

1 **Study on the effects of storm movement on rainfall-runoff modelling at the basin scale**

2 Shahram KhalighiSigaroodi^{1,3}, Qiuwen Chen^{2,3,*}

3 1. Faculty of Natural Resources, University of Tehran, Iran

4 2. CEER Nanjing Hydraulics Research Institute, Nanjing, 210023, China

5 3. RCEES Chinese Academy of Sciences, Beijing, 100085, China

6 * Correspondence to: Tel./Fax: +86 10 62849326, E-mail: qwchen@nhri.cn

7 **Abstract:** A number of studies have emphasized the effects of rainfall movement on runoff
8 simulation; nevertheless, due to the lack of rain gauges inside sub-basins, a method using a
9 hyetograph of the nearest gauges to a sub-basin is usually employed. This study investigated
10 the negative effects of neglecting rainfall movement on overland simulation results in even a
11 middle-sized basin. Simulations were carried out under two conditions: (1) stationary
12 conditions where the nearest gauge hyetograph was used and rainfall movement was ignored,
13 which is quite common in case of a lack of data; (2) moving conditions where a shifted
14 hyetograph based on hyetograph timing recorded in the basin was used. The simulation
15 results were compared with the measured discharge at the outlets. The results revealed that
16 using the shifted hyetograph, which could consider the rainfall movement over sub-basins,
17 decreased the mismatches between the simulated and observed hydrograph. In some of the
18 cases, the shifted hyetograph reduced the relative difference more than 20%.

19 **1. Introduction**

20 Since the first reports in the 1960s (Maksimov, 1964; Yen and Chow 1969) emphasized that
21 higher peak flows are generated whenever the precipitation moves from upstream toward
22 downstream, and conversely, rainfalls passing from down to upstream result in a rounded
23 hydrograph, a great deal of research has investigated the effects of rainfall movement on the
24 shape of the runoff hydrograph in the past half century. Most studies (Ngirane et al., 1985;
25 Singh, 1997, 1998) have applied mathematical approaches to obtain a better understanding of
26 the effects of storm speed and direction characteristics on the hydrograph shape. Their results
27 showed that hyetograph characteristics, such as rainfall pattern, duration, intensity, direction
28 and speed, significantly affected the hydrograph shape. Some researchers (Singh, 1998;
29 Mizumura, 2011) adopted a kinematic wave equation to model the hydrograph in the case of
30 a moving rainstorm. Their results showed that the maximum flow depth was generated when
31 the rainstorm speed equalled the flood movement toward the outlet, and the speed of the
32 storm had a greater impact for larger Manning's roughness coefficients. Recent studies have
33 preferred dynamic wave models based on Saint Venant equations to obtain flexible results
34 under varying conditions (Costabile, 2012). Kim and Seo (2013) applied a dynamic wave
35 model base on shallow water equations to study the effects of storm movement on runoff
36 generation in a V-shaped watershed experimentation system. The results revealed that storm
37 movement could generate a loop in the stage-discharge curve, and changes in storm
38 movement direction could invert the rotation of the loop. In addition, there has been some
39 research (De Lima et al. 2002) using rainfall simulators at laboratory scale to investigate the
40 effects of storm movement. Laboratory portable rainfall simulators and flumes were used to
41 simulate the hydrograph response to moving storms and subsequently soil erosion (De Lima
42 et al. 2003). They applied different hyetograph patterns to study the effects of rainfall
43 characteristics on the runoff hydrograph. The simulation outputs of hypothetical storms

44 moving upward and downward over a laboratory impervious plane revealed that the peak
45 discharges and hydrograph shape were highly affected by storm movement. Saghafian et al.
46 (1995) used a two-dimensional runoff model and a Monte Carlo method to investigate storm
47 movement effects on runoff. The results indicated that when storm movement is slow, a
48 stationary rainstorm could be used in simulations; while when storm movement is fast, a
49 stationary rainstorm was not acceptable. Ogden et al. (1995) showed that the runoff
50 hydrograph was more sensitive to storm speed than direction in two-dimensional basin
51 topography. Base on Manning's equation, the peak maximum occurred when the storm
52 moved toward downstream at a critical speed equalling half the flow velocity.

53 Although there is well-known background on the effects of moving storms on overland
54 flow generation, most of the interest has focused on laboratory experiments (Singh, 1997,
55 1998; De Lima et al. 2002, 2003) or mathematical approaches (Costabile, 2012; Kim and Seo
56 2013; Saghafian et al., 1995, Ogden et al., 1995). These studies emphasized the effects of
57 movement on runoff generation via a synthetic hyetograph whose direction, speed and
58 intensity were well-controlled by the researchers. However, few studies are available about
59 rainstorm movement effects on runoff in natural environments of real basins, especially in the
60 case of data deficiency. The objective of this study was to (1) precisely examine the effects of
61 moving storms on hydrograph simulation at the basin scale using natural recorded rainfall-
62 runoff; (2) provide an approach to determine the rainfall characteristics under the conditions
63 of data shortage in ungauged basins.

64 **2. Materials and methods**

65 **2.1 Study area and data availability**

66 Barandoozchay basin, one of the Urmia Lake sub-catchments, is located in the northwest of
67 Iran. The study area lies in between Urmia Lake and the Iran-Iraq-Turkey international

68 border from 44° 45' E to 45° 14' E and 37° 06' N to 37° 29' N. The area of the basin is about
69 1146 km².

70 The basin is divided into 7 sub-basins (B1 to B7), based on the river branches and
71 topographic features. Fig. 1 shows the Barandoozchay map and hydrometeorological gauges.
72 This mountainous basin is mostly covered by grasslands, followed by farmland and orchard
73 land. The humid air often (not always) comes from the west, originating from the
74 Mediterranean Sea.

75 There are 6 daily rain gauges and 4 stream gauges inside the basin (Fig. 1), and 3 hourly
76 rain gauges (35010, 34013 and 34019) around the basin.

77 [Fig. 1 is here]

78 Seven storm events, which were recorded in all rain gauges during 1995 to 2014, were
79 selected. These events have recorded rain data (daily and hourly) available from the nearby
80 rain gauges and the hydrometric runoff data from the stream gauges.

81 **2.2 Estimation of sub-basin hyetograph**

82 When the cloud is stationary, most of sub-basins that are covered by the cloud react to the
83 rainfall simultaneously, implying that the start time and end time of the rainfall event is
84 approximately the same for all sub-basins; while in the case of a moving cloud, the sub-
85 basins that are located in the wind direction start to generate runoff earlier than the others
86 (Fig. 2).

87 [Fig. 2 is here]

88 Since there is no record from the rain gauge inside the basin, the start and end time of the
89 events were unknown. Therefore, the residence time of the storm cloud over each sub-basin
90 and its role in outlet runoff generation were estimated and examined.

91 As the first step, the total daily rainfall of each sub-basin was estimated using Kriging
92 and IDW (Inverse Distance Weighted) methods, based on the rain gauges inside the basin.
93 Fig. 3 shows the raster map of generated rainfall for the event on May 12th, 2010.

94 [Fig. 3 is here]

95 The total daily rainfall was then disaggregated into hourly rainfall. Since there is no
96 hourly recording gauge inside the basin, the nearest recording gauges at Urmia, Oshnavieh
97 and Naghadeh (35010, 34013 and 34019) were used. The hourly rainfalls for sub-basins were
98 obtained through following steps:

99 Determine the best hyetograph from one of the stations for disaggregation. The best
100 hyetograph was selected based on daily rainfall amounts in stations and sub-basins.

101 Calculate the ratio of total rainfall in a sub-basin to the total daily rainfall recorded in the
102 selected station with the best hyetograph.

103 Multiply the calculated ratio to the best hyetograph to obtain hourly rainfalls of a sub-
104 basin (Choi, 2008; Gyasi-Agyei et al. 2005, 2007). Fig. 4 illustrates the procedures to
105 disaggregate the daily rainfall into each sub-basin's hyetograph.

106 [Fig. 4 is here]

107 Due to dynamic motion of the cloud, the rainfall duration, start and end time, and
108 intensity as well as other characteristics change. These parameters are known for the gauge
109 locations, but unknown in other locations as well as sub-basins. To determine the cloud
110 arrival time of each sub-basin and the time of rainfall occurrence (start, end and duration), the
111 recorded hyetograph was concentrated to a unique time named the Time of Gravity Centre of
112 Hyetograph (TGCH) (Khalighi 2009). Since the TGCH is specified in gauge locations, it can
113 be calculated for sub-basins through the following procedures:

114 (1) TGCH for recorded rainfall was calculated as a momentum of the rainfall component
115 around the horizontal and vertical axis. The Fig. 5 shows that the recorded event in station

116 35010 started at 4:00 am and ended at 2:30 pm, and the calculated TGCH was at 9:00 am
117 (8.981).

118 (2) When cloud moves over a basin, the rainfall time at a point depends on the point
119 location and cloud speed and direction. At least 3 gauges are necessary to determine the
120 occurrence time of rainfall at a point, although more gauges could increase the accuracy. As
121 there are only 3 recording gauges around the study basin, a flat plane passes through the
122 stations (Fig. 6). Therefore, the equation of the plane ($TGCH=aX+bY+c$) was applied to
123 calculate the TGCH at each point. The UTM coordinates of the stations (X, Y) are considered
124 as independent variables and the TGCH are considered as dependent variable, and then the
125 coefficients (a, b and c) of the flat plane are calculated using algebraic functions (Howard
126 2010).

127 (3) The coordinates of the sub-basin centroids were placed in the above equations to
128 determine the TGCH of each sub-basin.

129 (4) The previously derived hyetograph was shifted as its gravity centre conformed to the
130 TGCH of each sub-basin centroid (Fig. 7).

131 [Fig. 5 is here]

132 [Fig. 6 is here]

133 [Fig. 7 is here]

134 For example, the TGCH for event 95/04/22 was recorded at 8.98, 6.48 and 5.33 at the
135 stations 35010, 34019 and 34013 respectively (table 2), then the equation of the TGCH plane
136 of this event was: $TGCH = 0.000077 * X + 0.000069 * Y - 317.457$. Based on this
137 equation and the coordinates of the B1 sub-basin centroid, the TGCH was 8:00 am, implying
138 that the TGCH at B1 occurred almost one hour earlier than at station 35010, which was 8:59
139 am.

140 **2.3 Rainfall-runoff modelling**

141 The HEC-HMS model (TR-55, 1986) was used to investigate the effects of storm movement
142 on hydrograph simulations. The model was calibrated using 5 events (1995/04/22,
143 2002/04/21, 2003/04/03, 2006/04/18 and 2008/04/07). Based on sensitive analysis, the
144 relative initial abstraction ($R_a = I_a/S$) is the most sensitive parameter among the other
145 parameters such as curve number (CN), lag time (T_l), total storage (S) and initial abstraction
146 (I_a). Table 1 showed the primary and optimized parameters in sub-basins. The validation was
147 conducted using the events 2010/05/12 and 2014/04/22. The results of peak discharges were
148 shown in Table 2.

149 [Table 1 here]

150 [Table 2 here]

151 After the calibration and validation, the simulations were carried out for all events using
152 two hypotheses: (1) stationary cloud where the sub-basin hyetograph timing is equal to the
153 nearest recording gauge; (2) moving cloud where the sub-basin rainfall hyetograph shifted
154 base on cloud movement direction and sub-basin location.

155 A Taylor diagram (Taylor, 2001, 2005; Sigaroodi et al., 2014) and root mean squared of
156 relative difference (RD) were used to compare the results of two hypothesized conditions.

$$RD = \sqrt{\left(\frac{P_O - P_S}{P_O}\right)^2} + 100$$

157

158 where the P_O and P_S are observed and simulated peak discharge respectively.

159 **3. Results**

160 Fig. 8 shows the planes of TGCH for different events. Although the basin is mainly affected
161 by the eastern humid Mediterranean air, the results indicated that each selected rainfall event
162 had different directions and speeds.

163

[Fig. 8 is here]

164 Based on the gauge locations and TGCH of each event, a plane equation
165 $TGCH = aX + bY + c$ was obtained for each event. Table 3 shows the equation coefficients.

166 [Table 3 is here]

167 The gravity centre coordinate of each sub-basin is used in the equations to calculate the
168 TGCH for the sub-basin centroid of each event. Fig. 9 shows how the sub-basin hyetograph is
169 shifted to obtain the TGCH for the event on April 3rd, 2003. The measured TGCH at the
170 gauges and the calculated TGCH for sub-basins are shown in Table 4.

171 [Fig. 9 is here]

172 [Table 4 is here]

173 Fig. 10 presents the HEC-HMS modeled results for the event on April 22nd, 2014 at the
174 gauge 35005. The right part shows the model performance under stationary conditions where
175 all sub-basins react to the hyetograph simultaneously. The gray and brown lines are the
176 modeled outputs for upper sub-basins, which make the simulated total output (blue line). The
177 hydrograph is sharp and the time to peak is quite different compared to the observed
178 hydrograph (red line). The left part presents the modeled result using a shifted hyetograph,
179 which matches better with the observed hydrograph.

180 [Fig. 10 is here]

181 For comparison, the modeled peak discharges of the 7 selected events under the two
182 conditions are presented together with the observations in Table 5.

183 [Table 5 is here]

184 Fig.11 displays the standard deviation (SD) and correlation coefficient R^2 of the modeled
185 results under stationary and moving conditions on the Taylor diagram. It is clearly seen that
186 the moving condition results are closer to the observation points than the stationary condition
187 results.

188 [Fig. 11 is here]

189 **4. Discussion**

190 To achieve accurate hydrological modeling, high quality and spatially-explicit rainfall data
191 should be accessible; however, in many cases uniform hyetographs are used for all sub-basins
192 due to lack of sufficient gauges. If the cloud motion is neglected, it means that the differences
193 between the times of runoff generation are ignored. In this case, to compensate for the
194 difference and achieve better matching between simulated and observed runoff, other basin
195 factors such as curve number (*CN*) or time-lag have to be modified, which most probably
196 cause artifacts in the coefficients (Khalighi et al., 2006, 2009).

197 This study provided an approach that the rainfall time in ungauged sub-basins could be
198 precisely determined using the recorded rainfalls in around gauges. Although more rain
199 gauges can obtain better results, at least 3 gauges are necessary to record the rainfall event for
200 determining the cloud direction and speed, which is reflected in the TGCH plane.

201 When the cloud movement is slow, consideration of movement is more important
202 compared to fast movement conditions. In the event of April 22nd, 2014, the time difference
203 between gauges 35010 and 34019 (Table 4) shows that the cloud movement is very low, thus
204 the sub-basin B1 generates runoff much earlier than B7. This result was not consistent with
205 the findings of Saghafian (1995), who stated that a stationary rainstorm could be used in low
206 speed storms. This study showed that for small basins or laboratory scales where the cloud
207 covers the whole basin, the storm motion effect could be ignored; while in the case of
208 middle-size to large basins, the runoff of low speed storms has an obvious role in determining
209 hydrograph shape. It can then be concluded that when the time difference between the
210 recorded rainfalls around the area is small, the differences between stationary and moving
211 runoff simulations are slight. These results were consistent with the findings of previous
212 studies, which showed the impacts of cloud motion on hydrographs by using rainfall

213 simulators at different laboratory scales (Sing, 1997, 1998; de Lima and Singh, 2002; de
214 Lima et al., 2003; Marzen, 2015) or the kinematic wave method (Mizumura, 2011).

215 The results of this study also revealed that longer rainfalls are less affected by cloud
216 movement. In other words, for rapid and short rains, the runoff hydrograph is more strongly
217 affected by cloud movement speed and direction. These results were consistent with the
218 findings of previous studies (de Lima and Singh, 2002; Khalighi, 2009; Dae-Hong Kim, 2013)
219 in laboratory.

220 However, it should be noted that the effects of cloud movement on hydrograph modeling
221 become visible only when the study area is divided into smaller sub-basins. In addition, lack
222 of gauges in this study caused to use a flat plane to calculate the TGCH for the sub-basins;
223 but other interpolation methods such as IDW and Kriging could be more appropriate to obtain
224 surface data from the point data.

225 In conclusion, although there are many laboratory experiments on the effects of rainfall
226 movement on runoff simulation, more studies are necessary to determine how the spatial-
227 temporal dynamics of rainfall can be considered at the real watershed scale, in particular for
228 ungauged areas.

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Tables

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Table 1. Optimized parameter in sub-basins

Sub-basin	$R_a = I_a/S$		CN	T_1 (h)
	Primary	Optimized		
B1	0.2	0.197	68	7.6
B2	0.2	0.18	71	6.2
B3	0.2	0.23	78	3.7
B4	0.2	0.23	80	2.3
B5	0.2	0.23	78	3.1
B6	0.2	0.23	82	2.7
B7	0.2	0.164	77	5.9

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Table 2. Comparison of observed and simulated peak discharge in validation step

Hydrological Station	2010/05/12		2014/04/22	
	Observed	Simulated	Observed	Simulated
35003	12.2	14.4	--	--
35005	34.8	31.5	297.9	352

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Table 3. Obtained coefficients for the TGCH flat plane

Coefficient / Time	95/04/22	02/04/21	03/04/03	06/04/18	08/04/07	10/05/12	14/04/22
a	0.000077	0.000256	0.000222	0.000244	0.000047	-7.3E-05	-8.9E-05
b	0.000069	0.000008	0.000095	-3.4E-05	-0.00003	0.000074	-0.00019
c	-317.457	-144.736	-485.298	30.743	127.119	-236.65	855.542

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Table 4. TGCH measured at the gauges and calculated for the sub-basins

Location		UTM		Precipitation Events						
		X	Y	95/04/22	02/04/21	03/04/03	06/04/18	08/04/07	10/05/12	14/04/22
Gauges	35010	507361	4155960	8.98	20.4	7.3	14.7	3.0	8.3	15.9
	34019	534124	4091310	6.48	26.7 ^a	6.2	23.4	6.2	1.6	25.5
	34013	510374	4100492	5.33	20.7	1.1	17.3	4.8	4.0	25.9
Sub-basins	B1	510820 ^b	4139365	8.0	21.1	6.3	16.1	3.7	6.8	18.7
	B2	495670	4134355	6.5	17.2	4.5	12.6	3.1	7.6	20.9
	B3	493644	4123015	5.6	16.6	3.7	12.5	3.3	6.9	23.2
	B4	483585	4118992	4.5	14	2.8	10.1	3	7.3	24.9
	B5	486806	4113932	4.4	14.8	2.9	11.1	3.3	6.7	25.5
	B6	493217	4112538	4.8	16.4	3.2	12.7	3.6	6.1	25.2
	B7	508969	4120830	6.6	20.5	5.1	16.3	4.1	5.6	22.3

302 a: The numbers over 24 refer to the next day.

303 b: Coordinate of centroid of sub-basin

Table 5. Modelled peak discharges under two conditions and differences

		Peak Discharge			Difference (%)	
Hydrological						
Date	Station	Obs.	Stationary	Moving	Stationary	Moving
2014/04/22	35005	297.9	352	315.3	18.2	5.8
2010/05/12		34.8	31.5	34.4	9.5	1.1
2008/04/07		61.4	70.15	65.6	14.3	6.8
2006/04/18		96.15	100.5	100.13	4.5	4.1
2003/04/03		20.1	20.4	20.3	1.5	1
2002/04/21		65.9	42.9	41.6	34.9	36.9
1995/04/22		37.45	51.2	42.58	36.7	13.7
2010/05/12	35003	12.2	14.4	13.4	18	9.8
2008/04/07		51.9	65.16	63.4	25.5	22.2
2006/04/18		85.4	93.8	93.57	9.8	9.6
2003/04/03		3.7	3.5	3.8	5.4	2.7
2002/04/21		24.3	28.8	26.1	18.5	7.4
1995/04/22		113.2	127.7	127.3	12.8	12.5
1995/04/22	35001	83	83.3	83.3	0.4	0.4

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Figure captions

- Figure 1. Barandoozchay basin and hydrometeorological gauges
- Figure 2. Schematic of rainfall movement effect on runoff formation
- Figure 3. Spatial distribution of rainfall event 2010/05/12
- Figure 4. Schematic of rainfall hyetograph determination in sub-basin centroid. a) Hourly hyetograph at nearest gauge, b) Daily precipitation at nearest gauge, c) Daily precipitation in sub-basin centroid, d) Derived hyetograph for sub-basin
- Figure 5. HYGCH output for calculation of hyetograph centroid at 95/04/22 in station 35010 (Gx: Temporal coordinates of concentrated event, Gy: Average of incremental rainfall)
- Figure 6. Flat plane passing through the TGCH for the event 1995/04/22
- Figure 7. Shifting the hyetograph to the estimated TGCH
- Figure 8. Precipitation time occurrence plane in different events
- Figure 9. Hyetograph of sub-basins before shift (left) and after shift (right). (Red arrows show the timing position of TGCH before and after shifting)
- Figure 10. HEC-HMS output for rainfall event 2014/04/22, under two different conditions, moving simulation (left) and stationary simulation (right)
- Figure 11. Scatter plot of the simulated peak discharge for stationary and moving conditions on a Taylor diagram