



Tracer test modeling for local scale residence time distribution characterization in an artificial recharge site

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Abstract.

Artificial recharge of aquifers is a technique for improving water quality and increasing groundwater resources. Understanding the fate of a potential contaminant requires knowledge of the residence times distribution (RTD) of the water beneath the artificial recharge infrastructure. A simple way to obtain the RTDs is to perform a tracer test. We performed a pulse injection tracer test in an artificial recharge system through an infiltration basin to obtain the breakthrough curves, which yield directly the RTDs. These were very broad and we used a numerical model to interpret them, and to extend the characterization to other flow conditions. The model comprised nine layers at the site scaled to emulate the layering of aquifer deposits. Two types of hypotheses were considered: homogeneous (all flow and transport parameters identical for every layer) and heterogeneous (diverse parameters for each layer). The parameters were calibrated against the head and concentration data in both model types, which were validated quite satisfactory against 1,1,2-Trichloroethane and electrical conductivity data collected over a long period of time with highly varying flow conditions. We found that the broad RTDs were caused by both the complex flow structure generated under the basin (the homogeneous model produced broad RTDs) and the heterogeneity of the media (the heterogeneous model yielded much better fits). We conclude that acknowledging heterogeneity is required to properly assess mixing and broad RTDs, which are required to explain the water quality improvement of artificial recharge basins.

15 1 Introduction

The need to satisfy increasing demand for water is the main driver behind managed aquifer recharge, which is becoming a standard technique for replenishing and/or enhancing groundwater resources. One of the goals of managed aquifer recharge is to provide aquifers with good water quality, even when lesser quality water is used to recharge the aquifer (e.g., treatment plant effluents or runoff water).



Water quality is enhanced during passage through soil (Bouwer, 2002; Greskowiak et al., 2005) because the passage causes reduction in the concentrations of dissolved organic matter (Vanderzalm et al., 2006), nutrients (Bekele et al., 2011), pathogens (Dillon et al., 2006), and some trace organic contaminants (Dillon et al., 2006; Hoppe-Jones et al., 2010; Valhondo et al., 2014, 2015). The appropriate management of artificial recharge systems requires an understanding of the fate of the potential 5 contaminants. This is especially relevant for recharge through infiltration basins or river bank filtration, which typically involve larger volumes of poorer quality water than typically used for injection wells.

The fate of contaminants depends on hydraulics, which is the focus of this work, and biochemistry. Hydraulics control the residence time distribution (RTD) of the recharged water reaching the wells, which is required to (1) ascertain the removal of potential contaminants, (2) interpret actual observations of removal, and (3) foresee (and eventually correct) future changes in 10 groundwater quality.

Understanding hydraulics entails an understanding of the spatial and temporal distributions of water fluxes around the recharge system and the relationship between the recharge system and the aquifer (i.e., recharge affected area, mixing of recharged and native groundwater, travel times) (Clark et al., 2004, 2014; Massmann et al., 2008; Bekele et al., 2014). The flux distribution is affected by the complexity and heterogeneity of natural systems. Sedimentary deposits frequently have 15 alternating layers with varying grain size distributions that may cause the aquifer to behave locally as a multilayer system, where the actual flux distribution is not controlled as much by the hydraulic conductivity within the layers as by their continuity and inter-connectivity, particularly in the vertical dimension (Fogg, 1986; Martin and Frind, 1998). Characterizing heterogeneity in such systems at the recharge basin scale is required for proper representation of RTDs. But it is hard because the head differences are small and detailed hydraulic testing difficult to perform.

20 A reasonable and easy way to address heterogeneity and broad RTD is to perform tracer tests. Ironically, few tracer tests have been performed in the context of artificial recharge. Notable exceptions are the studies in Berlin, Germany (Massmann et al., 2008), which were restricted to environmental tracers due to the proximity to the water supply, and Orange County, California (Clark et al., 2004; Becker et al., 2015), which used environmental and deliberate (SF_6) tracers. In both cases, the goal was to monitor the recharge water plume. Both studies found a strong variation of groundwater age with depth. To the best of our 25 knowledge, however, no test has been performed to characterize the scale of a site. To this end, we performed a pulse injection at the Sant Vicenç site (Barcelona, Spain) (Valhondo et al., 2014, 2015) to obtain the RTDs by monitoring breakthrough curves.

The objective of this paper is to describe the tracer test and its interpretation using both heterogeneous and homogeneous models to assess the need for model complexity.

2 Materials and methods

30 2.1 Site description and instrumentation

The work was performed at the recharge basin owned by the Catalan Water Agency, located at Sant Vicenç dels Horts, approximately 15 km inland from the Mediterranean shore (Fig. 1 A) along the Llobregat Lower Valley aquifer (Barcelona, Spain).

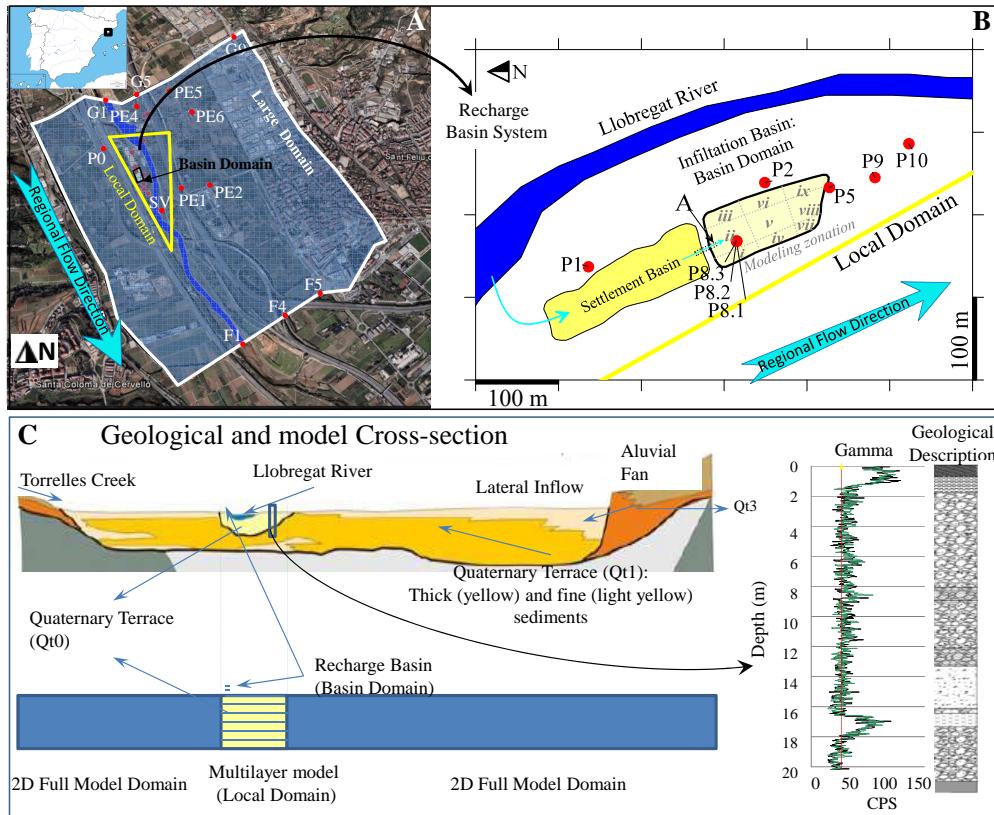


Figure 1. General (A) and local (B) plan views of the infiltration system, monitoring points, and model dimensions. (C) Geologic cross-section of the site and the conceptualization used in the numerical model, stratigraphic log and natural gamma measurements from the site. The area identified with the yellow triangle in A was modeled as a multilayer aquifer (see C).

Recharge water is taken from the Llobregat River, which is impacted by numerous treatment plant effluents (Köck-Schulmeyer et al., 2011). The Llobregat River was disconnected from the aquifer during the whole experiment period. The river water was diverted to a settlement basin ($\approx 5000 \text{ m}^2$), where it remained for 2 to 4 days. Thereafter, the water flowed to the infiltration basin ($\approx 5000 \text{ m}^2$), Fig. 1 B. Flow rate to the infiltration basin was measured hourly by the Llobregat groundwater users community (CUADLL). The infiltration rate averaged 1 m/d. The system has been operational since February 2009.

5 A 65 cm-thick reactive barrier was installed at the bottom of the infiltration basin in March 2011. The barrier comprises vegetable compost and aquifer sand in equal volumetric proportions and a very small fraction of clay and iron oxides. The vegetable compost aimed to release dissolved organic carbon into the infiltrating water to favor a broad range of redox conditions below the basin, thus increasing the diversity of microbial metabolic paths to enhance removal of organic contaminants (Li et al., 2013; Alidina et al., 2014; Valhondo et al., 2014, 2015). Clay and iron oxide were added to provide sorption sites for 10 cationic and anionic organic compounds, respectively. Details on the barrier are provided by Valhondo et al. (2014, 2015).



The aquifer beneath the basin comprises Quaternary alluvial sediments with a predominant portion of gravel and sand, and a small fraction of clay (Barbieri et al., 2011). The aquifer extends to a depth of 20 to 23 m below the ground at the site and is underlain by Pliocene marls that are assumed to be impervious. The saturated thickness during 2010–2014 ranged from 12 to 14 m. The groundwater flows from NNW to SSE with a natural gradient of 2.3‰ (Iribar et al., 1997). Figure 1 C shows a simplified cross-section of the site. The Llobregat River deposits are intertwined with colluvial deposits from lateral alluvial fans of local creeks, forming complex alternating layers of different compositions. The varying percentages of gravel, sand, 5 and clay of each layer causes a significant heterogeneity in the vertical dimension (Gámez et al., 2009). This layering was represented in the numerical model as multilayered local domains.

Eight piezometers were used as monitors (Fig. 1). Piezometer P1, located upstream from the infiltration basin, monitored the background regional groundwater flowing under the basin. Piezometers P8.3, P8.2, and P8.1, located in the middle of the infiltration basin at depths of 7 to 9 m, 10 to 12 m and 13 to 15 m, respectively monitored the depth-related changes of the 10 recharged water. Piezometers P2, P5, P9, and P10, located downstream along the flow path at increasing distances from the infiltration basin, monitored the recharged water at increasing travel times. Most of the piezometers were equipped with CTD-Diver (Schlumberger water services, Delft, The Netherlands) sensors for continuous recording of electrical conductivity (EC), temperature, and water level (as pressure). The CTD-Divers were installed prior to artificial recharge. An additional CTD-Diver was placed in the middle of the infiltration basin to measure EC, temperature, and level of the infiltration water. Samples from 15 monitoring points and infiltration basin were collected for analysis during several campaigns (Valhondo et al., 2014, 2015).

2.2 Tracer test

A natural flow tracer test experiment was performed between 9 July and 14 September, 2012. Amino-G acid was selected as the tracer because it can be detected at very low concentrations, is stable in the experimental pH range (Flury and Wai, 2003), and has relatively low sorption onto organic matter and clay (Trudgill, 1987; Smart and Smith, 1976). It is, however, 20 photo-degradable.

The recharge system was operating for three weeks before adding the tracer, while trying to maintain a 1 m column in the infiltration basin to sustain steady-state flow. On 9 July, referred to as "day 0", 8 kg of amino-G acid diluted in 0.9 m³ of water from the settlement basin was poured into the entrance of the infiltration basin over approximately 15 minutes (point A in Fig. 1B) in the late afternoon to minimize photo-degradation. Breakthrough curves were measured in situ with 3 portable 25 GGUN-FL fluorometers (Albillia Co, Neuchâtel, Switzerland) at six downstream monitoring points (P8.3, P2, P5, P8.1, P9, and P10) (Fig 1B). The fluorometers were calibrated with serial dilutions (1, 10, and 100 µg/L) prepared with the tracer and water from the settlement basin. Three fluorometers were initially installed at the monitoring points with the shortest expected travel times (P8.3, P2, and P5) and programmed to record a measurement every 5 minutes. Mean travel times were obtained from the EC recordings. Once most of the breakthrough curves had been recorded at these three points, the fluorometers were 30 moved to the monitoring points with longer travel times (P9, P10, and P8.1). Thus, one fluorometer was moved from P8.3 to P8.1 at midnight on 13 July, and the others were moved from P2 to P10 and P5 to P9 at 17:20 on 15 July. The recording interval was changed to 15 minutes on 25 July and maintained until the end of the test on 14 September.



2.3 Model construction

An integrated regional hydrogeologic model was built to improve the management of the water resources (Iribar et al., 1997; Abarca et al., 2006). The artificial recharge system is nested within the regional model domain. Therefore, to study the aquifer behavior in the proximity of the artificial recharge system, we created a numerical groundwater flow and transport model based on the previously cited (Abarca et al., 2006) regional one and we increased the local scale detail with information from the 5 recharge system.

2.3.1 Boundary conditions, and model parameterization

The model was bounded along the flow direction by two lines of frequently measured piezometers (G1-G5-G9 and F1-F4-F5 in Fig. 1 B), ≈ 3 km away, not affected by the artificial recharge, so that they could be used to prescribe the head at the northern and southern boundaries. The width of the model (≈ 2.5 km) was established by the fluvial deposits. Inflows from the eastern 10 and western local creeks were prescribed using updated time dependent inflows of the regional model with recent weather data. Time dependent pumping rates were prescribed on the drinking water wells with radial galleries (PE1, PE2, PE4, PE5, and PE6 in Fig. 1 A) using data from the water utility (AGBAR). Areal recharge was prescribed by updating the regional model functions for recharge in urban, rural, and irrigated areas. Infiltration from the river bed was taken from the regional model.

The multilayer nature of the system was modeled explicitly only in the area adjacent to the infiltration basin (local domain 15 $\approx 0.5 \times 1.5$ km² yellow triangle in Figure 1 A), where a high level of detail was needed. The rest (large domain) was modeled as a two-dimensional plane using linear triangular elements. The element size increased from 5 m at the infiltration basin to ≈ 185 m at the edges of the model.

We used nine layers (Ly1 to Ly9 starting from the bottom) that overlapped in the local domain and were linked by one-dimensional elements. The first seven layers were 2 m thick and emulated the vertical sequence of aquifer deposits. Layers 8 20 and 9 extended only over 50×100 m² of the infiltration basin (basin domain), were 0.3 m thick, and used to represent retention time at the reactive barrier.

Modeling comprised three steps: (1) Calibration of the large domain flow model using four years of head data, (2) calibration of the local domain flow and transport model using tracer test data, and (3) validation by comparison with 1,1,2-trichloroethane (TCA) and EC data collected under different flow conditions from those used for the calibration. The model was built using 25 the inversion capabilities of Transdens (Medina and Carrera, 1996, 2003; Hidalgo et al., 2004).

2.3.2 Large scale flow model

Hydraulic parameters (hydraulic conductivity and storativity) of the local and large domain models were calibrated against the head data at the piezometers shown in Figure 1 from January 2010 through December 2013. Several infiltration episodes took place during these four years, which were discretized into daily time steps. The initial parameters were obtained from the 30 regional flow model (Abarca et al., 2006) and completed with local tests.



2.3.3 Local scale flow and transport model

The estimated flow parameters in the large scale model were used to calibrate transport parameters and recalibrate flow parameters at the local and basin domains against the heads and concentrations recorded during the tracer test (from 9 July to 14 September 2012).

A 15 min wide tracer input was added to the inflow at the start of the test. The tracer was poured at the entrance of the infiltration basin (point A in Fig. 1 B) and afterwards clean water continued to flow to the basin. Thus, the tracer was not homogeneously diluted in the whole basin water volume, as a consequence the maximum concentration measured at P8.3 was 2.75 times higher than the concentration assuming complete dilution of the tracer in the basin volume (1.6 mg/l). This, together with the potential photodegradation of the amino-G, caused the expected distribution of the amino-G concentration to decrease from north to south in the basin. To resolve this issue, we divided the basin into nine zones (zones i through ix in Fig. 1 B) and estimated the concentration of the tracer at each zone as a multiple of the amino-G acid concentration function.

The time discretization was quite irregular, with the shortest time steps (5 minutes) between the tracer discharge and the arrival of the breakthrough curve at P8.3 and the longest steps (3 days) at the end of the test. The effect of preferential flow that was apparent from the redox sensitive species (Valhondo et al., 2014, 2015) was reproduced by distributing the time dependent measured inflow data between the surface of the infiltration basin in Ly9 and Ly7.

The standard deviation assigned to all concentration measurements at each observation point was 1% of the maximum concentration at each point, which was to ensure that a comparable weight was given to each point during calibration (maximum concentration varied by ≈ 2 orders of magnitude).

The calibration yielded three models: a homogeneous one ("Hom"), where K_x and K_z were constant throughout the model domain, and two heterogeneous ones ("Het-1" and "Het-2") with different hydraulic conductivities for each layer. These last two models represented two different convergence points of the calibration process and both were used to highlight the non-uniqueness of the solution and to assess uncertainty.

2.4 Validation

The three models were tested against TCA and EC data measured at the P piezometers. The TCA simulations covered eight months (from April 2011 to December 2011). Initial concentration and lateral inflows of TCA concentrations were fixed as the maximum concentration observed at P1 (upstream of the infiltration basin). Artificial recharge concentration was zero. The EC simulations covered four years (2010–2013). Initial and lateral inflows of EC were fixed at $1200 \mu\text{S}/\text{cm}$, the mean measured during the period. Recharge water EC was prescribed using a time function based on the measured EC. TCA and EC concentrations in the northern border of the local domain were prescribed based on the TCA and EC measured at P1 and adjusted for the travel time from the northern border to P1.



30 3 Results and discussion

3.1 Flow model

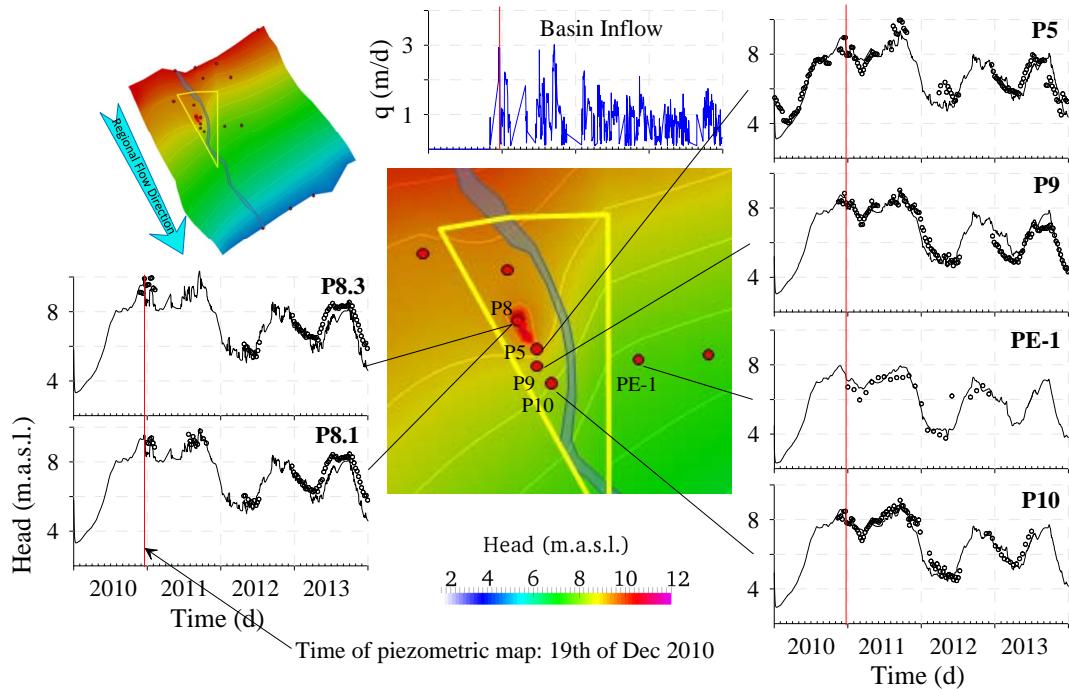


Figure 2. Calculated head level surface on 19 of December 2010, when the system was working, and measured (circles) and calculated (line) heads (m.a.s.l.) vs. time (d) at five monitoring piezometers (P8.3, P8.1, P5, P9, and P10) and one extraction well (PE-1).

Artificial recharge creates a smooth dome below the infiltration basin and modifies the piezometric surface (Fig. 2). Figure 2 displays head fits at five monitoring piezometers located around local domain. The fit was good, which suggests that the size of the multilayer local domain was sufficient to reproduce head variations at the monitoring piezometers close to the basin where the gradient is mainly vertical ($\approx 10\%$). The model also reproduces measurements at extraction wells outside of the multilayer local domain (e.g. PE-1 in Fig. 2), suggesting that the two-dimensional model was sufficient to create the appropriate head frame for simulating transport.

3.2 Tracer test model

Conservative transport parameters were estimated for the nine layers of the local and basin domains using the tracer test data. Flow parameters of the nine layers and the one-dimensional elements linking them were also re-calibrated because the concentrations were more sensitive to vertical layering than the heads, which were only mildly sensitive to the degree of hydraulic connection (Fogg, 1986). Three sets of parameters (Het-1, Het-2, and Hom) were obtained. Estimated parameters fit,



measured by the root mean square-weighted error (RMSWE), and amino-G input mass for these three outcomes are shown in Table 1.

Table 1. Parameters (Hydraulic conductivity, K (m/d), and porosity, ϕ), RMSWE, and input mass in outcomes Het-1, Het-2, and Hom, after calibration.

		K (m/d)			ϕ		
		Het-1	Het-2	Hom	Het-1	Het-2	Hom
Basin Domain (50 x 100 m)	Ly 9 (K_h)	1	1	1.4	0.5	0.5	0.5
	Ly 9 (K_z)	5.0	5.0	3.0	0.0001	0.002	0.0001
	Ly 8 (K_h)	1	1	1.4	0.5	0.5	0.5
Local Domain (1000 x 500 m)	Ly 8 (K_z)	5.0	5.0	3.0	0.0001	0.002	0.0001
	Ly 7 (K_h)	0.2	0.2	1.4	0.17	0.13	0.2
	Ly 7 (K_z)	2.4	3.4	3.0	0.0001	0.01	0.0001
	Ly 6 (K_h)	139.1	158.2	289.9	0.17	0.17	0.2
	Ly 6 (K_z)	2.4	3.4	3.0	0.0001	0.01	0.0001
	Ly 5 (K_h)	1042.1	1187.3	289.9	0.18	0.17	0.2
	Ly 5 (K_z)	93.5	190.5	3.0	0.0001	0.01	0.0001
	Ly 4 (K_h)	203.4	253.9	289.9	0.16	0.20	0.2
	Ly 4 (K_z)	42	210.7	3.0	0.0001	0.01	0.0001
	Ly 3 (K_h)	330.9	479.0	289.9	0.27	0.33	0.2
	Ly 3 (K_z)	0.2	0.2	3.0	0.0001	0.01	0.0001
	Ly 2 (K_h)	131.1	75.5	289.9	0.25	0.22	0.2
	Ly 2 (K_z)	2.1	29.1	3.0	0.0001	0.01	0.0001
	Ly 1 (K_h)	162.3	478.2	289.9	0.10	0.20	0.2
Model Fit (RMSWE)		1979	2000	9358			

5 Measured and calculated breakthrough curves at monitoring piezometers are displayed in Figure 3, which provides room for some insights. Monitoring piezometers P8.3, P2, and P5 had faster responses with maximum concentrations higher than those of piezometers P8.1, P9, and P10, in which dispersion and mixing generated longer tails. P8.3, located below the basin, was the first monitoring point reached by the tracer, and was where the observed maximum concentration was the highest, more than an order of magnitude higher than the next monitoring point P2. We assume that the early arrival at P8.3 occurred through
 10 preferential flow paths. The breakthrough curve at this point was very narrow, as the follow-up water without tracer reached this point equally fast. First arrival at P2 and P5 (1 day) was much faster than at P8.1 (3 days), only 6 m below the phreatic surface, despite the fact that vertical gradient ($\approx 10\%$) was much larger than the horizontal gradient (less than 4%). This observation implies that recharged water spreads laterally much faster than vertically and confirms the importance of layering.

Breakthrough curves at P10, P8.1, and P9 exhibited longer tails than those at P8.3, P2, and P5. The short tails were consistent
 15 with the fast arrival. The long tails might suggest the impact of heterogeneity (dispersion) and mixing away from the entrance. The fact that the homogeneous model reproduced tailing quite well (at least for P8.1 and P9), however, implies that broad RTDs are caused not only by heterogeneity but also by the mean flow structure. The "shower" effect of recharge ensures that water flowing initially upstream or falling on the dome top will eventually mix with recently recharged water further downstream. This effect is illustrated by the spatial distribution of the concentration shown in Figure 4.

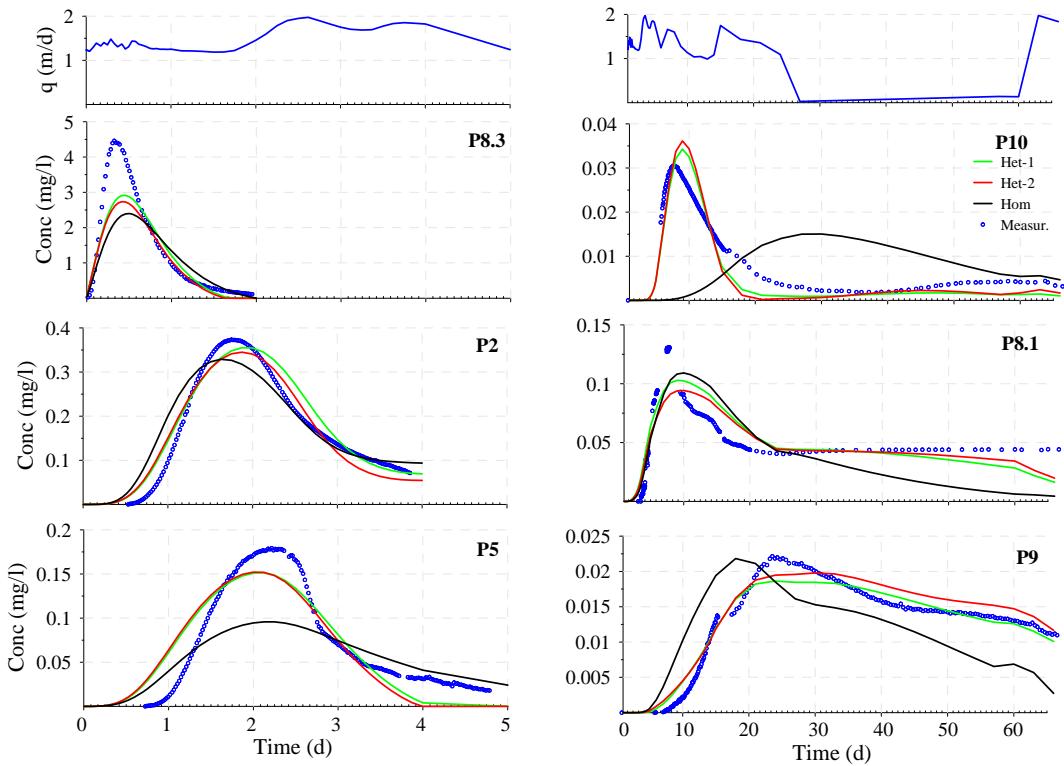


Figure 3. Inflow rate to the infiltration basin and measured (blue circles) and calculated breakthrough curves from the piezometers monitoring the amino-G acid concentration (mg/l) vs. time (d) using the estimated set of parameters of Het-1 (green line), Het-2 (red line), and Hom (black line).

Several features are apparent from the spatial distribution of the concentrations. First, the distribution was balloon-like. 5 The tracer was distributed along an outer crust that grew by filling with the tracer-less water that kept entering through the basin. Second, the portion of tracer that initially flowed upstream was transported laterally and downstream. This promoted shear and lateral mixing. Third, the Het-2 model provided another shear mechanism (Fig. 4 B, D and F) by the fluctuations in horizontal hydraulic conductivity (K_x) among the different layers. Note that the plume in layer 5 traveled much faster than in the remaining layers, to the point that it had virtually disappeared from the local domain after 60 days. The plume also almost 10 disappeared in Layers 6 and 7, but in this case the disappearance reflected vertical, rather than horizontal, displacement.

We contend that these shear mechanisms promoted mixing and that they were more marked in the heterogeneous models than in the homogeneous model. In fact, this observation was confirmed by the plumes shown in Figure 4. Those of the heterogeneous models were much more diluted than those of the homogeneous model.

This kind of shear and mixing promoted broad RTDs and caused recently recharged water (possibly aerobic and loaded with dissolved organic carbon) to mix with more than 60 days old water (possibly anaerobic and depleted of dissolved organic carbon) at monitoring points P8.1, P9, and P10. Such mixing contributes to favoring the presence of a primary substrate to be

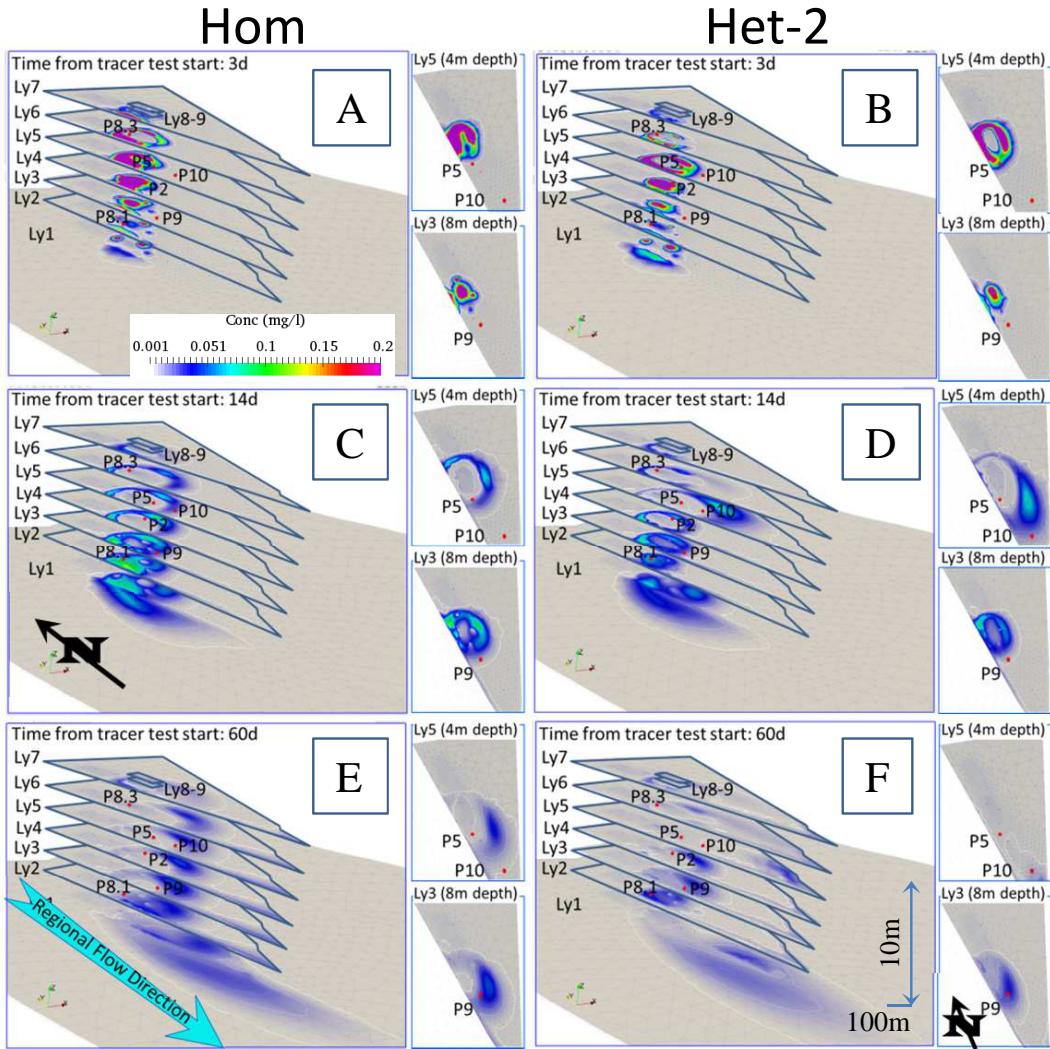


Figure 4. Distribution of amino-G acid concentration at three time points (3 d, 14 d, and 60 d) since the test started, calculated with the Hom (A, C, and E) and Het-2 (B, D, and F) models. The concentrations in layers 3 and 5 are shown from below on the side of each frame. Note that the vertical scale is 100 times the horizontal scale.

metabolized by microorganisms, which increases the biotransformation of emerging contaminants, by co-metabolism. It may 5 also explain, at least in part, the excellent performance of the system in eliminating a broad range of emerging contaminants (Valhondo et al., 2014, 2015).

Mixing at the edges of the local domain is unrealistic. As shown in Figure 1, only the local domain was treated as a multilayer. The rest was treated as two-dimensional. This implies that, as the plume left this domain, the outflow of all the layers was mixed. This causes the plume that departs from the western edge of the local domain (plotted as Ly1 in Figure 4).



Whereas this mixing was an artifact of the model structure, it does not affect the computed breakthrough curves because all
5 observation points belong to the local domain.

A final remark on the validity of the models can be drawn from the fact that monitoring point P10 was further away from the basin than monitoring point P9. Nevertheless, P10 was reached by the tracer faster than P9. Breakthrough curves of P10 and P5 were poorly reproduced under the homogeneous medium hypothesis. Both heterogeneous models, Het-1 and Het-2, reproduced the measured concentrations with better accuracy than Hom. The RMSWE values were 1979 and 2000 for Het-1
10 and Het-2, respectively, and 9358 for Hom. The main difference between Het-1 and Het-2 was the distribution of conductivity and porosity, because the aquifer transmissivity in the local domain was ultimately the same. Model Het-2 was more consistent with the field observations regarding the materials distribution (Fig. 1 C) than model Het-1.

These observations suggest that the Het models were better than the Hom model. But they may be overparameterized (Poeter and Hill, 1997; Carrera et al., 2005)). It is clear that the heterogeneity assumption was required to reproduce the geologic
15 observations, which was a valuable finding in itself (D'Agnese et al., 1999), and to model mixing (Le Borgne et al., 2010). It is also clear, however, that parameterizing heterogeneity causes non-uniqueness. In fact, the fast arrival at P10, which we reproduced by the high hydraulic conductivity in layer 5, might reflect other causes (e.g., a high-K paleochannel within layer 5). We examine the model validity below.

3.2.1 TCA and EC validation models

20 Figure 5 displays the changes in the measured and calculated concentrations of TCA ($\mu\text{g/L}$) at the monitoring piezometers. Measured TCA concentrations approached the background concentration of the aquifer (some hundreds of $\mu\text{g/L}$ but varying) when the artificial recharge system was not operating. Concentrations of TCA decreased and fell below the detection limits at most monitoring points when the recharge with TCA-free water was activated. These trends were generally reproduced by the three models and confirmed that the observation points sampled recharge water. The models were far slower in reacting
25 to changes in recharge rate, however, than the actual observations. In particular, they were too slow to reproduce the TCA concentration rebound after the recharge stopped in Nov-Dec.

Figure 6 displays the changes in measured and calculated EC ($\mu\text{S/cm}$) during two years. The three models reproduced the observations quite accurately, except during the low recharge period at the end of 2012 and the beginning of 2013. Measured EC at P2, P5, and P10 during this period fell below both the recharge water EC (similar to that of P8.3) and aquifer water EC
30 (similar to P1). Therefore, the error must be attributed to some unaccounted inflow of low EC water rather than to poor model structure.

In summary, the three models reproduced quite well the change in TCA, which was present in the aquifer but not in the recharge water, and EC, which fluctuates in both. On the one hand, this implies that the velocity field, imposed by recharge and natural aquifer flow, was not overly sensitive to local hydraulic conductivities. On the other hand, it implies that it would be difficult to accurately estimate the layering structure solely based on the concentration breakthrough curves. In fact, the RMSWE for EC with the Hom model (4629) was slightly smaller than that for the Het-1 and Het-2 models (4682 and 4689, respectively), which suggests that the Hom model, having less uncertain parameters, was more robust than the Het models.

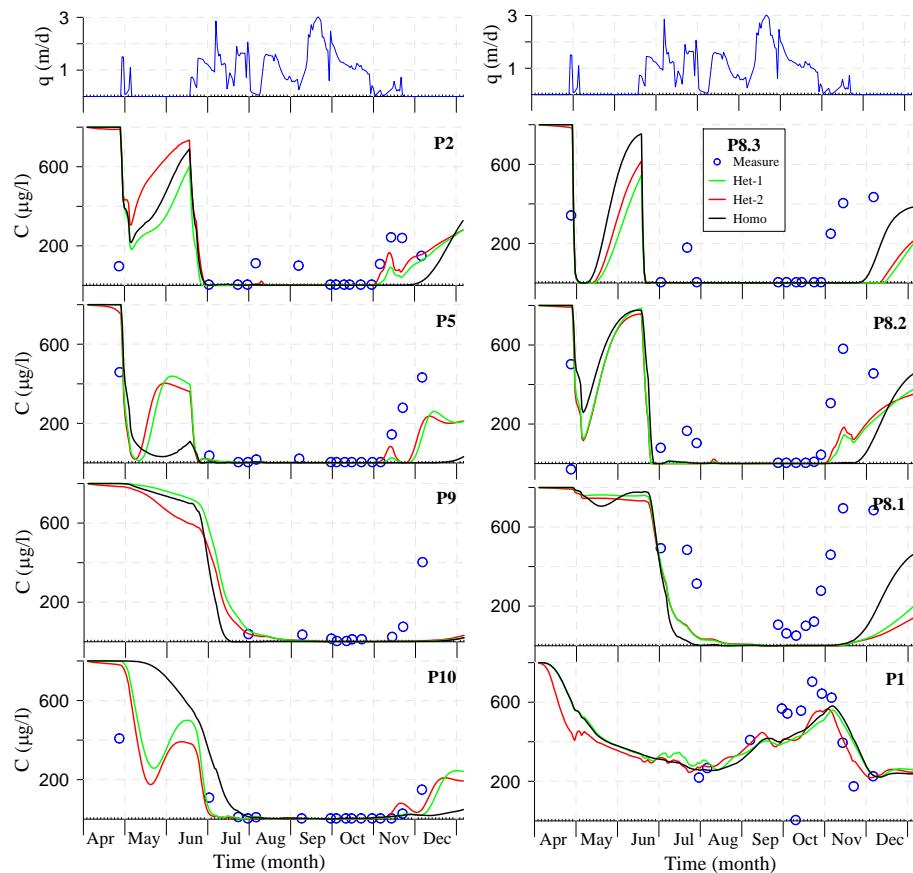


Figure 5. Measured (dark blue circles) and calculated TCA concentration ($\mu\text{g/l}$) changes over time at monitoring piezometers for the Het-1 (green line), Het-2 (red line) and Hom (black line) models. The infiltration rate is also shown (top).

5 One might be tempted to use multicontinua models (Haggerty and Gorelick, 1995), which reproduce the effect of heterogeneity (Silva et al., 2009; Dentz et al., 2011). In fact, such models would probably capture the fast rebound of the TCA concentration when the recharge stopped, as observed in Figure 5. In view of the calibration non-uniqueness of these models, however, would probably worsen identifiability.

4 Conclusions

10 This work provides useful insight on both tracer testing for characterization of artificial recharge and on transport modeling. The tracer test was successful in identifying RTDs at a number of piezometers. These distributions were quite narrow at points immediately adjacent to the basin (P8.3, P2, P5) but were very broad (more than 60 days) at points slightly further away (P8.1, P9, P10). Broad RTDs imply significant mixing of recently recharged water with water recharged some time before.

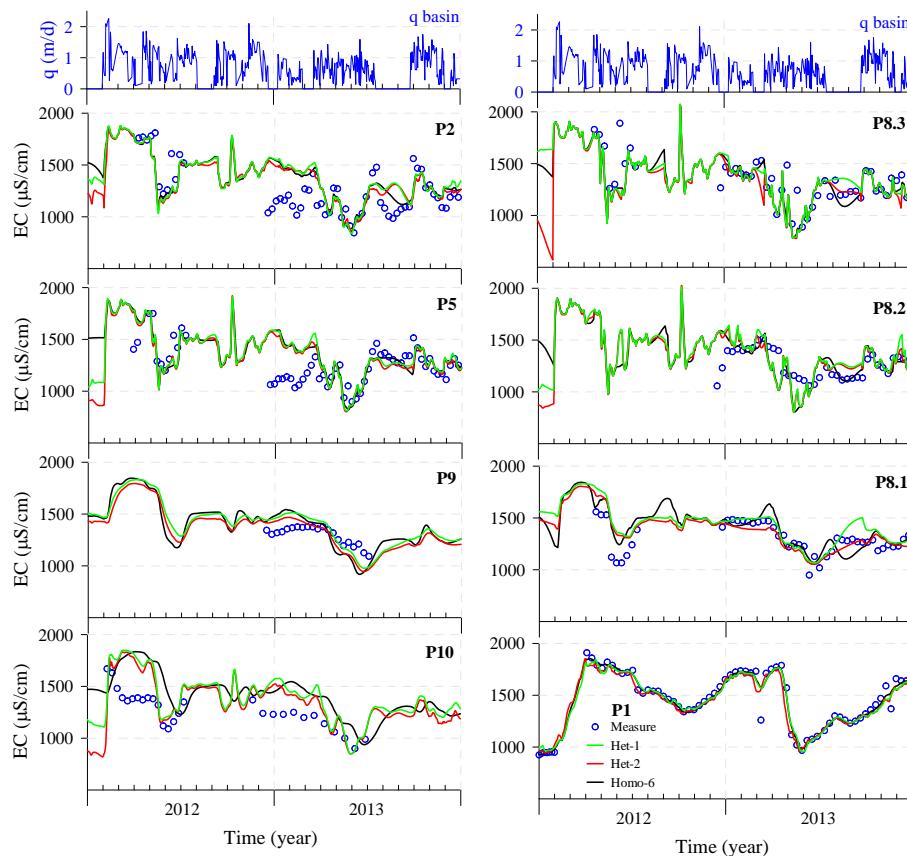


Figure 6. Changes in measured (dark blue circles) and calculated EC ($\mu\text{S/cm}$) at monitoring piezometers for the Het-1 (green line), Het-2 (red line) and Hom (black line) models. The infiltration rate is also shown (top).

Such mixing, together with the conditions imposed by the reactive layer, promote diverse metabolic paths and helps to explain
 5 the effective removal of a wide range of emerging contaminants at this site (Valhondo et al., 2014, 2015).

The model suggests that the broad RTDs are the result of both the flow structure, which is complex, and heterogeneity. Recharged water flows initially upstream and then laterally around and below recently recharged water. This complexity stretches flow tubes and favors mixing (Dentz et al., 2011). This effect was observed in both the homogeneous and heterogeneous models. Further shear, stretching, and mixing was caused by the variability in the hydraulic conductivity among
 10 layers.

Regarding transport modeling, it is clear that the collected breakthrough curves were not sufficient to identify the hydraulic conductivities of the modeled layers or, even less, more complex heterogeneous structures. In fact the simple homogeneous model, which did not perform as well as the two heterogeneous models during the tracer test calibration, yielded similar (if not better) blind predictions of TCA and EC under varying flow conditions.



The introduction of heterogeneity is justified not by the quantitative head or concentration data, but by geologic understanding. Ultimately, the actual RTDs that are required for proper interpretation of pollutant removal, were well reproduced by the
5 heterogeneous models. Therefore, these models should be used for interpreting and predicting the fate of recharge water. Yet, given the importance of artificial recharge and that its clean-up potential can be enhanced by time fluctuations of recharge rate (de Dreuzy et al., 2012), much can be gained by the detailed characterization of recharge sites. To this end, tracer tests are useful, but insufficient. They must be complemented with cross-hole inter-layer testing.

Author contributions. All authors have contributed to the work. Jesús Carrera, Carlos Ayora, and Cristina Valhondo designed the tracer test
10 experiment and the three of them together with Katrien De Pourcq and Alba Grau-Martínez carried it out. Jesús Carrera and Juan Hidalgo developed the model code, and advised Cristina Valhondo, Lurdes Martínez-Landa, and Isabel Tubau performing simulations and calibrating flow and transport parameters. Cristina Valhondo and Lurdes Matínez-Landa prepared the manuscript counseled by Jesús Carrera.

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