

Voxel inversion of airborne electromagnetic data for improved groundwater model construction and prediction accuracy

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Abstract

We present a workflow for efficient construction and calibration of large-scale groundwater models that includes the integration of airborne electromagnetic (AEM) data and hydrological data. In the first step, the AEM data are inverted to form a 3D geophysical model. In the second step, the 3D geophysical model is translated, using a spatially dependent petrophysical relationship, to form a 3D hydraulic conductivity distribution. The geophysical models and the hydrological data are used to estimate spatially distributed petrophysical shape factors. The shape factors primarily work as translators between resistivity and hydraulic conductivity, but they can also compensate for structural defects in the geophysical model.

The method is demonstrated for a synthetic case study with sharp transitions among various types of deposits. Besides demonstrating the methodology, we demonstrate the importance of using geophysical regularization constraints that conform well with the depositional environment. This is done by inverting the AEM data using either smoothness (smooth) constraints or minimum gradient support (sharp) constraints, where the use of sharp constraints conform best with the environment. The dependency on AEM data quality is also tested by inverting the geophysical model using data corrupted with four different levels of background noise. Subsequently, the geophysical models are used to construct competing groundwater models for which the shape factors are calibrated. The performance of each groundwater model is tested with respect to four types of prediction that are beyond the calibration base: a pumping well's recharge area and groundwater age, respectively, are predicted by applying the same stress as for the hydrologic model calibration; and head and stream discharge are predicted for a different stress situation.

1 As expected, in this case the predictive capability of a groundwater model is better when it is based on
2 a sharp geophysical model instead of a smoothness constraint. This is true for predictions of recharge
3 area, head change, and stream discharge, while we find no improvement for prediction of groundwater
4 age. Furthermore, we show that the model prediction accuracy improves with AEM data quality for
5 predictions of recharge area, head change and stream discharge, while there appears to be no accuracy
6 improvement for the prediction of groundwater age.

7 **1 Introduction**

8 Large-scale geological and groundwater models are used extensively to support aquifer management.
9 (Here “large scale” refers to an area of from tens to thousands of square kilometers.) Determining the
10 distribution of hydraulic properties and the geometry and connectivity of the groundwater system is of
11 significant importance because these features control the flow paths (Desbarats and Srivastava 1991;
12 Fogg et al. 1999; Weissmann and Fogg 1999). Incorrect reconstruction of the geological structures has
13 thus been recognized as an important source of uncertainty when a groundwater model is used to make
14 predictions outside its calibration base (Refsgaard et al. 2012; Seifert et al. 2012; Zhou et al. 2014).
15 The data traditionally used for structural mapping include lithological logs from boreholes,
16 hydrological data, and hydraulic testing results, but these data are often sparse and unevenly
17 distributed within an investigated domain. In these (very common) cases, data scarcity becomes a
18 major obstacle for structural mapping in relation to large scale groundwater modeling (Refsgaard et al.
19 2012; Zhou et al. 2014).

20 Ground-based and airborne electromagnetic methods have shown great potential for mapping
21 geological structures (Jørgensen et al. 2003; Thomsen et al. 2004; Abraham et al. 2012; Oldenborger
22 et al. 2013; He et al. 2014; Munday et al. 2015). For large scale mapping, the airborne electromagnetic
23 method (AEM) is efficient and cost-effective, supplementing traditional data with dense estimates of
24 electrical resistivity which, in some environments, inform about the lithology and thereby about
25 structure (Robinson et al. 2008; Binley et al. 2015). AEM measurements can be made quickly over
26 large areas, and the resolution can be as fine as 25 m in the horizontal direction and 5 m in the vertical
27 (Schamper et al. 2014) with a penetration depth of up to several hundred meters (Siemon et al. 2009).

28 Various methods to incorporate resistivity estimates (hereafter referred to as resistivity models) in
29 groundwater model construction have been reported. Manual and knowledge-driven approaches have
30 been used to combine geological, hydrological and geophysical data with expert knowledge
31 (Jørgensen et al. 2013). However, the manual approach is subjective and can be very time consuming
32 and expensive to use when resistivity models from large AEM surveys are to be incorporated in model
33 construction. Alternatively, more objective and cost-efficient geostatistical modeling approaches

1 (Carle and Fogg 1996; Deutsch and Journal 1998; Strebelle 2002) are available for generating models
2 from a combination of borehole information and AEM-determined resistivity models. For example:
3 He et al. (2014) used a transition probability indicator simulation approach (Carle and Fogg 1996),
4 while Gunnink and Siemon (2015) used sequential indicator simulation (Deutsch 2006). Marker et al.
5 (2015) used a deterministic strategy for the integration of AEM resistivity models into the
6 hydrological modeling process.

7 The just mentioned studies all used sequential hydrogeophysical inversion approaches (SHI; as
8 defined by Ferré et al. 2009). In SHI the geophysical data are inverted first and independently from the
9 later inversion of the hydrological data. For large scale groundwater modeling, Herckenrath et al.
10 (2013) and Christensen et al. (2016) used both SHI and joint hydrogeophysical inversion approaches
11 (JHI; as defined by Ferré et al. 2009). In JHI, the geophysical and hydrological data are inverted
12 jointly by linking the geophysical and hydrological models directly through some of their parameters.
13 The linking can, for example, be done by using an Archie's law inspired petrophysical relationship
14 (Archie 1942) to translate between the geophysical and hydrologic parameters.

15 In general, petrophysical relationships are difficult to establish because such translation tends to be
16 site-, scale- and facies-specific (Chen et al. 2001; Hyndman and Tronicke 2005; Slater 2007) and
17 uncertain (Mazáč et al. 1985; Slater 2007). The studies by Herckenrath et al. (2013) and Christensen et
18 al. (2016) used a fixed petrophysical relationship throughout the model domain. Better results can be
19 obtained by using a spatially variable relationship, which allows for local translation between
20 hydraulic conductivity and electrical resistivity, and by including the spatially dependent petrophysical
21 parameters in the optimization process (Linde et al. 2006).

22 There are two other challenges for incorporating resistivity models into large scale groundwater
23 modeling: differences in model discretization, and choice of geophysical regularization methodology.
24 Groundwater models are often discretized in a regular voxel grid while the traditional resistivity
25 models are 1D and placed at the respective sounding location. For airborne surveys, for example, the
26 resistivity models are normally located along the flight lines (Christiansen et al. 2006). Such resistivity
27 models therefore need to be relocated to conform to the grid of the groundwater model. The relocation
28 will often be a subtle process where information can be lost. To address this issue, Fiandaca et al.
29 (2015) presented a geophysical modeling approach referred to as "voxel inversion", which decouples
30 the geophysical inversion model space from the geophysical measurement positions. This allows
31 estimation of a 3D geophysical model that is discretized on the same voxel grid as the groundwater
32 model.

1 Traditionally, geophysical regularization includes horizontal and vertical smoothing constraints
2 (Constable et al. 1987) or is limited to a few-layer inversion (Auken and Christiansen 2004), whereas a
3 groundwater system often has sharp layer or body boundaries. It has therefore been recognized, e.g. by
4 Day-Lewis (2005) and others, that the regularization used to stabilize the geophysical inversion may
5 not reflect the actual hydrologic conditions unless it is chosen carefully. If, for example, smooth
6 regularization is used to estimate resistivity models in a sharply layered system, it will produce a
7 blurred resistivity distribution from which one should be careful with inferring the spatial distribution
8 of hydraulic conductivity to be used in a groundwater model. In this case, it would be better to use
9 minimum gradient support regularization (Portniaguine and Zhdanov 1999; Blaschek et al. 2008;
10 Vignoli et al. 2015) for the geophysical inversion because the estimated resistivity distribution will
11 tend to consist of fewer, more sharply defined layer boundaries (vertically and horizontally). However,
12 it is often ignored that geophysical data can be inverted using alternative regularization schemes, and
13 to test whether the alternative geophysical models affect the predictive capability of a groundwater
14 model.

15 The main objective of the present study is to present a novel sequential hydrogeophysical approach
16 whereby a voxel based 3D resistivity model is used to parameterize and calibrate a groundwater
17 model. The model parameterization methodology allows the calibration to compensate for errors in the
18 resistivity model. Furthermore, we will demonstrate that it is important for groundwater flow
19 simulations that the underlying resistivity model is estimated using regularization constraints that
20 conform well to the geological environment. Finally, we analyze how groundwater model prediction
21 accuracy depends on the quality of the geophysical data that was used to estimate the resistivity
22 model. Section 2 of the paper presents the methodology. Section 3 describes the synthetic test case
23 used for our demonstration purposes. Section 4 presents the results, while sections 5 and 6 present
24 discussions and conclusions of the work, respectively.

25

26

1 **2 Methodology**

2 Conceptually, we define a translator function that describes the petrophysical relationship between
3 electrical resistivity and hydraulic conductivity. The petrophysical relationship can vary horizontally
4 and vertically, thereby adapting to the local conditions in translation from the geophysical model space
5 to the hydrological model space. Through inversion, the 3D spatially dependent optimal parameters of
6 the petrophysical relationship are estimated for each layer interval, thereby covering the entire three-
7 dimensional model space.

8 Figure 1 provides a workflow for the method. First, the gathered airborne electromagnetic (AEM) data
9 from the survey area are inverted with smooth or sharp horizontal and vertical constraints (Vignoli et
10 al. 2015). This is done by using a recently developed voxel inversion scheme which decouples the
11 geophysical model from the position of the acquired data (Fiandaca et al. 2015). The geophysical
12 model space thus corresponds to the full 3D hydrological model grid. Secondly, the geophysical voxel
13 based resistivity model is used as input for the sequential hydrological inversion. The geophysical
14 model parameter (resistivity) is linked to the main investigated parameter (hydraulic conductivity)
15 through a petrophysical relationship that has unknown shape factor values. The shape factor values are
16 estimated through a hydrological inversion which minimizes an objective function describing the
17 misfit between simulated groundwater model responses and corresponding observed hydrological
18 data. Finally, the calibrated groundwater model can be used to make a set of relevant hydrologic
19 predictions. The various steps of the methodology are explained in more detail in the following.

20

21 **2.1 Geophysical voxel inversion**

22 In the first step (Figure 1, box 1), the AEM data undergoes constrained deterministic inversion using a
23 recently developed voxel inversion approaches. This approach allows the geophysical model spaces to
24 be spatially decoupled from the geophysical measurement positions (Fiandaca et al. 2015). In most
25 inversion schemes, the forward and inverse formulations use the same model discretization. In the
26 voxel formulation, the two model discretizations are decoupled. The voxel model space thus defines
27 the geophysical properties on the nodes of a regular 3D grid.

28 For calculating the forward responses, a “virtual” 1D model is a built at each sounding position. The
29 “virtual” 1D model is defined by a number of layers, and layer thicknesses. The geophysical properties
30 are interpolated from the voxel model space into the layer centers of the virtual model that is
31 subsequently used to simulate the forward response for the corresponding sounding.

1 The voxel inversion approach thus allows for inversion of AEM data into a geophysical model defined
 2 on a 3D regular grid, regardless of the sounding positions. As a result, the geophysical inversion can
 3 be conducted using the same grid as that defined for a 3D groundwater model, thereby minimizing
 4 scaling issues in the coupling of geophysical and hydrological models.

5 The general solution to the non-linear geophysical inversion problem can be found in Auken et al.
 6 (2014). To stabilize the inverse problem, either of two types of regularization methods can be applied.
 7 The first regularization method is commonly referred to as smoothness-constrained inversion
 8 (Constable et al. 1987). The smoothness-constrained inversion tends to reduce contrasts and the
 9 resulting geophysical model may appear blurred. The reason for this is found in its minimum-structure
 10 L2 norm inversion formalism (Constable et al. 1987; Menke 2012). Following the notation used by
 11 Vignoli et al. (2015), this can be expressed as:

$$(m_i - m_j)^2 / \sigma_{i,j}^2 \quad (1)$$

12 where the m_i and m_j are the constrained parameters and $\sigma_{i,j}$ defines the constraint strength. The
 13 penalization of structures is clearly seen in eq. (1), where $(m_i - m_j)^2 / \sigma_{i,j}^2$ is proportional to the
 14 square of the value of the variation $(m_i - m_j)$. This implies that an increase in model parameter
 15 variation will always result in a penalization in the stabilizer. The smoothness regularization thus
 16 prevents reconstruction of sharp transitions.
 17

18 The second regularization method is the minimum gradient support (Portniaguine and Zhdanov 1999;
 19 Blaschek et al. 2008; Vignoli et al. 2015), which allows for large sharp vertical and horizontal model
 20 transitions. The minimum gradient support regularization seeks to minimize the spatial variations
 21 vertically and laterally by penalizing the vertical and horizontal model gradients through the stabilizer
 22 expressed as (Vignoli et al., (2015)):
 23

$$\frac{(m_i - m_j)^2 / \sigma_{i,j}^2}{(m_i - m_j)^2 / \sigma_{i,j}^2 + 1} \quad (2)$$

24 In eq. (2), $\sigma_{i,j}$ is a parameter used to control the sharpness of the regularization constraints. The
 25 stabilizer contribution to the objective function is thus one when $|m_i - m_j| \gg \sigma_{i,j}$ and zero when
 26

1 $\sigma_{i,j} \gg |m_i - m_j|$. The minimum gradient support functional thus counts the number of model
2 variations larger than $\sigma_{i,j}$ for the stabilizer term of the objective function. This formalism allows sharp
3 vertical and horizontal model transitions, which are penalized excessively by the smoothness-
4 constrained inversion.

5

6 **2.2 Hydrological model parametrization**

7 In the second step (Figure 1, box 2), the three dimensional distribution of electrical resistivity values is
8 linked to the hydrological parameters (i.e. hydraulic conductivity) through a spatially varying
9 petrophysical relationship. Shape factors of this relationship are calibrated.

10 Linking hydraulic conductivity and electrical resistivity is not trivial because the parameter values and
11 the form of the petrophysical relationship may vary dramatically between different types of
12 environments. In addition, there can be fundamental questions about how the effective properties
13 controlling electrical current flow are related to the effective properties controlling fluid flow (Slater
14 2007). The primary factors controlling this relationship are porosity, pore water conductivity,
15 tortuosity, grain size, degree of saturation, amount of clay minerals, etc. (McNeill 1980). The simplest
16 petrophysical relationship is the empirical relationship known as Archie's law (Archie 1942), which
17 relates porosity, pore water conductivity, and the degree of saturation to bulk electrical conductivity.
18 However, this type of relationship does not take the electrical surface conductance of clay minerals
19 into account. The Waxman and Smits model (Waxman and Smits 1968) combined with the dual-water
20 model by Clavier et al. (1984) provides a basis for establishing empirical relationships for shaly sand
21 and sediments containing clays (Revil and Cathles 1999; Revil et al. 2012). For glacial sedimentary
22 environments, it is reported that clay has low electrical resistivity and also low hydraulic conductivity,
23 and sand has high electrical resistivity and high hydraulic conductivity (Mazáč et al. 1985). For these
24 environments, it is common to use a power law relationship which is given some theoretical support
25 by Purvance and Andricevic (2000). The relationship is expressed as

$$K = \alpha \cdot \rho^\beta \quad (3)$$

26

27 where K is the hydraulic conductivity (m/s), ρ is the electrical resistivity (ohm-m), and α and β are
28 two empirical shape factors. To compute K for each element in the groundwater model grid, α and β
29 need to be parameterized and estimated. We suggest to make the parameterization by pilot points
30 placed in a regular grid in each layer of the groundwater model (Certes and De Marsily 1991; Doherty
31 2003). Each pilot point holds a set of α and β parameters, and kriging is used for spatial interpolation

1 of α and β from the pilot points to the model grid. This kind of parametrization creates smooth
2 transitions in the parameter fields and allows for variation in both the horizontal and vertical direction
3 of the ρ to K translation. Hydraulic conductivity can thus be calculated by eq. (3) for every element
4 in the groundwater model grid.

5

6 **2.3 Hydrological Inversion**

7 The model parameters, α and β at the pilot points, are calibrated by fitting the groundwater model to
8 hydrological data. When the number of model parameters is large compared to the number of
9 observation data, the minimization must be stabilized by regularization. The total objective function
10 to be minimized is therefore a balanced compromise between a measurement term (Φ_m) and a
11 regularization term (Φ_r). The combined objective function has the form

$$\Phi_{total} = \Phi_m + \mu \cdot \Phi_r = \sum_{i=1}^{n_d} \omega_{d,i} (d_{obs,i} - d_{sim,i})^2 + \mu \cdot \Phi_r \quad (4)$$

12

13 where Φ_{total} is the total objective function, $d_{obs,i}$ and $d_{sim,i}$ are measured and equivalent simulated
14 data values, ω_{di} is a data dependent weight, μ is a weight factor, and ϕ_r is a Tikhonov regularization
15 term. Here, ϕ_r is defined as preferred difference regularization, where the preferred difference
16 between neighboring parameter values is set to zero. Φ_{total} is minimized iteratively, and the
17 regularization weight factor, μ , is calculated during the iteration to ensure that Φ_m , the measurement
18 part of the objective function, becomes approximately equal to a user specified target value (Doherty
19 2010).

20

21 **3 Synthetic example**

22 For illustrative purposes, we use a three dimensional synthetic system very similar to that presented by
23 Christensen et al. (2016). The only difference is that the active part of the groundwater system only
24 consists of 5 layers whereas Christensen et al. (2016) used a 20 layer model.

25 **3.1 Groundwater reference system and hydrological data**

26 The groundwater system is intended to mimic a glacial landscape and covers an area that is 7000 m
27 (N-S) by 5000 m (E-W). The geology of the system was generated using T-PROGS (Carle 1999) as

1 having a horizontal discretization of 25 m x 25 m, and a vertical discretization of 10 m. The system
2 extends 50 m in the vertical direction where it reaches impermeable clay with a horizontal surface.
3 The T-PROGS generated geology above the impermeable clay consists of categorical deposits of sand,
4 silt and clay. Within each of the three types of deposits, hydraulic conductivity, recharge and porosity
5 were generated as horizontally correlated random fields using FIELDGEN (Doherty 2010). All
6 boundaries of the domain were defined as having no-flow conditions except the southern boundary
7 where hydraulic head was defined as constant, $h = 0$ m. The local recharge depends on the type of
8 sediment at the uppermost layer. Most groundwater discharges through the southern boundary, but
9 approximately 35% discharges into a river running north to south in the middle of the domain (Figure
10 2). Groundwater flow was simulated as confined steady-state flow employing MODFLOW-2000
11 (Harbaugh et al. 2000) with the spatial discretization equal to the geological discretization.
12 Groundwater is pumped at a rate of $0.015 \text{ m}^3\text{s}^{-1}$ from a well located at $x=2487.5\text{m}$ and $y=1912.5 \text{ m}$ and
13 the well screens the deepest 10 meters of the groundwater system. In the following, this system is
14 called the *reference system*.

15 Thirty-five boreholes are found within the domain (Figure 2). Each borehole contains a monitoring
16 well that screens the deepest 10 m of sand registered in the borehole. For each system realization,
17 hydraulic head in the 35 wells and the river discharge at the southern boundary were extracted from a
18 forward simulation made by MODFLOW-2000. The 35 simulated hydraulic head values were
19 contaminated by independent Gaussian error with zero mean and 0.1 m standard deviation. The river
20 discharge was corrupted with independent Gaussian error with zero mean and a standard deviation
21 corresponding to 10% of the true river discharge. The 36 contaminated values constitute the
22 hydrological data used for groundwater model calibration.

23

24 **3.2 Geophysical reference system and data**

25 The geophysical reference system was designed so that there is perfect correlation between hydraulic
26 conductivity and electrical resistivity. This implies that a relationship between hydraulic conductivity
27 and measured electrical resistivity is likely to exist. The true relationship is of the same form as eq. (3
28), and it uses constant shape factor values $\alpha = 1e^{-12}$ and $\beta = 4$. This corresponds to conditions
29 where clay has low electrical resistivity and also low hydraulic conductivity, and sand has high
30 electrical resistivity and high hydraulic conductivity. The impermeable clay at the base of the
31 reference system was assigned a constant value of 5 ohm-m.

32 The AEM data were simulated using AarhusInv (Auken et al. 2014) for a system setup similar to a
33 typical dual-moment SkyTEM-304 system (Sørensen and Auken 2004). The simulated survey consists

1 of 35 E-W flight lines with 200 meter spacing between the flight lines. AEM system responses were
 2 simulated for every 25 m along the flight lines giving a total of 6300 sounding locations for both the
 3 transmitted high and low moments. AarhusInv is a 1D modeling code. To mimic the loss of resolution
 4 with layer depth we simulated the responses using the 2D logarithmic average resistivity of all model
 5 cells inside the radius of the foot print at a given depth. To obtain the geophysical data set, the
 6 simulated data were contaminated with noise according to the noise model suggested by (Auken et al.
 7 2008):

$$V_{resp} = V \cdot \left(1 + G(0,1) \cdot \left[STD_{uni}^2 + \left(\frac{V_{noise}}{V} \right)^2 \right]^{1/2} \right) \quad (5)$$

8
 9 where V_{resp} is the perturbed synthetic data, V is the synthetic noiseless data, $G(0,1)$ is standard
 10 Gaussian noise (with zero mean and unit standard deviation), and STD_{uni}^2 is uniform noise variance.
 11 V_{noise} is the background noise contribution given by

$$V_{noise} = b \cdot \left(\frac{t}{10^{-3}} \right)^{-1/2}, \quad (6)$$

12
 13 where t is the gate center time in seconds, and b is the background noise level at 1 ms. For the
 14 following analysis we generated geophysical datasets with four levels of background noise, i.e. b equal
 15 to 1, 3, 5, and 10 nV/m², respectively. The uniform standard deviation, which accounts for instrument
 16 and other non-specified noise contributions, was set to 3% for $d\mathbf{B}/dt$ responses. After the data were
 17 perturbed with noise, it was processed as a field data set (Auken et al. 2009), resulting in an uneven
 18 number of gates per sounding. Figure 3 illustrates the resulting low and high moment AEM sounding
 19 data, respectively, for the different background noise levels.

20

21 **3.3 Geophysical voxel inversion**

22 The geophysical data were inverted by voxel inversion (Fiandaca et al. 2015) using AarhusInv (Auken
 23 et al. 2014). The voxel inversion was conducted in two different ways: by using L2-norm “smooth”
 24 constraints, or by using minimum gradient support “sharp” constraints (both implemented in
 25 AarhusInv; Auken et al. 2014).

1 To avoid the influence of numerical discretization errors, the geophysical voxel inversion uses the
2 same spatial discretization as the reference system and the groundwater model. For both smooth and
3 sharp inversions, a 40 ohm-m uniform half-space was used as the starting model and spatial
4 regularization was applied using the same settings throughout all inversions. Considering the small
5 number of layers and the shallow discretization, it was unnecessary to apply vertical constraints for
6 any of the inversions. On the contrary, depth and direction dependent horizontal constraint factors
7 were used for both smooth and sharp inversions. The strength given to the horizontal constraints is
8 based on experience, keeping in mind that the constraint factors should not prevent data fitting, but
9 promote model consistency. Therefore, a few experiments were made to “manually” tune the
10 magnitude of the constraint factors. Different values along the flight lines and perpendicular to them,
11 respectively, were found to give better results. This is a result of having higher data density along the
12 flight lines, compared to the perpendicular direction. In these synthetic tests (similar to what is done
13 with field data with analogous data density) the smooth regularization constraint factors of 1.9 along
14 the flight lines and 1.05 perpendicular to the flight lines were used for the first layer.

15 Contrary to the conventional inversion of geophysical data, where the vertical discretization of the
16 geophysical model is normally characterized by logarithmically increasing layer thicknesses, in this
17 study fixed layer thicknesses were used in the geophysical models. To account for the loss of
18 resolution with depth without increasing the layer thicknesses, the horizontal constrain factors were set
19 to decrease linearly with depth (tighter bands for the deeper layers), resulting in constraint factors of
20 1.4 along the flight lines and 1.02 perpendicular to the flight lines for the sixth layer.

21 The same directional and depth-dependent tuning used for smooth regularization was also applied to
22 the sharp inversion. In this case constraint factors of 1.0625 along the flight lines and 1.01
23 perpendicular to the flight lines were used for the first layer, while factors of 1.025 along the flight
24 lines and 1.01 perpendicular to the flight lines were used for the sixth layer. The smaller values of the
25 constraint factors in the sharp inversion are due to the different role that the factors play in the
26 regularization definition, as evident when comparing eq. (1) and eq. (2). The difference in constraint
27 values between smooth and sharp inversion is analogous to what has been used in other studies (e.g.
28 Vignoli et al., 2015).

29

30 **3.4 Groundwater model parametrization and calibration**

31 In the following, the groundwater model will be parameterized in two different ways. Both approaches
32 treat the shape factors between hydraulic conductivity and resistivity, α and β , in relationship (3), as

1 spatially dependent parameters to be estimated. The two parameterizations differ by the resistivity
 2 model that is used to calculate the hydraulic conductivity field of the groundwater model:

- 3 • The first type of parameterization uses a resistivity model estimated by smooth voxel inversion of
 4 AEM data collected with a background noise level of 3 nV/m². These models will be referred to
 5 as SHI-smooth-3.
- 6 • The second type of parameterization uses a resistivity model estimated by sharp voxel inversion of
 7 AEM data collected with a background noise level of either 1, 3, 5, or 10 nV/m². These models
 8 will be referred to as SHI-sharp-1, SHI-sharp-3, SHI-sharp-5, and SHI-sharp-10, respectively.

9 The shape factors, α and β , of the petrophysical relationship are parametrized by placing pilot points in
 10 a uniform grid, with 5 nodes in the x direction and 7 in the y direction. Hence, in total the groundwater
 11 model is parameterized by $5 \times 7 \times 5 = 175$ petrophysical relationships each having two parameters (the
 12 shape factors).

13 The parameter values are estimated by fitting the available hydrological data consisting of the 35
 14 observations of hydraulic head and one river discharge observation. Calibration is done by
 15 minimization the total objective function given by eq. (4), where the measurement objective function
 16 is computed as

$$\Phi_m = n_h^{-1} \sum_{i=1}^{n_h} \omega_h (h_{obs,i} - h_{sim,i})^2 + n_r^{-1} \sum_{i=1}^{n_r} \omega_r (r_{obs,i} - r_{sim,i})^2 \quad (7)$$

17

18 where, n_h and n_r are the number of head and river measurements, respectively; h_{obs} and h_{sim} are
 19 observed and corresponding simulated hydraulic heads; r_{obs} and r_{sim} are observed and
 20 corresponding simulated river discharge; and ω_h and ω_r are subjectively chosen weights for head and
 21 discharge data, respectively. If a model is expected not to have structural defects then it would be ideal
 22 to choose the weights $\omega_h = \sigma_h^{-1}$ and $\omega_r = \sigma_r^{-1}$, where σ_h and σ_r is the standard deviation of
 23 measurement error for head and river measurements, respectively. However, in this case (as in all real
 24 cases) the model has structural errors that make the misfit between hydraulic head data and equivalent
 25 simulated values much larger than what can be explained by measurement error. In accordance with
 26 common groundwater modeling practice (e.g. Christensen et al. 1998), we therefore conducted
 27 residual analysis and a few experiments to estimate the magnitude of the total head error (which is the
 28 sum of observation error and structural error). This indicated that the standard deviation for the total
 29 error on hydraulic head is approximately $10 \cdot \sigma_h$, while the total error for the river discharge is totally
 30 dominated by measurement error. As weights we therefore used $\omega_h = (10 \cdot \sigma_h)^{-2} = 1.0$ and

1 $\omega_r = (\sigma_r)^{-2} = 1.38 \cdot 10^5$, respectively. Using these weights, and averaging over the 20 system
2 realizations, gave a minimized objective function value of $\bar{\phi}_m = 2.5$. This is close to the value of 2.0 ,
3 which would be expected from (7) if the weighting used reflects the error magnitudes.

4 Calibration was performed using BeoPEST, a version of PEST (Doherty 2010) that allows the
5 inversion to run in parallel using multiple cores and computers.

6 It should be noted that for calibration and model prediction we applied the recharge field and boundary
7 conditions of the reference system.

8 **3.5 Model predictions**

9 In step 3 (Figure 1, box), the calibrated groundwater model is used to make predictions.

10 In the following synthetic demonstration study, the calibrated SHI-smooth and SHI-sharp groundwater
11 models are evaluated by comparing their simulated model predictions with corresponding predictions
12 simulated for the (synthetic and, therefore, known) reference system. The former are called “model
13 predictions”, the latter are called “reference predictions”.

14 Prediction types 1 and 2 relate to steady-state flow when groundwater is pumped from the well. This is
15 also the condition for which the hydrologic data used for calibration were sampled. Type 1 is the
16 average age of the groundwater pumped from the well. Type 2 is the size of the recharge area of the
17 pumping well. Both of these predictions differ in type from the calibration data. For these model
18 predictions, we used a homogeneous porosity of 0.2 (the average value of the reference system
19 porosity fields is 0.184).

20 Prediction types 3 and 4 relate to a new stress situation long after pumping from the well has ceased:
21 type 3 is groundwater discharge into the stream, and type 4 is head recovery for a well screening a
22 layer north-east of the pumping well (the location is shown on Figure 2).

23 The reference and model prediction types 3 and 4 were simulated by MODFLOW-2000 (Harbaugh et
24 al. 2000), while type 1 and 2 were simulated by forward particle tracking using MODPATH version 5
25 (Pollock 1994) and MODFLOW-2000 results.

26 The first two types of prediction are interesting from the perspectives of protection and resource-
27 management of a well field, while the latter two are relevant in the case of possible change of
28 management practice resulting in a new stress.

29

3.6 Evaluation of prediction performance

As said in the beginning of section 2, steps 1-3 of the framework can be repeated for a number of system realizations to provide consistent statistical interference regarding the model prediction results. Here, 20 different reference system realizations were used. For each prediction, we therefore have 20 corresponding sets of reference predictions and model predictions that can be used to evaluate the performance of a calibrated model with respect to that prediction. The performance is evaluated for SHI-smooth and SHI-sharp models, respectively, and it is done in the following ways.

Prediction error characteristics are quantified by the mean absolute error (MAE), the mean error (ME) following:

$$MAE = \frac{1}{N} \sum_{i=1}^N |x_i - t_i| \quad (8)$$

$$ME = \frac{1}{N} \sum_{i=1}^N x_i - t_i \quad (9)$$

11

where x_i is the model prediction of realization i , t_i is the reference prediction of realization i , and $N = 20$ is the number of system realizations. MAE measures how close the model prediction tends to be to the reference prediction; ME measures the tendency of positive or negative bias in the model prediction.

16

4 Results

4.1 Geophysical results

Figure 4 shows a representative cross-section for one of the 20 system realizations. Both geophysical models in Figure 4 were inverted using data perturbed with a background noise level of 3nV/m^2 . Comparing the geophysical model results with the reference model, we find that the SHI-smooth-3 resolves the main features reasonably well for the upper layers. The main discrepancy is found in the fifth layer, where the sand bodies are not resolved. In general, the resistivity of the sand bodies (dark orange in the reference system) is underestimated, and the transitions between the categorical deposits are artificially smooth.

1 Figure 4 shows that SHI-sharp-3 resolves the sand body in layer 5 much better than SHI-smooth-3.
2 Moreover, the locations and boundaries of the geological deposits tend to be less smeared out when
3 using the sharp constraints. Inspection of the histograms at the bottom of Figure 4 shows that the SHI-
4 sharp-3 model tends to produce resistivity distributions that are more similar to the reference
5 distributions than the SHI-smooth-3 model. This improvement could allow for easier translation from
6 electrical resistivity into hydraulic conductivity and correspondingly more faithful representation of
7 hydrogeologic structure and connectivity.

8 Figure 5 shows voxel by voxel density plots of reference versus estimated electrical resistivity for a
9 SHI-smooth model and corresponding SHI-sharp models. Pearson's correlation coefficient (PCC;
10 Cooley and Naff 1990) is shown on top of the density plot for each layer. A comparison of the density
11 plots and the PCC values of the SHI-smooth-3 and SHI-sharp-3 models shows that using sharp instead
12 of smooth constraints improves the inverted geophysical model. The improvement is seen most clearly
13 for the sand deposits

14 For both SHI-smooth and SHI-sharp models there is a strong correlation between the electrical
15 resistivity estimates and the true electrical resistivities of the first layer, but the SHI-smooth model has
16 weaker correlation than the SHI-sharp models. For both types of models, the correlation weakens with
17 depth and background noise. The former is caused by the resolution limitations of AEM data.
18 However, the depth and resistivity of the low-resistivity clay at the base of the model are well resolved
19 by both the SHI-smooth and SHI-sharp models inversions (results not shown).

20 **4.2 Hydrological calibration results**

21 The calibration results for the 20 different system realizations are shown in Figure 6. The figure shows
22 that the measurement objective function value, Φ_m , for most system realizations is close 2.0. This is
23 the case for almost all of the SHI-Sharp model realizations, even for large background noise levels.
24 For many of the realizations, the SHI-Smooth model also fits the data well; but, several realizations
25 lead to higher misfit than desired. This makes $E[\Phi_m]$ equal to 5.8 for SHI-Smooth-3 models while it
26 is 2.5 for the SHI-Sharp-3 models. That is, the estimated hydraulic conductivity field tends to be better
27 for sharp models than for smooth models.

28

29 **4.3 Parameter estimation**

30 Figure 7 shows a cross section of the estimated K -, α - and β - fields for one of the system realizations.
31 The two columns show estimates for the SHI-smooth-3 and SHI-sharp-3 models. Figure 8 shows a
32 density plot of the reference hydraulic conductivity distribution and the estimated hydraulic

1 conductivity distributions. The results in Figure 7 and Figure 8 are typical for all 20 system
2 realizations.

3
4 From Figure 7 a) and Figure 7 b) it is seen that the estimated α and β parameter values change
5 smoothly in the horizontal direction but have sharp transitions in the vertical direction. The second
6 row of Figure 7 shows the corresponding estimated K fields whose main features are determined by
7 the underlying resistivity models (Figure 4), but they are “corrected” during model calibration to make
8 the groundwater model fit the hydrological data.

9
10 For the SHI-smooth-3 model, α and β are taking compensatory roles particularly in the first layer.
11 Here, the estimated α and β values are higher than the shape factors of the true relationship that was
12 used to construct the geophysical reference system. This increases the hydraulic conductivity in layer
13 1 to compensate for the too low hydraulic conductivity (and resistivity, Figure 4) in layer 2 and deeper
14 layers. The estimated α and β values are not sufficient to compensate for the missing deep high-
15 resistivity body in in layer 5 of the SHI-Smooth-3 model (Figure 4).

16
17 For the SHI-sharp-3 model, the estimated α and β parameter values only vary slightly from the shape
18 factor values of the true relationship except for layer 5 (Figure 7 b). This indicates that for the
19 shallower layers the sharp inversion of AEM data sufficiently resolves the resistivity of features that
20 are important for groundwater model calibration. In layer 5 the estimate of shape factor β turns out to
21 be fairly high to compensate for the too low resistivity estimates in this layer (Figure 4).

22 Figure 8 shows voxel by voxel density plots of reference versus estimated hydraulic conductivity for
23 SHI-smooth and SHI-sharp models. The results confirm that the K field tends to be overestimated for
24 the first layer, in particular for the SHI-smooth-3 model. From the second layer and deeper, the
25 hydraulic conductivity values tend to be underestimated for sand but overestimated for silt and clay.
26 Moreover, the distributions of estimated K smear out with depth. Judged by PCC values and visual
27 inspection of Figure 8 (highlighting connectivity of the K field), the hydraulic conductivity field
28 estimated for any SHI-sharp model is in better agreement with the reference field than the field
29 estimated by the SHI-Smooth-3 model.

30 Model structural accuracy is quantified in

1 Table 1 for both the SHI-smooth and SHI-sharp models. Structural accuracy is calculated here as the fraction
2 of total number of voxels for which the estimated \log_{10} -hydraulic conductivity plus/minus twenty percent
3 contains the true \log_{10} -hydraulic conductivity value of the reference model. The results are averaged over the
4 20 system realizations. From

1 Table 1 it is seen that all SHI-sharp models outperform the accuracy of the SHI-smooth models except
2 for layer 5. The exception occurs because the SHI-smooth models are fairly good at estimating the K
3 distributions for silt and clays, but underestimates K for sand (Figure 8). On the contrary, SHI-sharp
4 models overestimate the K distributions for silt and clays, but only slightly underestimate K for sand
5 (Figure 8). Therefore, for layer 5, the model structural accuracy appears to be better for SHI-smooth
6 than for SHI-sharp models.

7

8 **4.4 Prediction results**

9 For each of the 20 system realizations, the calibrated groundwater models were used to make the
10 model predictions described in section 3.5. Figure 9 shows scatter plots of reference prediction versus
11 the calibrated model prediction; each plotted point corresponds to a particular system realization and
12 corresponding SHI-smooth-3 or SHI-sharp-3 model. The mean error (ME) and mean absolute error
13 (MAE) of the prediction are also given in Figure 9. Figure 10 shows a MAE contour map for head
14 recovery predictions.

15

16 **4.4.1 Particle tracking predictions**

17 The first column of Figure 9 shows results for prediction of average age of the groundwater pumped
18 from the pumping well. The scatter plot illustrates that SHI-sharp models tend to over-predict average
19 age. This is seen by the majority of points plotting above the identity line as well as by the value of
20 $ME = 32$ (Figure 9). The age prediction results are similar for the SHI-smooth models although the
21 spread of points is larger than for SHI-sharp-3 (e.g. quantified by the larger value of MAE). There are
22 two major explanations for these relatively “poor” predictive performances. First, the calibrated K -
23 fields underestimate hydraulic conductivity of sand deposits in the deeper layers (Figure 8), which
24 results in too slow particle travel times at depth. Secondly, the reconstruction of the deepest layers is
25 too smooth for both SHI-smooth and SHI-sharp models (Figure 7) and does not resolve the small-scale
26 variability that controls the transport of particles.

27 The second column of Figure 9 reports results related to prediction of the recharge area of the
28 pumping well. The scatter plot shows that the SHI-smooth models under-predicts the recharge area.
29 This happens because the smooth models lead to estimation of hydraulic conductivities in the deepest
30 layers that are too low. This creates a deep cone of depression around the pumping well that extends
31 upward locally to reach the river bed. This induces a local discharge of water from the stream through
32 the groundwater system to the pumping well. These models thus predict that a significant proportion

1 of the pumping comes from local discharge from the river. (This is compensated by increased model
2 predicted groundwater discharge to other parts of the river.) For the true, reference system used to
3 generate the data, the river is not losing water, and all water pumped from the well originates from
4 groundwater recharge.

5 The SHI-sharp models are better predictors of the recharge area, but also these models tend to predict
6 an area that is too small. These models also predict local discharge from the river to the groundwater
7 system, but to a lesser degree than the SHI-smooth models. This is likely because the main features of
8 the reference system are better reconstructed by the SHI-sharp-3 models.

9

10 **4.4.2 Head recovery and discharge predictions**

11 The prediction of head recovery at an observation well (location shown in Figure 10) is done poorly
12 by the SHI-smooth-3 (Figure 9). The predicted head recovery is very small for most of these models
13 because they tend to have too little hydraulic connectivity between the deepest layers, the estimated
14 hydraulic conductivities are too low in the deep sand layers, and the simulated cone of depression is
15 therefore too deep and too local.

16 The SHI-sharp-3 models make less biased, fairly reasonable predictions of the head recovery (Figure
17 9) because they resolve the variations of hydraulic conductivity at depth better than the SHI-smooth-3
18 models. The superiority of SHI-sharp-3 models for recovery prediction is also seen from the *MAE*
19 contour maps in Figure 9. The *MAE* is seen to be spatially dependent: it is largest at the pumping well,
20 and smallest at the constant head boundary to the south

21 The fourth column of Figure 9 shows that both types of models are good predictors of discharge to the
22 river after cessation of pumping. However, the SHI-sharp-3 model prediction is superior (its points
23 plot closer to the identity line). For SHI-smooth-3, the prediction tends to be positively biased and
24 more spread than for SHI-sharp-3.

25

26 **4.4.3 Prediction error as function of data quality**

27 In Figure 11 *MAE* is used as a metric to evaluate how the prediction performance of SHI-sharp
28 models depends on the level of background noise for the geophysical data. The noise levels were kept
29 unchanged for the hydrological data.

30 Figure 11 shows that the average age prediction made by SHI-sharp models are nearly unaffected by
31 the quality of the geophysical data. It is speculative, but this result may be because this prediction is

1 highly dependent on small scale variability in hydraulic conductivity and porosity that cannot be
2 resolved from any of the geophysical data sets. That is, even the highest quality geophysical data are
3 not highly informative, so reducing the data quality further has little effect.

4 It is different for the recharge area prediction (Figure 11): MAE increases for this by approximately
5 25% when the level of background noise is increased from 1 nV/m² to 10 nV/m². This happens
6 because the variations of resistivity (and thus hydraulic conductivity) are less well resolved when
7 using the poor quality geophysical data.

8 The third and fourth rows of Figure 11 show the head recovery and river discharge prediction after
9 cessation of the pumping well. Head recovery and discharge predictions also tend to depend on the
10 quality of the geophysical data. The MAE increases by 17 % for recovery prediction and 23 % for
11 discharge prediction when the noise level of the geophysical data increases from 1 nV/m² to 10 nV/m².

12

1 **5 Discussion**

2 **5.1 Estimation of Parameters in the Petrophysical Relation**

3 Parameterizing the groundwater model by assuming a spatially dependent petrophysical relationship
4 between resistivity and hydraulic conductivity makes it possible to use a resistivity voxel model for
5 construction and calibration of a groundwater model. Assuming that the relationship is spatially
6 dependent can account for two challenges: i) there may be actual changes in the petrophysical
7 relationship within an investigated domain, and ii) there may be resolution limitations in the estimated
8 resistivity model.

9 Challenge i) is likely to be the rule for many environments, especially sedimentary environments,
10 where the formation resistivity is primarily controlled by the pore water resistivity and the clay
11 content. In the case of spatial changes of pore water resistivity and/or content of various clay mineral
12 content, the discrimination between clay and sands may be less clear in the estimated resistivity
13 values. For large-scale groundwater system, the variation of pore water resistivity (e.g. saline pore
14 water) is expected to vary smoothly, which would be accounted for by the spatially varying
15 petrophysical relationship. However, the procedure only works as applied here if the underlying
16 assumption that clay rich deposits have lower electrical resistivity than sand deposits is valid, .

17 Challenge ii) concerns the geophysical model resolution of the true formation resistivity. EM methods
18 are, by nature, more sensitive to deposits of low electrical resistivity than to deposits of high
19 resistivity, and their vertical and horizontal resolutions decrease with depth. This challenge affects the
20 resistivity models estimated in the present synthetic study. Spatially dependent shape factors can take
21 a compensatory role for the resolution issues of the estimated geophysical voxel model. The calibrated
22 shape factors may thus no longer have firm physical meaning because they mainly act as correction
23 parameters for absorbing structural errors of the geophysical model. This is acceptable as long as the
24 resulting hydraulic conductivity values are reasonable. The idea of calibrating the shape factors is
25 related to the concept of compensatory parameters in highly parameterized calibration described by
26 Doherty and Welter (2010) and by Doherty and Christensen (2011).

27 Finally, Auken et al. (2008) showed that using borehole data as a priori information in the geophysical
28 inversion improves the reconstruction of the model features significantly. Estimation of EM-based
29 resistivity models should therefore, wherever possible, be supported by borehole information to
30 improve the decreasing spatial resolution of the EM methods.

31

5.2 Geophysical inversion strategy and data quality

Inversion of AEM data using a 1D geophysical model usually applies smoothness constraints in order to regularize the inversion (Auken and Christiansen 2004; Viezzoli et al. 2008). Traditionally, the regularization includes both lateral and vertical smoothing constraints (Constable et al. 1987) or a few layer parametrization (Auken et al. 2008). Inversion using the former type of regularization produces smooth images with blurred formation boundaries which can be problematic when it is important to resolve structural connections in a complex geological system. The latter few-layer inversion may also be prone to produce artifacts when used to map such systems. Day-Lewis (2005) and others therefore recognized that regularization used to stabilize the geophysical inversion can lead to artifacts that do not reflect the actual hydrogeological conditions. Thoughtless use of such results to construct groundwater models can have serious ramifications.

For the present case study, the number of vertical transitions is a great challenge for the AEM method due to the principle of high resistivity equivalence. That is, it is difficult to resolve a high-resistivity layer between two low-resistivity layers because the energy loss, and therefore the sensitivity, is concentrated in the less resistive layers. This will result in layer suppression, because the data sensitivity to the high resistive layer is low (Christiansen et al. 2006). This effect is present for both the smooth and sharp inversion, but in the sharp inversion the effect is less fuzzy and features, especially for the fifth layer, could be more clearly reconstructed (Figure 4). When the sensitivity of the AEM method is too low, the regularization may make information migrate from areas with higher measurement sensitivity (Vignoli et al. 2015). In contrast to the smooth regularization scheme, the sharp regularization method is designed to penalize smooth transitions which eventually improves the reconstruction of the deeper sand bodies in the present study. Therefore, for the studied case, the sharp regularization methodology should be preferred over smooth regularization, because the sharp constraints correspond better to the actual structures of the reference system (sharp transitions between categorical deposits; Figure 4). Moreover, because the sharp regularization methodology leads to improved reconstruction of subsurface structures, these models lead to greater accuracy and improvement of most groundwater model predictions (Figure 9).

The groundwater system considered here is relatively shallow, at least as seen from the perspective of the AEM system used in the demonstration example. This is evident from the transmitted EM signal (Figure 3). The background noise is primarily affecting the last time-gates (10^{-4} - 10^{-3} s) of the low-moment and only to a small degree the high moment time gates (even for low quality data). This implies that the resolution of the AEM data is generally high for the upper layers. Therefore, in the present case the upper layers of all the geophysical models (both SHI-smooth and SHI-sharp) are well-

1 resolved and to a large extent unaffected by AEM data quality (Figure 5). However, the deep sand
2 units are difficult to resolve because they give only a weak signature in the AEM data (Figure 3,
3 Figure 5). This is particularly true for the poorest AEM data quality cases where the late time gates for
4 the low moment measurements are disturbed by background noise.

5 **6 Summary and Conclusion**

6 We present a workflow for efficient construction and calibration of large-scale groundwater models
7 using a combination of airborne electromagnetic (AEM) data and hydrological data. Other types of
8 data could be integrated as well following the same procedure. First, the AEM data are inverted to
9 form a 3D geophysical model. Subsequently, the geophysical model is translated to a 3D model of
10 hydraulic conductivity by using a spatially dependent petrophysical relationship for which the shape
11 parameters are estimated by fitting the groundwater model to hydrological data. The estimated shape
12 factors of the petrophysical relationship primarily work as translators between resistivity and hydraulic
13 conductivity, but they can also compensate for structural defects in the model.

14 The method is demonstrated for a synthetic case study where the subsurface consists of categorical
15 deposits with different geophysical and hydraulic properties. The AEM data are inverted using both
16 smooth and sharp regularization constraints, resulting in two competitive geophysical models.
17 Furthermore, the influence of the AEM data quality is tested by inverting the sharp geophysical
18 models using data corrupted with four different levels of background noise. The resulting groundwater
19 models are each calibrated on basis of head and discharge data, and their predictive performance is
20 tested for four types of prediction beyond the calibration base. Predictions of a pumping well's
21 recharge area and groundwater age are applying the same stress situation as applied during hydrologic
22 model calibration, while predictions of head and stream discharge is done for a changed stress
23 situation.

24 It is found that a geophysical model inverted with sharp constraints (SHI-sharp) leads to a more
25 accurate groundwater model than one that is based on a geophysical model inverted with smooth
26 constraints (SHI-smooth). The SHI-sharp model leads to an estimated hydraulic conductivity field of
27 greater accuracy and to improvement of most groundwater model predictions. The explanation is that
28 the reference system (like many real hydrogeologic systems) is characterized by sharp transitions
29 between categorical deposits; this is resolved better by the SHI-sharp resistivity model than by the
30 SHI-smooth model.

31 Finally, it is shown that prediction accuracy improves with AEM data quality for predictions of
32 recharge area, head change and stream discharge, while the accuracy appears to be unaffected for

- 1 prediction of groundwater age, which cannot be predicted accurately even with high quality
- 2 geophysical data.

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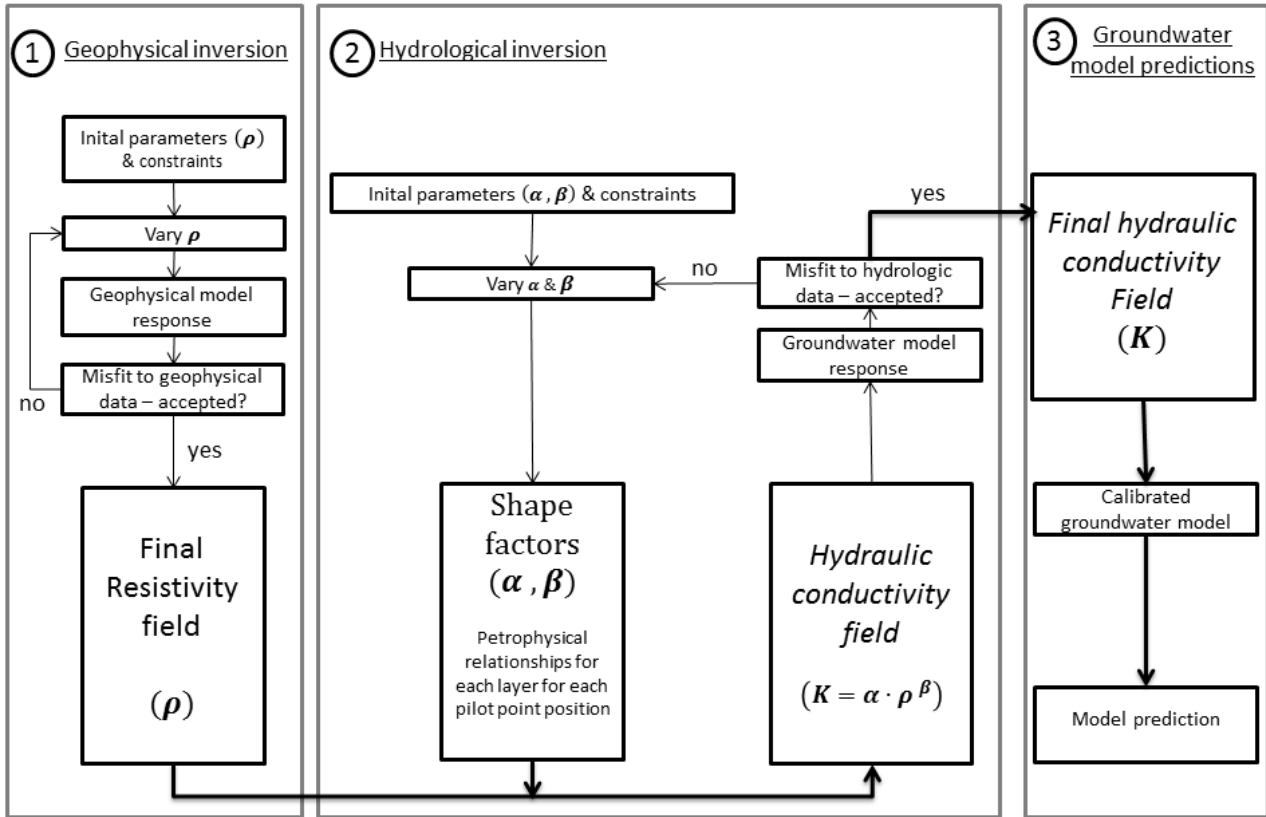
9

1 Table 1. Model structural accuracy comparison for groundwater model using both smooth or sharp
 2 geophysical models and different background noise levels. The results are averaged over the 20
 3 system realizations. A value of 1.0 means that the model’s hydraulic conductivity field is in good
 4 agreement with the reference field; a value of 0.0 means no agreement (see body text for exact
 5 definition of “structural accuracy”).

	Layer 1	Layer 2	Layer 3	Layer 4	Layer 5
SHI-1 Smooth	0.89	0.79	0.56	0.54	0.64
SHI-1 Sharp	0.96	0.91	0.81	0.61	0.48
SHI-3 Sharp	0.96	0.92	0.82	0.64	0.5
SHI-5 Sharp	0.96	0.91	0.78	0.64	0.49
SHI-10 Sharp	0.96	0.9	0.78	0.6	0.46

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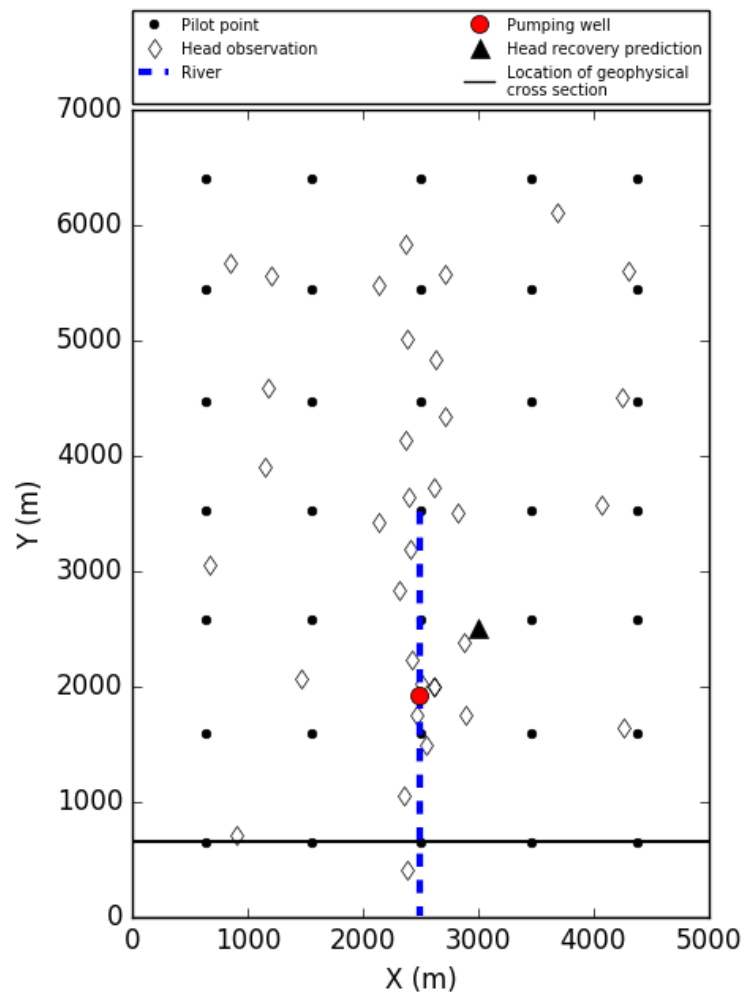


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3 Figure 1. Conceptual flowchart for the sequential hydrogeophysical inversion. First step (box 1):
4 geophysical inversion. Second step (box 2), groundwater model calibration where shape factors of the
5 petrophysical relationship is estimated using hydrological data. Third step (box 3): The calibrated
6 groundwater model is used for predictive modeling.

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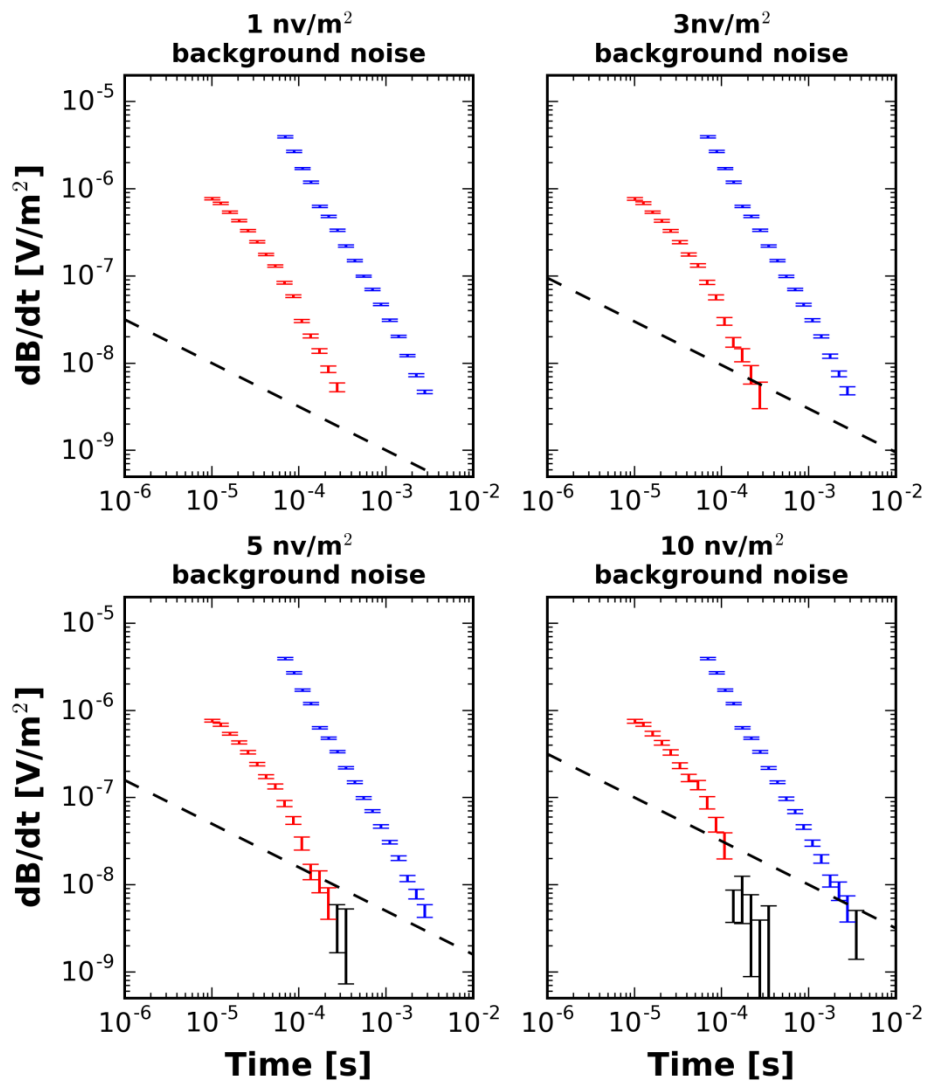
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2 Figure 2. A map of locations of boreholes, a pumping well, pilot points, head recovery prediction and
 3 location of a geophysical cross-section.

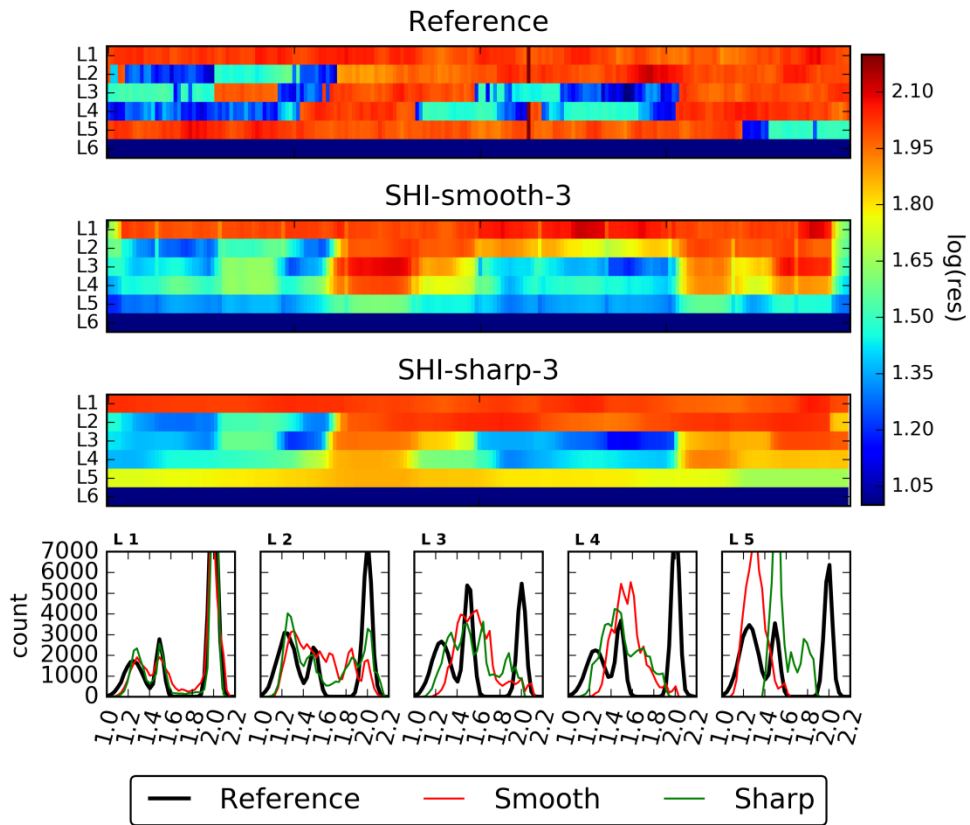
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2 Figure 3. AEM sounding data corrupted by four levels of background noise. The values on top of each
 3 subplot corresponds to the noise level at 1 ms and to the b -value in eq. 6. The black dashed curves
 4 indicate the background noise levels, low and high moment earth responses are illustrated as red and
 5 blue error bars, respectively, and the black error bars illustrate data which are removed by the data
 6 processing

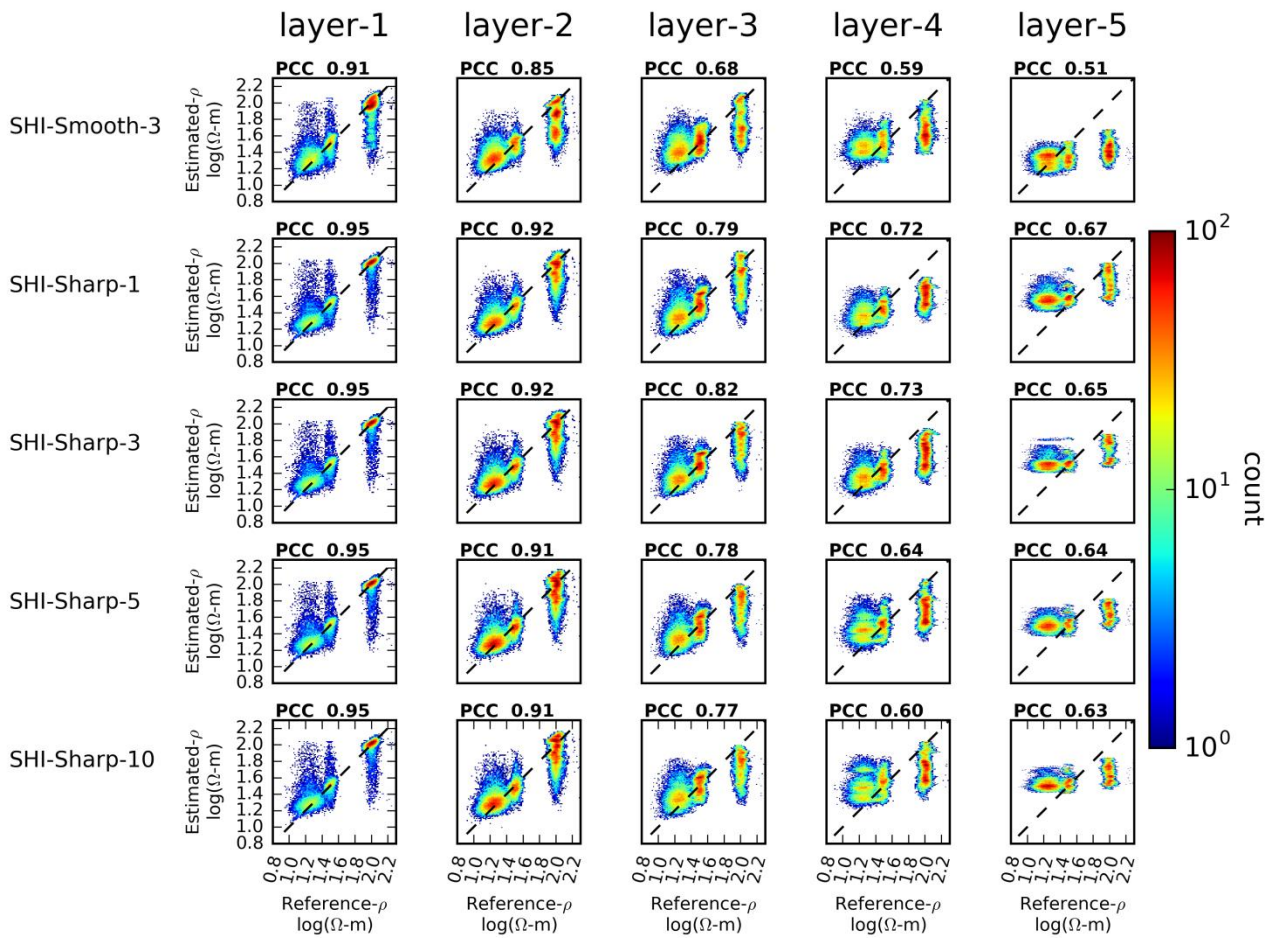
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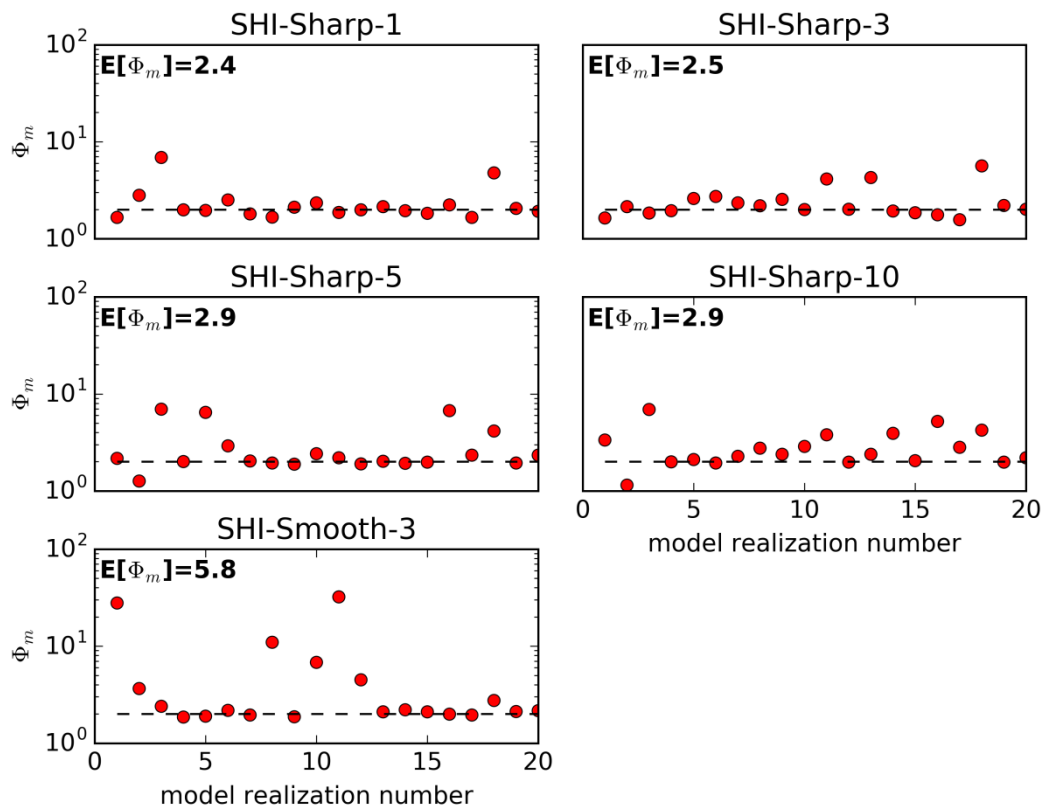
2 Figure 4. The figure shows an East-West cross section of resistivity for the reference system
 3 (realization number 20), and inversion results for Smooth and Sharp inversion, respectively. The last
 4 row shows at histogram of resistivity for each layer. The black curve is the resistivity distribution for
 5 the reference system, the red curve shows the resistivity distribution for the smooth inversion, and
 6 finally the green curve shows the resistivity distribution for the smooth inversion.

7



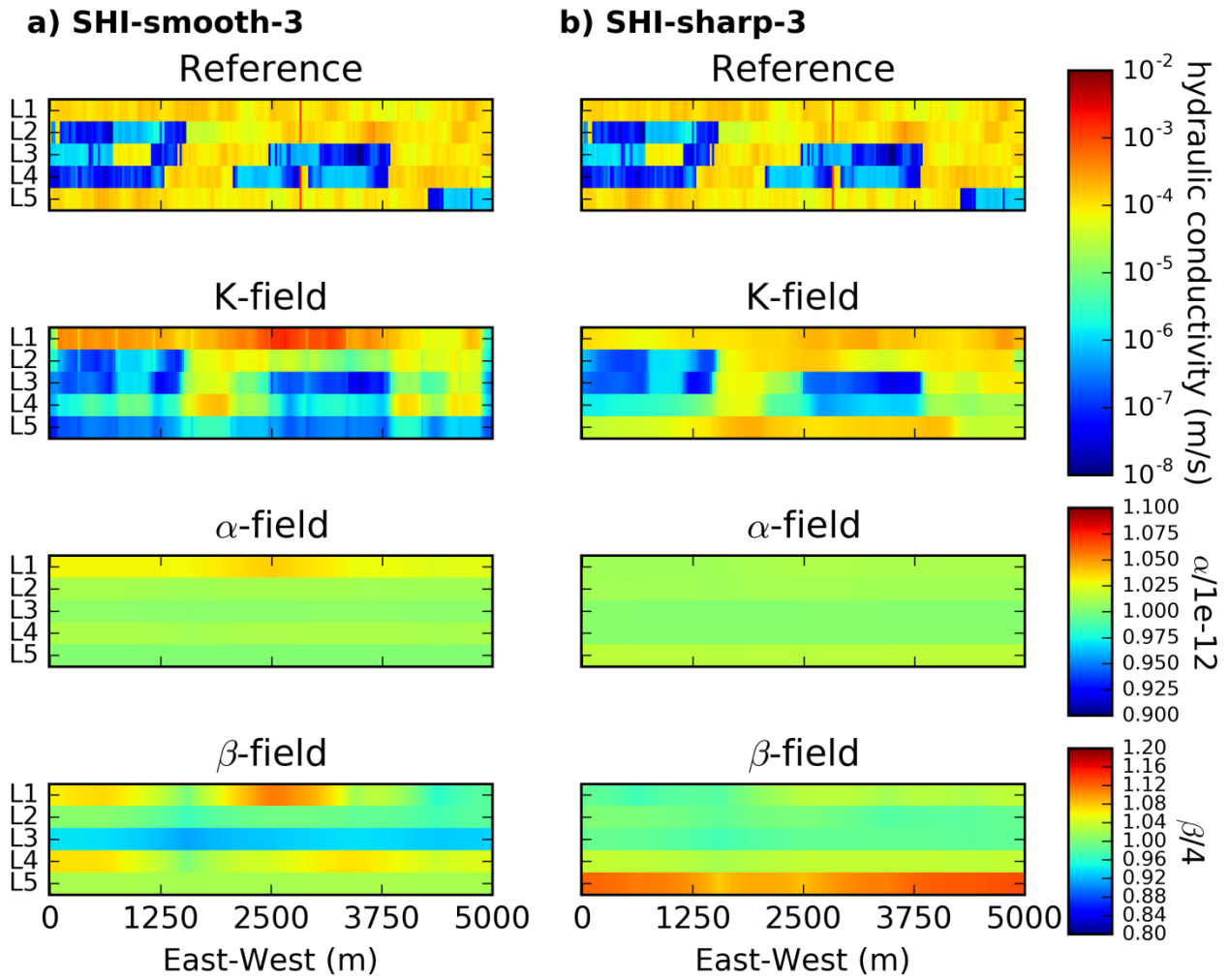
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2 Figure 5. Scatterplot of true and estimated electrical resistivity field for smooth geophysical inversion
 3 and sharp geophysical inversion for different data quality of the AEM data for model realization
 4 number 20. On top of each window is Pearson correlation coefficient (PCC) calculated.



1

2 Figure 6. Measurement objective function value obtained for the various groundwater model
 3 calibration cases, while $E[\Phi_m]$ is the mean value across all 20 different system realizations. The
 4 dashed line indicates the expected target value for the model calibrations.



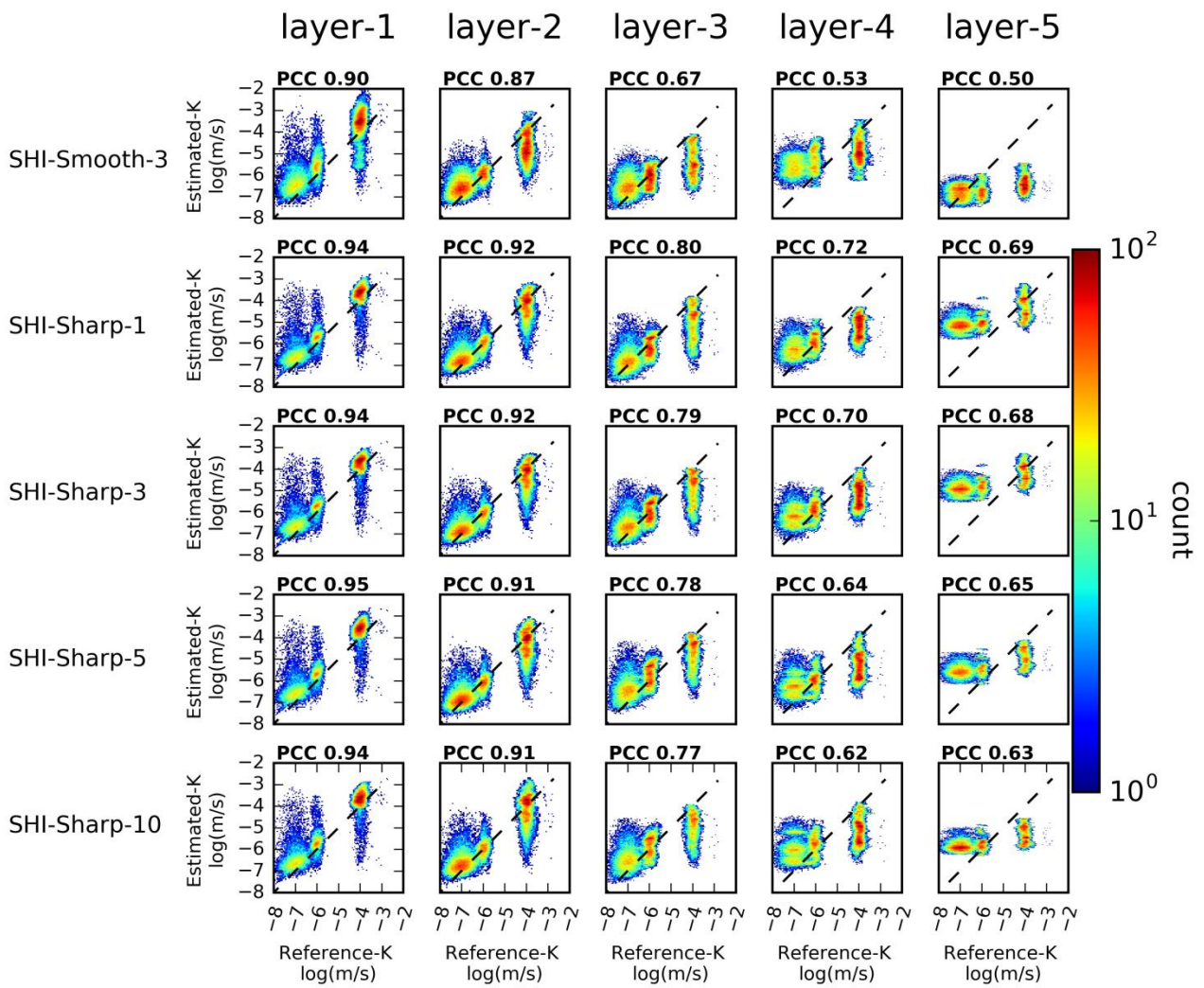
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2 Figure 7. East-West cross-section for model realization number 20. a) shows the parameters fields for
 3 the SHI-smooth-3 calibrated model. b) Shows the parameters fields for the SHI-sharp-3 calibrated
 4 model. First row shows the reference K-field, second row shows the estimated K-field, third and
 5 fourth row shows shape factors of the petrophysical relationship for alfa and beta, respectively.

6

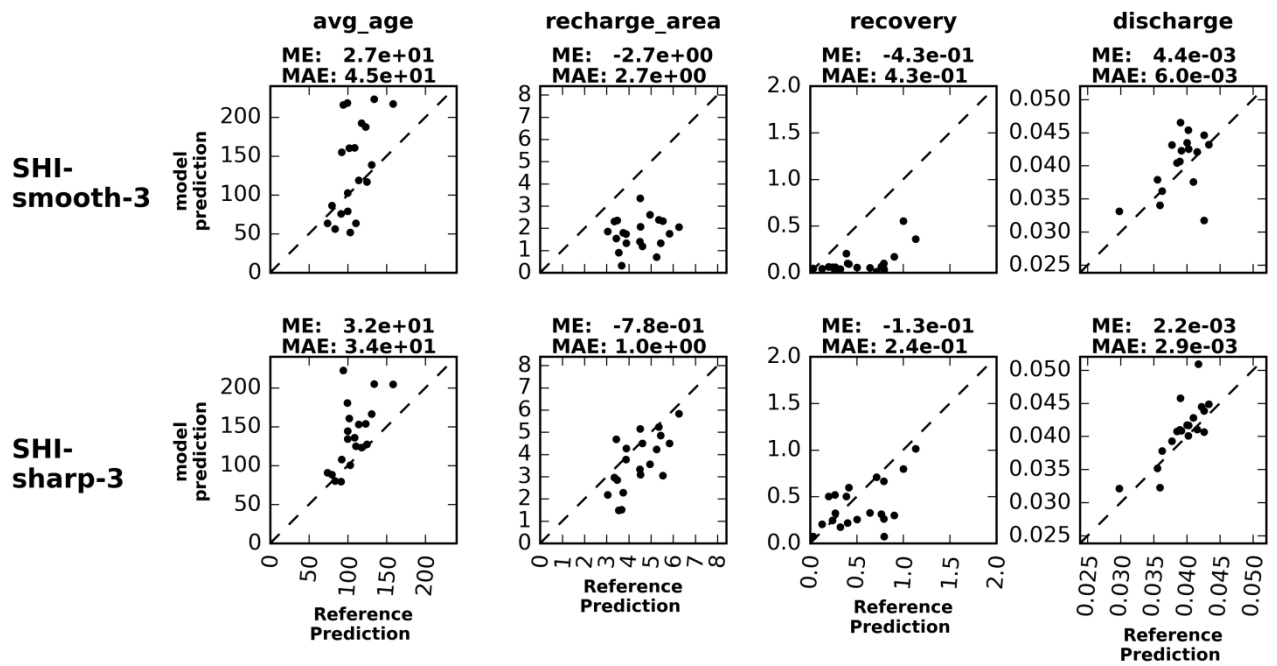
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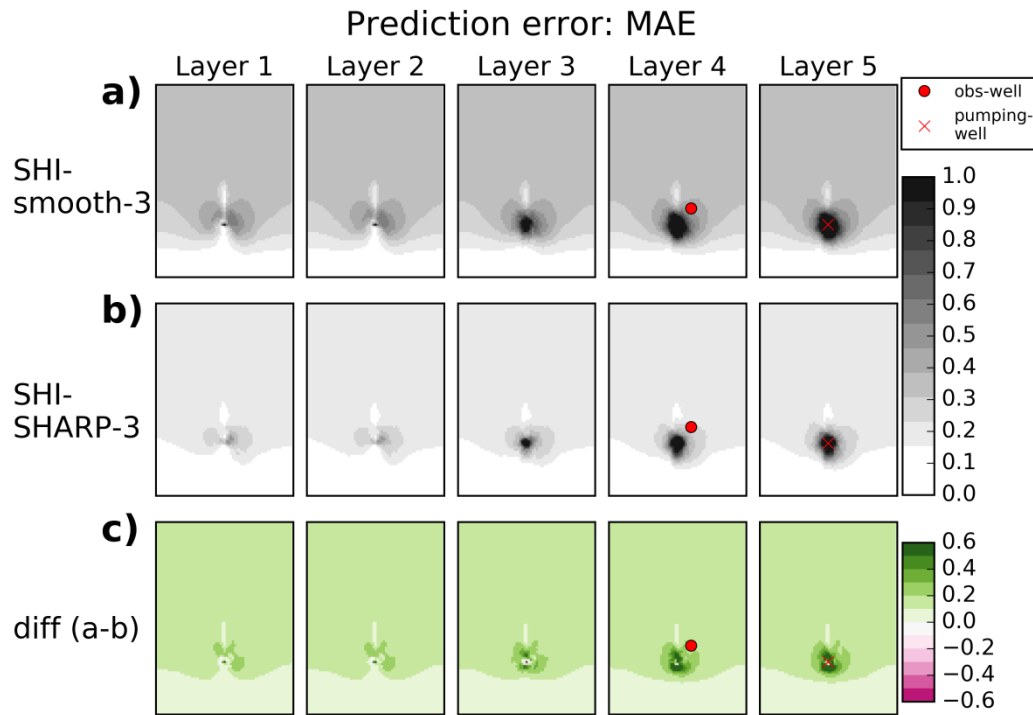
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2 Figure 8. Scatterplot of true and estimated hydraulic conductivity field for smooth geophysical
 3 inversion and sharp geophysical inversion for different data quality of the AEM data for model
 4 realization number 20. On top of each window is Pearson correlation coefficient (PCC) calculated.



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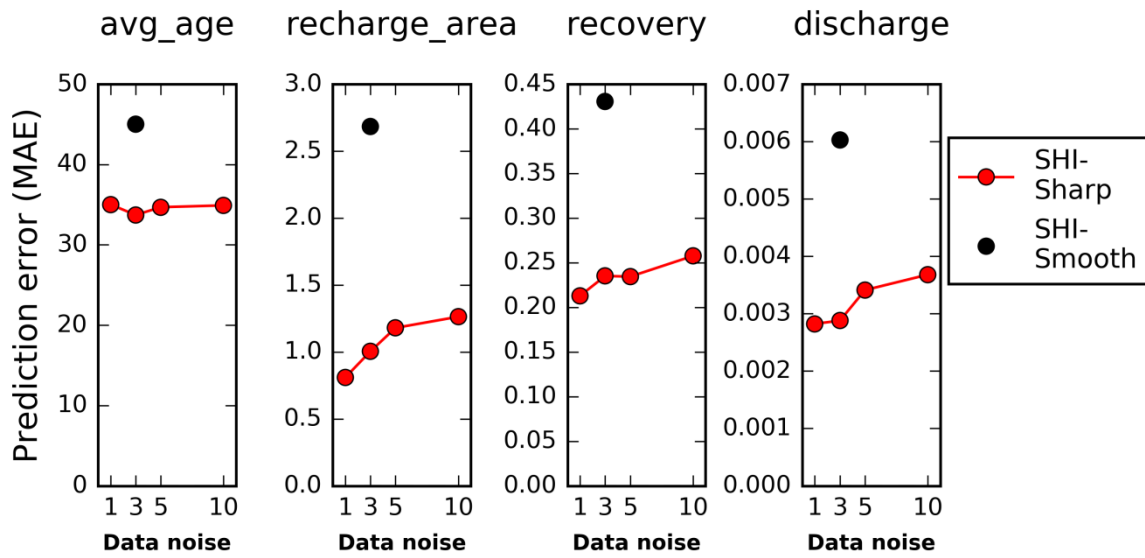
2 Figure 9. Scatter plots of calibrated model prediction versus the reference model prediction using
 3 results from the 20 system realizations. The plots in the first and second columns are the average
 4 groundwater age and recharge area, respectively, of the pumping well. Column three is for head
 5 recovery when pumping has stopped in the observation well shown in Figure 10, and column four is
 6 for groundwater discharge to the river after pumping has ceased *ME* and *MAE* are used to quantify the
 7 prediction error on basis of the 20 realizations.



1

2 Figure 10. MAE contour map for head recovery prediction. a) For predictions using the SHI-smooth
 3 models. b) For predictions using the SHI-smooth models. c) Difference between maps shown in a)
 4 and b). Red dot marks the location of the observation well for the head recovery prediction shown in
 5 Figure 9. The red cross marks the location of the pumping well.

6



1

2 Figure 11. Prediction error as function of the background noise on the geophysical data. The black dot
 3 is the SHI-smooth models using a background noise level of 3nV/m^2 . The red dots are the SHI-sharp
 4 models as a function of background noise level.