peak and the corresponding rainfall structure exponent vs. dimensionless time-to-peak in case of variable runoff coefficient; Dimensionless Instantaneous Unit Hydrograph and the corresponding dimensionless Unit Hydrographs for $d_*=1.0$. Note that the shape parameter α is equal to 3.**

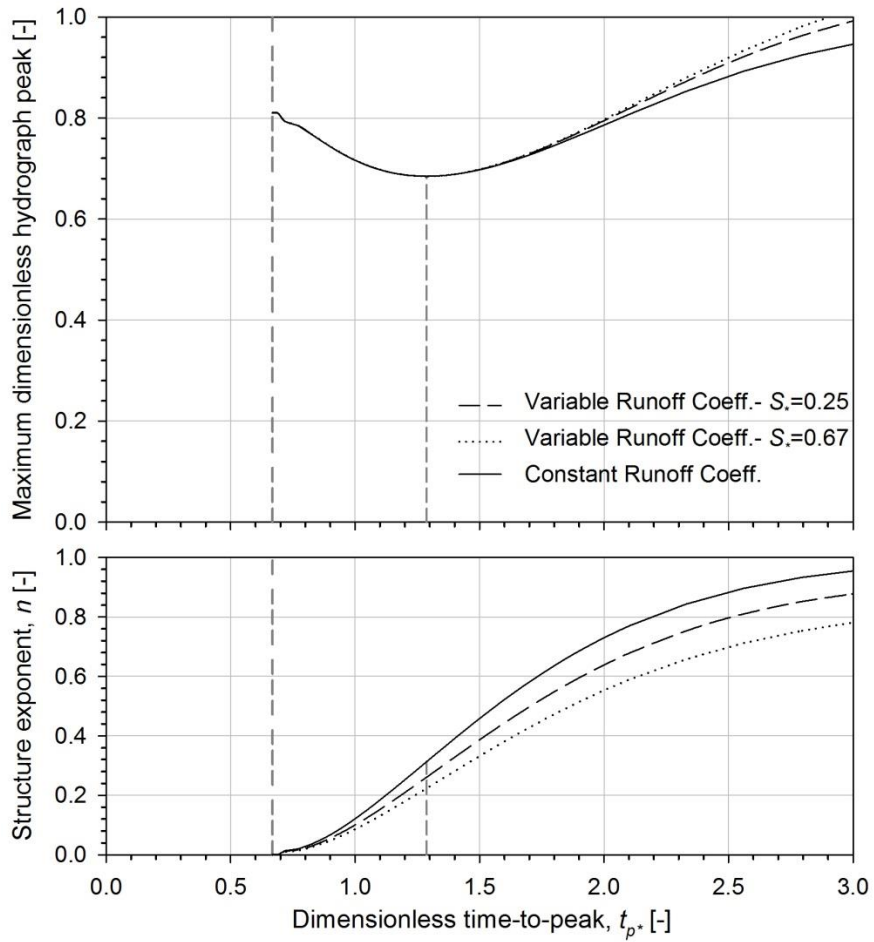


Figure 9: Maximum dimensionless hydrograph peak and the corresponding rainfall structure exponent vs. dimensionless time-to-peak in case of variable runoff coefficients with respect to dimensionless maximum retention S_r of 0.25 and 0.67. The comparison to the case of constant runoff coefficient is also reported.

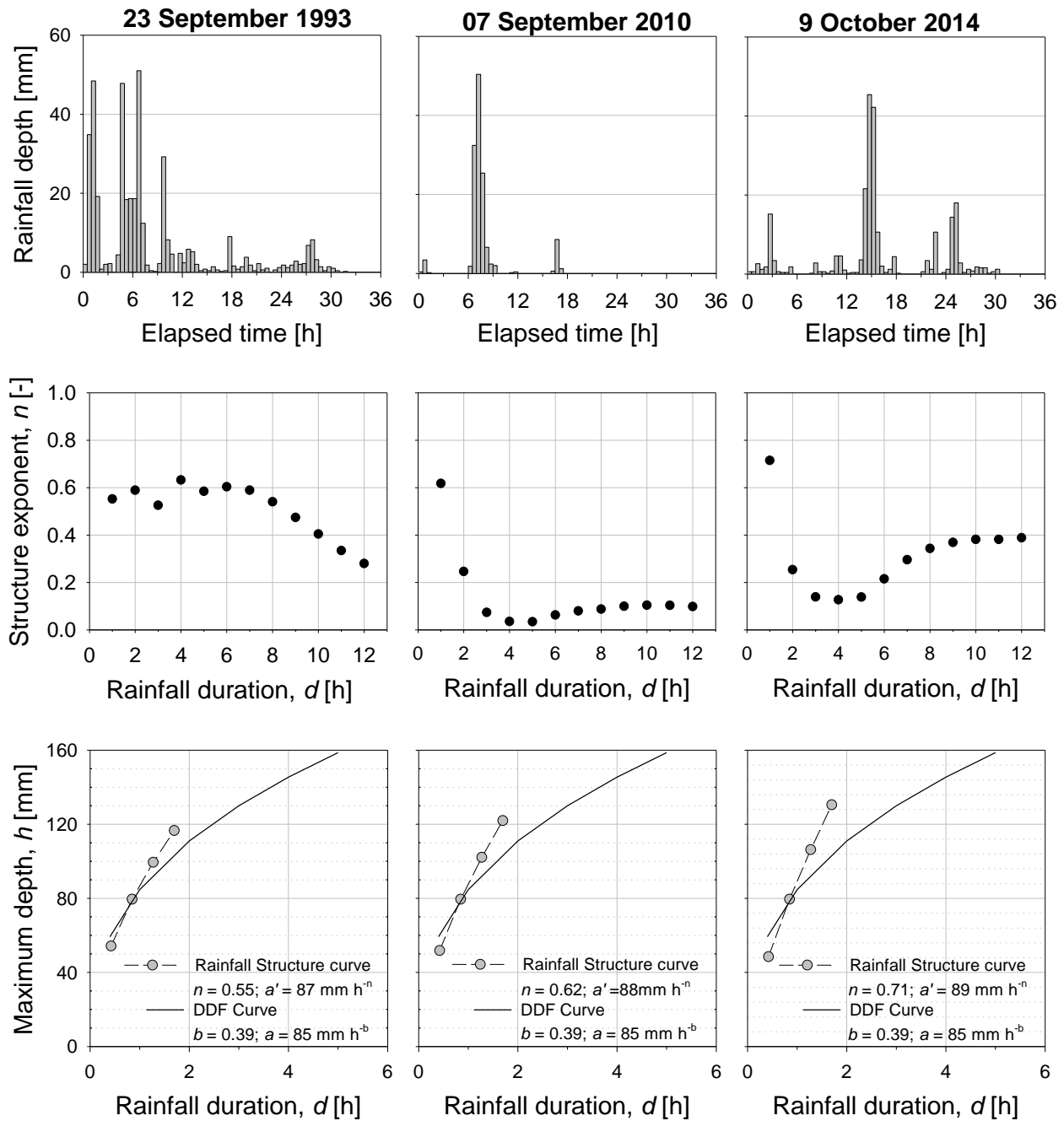


Figure 10: Rainfall event structure of three events observed in Genoa (IT): the observed rainfall depths (at the top) and the estimated rainfall structure exponents (at the centre) are reported. At the bottom, the rainfall structure and Depth-Duration-Frequency curves, evaluated for the reference time of the Bisagno – La Presa catchment, are reported.

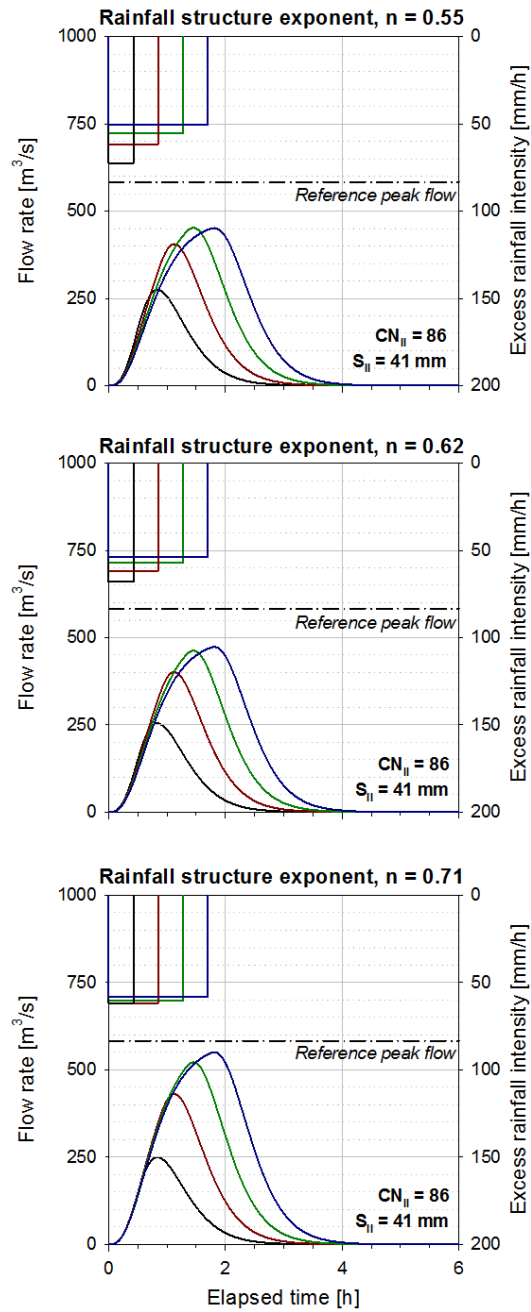


Figure 11: The excess rainfall hyetographs, the corresponding hydrographs and the reference value of the hydrograph peak flow for the Bisagno – La Presa catchment evaluated for three rainfall structure exponents. Note that each graph includes four rainfall durations (i.e. 0.5, 1.0, 1.5, and 2.0 times the reference time).

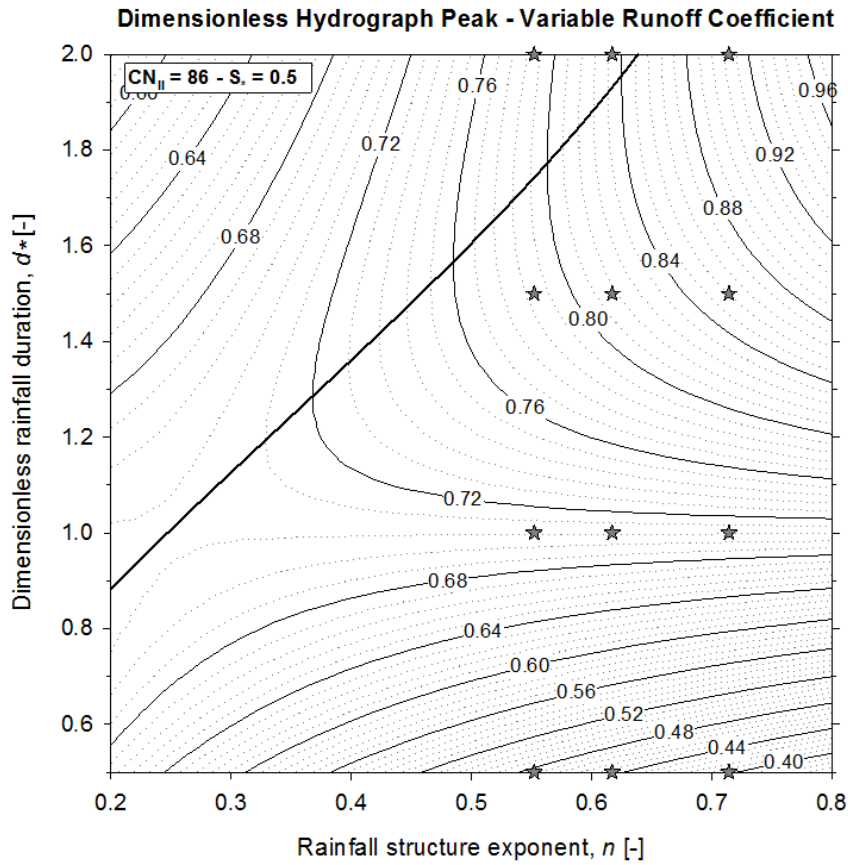


Figure 12: Contour plot of the dimensionless hydrograph peak evaluated for the Bisagno – La Presa catchment in case of variable runoff coefficient ($S_* = 0.5$). The maximum dimensionless runoff peak curve is also reported (bold line) together with the dimensionless hydrograph peaks (grey-filled stars) for the selected rainfall structure exponents ($n = 0.55, 0.62, 0.71$) and durations ($d_* = 0.5, 1.0, 1.5, \text{ and } 2.0$).