



Evaluation of snow data assimilation using the Ensemble Kalman Filter for seasonal streamflow prediction in the Western United States

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- 10 Abstract. In this study we examine the potential of snow water equivalent data assimilation (DA) using the ensemble Kalman Filter (EnKF) to improve seasonal streamflow predictions. There are several goals of this study. First, we aim to examine some empirical aspects of the EnKF, namely the observational uncertainty estimates and the observation transformation operator. Second, we use a newly created ensemble forcing dataset to develop our ensemble model states (e.g. model state uncertainty). Finally, we also examine the impact of varying the observation and model state uncertainty on forecast skill. We use basins
- 15 from the Pacific Northwest, Rocky Mountains, and California in the western United States with the coupled Snow17 and Sacramento Soil Moisture Accounting (SAC-SMA) models. Results show that most EnKF implementation variations result in improved streamflow prediction, but the methodological choices in the examined components impact predictive performance in a non-uniform way across the basins. Finally, basins with relatively higher calibrated model performance (> 0.80 NSE) without DA generally have lesser improvement with DA, while basins with poorer historical model performance show greater
- 20 improvements.

Keywords:

Hydrological data assimilation; SWE; EnKF; Snow-17; SAC





1 Introduction

In the snow-dominated watersheds of the Western US, spring snowmelt is a major source of runoff (Barnett et al., 2005; Clark and Hay, 2004; Singh and Kumar, 1997; Slater and Clark, 2006). In such basins, the initial conditions of the basin, primarily in the form of snow water equivalent (SWE), drive predictability out to seasonal time scales (Wood et al., 2005; Wood and Lettenmaier, 2008; Harrison and Bales, 2015; Wood et al. 2015). Thus better estimates of basin mean initial SWE should lead to better seasonal streamflow predictions (Clark and Hay, 2004; Slater and Clark, 2006; Wood et al. 2015). For various reasons (e.g., the uncertainty in model parameters, forcing data, model structures), simulated SWE in hydrological models can be very

30 different from reality (Pan et al., 2003). Fortunately, a variety of snow observations (including point gauge and spatial satellite data) contain valuable information (Andreadis and Lettenmaier, 2006; Barrett, 2003; Mitchell et al., 2004; Su et al., 2010; Sun et al., 2004). Many studies have explored the role of snow data assimilation in different modeling frameworks (Kerr et al., 2001; Moradkhani, 2008; Takala et al., 2011; McGuire et al, 2006; Wood and Lettenmaier, 2006).

Of particular focus here are papers that have examined the impact of SWE data assimilation (DA) on runoff modelling and prediction (e.g. Wood and Lettenmaier, 2006; Franz et al., 2014; Jörg-Hess et al., 2015; Moradkhani, 2008; Slater and Clark, 2006). Among the major challenges facing SWE-based DA are that the time-space resolution of remote sensing SWE data are too coarse or period-limited for many watershed-scale hydrological applications in mountainous regions (Dietz et al., 2012; Jörg-Hess et al., 2015), and point gauge snow data have sparse and uneven spatial coverage. For point measurements, spatial interpolation based on distance are typically used to estimate observed SWE state in a watershed of interest (e.g.,Franz et al.,

- 2014; Jörg-Hess et al., 2015; Slater and Clark, 2006; Wood and Lettenmaier, 2006).
 Here we use the Ensemble Kalman Filter (EnKF) method for DA using an implementation that allowing for seasonally varying estimates of observation and model error variances (Evensen, 1994, 2003; Evensen et al., 2007). The EnKF framework has been successfully implemented in research basins in several previous studies (Clark et al., 2008; Franz et al., 2014; Moradkhani et al., 2005; Slater and Clark, 2006; Vrugt et al., 2006). The EnKF provides an objective analytical framework to optimize the
- 45 update of model states based on observed values and their corresponding uncertainties. While the EnKF approach has a formal theory, its overall objectivity in an application (contrasting with an arbitrary DA approach such as direct insertion) nonetheless depends on several methodological choices that are often empirical when applied to SWE DA. Here we examine DA performance sensitivities related to three elements: 1) the estimation of watershed mean SWE from surrounding point measurements; 2) the transformation operator that relates watershed mean SWE to model mean SWE; and 3) sensitive analyses
- 50 of the relative size of observed and model error variance. Following Slater and Clark (2006), this study uses two slightly different approaches to estimate ensemble SWE observations with point gauge SWE data from surrounding gauge sites for study basins. When using calibrated hydrologic modeling systems, model SWE states may exhibit systematic biases from observed SWE estimates for a number of reasons – e.g., all





hydrologic models must simplify real watershed physics and structure, and model parameter estimation (calibration) may result

- 55 in SWE behavior that in part compensates for forcing or model errors (e.g. Slater and Clark, 2006). Therefore, transformation of snow observations to model space is needed before they are used to update the model states to ensure that the model ingests SWE estimates that are as close to unbiased relative to the model climatology as possible. We explore two variations on an approach using cumulative density function (CDF) transformations of observations to model space (following Wood and Lettenmaier, 2006, among others). Additionally, we undertake a sensitivity analysis to highlight the importance of robust
- 60 observations and model uncertainty estimates. We focus on the impacts of updates made just once per snow accumulation season, noting that an important choice that is not examined as a result is the selection of DA dates and frequency. For a given generally optimal selection of the EnKF system, the Ensemble Streamflow Forecast (ESP) approach is used to test the impact of SWE DA on subsequent streamflow forecasts.

We apply the EnKF system to nine river basins in the Western US that have a range of basin features and environmental

65 conditions. This is distinct from many previous studies that focus on one or two basins in more detail (e.g., Clark et al., 2008; Franz et al., 2014; He et al., 2011; Moradkhani et al., 2005). We also use ensemble simulations driven by a new probabilistic forcing dataset (Newman et al, 2015) as a basis for estimating model SWE uncertainty, in contrast to prior studies which used more arbitrary distributional assumptions. The following sections discuss the study basins and data sets, and the model and EnKF DA approach, before the presenting study results, and discussion and conclusions.

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2 Study basins and data

In this study, nine basins across the Western US are selected for SWE DA evaluation. They are in the Pacific Northwest, California (Sierra Nevada Mountains), and central Rocky Mountains. We focus on these three areas as they span a range of snow accumulation and melt conditions of the Western US and are in areas with active seasonal streamflow prediction and

- 75 water resource management. Note we do not examine rain driven low-lying basins as they do not have significant SWE contributions to runoff. The locations of the basins and nearby SWE gauge sites are shown in Figure 1, illustrating that all of the study watersheds have SWE measurements distributed in and/or around the basins. The main features of these basins are shown in Table 1. The basin areas range from 16 to 1163 km² and the mean elevations of the basins range from 998 to 3459 m with a large spread in basin mean slopes (as estimated from a fine-resolution digital elevation model) and forest percentage.
- 80 Two sources of SWE observations are used in this study: (1) the widely used Snow Telemetry (Snotel) network for Natural Resources Conservation Service (NRCS) (www.wcc.nrcs.usda.gov/snow/), which covers most of the western US; and (2) the California Department of Water resources (DWR) (cdec.water.ca.gov/snow) (denoted as CADWR sites hereafter), which maintains a snow pillow network for California. The SWE data from CADWR sites have frequent missing data and some unrealistic extreme values, thus extensive manual quality control was required before using the CADWR data in the study.





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3 Methodology

3.1 Models and calibration

The Snow-17 temperature index snow model is coupled to the Sacramento Soil Moisture Accounting (SAC-SMA) conceptual hydrologic model (Anderson, 2002; Anderson, 1973; Burnash and Singh, 1995; Burnash et al., 1973; Franz et al., 2014;

- 90 Newman et al., 2015a) to simulate streamflow in this study. This model combination has been in operational use by US National Weather Service (NWS) River Forecast Centers (RFCs) since the 1970s (Anderson, 1972; 1973). The Snow-17 model is a conceptual snow pack model that employs an air temperature index to partition precipitation into rain and snow and parameterize energy exchange and snowpack evolution processes. The only required forcing inputs are near-surface air temperature and precipitation. The output rain-plus-snowmelt (RAIM) time series from Snow-17 is part of the forcing input
- 95 of the SAC-SMA model. SAC-SMA is a conceptual hydrologic model that uses five moisture zones to describe the movement of water through watersheds. The required forcing input is the potential evaporation and the surface water input from Snow-17.

Daily streamflow data from United States Geological Survey (USGS) National Water Information System server (http://waterdata.usgs.gov/usa/nwis/sw) are used to calibrate 20 parameters of Snow-17 and SAC-SMA model. The calibration

- is obtained using the shuffled complex evolution global search algorithm (SCE; Duan et al, 1992) via minimizing daily simulation Root Mean Square Error (RMSE). UGSS streamflow data are also used to verify the model predictions.
 Model uncertainty arises from model parameter and structural uncertainty (e.g. Clark et al., 2008) and forcing input uncertainty (e.g., Carpenter and Georgakakos, 2004). Focusing on the latter, we drive the hydrology models with 100 equally likely members of meteorological data ensemble generated as described in Newman et al. (2015b), producing an 100 member
- 105 ensemble of model moisture states, including SWE, and streamflow. The daily-varying spread of the ensemble model states serve as the estimate of model uncertainty. Because this method estimates SWE uncertainty without also considering sources other than forcing input uncertainty, and therefore may underestimate model uncertainty in initial SWE (e.g. Franz et al. 2015), we also include a sensitivity analysis to explore the sensitivity of DA results to variations in the estimated observation and model uncertainty magnitudes.

110 3.2 Generating ensembles of estimated observed watershed SWE

Since the SWE observations are point measurements that do not represent the watershed mean conditions and have observation error, observation uncertainty needs to be robustly estimated to ensure reasonable DA performance. In this study, we follow Slater and Clark (2006) to generate ensemble SWE observations using a multiple linear regression in which the predictors are the attributes of SWE gauge sites (longitude, latitude and elevation). The observation uncertainty is estimated by leave-one-

115 out (LOO) cross validation: i.e., each station is left out of the regression training and then its SWE is predicted and verified





against its actual measurement. The SWE values are transformed into percentiles or Z-score before the regression is performed, and the corresponding inverse transformations are used to convert them back to SWE values. These two approaches are denoted as percentile and Z-score interpolation respectively and detailed descriptions for them are as follows.

3.2.1 Percentile interpolation

120 First, the non-exceedance percentile $p_y^o(k)$ of each SWE observation (observation based values noted with superscript o) at gauge site k on DA date in year y is calculated based on its rank, or percentile, within a sample of all SWE observations at the same site within a time-window of +/- n days centered on the date of the observation.

Then we use the percentiles to do linear regression on geographic features latitude, longitude and elevation to estimate the SWE percentile for the target basin: \hat{p}_y^o , where the hat indicates the basin mean estimate. By LOO cross validation, the

125 interpolation error of the linear regression is estimated as \hat{e}_y^o . We sample from normal distribution $N(\hat{p}_y^o, \hat{e}_y^o)$ to get the ensemble percentiles $\{\hat{p}_y^o(j)\}$, where j = 1, ..., 100 represents ensemble member.

Finally, we take the corresponding $\hat{p}_{y}^{o}(j)$ percentile from the full ensemble model SWE within the time-window of +/- n days centered on the DA date in year y, denoted as $\hat{S}_{y}^{f}(j)$. The final ensemble SWE observations on DA date at year y for the target basin are $\{\hat{S}_{y}^{f}(J)\}$, where J = 1,..., 100.

130 **3.2.2** *Z*-score interpolation

First, we use the observed SWE at gauge site k on DA date in year y to calculate the Z-score:

$$Zscore_{y}(k) = \frac{S_{y}^{o}(k) - \overline{S^{o}(k)}}{\sigma(S^{o}(k))},$$
(1)

where $\overline{S^o(k)}$ and $\sigma(S^o(k))$ are the are the long-term mean and standard deviation of a sample of all non-zero SWE observations at the same site within a time-window of +/- n days centered on the date of the observation respectively. Here

135 we use the *Z*-score in the linear regression and again use LOO cross validation to estimate the mean and interpolation error of the *Z*-score for a target basin. Then we sample from normal distribution to get ensemble *Z*-scores for target basin, denoted as $\{\hat{Z} - \text{score}_{y}^{o}(j)\}$, where j = 1,..., 100 represents ensemble member. Finally we use the following equation to transform *Z*score to back to SWE values:

$$\hat{S}_{y}^{o}(j) = \hat{Z}score_{y}^{o} \times \sigma\left(S^{f}(k)\right) + \overline{S^{f}(k)},\tag{2}$$

140 where $\overline{S^f(k)}$ and $\sigma(S^f(k))$ are the long-term non-zero mean and standard deviation of the full ensemble model SWE within the time-window of +/- n days centered on the DA date in year y respectively. The final ensemble SWE observations on DA date at year y for the target basin are $\{\hat{S}_x^o(j)\}$, where j = 1,..., 100.

3.3 EnKF approach and experimental design





For evaluating the relative performance of DA and for re-initializing the soil moisture of DA runs at the beginning of each water year (WY), an open loop or 'control' retrospective simulation (denoted No DA) is performed using the calibrated model parameters with ensemble forcing data. This control run is one continuous simulation per ensemble member for the entire hindcasting and evaluation period (1981-201X) for each basin.

In operational practice, manual adjustments to model SWE are applied repeatedly throughout the water year, and particularly before initializing seasonal forecasts. Because this study is focusing on assessing variations in methodological aspects of the

150 DA approach, we simplify this configuration by applying DA updates only once per year, using the date on which the SWE correlation with future runoff is highest for the study basin, but no later than 1 April, a common date for initiation of spring seasonal runoff forecasts.

The EnKF method used in this study is a time-discrete forecast and linear observation system described by two relationships (generally following the notation of Ide et al. (1997) and Wu et al. (2012) :

155	$\boldsymbol{x}_{i+1}^t = \boldsymbol{M}(\boldsymbol{x}_i^t) + \boldsymbol{\eta}_i,$	(3)
	$\boldsymbol{y}_i^o = \boldsymbol{h}(\boldsymbol{x}_i^t) + \boldsymbol{\varepsilon}_i,$	(4)

where *i* is the time step, M is the coupled Snow17 and SAC-SMA model system, *x* is the state variable and *y* is the observation variable (in this study both *x* and *y* are the one-dimensional vector containing basin mean SWE for the target watershed across all ensemble members), the superscripts *t* and *o* stand for truth and observed respectively, η and ε are the model and observation

160 errors respectively, and **h** is the observation operator that maps the model states to the observation variable. In this study, **h** is simply the identity vector as we regard the SWE estimates that have been transformed to model space as observation *y*, as a pre-processing step.

The SWE DA approach is implemented via the following procedure:

1) Run the watershed model once for each ensemble forcing member from the beginning of a WY until the DA date with initial states x_0 taken from the retrospective control runs, producing the ensemble forecast states x_i^f . The superscript f

denotes forecast.

2) Calculate the ensemble analysis states:

$$\boldsymbol{x}_{i}^{a} = \boldsymbol{x}_{i}^{f} + \boldsymbol{s}_{i}\boldsymbol{h}_{i}^{T}(\boldsymbol{h}_{i}\boldsymbol{s}_{i}\boldsymbol{h}_{i}^{T} + \boldsymbol{o}_{i})^{-1}\boldsymbol{d}_{i},$$

$$(5)$$

where superscript a means analysis, $\mathbf{0}$ and \mathbf{s} are the observed and model simulation error variances (estimated by the variance

of ensemble observations and model states respectively) respectively, and the innovation vector (residual) is calculated as:

$$\boldsymbol{d}_i = \boldsymbol{y}_i^o - \boldsymbol{h}_i(\boldsymbol{x}_i^r), \tag{6}$$

3) Update the Snow-17 SWE states with the analysis states to use for initialization of forecasts through the end of the WY.

Steps 1-3 are repeated for all WY available in the hindcast period (1981-201X). Soil states are re-initialized using the states





175 from the retrospective (No DA) run at the start of every WY (October 1), when there is no SWE.

3.4 Model and observation error variance

In this study, only the uncertainty of the forcing data is taken into account in our model uncertainty, and uncertainty that arises from model structural and parameter errors could cause the true model error to be larger. Thus we assess the impacts of inflating model error variance to evaluate the relative size of observed and forecast error variance. We simply set the model SWE error

180 variance to 1/2 and 2 times of the original size to see how the DA performances change. If increasing the model error variance results in DA performance improvements, it would indicate that the model error variance is underestimated, and vice versa. This sensitivity analysis underscores the importance of a careful effort to properly estimate both model and observational uncertainty when using the EnKF – a challenge that is well known in the DA community.

3.5 Seasonal Ensemble Streamflow Prediction

- Although the impacts of the SWE DA on forecast accuracy can be assessed through verification of post-adjustment simulations using 'perfect' future forcing, we demonstrate the performance of SWE DA by initializing seasonal ESP forecasts for a streamflow forecast product that is widely used in water management, the snowmelt-period runoff volume from April through July. ESP uses historical climate data to represent the future climate conditions each year from the start point of forecast period to predict streamflow. Two typical ESP applications are tested in this study. Because we have an ensemble of historical forcing
- 190 instead of the traditional application in which only a single historical forcing time series is available, there are different ways to construct an ESP. We adopt two: (1) We construct the ESP forcing ensemble by randomly selecting one year of the historical ensemble forcing data for each historical member of the ESP; and (2) We use all historical years of ensemble mean forcing data for each ESP historical year member, yielding a 30*100 member ensemble for an ESP based on meteorology from 1981-2010 (variations are noted ens forcing and ens mean forcing respectively in subsequent figures discussing ESP results).

195 **3.6 Verification metrics**

In this study, five frequently used statistics are calculated for evaluating the two DA approaches. The Bias, Correlation coefficient (R), Relative root mean squared error (R-RMSE), Nash-Sutcliffe efficiency (NSE) are based on the ensemble averages. And Continuous Ranked Probability Score (CRPS) is a measurement of error for probabilistic prediction (Murphy and Winkler, 1987). It is defined as the integrated squared difference between cumulative distribution function (CDF) of forecasts and observations:

$$\operatorname{CRPS} = \int_{-\infty}^{+\infty} \left[F^{\mathrm{f}}(x) - F^{\mathrm{o}}(x) \right]^2 dx, \tag{7}$$

where F^{f} and F^{o} are CDF for forecasts and observations respectively. Smaller CRPS means more accurate forecasts.

4 Results

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205 4.1 Overall performance in the case basins





Using the two approaches described in Section 3.2 with three different window lengths (7 days, 3 months, 1 year), a sample comparison from one year (2004) of the results for estimated watershed SWE from the two methods versus the model SWE ensemble on DA date (DA dates for the case basins are listed in Table 1) for the case basins are shown in Figure 2. The distributions of SWE from the model ensemble and from the percentile and *Z*-score interpolation methods differ in ways that

- 210 are not consistent across all watersheds. The variance of the estimated observed SWE for both methods is generally largest for the 1-year, an effect that is more pronounced for the *Z*-score interpolation. However, we also note that the ensemble observations of 7-day window can have a large variance, likely due to the more limited sample size for the regression, which can negatively impact DA performance (see Supplement Tables S1.1 and S1.2). Therefore, a 3-month window is recommended for both approaches..
- The evaluation statistics for ensemble SWE observations estimated by percentile and Z-score interpolation approach are shown in Table 2 and Table 3 respectively. Only results for the 3-month window are shown in these two tables, the tables for 7-day and 1-year windows are in supplement S1. In Tables 2 and 3, the 2nd column shows the forecast error variance used to calculate analysis states, where "No DA" means the open loop control run (see Section 3.3), and the P, 1/2·P and 2·P refer to the DA runs with the model error variance estimated by 1, 1/2 and 2 times the original size of the ensemble model variance. Both
- 220 percentile and Z-score interpolation approach exhibit enhanced DA performances among the case basins, indicating that both these approaches are effective in adding observation based information to the model simulations. The percentile interpolation and Z-score interpolation methods vary in performance across the basins with both performing better in some basins and not others. The results of forecast error variance inflation shows that for both percentile and Z-score interpolation, "2·P" has better performance than "P" in most of the case basins – i.e., increasing the model error variance leads the assimilation to trust
- 225 observations more and improves the DA performance. This indicates that the model error variance tends to be underestimated or our observation uncertainties tend to be overestimated.

The evaluation statistics of Table 2 and 3 are also presented as scatter plots in Figures 3 and 4 respectively. The metrics R-RMSE, R and NSE indicate that both approaches bring enhancements to the ensemble mean streamflow in most basins, although the DA does not help correct forecast biases. Post-processing procedures (e.g. bias correction) could be used to further

- enhance the system performance, but is not a focus of this study. These figures also show that No DA forecasts with relatively better performance, mostly due to better simulations of forecast initial conditions, benefit less from DA. Figure 5 summarizes the ESP evaluation statistics. For simplicity, only the percentile interpolation approach with a 3-month window is shown without forecast error inflation. It shows that for both ESP forcing methodologies used (Section 3.5) in all the case study watersheds, SWE DA enhances seasonal runoff prediction skill, including the probabilistic prediction metric
- 235 CRPS. Again, higher skill No DA watersheds saw smaller DA improvements. The DA evaluation metric improvement increment versus the corresponding No DA evaluation metric score for the case basins are shown in Figure 6. It can be seen





that the DA improvements in all evaluation metrics have a general weak negative correlation with No DA performance, which again highlights that better simulated basins benefit less from SWE DA.

4.2 Case study analyses

- 240 To provide a more in-depth examination of the SWE DA impacts to the watershed model states and fluxes, time series of runoff and SWE are shown in Figures 7, 8 and 9 for three example basins, one for each region (the same figures for the other six basins are included in the supplemental material), and for one hindcast year. The feedback from the change of SWE on DA date to seasonal runoff is readily apparent. Increasing the ensemble model SWE through DA will lead to increased model runoff, and vice versa. For basins with a strong seasonal cycle of streamflow (e.g. Greys and Merced River), SWE DA generally
- 245 improves daily runoff forecasts in addition to seasonal volume forecast improvements, although this is not true in every watershed (e.g. Tolt River).

Figures 10, 11 and 12 show several scatter plots of forecast period runoff for the ESP ensemble forcing and perfect forcing forecasts, versus observed runoff, in the three case basins for all of the hindcast years. The left two columns show the comparison for No DA and DA simulated seasonal runoff vs observed runoff for perfect (top row) and ESP ensemble forcing

- (bottom row) respectively. The 1:1 lines are shown as grey dashed lines and regression lines for the results are shown as green solid line. The results after DA have higher correlation and are closer to the 1:1 line, which indicates that for both forcing types SWE DA improves seasonal runoff simulation and prediction skill. The rightmost columns in these three figures show the scatter plots of SWE increment (i.e., SWE analyses states minus model SWE without DA) vs runoff error (i.e., the simulated seasonal runoff without DA minus the observed seasonal runoff). If the runoff errors are positive (the seasonal runoff is
- 255 overestimated), we expect the SWE increment to be negative in order to decrease the model seasonal runoff (counteract model error) and vice versa. Thus the ideal results are that the points fall onto different sides of y=0 and x=0 lines (shown as grey dashed lines in this panel), i.e., the points all fall into the 2nd (upper left) and 4th (lower right) quadrants. This is generally the case for our case basins for both perfect and ESP forcing, which again shows that the SWE DA approach is successful in reducing model and forecast error.

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5 Discussion and Conclusions

SWE observations generally contain valuable information that has potential to enhance seasonal runoff forecasts. However, relating point SWE measurements that have uneven spatial distribution and varying environmental conditions to watershed mean conditions is a challenge that is often met by empirical solutions. Two approaches of constructing SWE ensemble observations are examined in this study in an effort to reduce the spatial variability and decrease the interpolation uncertainty while also transforming the observations to model space (e.g., the range of the model climatology). Both percentile and Z-score transformations normalize the original SWE values in a way to decrease the spatial variability (Slater and Clark 2006;





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Wood and Lettenmaier, 2006). The former ensures the ensemble observations have the same mean as the ensemble model SWE and the variance of ensemble observations is proportional to ensemble model SWE variance. The latter emphasizes the shape of the observation time series. SWE observations in and near a watershed but at different elevations may have greatly varying values, but their percentile and Z-score statistics will show reduced variation because they arise from similar relative weather conditions with respect to conditions in other years. Using normalized statistics significantly reduces the interpolation uncertainty and systematic biases relative to the watershed's SWE climatology.

Sensitivity analyses of model uncertainty impacts on DA performance suggest that either the forcing-alone based estimation

- 275 of model errors underestimate the total model error variance, or the observed SWE error estimation approaches (interpolation plus the SWE regression) tend to overestimate observation uncertainty, or both. It is likely we are underestimating model uncertainty because we have not taken model structural and parameter uncertainty into consideration. Future work should examine this in more detail, as this work clearly indicates that uncertainty scaling approaches (for the model and/or the observations) are likely to be a valuable step for achieving successful DA performance.
- 280 In this study, a 3-month window of SWE observations generally gives the best performance. However, in some basins a different window length may bring larger improvements. Generally, longer windows mean that more SWE values are used for transformation, and the transformation tends to be more statistically representative of the long-term model-observation climatology. However, shorter time windows mean that the model SWE values used for transformation are more relevant to a specific seasonal time period, avoiding aliasing for seasonality, but have much smaller sample sizes and may not properly
- 285 represent the relationship between model and observation climatologies. The window length must be a balance between these two considerations.

The ESP-based assessment of SWE DA in the case study watersheds shows clearly the potential for SWE DA to enhance seasonal runoff forecasts in an automated fashion, which is notable as this has been a long-standing challenge in operational forecasting. We show at least minor improvement in seasonal runoff forecasts in all nine basins. A notable finding is also that the benefits of SWE are linked to the quality of the model simulations of the basin, which can help to target the application of

DA to locations where it will have the most benefit. We chose a DA update frequency of once per year, the date of climatological maximum correlation of modeled and observed runoff. In operational practice, updates would be applied more frequently, pointing to an area for future research. We note also that this study was conducted using conceptual lumped watershed models, similar to those used in operational practice in the

295 US. As a result, this study did not shed light on how to address additional challenges that may be associated with using SWE DA for the spatially distributed models, or with spatially continuous datasets (e.g., satellite and remote sensing SWE estimates) that are increasingly being developed or applied in streamflow forecasting contexts. Although SWE DA has been implemented in distributed models in limited experimental contexts (e.g., Wood and Lettenmaier, 2006), a systematic examination of EnKF





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DA in spatially distributed hydrological models, coupled with a thoughtful accounting for model parameter and structural errors, remains a potentially fruitful area of research and development.

Data Availability

All data used in this study are publicly available. The watershed shapefiles and basin information are described in Newman et al. (2015a) at: doi:10.5065/D6MW2F4D. The forcing ensemble is described in Newman et al. (2015b) and are available at: doi:10.1065/D6TH8JR2. The streamflow data are available through the USGS via: http://waterdata.usgs.gov/usa/nwis/sw and in doi:10.5065/D6MW2F4D. The Snotel observations are available at: www.wcc.nrcs.usda.gov/snow/ while the California SWE observations are available at: cdcc.water.ca.gov/snow.

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Table 1 Basin features of nine case basins.

Region	Basin ID	Elevation (m)	DA date	Basin area (km ²⁾	Slope	Forest percent	Basin name
14	09081600	3092.15	April 1	436.88	150.58	0.6136	Crystal River
14	09352900	3459.15	April 1	187.74	156.09	0.5199	Vallecito Creek
17	13023000	2468.57	March 1	1163.72	98.51	0.6753	Greys River
17	12147600	998.25	April 1	16.07	159.37	1	SF Tolt River
17	13235000	2077.16	April 1	1158.47	126.25	0.8604	SF Payette River
17	14158790	1210.48	March 15	40.76	116.44	1	Smith River
16	10336645	2180.92	April 1	20.09	118.27	0.7136	General Creek
16	10336660	2188.08	April 1	32.46	83.46	0.7908	Blackwood Creek
18	11266500	2576.54	April 1	836.15	140.18	0.6741	Merced River

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(\mathbf{x})	•
	BY

Variance (P) Pion DA 0.124 0.931 0.832 -16.64 34.92 Crystal River P 0.118 0.942 0.847 -12.34 33.12 Vallecito River No DA 0.115 0.944 0.886 -9.21 32.7 Vallecito River P 0.103 0.975 0.89 -30.69 28.37 I/2·P 0.106 0.974 0.883 -32.11 28.87 2·P 0.099 0.975 0.898 -28.34 27.85 Greys River P 0.082 0.974 0.94 -3.91 15.55 1/2·P 0.09 0.969 0.929 -3.8 16.3 2·P 0.090 0.969 0.929 -3.8 15.06 SF Tolt River P 0.234 0.823 0.505 120.04 156.87 1/2·P 0.234 0.812 0.503 111.55 164.73 2·P 0.234 0.812 0.503 12.7	Basin name	Forecast error	R-RMSE	R	NSE	Bias	CRPS
No DA 0.124 0.931 0.832 -16.64 34.92 Crystal River P 0.118 0.942 0.847 -12.34 33.12 1/2·P 0.115 0.944 0.842 -14.29 33.39 2·P 0.115 0.944 0.856 -9.21 32.7 Vallecito River P 0.103 0.975 0.89 -30.69 28.37 Vallecito River P 0.106 0.974 0.883 -32.11 28.87 2·P 0.099 0.975 0.898 -28.34 27.85 Greys River P 0.082 0.974 0.94 -3.91 15.55 1/2·P 0.09 0.969 0.929 -3.8 16.3 2·P 0.077 0.977 0.948 -3.83 15.06 SF Tolt River P 0.234 0.823 0.505 120.04 156.67 J2·P 0.234 0.812 0.503 111.55 164.73 J2·P<		variance (P)					
$ \begin{array}{c} \mbox{Crystal River} & P & 0.118 & 0.942 & 0.847 & -12.34 & 33.12 \\ 1/2 \cdot P & 0.12 & 0.94 & 0.842 & -14.29 & 33.39 \\ \hline 2 \cdot P & 0.115 & 0.944 & 0.856 & -9.21 & 32.7 \\ \hline No DA & 0.11 & 0.971 & 0.874 & -33.89 & 29.95 \\ \hline P & 0.103 & 0.975 & 0.89 & -30.69 & 28.37 \\ \hline 1/2 \cdot P & 0.106 & 0.974 & 0.883 & -32.11 & 28.87 \\ \hline 2 \cdot P & 0.099 & 0.975 & 0.898 & -28.34 & 27.85 \\ \hline P & 0.082 & 0.974 & 0.94 & -3.91 & 15.55 \\ \hline 1/2 \cdot P & 0.09 & 0.969 & 0.929 & -3.8 & 16.3 \\ \hline 2 \cdot P & 0.09 & 0.969 & 0.929 & -3.8 & 16.3 \\ \hline 2 \cdot P & 0.077 & 0.977 & 0.948 & -3.83 & 15.06 \\ \hline P & 0.234 & 0.823 & 0.505 & 120.04 & 156.87 \\ \hline 1/2 \cdot P & 0.234 & 0.812 & 0.503 & 111.55 & 164.73 \\ 2 \cdot P & 0.234 & 0.812 & 0.504 & 132.76 & 150.56 \\ \hline No DA & 0.124 & 0.963 & 0.895 & -26.77 & 29.43 \\ \hline SF Tolt River & P & 0.118 & 0.967 & 0.905 & -24.91 & 28.81 \\ River & 1/2 \cdot P & 0.117 & 0.968 & 0.906 & -25.88 & 28.07 \\ \hline 2 \cdot P & 0.117 & 0.968 & 0.906 & -25.88 & 28.07 \\ \hline Smith River & P & 0.118 & 0.907 & 0.803 & -26.25 & 65.92 \\ \hline 1/2 \cdot P & 0.187 & 0.898 & 0.781 & -28.04 & 69.24 \\ \hline 2 \cdot P & 0.178 & 0.907 & 0.803 & -26.25 & 65.92 \\ \hline 1/2 \cdot P & 0.187 & 0.898 & 0.781 & -28.04 & 69.24 \\ \hline 2 \cdot P & 0.168 & 0.916 & 0.824 & -22.91 & 62.07 \\ \hline No DA & 0.112 & 0.975 & 0.935 & -15.54 & 36.64 \\ \hline P & 0.092 & 0.99 & 0.971 & -17.36 & 29.76 \\ \hline 1/2 \cdot P & 0.187 & 0.898 & 0.781 & -28.04 & 69.24 \\ \hline 2 \cdot P & 0.168 & 0.916 & 0.824 & -22.91 & 62.07 \\ \hline No DA & 0.112 & 0.976 & 0.935 & -52.44 & 55.12 \\ \hline Blackwood & P & 0.118 & 0.976 & 0.935 & -52.44 & 55.12 \\ \hline Blackwood & P & 0.118 & 0.982 & 0.944 & -49.95 & 52.8 \\ \hline Creek & 1/2 \cdot P & 0.117 & 0.98 & 0.945 & -14.53 & 55.04 \\ \hline P & 0.0253 & 0.963 & 0.762 & -105.95 & 84.98 \\ \hline Averced River & P & 0.258 & 0.961 & 0.752 & -107.76 & 86.62 \\ \hline 2 \cdot P & 0.258 & 0.961 & 0.752 & -107.76 & 86.62 \\ \hline 2 \cdot P & 0.245 & 0.964 & 0.776 & -102.65 & 81.87 \\ \hline \end{array}$		No DA	0.124	0.931	0.832	-16.64	34.92
Number 1/2·P 0.12 0.94 0.842 -14.29 33.39 2·P 0.115 0.944 0.856 -9.21 32.7 Vallecito River P 0.103 0.971 0.874 -33.89 29.95 P 0.103 0.975 0.89 -30.69 28.37 1/2·P 0.106 0.974 0.883 -32.11 28.87 2·P 0.099 0.975 0.898 -28.34 27.85 Greys River P 0.082 0.974 0.94 -3.91 15.55 1/2·P 0.09 0.969 0.929 -3.8 16.3 2·P 0.077 0.977 0.948 -3.83 15.06 SF No DA 0.242 0.76 0.467 81.64 176.98 SF Tolt River P 0.234 0.823 0.505 120.04 156.87 1/2·P 0.234 0.812 0.503 111.55 164.73 2·P 0.	Crystal River	Р	0.118	0.942	0.847	-12.34	33.12
$ \frac{2 \cdot P}{Vallecito River} = \frac{2 \cdot P}{Vallecito River} = \frac{2 \cdot P}{P} = \frac{0.115}{0.103} = \frac{0.971}{0.971} = \frac{0.874}{0.874} = \frac{-33.89}{-30.69} = \frac{29.95}{28.37} \\ \frac{P}{1/2 \cdot P} = \frac{0.106}{0.106} = \frac{0.974}{0.974} = \frac{0.883}{0.883} = \frac{-32.11}{-32.11} = \frac{28.87}{28.87} \\ \frac{2 \cdot P}{2 \cdot P} = \frac{0.099}{0.997} = \frac{0.898}{0.888} = \frac{-2.834}{-2.98} = \frac{27.85}{18.67} \\ \frac{P}{1/2 \cdot P} = \frac{0.082}{0.994} = \frac{0.974}{0.94} = \frac{0.944}{-3.91} = \frac{15.55}{15.55} \\ \frac{P}{1/2 \cdot P} = \frac{0.097}{0.997} = \frac{0.929}{0.929} = \frac{-3.8}{-3.8} = \frac{16.3}{16.3} \\ \frac{2 \cdot P}{2 \cdot P} = \frac{0.077}{0.977} = \frac{0.948}{0.948} = \frac{-3.83}{-3.83} = \frac{15.06}{15.687} \\ \frac{P}{1/2 \cdot P} = \frac{0.234}{0.234} = \frac{0.823}{0.823} = \frac{0.505}{0.505} = \frac{120.04}{156.87} = \frac{156.87}{164.73} \\ \frac{2 \cdot P}{2 \cdot P} = \frac{0.234}{0.234} = \frac{0.812}{0.837} = \frac{0.504}{0.504} = \frac{132.76}{150.56} \\ \frac{No DA}{2 \cdot P} = \frac{0.118}{0.234} = \frac{0.967}{0.905} = \frac{-24.91}{24.91} = \frac{28.81}{28.07} \\ \frac{No DA}{2 \cdot P} = \frac{0.117}{0.117} = \frac{0.968}{0.906} = \frac{-25.88}{-28.77} = \frac{29.43}{24.91} \\ \frac{No DA}{0.211} = \frac{0.872}{0.977} = \frac{0.803}{0.903} = \frac{-22.5}{-26.77} = \frac{29.43}{24.91} \\ \frac{P}{2 \cdot P} = \frac{0.118}{0.118} = \frac{0.977}{0.977} = \frac{0.803}{0.903} = \frac{-22.5}{-26.25} = \frac{65.92}{1.2 \cdot P} \\ \frac{No DA}{0.118} = \frac{0.978}{0.997} = \frac{0.824}{0.824} = \frac{-22.91}{-22.91} = \frac{62.07}{62.07} \\ \frac{P}{1.2 \cdot P} = \frac{0.118}{0.916} = \frac{0.915}{0.992} = \frac{-15.54}{-36.64} = \frac{36.64}{9.24} \\ \frac{P}{2 \cdot P} = \frac{0.108}{0.118} = \frac{0.976}{0.935} = \frac{-52.44}{-22.91} = \frac{52.8}{52.12} \\ \frac{P}{1.2 \cdot P} = \frac{0.118}{0.916} = \frac{0.943}{0.925} = \frac{-52.54}{-52.44} = \frac{52.8}{55.12} \\ \frac{P}{0.018} = \frac{0.996}{0.999} = \frac{0.971}{0.973} = \frac{-52.34}{-52.23} = \frac{52.44}{-52.91} \\ \frac{P}{2 \cdot P} = \frac{0.118}{0.982} = \frac{0.944}{0.943} = \frac{-52.8}{55.04} \\ \frac{P}{0.253} = \frac{0.963}{0.963} = \frac{0.762}{0.737} = \frac{-105.4}{-105.95} = \frac{84.98}{1.2 \cdot P} \\ \frac{No DA}{0.266} = \frac{0.952}{0.737} = \frac{-105.4}{-105.95} = \frac{84.98}{1.2 \cdot P} \\ \frac{No DA}{0.255} = \frac{0.964}{0.776} = \frac{-105.95}{-105.95} = \frac{84.98}{1.2 \cdot P} \\ \frac{P}{0.253} = \frac{0.964}{0.776} = \frac{-102.65}{0.955} = \frac{84.98}{0.961} \\ \frac{P}{0.$		$1/2 \cdot P$	0.12	0.94	0.842	-14.29	33.39
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		2·P	0.115	0.944	0.856	-9.21	32.7
P 0.103 0.975 0.89 -30.69 28.37 $1/2 \cdot P$ 0.106 0.974 0.883 -32.11 28.87 $2 \cdot P$ 0.099 0.975 0.898 -28.34 27.85 $Regines River$ P 0.002 0.974 0.944 -3.91 15.55 $1/2 \cdot P$ 0.09 0.969 0.929 -3.8 16.3 $2 \cdot P$ 0.077 0.977 0.948 -3.83 15.06 SF $No DA$ 0.242 0.76 0.467 81.64 176.98 P 0.234 0.823 0.505 120.04 156.87 $1/2 \cdot P$ 0.234 0.812 0.503 111.55 164.73 $2 \cdot P$ 0.234 0.837 0.504 132.76 150.56 No DA 0.124 0.963 0.895 -26.77 29.43 SF Payette P 0.118 0.967 0.903		No DA	0.11	0.971	0.874	-33.89	29.95
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	Vallecito River	Р	0.103	0.975	0.89	-30.69	28.37
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	valicento River	$1/2 \cdot P$	0.106	0.974	0.883	-32.11	28.87
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		2·P	0.099	0.975	0.898	-28.34	27.85
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		No DA	0.113	0.952	0.887	-2.98	18.67
	Carros Discon	Р	0.082	0.974	0.94	-3.91	15.55
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	Greys River	$1/2 \cdot P$	0.09	0.969	0.929	-3.8	16.3
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		2·P	0.077	0.977	0.948	-3.83	15.06
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		No DA	0.242	0.76	0.467	81.64	176.98
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $		Р	0.234	0.823	0.505	120.04	156.87
$\frac{2 \cdot P}{Merced River} = \begin{array}{c ccccccccccccccccccccccccccccccccccc$	SF Tolt River	$1/2 \cdot P$	0.234	0.812	0.503	111.55	164.73
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		2·P	0.234	0.837	0.504	132.76	150.56
$\begin{array}{c c c c c c c c c c c c c c c c c c c $		No DA	0.124	0.963	0.895	-26.77	29.43
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	SF Payette	Р	0.118	0.967	0.905	-24.91	28.81
$\frac{2 \cdot P}{Merced River} = \begin{array}{c ccccccccccccccccccccccccccccccccccc$	River	1/2·P	0.117	0.968	0.906	-25.88	28.07
		2·P	0.119	0.963	0.903	-23.41	30.02
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		No DA	0.211	0.872	0.722	-28.5	76.42
		Р	0.178	0.907	0.803	-26.25	65.92
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	Smith River	1/2·P	0.187	0.898	0.781	-28.04	69.24
$ \frac{No DA}{General Creek} \begin{tabular}{ c c c c c c c c c c c c c c c c c c c$		2·P	0.168	0.916	0.824	-22.91	62.07
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $		No DA	0.118	0.978	0.952	-15.54	36.64
		Р	0.092	0.99	0.971	-17.36	29.76
$\begin{tabular}{ c c c c c c c c c c c c c c c c c c c$	General Creek	1/2·P	0.097	0.988	0.968	-18.19	30.29
$\begin{tabular}{ c c c c c c c c c c c c c c c c c c c$		2·P	0.089	0.99	0.973	-15.22	30.86
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		No DA	0.127	0.976	0.935	-52.44	55.12
$\begin{tabular}{ c c c c c c c c c c c c c c c c c c c$	Blackwood	Р	0.118	0.982	0.944	-49.95	52.8
2·P 0.117 0.98 0.945 -44.53 55.04 Merced River No DA 0.266 0.952 0.737 -105.4 83.41 P 0.253 0.963 0.762 -105.95 84.98 1/2·P 0.258 0.961 0.752 -107.76 86.62 2·P 0.245 0.964 0.776 -102.65 81.87	Creek	1/2·P	0.119	0.982	0.943	-53.23	51.11
No DA 0.266 0.952 0.737 -105.4 83.41 P 0.253 0.963 0.762 -105.95 84.98 I/2·P 0.258 0.961 0.752 -107.76 86.62 2·P 0.245 0.964 0.776 -102.65 81.87		2·P	0.117	0.98	0.945	-44.53	55.04
P 0.253 0.963 0.762 -105.95 84.98 1/2·P 0.258 0.961 0.752 -107.76 86.62 2·P 0.245 0.964 0.776 -102.65 81.87		No DA	0.266	0.952	0.737	-105.4	83.41
Merced River 1/2·P 0.258 0.961 0.752 -107.76 86.62 2·P 0.245 0.964 0.776 -102.65 81.87		Р	0.253	0.963	0.762	-105.95	84.98
2·P 0.245 0.964 0.776 -102.65 81.87	Merced River	1/2·P	0.258	0.961	0.752	-107.76	86.62
		2·P	0.245	0.964	0.776	-102.65	81.87

Table 2 Evaluation statistics for percentile interpolation before and after DA.





D.	Forecast error	D DMCE	R	NSE	Bias	CRPS
Basin name	variance (P)	K-KIVISE				
	No DA	0.124	0.931	0.832	-16.64	34.92
Crustal Divor	Р	0.119	0.942	0.845	-11.22	33.94
Crystal Kivel	$1/2 \cdot P$	0.12	0.939	0.841	-13.4	34.05
	$2 \cdot P$	0.117	0.945	0.85	-8.57	33.56
	No DA	0.11	0.971	0.874	-33.89	29.95
Vallecito	Р	0.1	0.975	0.896	-29.84	28.7
River	$1/2 \cdot P$	0.103	0.975	0.89	-31.15	29.06
	2 · P	0.098	0.974	0.9	-28.27	28.19
	No DA	0.113	0.952	0.887	-2.98	18.67
Crown Divor	Р	0.092	0.969	0.926	-3.5	16.22
Gleys River	$1/2 \cdot P$	0.098	0.965	0.916	-3.41	16.8
	2 · P	0.087	0.972	0.933	-3.48	16.02
	No DA	0.242	0.76	0.467	81.64	176.98
OF T-14 Dime	Р	0.182	0.879	0.7	102.55	127.9
SF Toll Kiver	$1/2 \cdot P$	0.193	0.857	0.661	97.22	137.26
	2·P	0.174	0.896	0.725	109.53	121.26
	No DA	0.124	0.963	0.895	-26.77	29.43
SF Payette	Р	0.117	0.965	0.907	-22.69	28.69
River	$1/2 \cdot P$	0.115	0.967	0.909	-22.99	27.89
	2·P	0.122	0.961	0.897	-23.36	30
	No DA	0.211	0.872	0.722	-28.5	76.42
Carith Discon	Р	0.173	0.911	0.813	-30.55	63.69
Smith River	$1/2 \cdot P$	0.183	0.902	0.792	-31.22	67.56
	2·P	0.163	0.921	0.835	-29.45	59.99
	No DA	0.118	0.978	0.952	-15.54	36.64
Conversit Convelo	Р	0.096	0.99	0.968	-19.96	29.97
General Creek	$1/2 \cdot P$	0.1	0.988	0.966	-18.65	30.95
	2·P	0.096	0.992	0.969	-21.17	30.4
	No DA	0.127	0.976	0.935	-52.44	55.12
Blackwood	Р	0.118	0.984	0.944	-53.46	51.82
Creek	$1/2 \cdot P$	0.118	0.983	0.944	-53.17	50.62
	2 · P	0.12	0.983	0.942	-53.52	53.74
	No DA	0.266	0.952	0.737	-105.4	83.41
Margad Diver	Р	0.24	0.967	0.787	-99.26	79.81
wierced Kiver	$1/2 \cdot P$	0.246	0.964	0.775	-101.37	83.11
	2 · P	0.234	0.969	0.797	-96.91	76.03

Table 3 Evaluation statistics for Z-score interpolation before and after DA.







Position of 9 case basins and SWE gauge sites

Figure 1. Location of nine case basins in the Western US and SWE gauge sites.







420 Figure 2. Boxplots of ensemble model SWE and estimated ensemble SWE observations for the nine case basins on the DA date in 2004, for three window lengths – 7 days, 3 months, and 1 year.





425



Figure 3. Evaluation metrics for April-July ensemble mean streamflow from the percentile-based interpolation method for the nine case basins.







Figure 4. Evaluation metrics for April-July ensemble mean streamflow from the Z-score interpolation for the nine case

430 basins.







Figure 5. Evaluation statistics of percentile interpolation for the nine case basins with the two variations on ESP and with perfect forcing data (ens in the legend denotes ensemble).

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440 Figure 6. Incremental change in evaluation statistics for ESP and perfect forcing forecasts using percentile-based interpolation for the nine case basins.







Region: 17 Basin ID: 13023000 Name: Greys River

445 Figure 7. Time series plots for runoff and SWE for Greys River. Light blue lines indicate individual ensemble member traces. Vertical black dashed line denotes data assimilation date.







Region: 17 Basin ID: 12147600 Name: SF Tolt River

450 Figure 8. Time series plots for runoff and SWE for SF Tolt River following Figure 7.







Region: 18 Basin ID: 11266500 Name: Merced River

Figure 9. Time series plots for runoff and SWE for Merced River following Figure 7.

455







Figure 10. Scatter plots for seasonal runoff and SWE on DA date in the DA years for Greys River. Black dashed diagonal lines is the 1:1 line, while the green lines indicate linear regression fits to data.







Figure 11. Scatter plots for seasonal runoff and SWE on DA date in the DA years for SF Tolt River following Figure 10.







465 Figure 12. Scatter plots for seasonal runoff and SWE on DA date in the DA years for Merced River following Figure

10.