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Estimation of overland flow metrics at semiarid condition: Patagonian Monte

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Abstract

Water infiltration and overland flow (WIOF) processes are relevant in considering water partition among plant life forms, the sustainability of vegetation and the design of sustainable hydrological management. WIOF processes in arid and semiarid regions present regional characteristic trends imposed by the prevailing physical conditions of the upper soil as evolved under water-limited climate. A set of plot-scale field experiments at the semi-arid Patagonian Monte (Argentina) was performed in order to estimate infiltration-overland descriptive flow parameters. The micro-relief of undisturbed field plots at z-scale <1 mm was characterized through close-range stereo-photogrammetry and geo-statistical modelling. The overland flow areas produced by experimental runoff events were video-recorded and the runoff speed was measured with ortho-image processing software. Antecedent and post-inflow moisture were measured, and texture, bulk density and physical properties of the soil at the upper vadose zone were estimated. Field data were used to calibrate a physically-based, time explicit model of water balance in the upper soil and overland flows with a modified Green-Ampt (infiltration) and Chezy's (overland flow) algorithms. Modelling results satisfy validation criteria based on the observed overland flow areas, runoff-speed, water mass balance of the upper vadose zone, infiltration depth, slope along runoff-plume direction, and depression storage intensity. The experimental procedure presented supplies plot-scale estimates of overland flow and infiltration intensities at various intensities of water input which can be incorporated in larger-scale hydrological grid-models of arid regions. Findings were: (1) Overland flow velocities as well as infiltration-overland flow mass balances are consistently modelled by considering variable infiltration rates corresponding to depression storage and/or non-ponded areas. (2) The statistical relations presented allow the estimation of theoretical hydrodynamic parameters (Chezy's frictional C , average overland flow depth d^*) through measurable characteristics of the surface soil and overland flow kinetics. (3) A protocol of field experiments and coupled

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time-distributed modelling to 1–2 above is described. The methodology and results obtained in this study are probably relevant to similar arid-semiarid areas of the world.

1 Introduction

The complexity of interactions between overland flow and infiltration has long received attention in hydrological studies. The spatial variability of some of the upper soil properties that are relevant in those processes has been identified as a major determinant of infiltration rates and the hydrological response of watersheds (Abrahams et al., 1990; Köhn et al., 2009; Darboux et al., 2001). Usual procedures to study runoff-infiltration processes involve the use of hydrological models based on hydrograph records. Although a large body of literature has been devoted to the criteria used to inspect hydrograph records (Ewen, 2011) less attention has been paid to the fact that many hydrological models that can accurately reproduce hydrograph records, produce severely biased estimates of overland flow velocities (Mügler et al., 2011).

Infiltration-runoff models need to calculate the overland flow velocity and depth to be able to simulate the flow of water over the land surface. Frictional effects must be accounted for, usually through simplifying equations of the fundamental hydrodynamic laws on continuity and momentum balance involved. To this aim, most field and laboratory studies on overland flow use the Darcy-Weisbach's f and Manning's n (Darboux et al., 2001; Hessel et al., 2003; Li, 2009). Both of these are calculated from the same variables and both suffer from limitations imposed by the high variability of the represented effects both in space and time. Abrahams et al. (1990) studied Darcy-Weisbach's f for desert hill slopes and found that it varies with the rate of flow. Since the rate of flow is highly variable in space, so too is f . Analogous limitations can be expected in relation to Manning's n .

In the case of arid and semiarid areas, complex interactions between runoff generation, transmission and re-infiltration over short temporal scales (Li et al., 2011; Reaney, 2008) add difficulties in the estimation of infiltration-overland flows. In these areas, the

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soil surface is predominantly flat and gently sloped at extended spatial scales, and the excess rainfall moves at the surface of the soil in very shallow structures combining small overland flow areas interspersed with finely ramified fingering patterns of small channels. Infiltration in arid and semiarid regions can also be modified by the development of water-repellent areas (Lipsius and Mooney, 2006) which in cases occur in relation to soil microtopography (Biemelt et al., 2005).

This kind of composite patterns of overland flow can be expected to drive a spatially heterogeneous pattern of water infiltration in the upper soil (van Schaik, 2009). Results obtained by Esteves et al. (2000) in laboratory runoff plots, indicate that despite the soil being represented by only one set of parameters, infiltration was not homogeneous all over the plot surfaces. They concluded that the microtopography had a strong effect on observed flow directions, producing small ponds along the paths of flow and altering the flow depths and velocity fields. Similar flow distributions were observed by Parsons et al. (1990) in plots under semiarid conditions. They advanced two explanations for the observed distributions. First, following the application of Darcy's law the infiltration is an increasing function of the applied water depth at the soil surface. Secondly, the water available (supply rate) to satisfy the infiltration is greater on runoff preferential paths.

Descroix et al. (2007) observed that catchments and small plot studies in arid regions usually show a so-called "hortonian" trend, with characteristic absence of base flow. This was the case in semi-arid Northern Mexico for catchments as large as tens of thousands km², and for approximately 500 km² in the Western Sierra Madre. Latron and Gallart (2007) determined that there is a strong relationship between the base flow and the extension of saturated areas at the basin scale. In Australia's semiarid rangelands, patches of bare soils were determined to be the main runoff controlling factor (Bartley et al., 2006).

Thompson et al. (2011) observed that the lateral redistribution of ponded water generated during intense rainstorms plays an important role in the hydrological, biogeochemical and ecological functioning of patchy arid ecosystems. Lateral redistribution

hydrograph quality, while the actual travel time of water is ignored. In extended hydrological practice, frictional factors are usually taken from published tables or assumed spatially constant (Esteves et al., 2000).

Plot experimentation has long been relevant in developing physically based models of water flow (Köhne et al., 2009). The term usually refers to laboratory (Antoine et al., 2011; Dunkerley, 2002, 2001) or field (Esteves et al., 2000; Mengitsu et al., 2012) studies involving areas in the scale of some few square meters, where water flow conditions can be manipulated with varying success. Plot experiments were often used to obtain estimates of depression storage (DS) (Antoine et al., 2011) in studies of overland flow, in the context of interpreting DS retention effect on hydrograph records (Govers et al., 2000; Hansen, 2000). The underlying concept is that DS areas would predominantly behave as a temporary passive storage of overland flow that would result in a delay in the hydrograph signals. This in turn prompts interest in its direct measurement at the plot scale through geometrical analysis of elevation models (Planchon and Darboux, 2002) or the analysis of the amounts of water retained in soil surface castings (Antoine et al., 2011).

This study is part of a larger project at the National Patagonic Centre (Chubut, Argentina) to develop tools for the eco-hydrological analysis of desertification processes. It builds on previous results obtained by Ares et al. (2003) indicating that early desertification processes involve modifications of the overland flow patterns and its distribution among the various vegetation life forms in the Patagonian Monte (Argentina). Here, an experimental protocol of field plot experiments and coupled simulation model to estimate infiltration and overland flow parameters in the Monte environment is presented. The protocol involves parameters that can be quantitatively monitored with an adequate degree of precision (overland flow movement, infiltration depth, soil water content) as well as others that can indirectly be estimated (surface frictional effect, average overland flow depth, depression storage). Specifically, this study addresses: (1) the effect of micro-topography at plot scale on the infiltration-overland flow intensities, (2) the estimation of overland flow parameters needed to calculate water movement

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at the soil surface. The methodology and results obtained in this study are probably relevant to similar arid-semiarid areas of the world.

2 Field experiments

The study was carried out at the wildlife refuge “La Esperanza”, a representative site of the Patagonian Monte (southern portion of the Monte Phytogeographic Province) which occupies an area of about 42 000 km² in South-Eastern Argentina (León et al., 1998; Soriano, 1950). The mean annual temperature is 13.4 °C and the mean annual precipitation is 235.9 mm (1982–2001). Most of the rainfall occurs during the cold season (April–September), but heavy rainfall events may also occur during the warm season. The local vegetation covers about 40% and 60% of the soil in a random patchy structure with three vegetation layers: the upper layer (1–2 m height) dominated by tall shrubs (*Larrea divaricata*, *Schinus johnstonii* Barkley, *Chuquiraga erinacea* Don., *Atriplex lampa* Gill. ex Moq., and *Lycium chilense* Miers ex Bert.), the intermediate layer (0.5–1.2 m height) also composed by dwarf shrubs (*Nassauvia fuegiana* (Speg.) Cabrera, *Junellia seriphioides* (Gillies and Hook) Mold., and *Acantholippia seriphioides* (A. Gray) Mold.), and the low layer (0.1–0.5 m height) dominated by perennial grasses (*Nassella tenuis* (Phil.) Barkworth, *Pappostipa speciosa* (Trin. and Rupr.) Romasch, and *Poa ligularis* (Nees ex Steud), Ares et al., 1990; Carrera et al., 2008). Soils are a complex of Typic Petrocalcids–Typic Haplocalcids with a fractured calcium carbonate layer from 0.45 to 1 m below the soil surface (del Valle, 1998). Upper soil texture types (USDA) are sandy or loamy sand.

Ten field plot experiments in undisturbed bare soil patches (800 × 800 mm) along a transect from S: –42.17046, W: –64.96343 to S: –42.21366, W: –64.99225 were conducted during the dry season in days with air temperatures in the range 20–32 °C. Water inflow to plots was achieved through a portable, wind protected, single-nozzle system mounted on a wooden frame and calibrated with a pressure gauge to supply water input rates in the range 225–4500 mm³ s⁻¹ at a small area of each plot. This

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created two plot sub-regions: one directly receiving the water inflow and another receiving the overland flow resulting from the excess of water input to the former. The water inflow range corresponded to usual values in the region as estimated with algorithms (Tarboton and Ames, 2001), based on the analysis of local terrain elevation models at the mentioned spatial scale. The frame also supported a Kodak EasyShare Z712 IS 7.1 MP camera (© Eastman Kodak Company, Rochester, New York) with video mode, at near zenith position over the centre of the plot and about 1 m above ground level. Video records of the expanding overland wetted areas were obtained during experiments of varying time length (72–1128 s) until the wet front reached some of the borders of the camera image in any direction, thus creating a zero-discharge condition at the plot micro basin.

Soil moisture at 0–30 mm depth was evaluated immediately after the water inflow ceased with a TDR pin (50 mm) probe (Time-Domain Reflect meter, TRIME®-FM, Ettlingen) inserted along a proper angle at equidistant (30–35 mm) points of a grid covering the whole overland flow area and neighbour dry points around it. TDR estimates of soil moisture were calibrated with gravimetric estimates obtained by extracting 2–5 soil cores (Ø: 40 mm) along the main axes of the overland flow plume, (at the centre of the directly wetted area and equidistant positions up to the wetted border and control at nearby dry soil, Fig. 1a). Soil cores were extended to depths 0–30, 30–90 and 90–180 mm, brought to the laboratory in sealed containers and wet-dry (105 °C) analysed for moisture content. Gravimetric moisture was expressed in volumetric terms through correction with soil bulk density measured through the excavation method (ISO 11272, 1998) at a nearby location. Maps of soil moisture in the wetted area at the end of the water inflow period were based on calibrated TDR estimates ($y = 1.166x + 0.05$; $R^2 = 0.92$, $n = 101$; x : gravimetric θ estimate). Map values were obtained by interpolation of 4–6 closest TDR estimates, based on a specified search radius algorithm (Idrisi v. 14.02, Clark Labs, Worcester).

The texture of the extracted soil core samples was measured with Lamotte's 1067 texture kit (LaMotte Co., Chestertown) calibrated with replicate estimates obtained at

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two reference laboratories (National Patagonic Centre, National University of the South, Argentina) with the Robinson's protocol (Gee and Bauder, 1986). Parameters θ_r , θ_s , K_{sat} , L , $\log_{10} \alpha$ and $\log_{10} n$ (Van Genuchten, 1980) corresponding to the soil moisture-water head function of the obtained soil samples were estimated based on textural data through an ANN (Artificial Neural Net) pedotransfer algorithm (Rosetta v. 1.2, US Salinity Laboratory, Riverside) developed by Shaap and Leij (1998).

A methodology (Rossi and Ares, 2012) combining Close-Range Stereophotogrammetry (CRS) and Geo-Statistical Modelling (GSM) was applied to obtain Digital Elevation Models (DEM) of all plots at millimetre precision along x - y - z coordinates. The coordinates of 9 control points and 64 targets obtained through CRS were loaded to an application (I-Witness v. 1.4, DCS Inc., Bellevue, WA, USA) for ortho-rectification and further to a krigging algorithm (Surfer v. 7, Golden Software Inc., Colorado) to obtain plot DEMs and DEM-based flow vector fields. At one control plot, the DEM obtained with the CRS-GSM procedure was further compared with another obtained through a regular optical level/staff procedure. Digital video-images of the overland flowing water plume were selected at regularly spaced time intervals (6–240 s), exported to an image processing application (Idrisi v. 14.02, Clark Labs, Worcester), ortho-rectified and overlaid to their corresponding plot DEM and map of flow vector fields. The terrain height change corresponding to the overland flow advance at each time interval was evaluated by averaging the altitude differences between all pixels around the borders at successively wet areas. At each time interval the area advanced by the overland flow plume was measured, discriminating between: (1) area advanced down slope, and (2) area advanced in direction opposed to the expected gravity flow. The magnitude of depression storage (DS) (Antoine et al., 2011) occurring within the wet plume area was then estimated through the ratio of areas $2/(1 + 2)$ above mentioned (Fig. 1b).

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3 Model

The changes observed at the overland flow areas and subsurface soil water content at the plots during the experiments were modelled with a physically based continuity model consisting in a set of non-linear ordinary differential and continuity equations with time variable parameters, each representing the storage of overland water and at the upper vadose zone, respectively:

$$\frac{dQ_o}{dt} = W - IN(t) - O(t) \quad (1)$$

$$\frac{dQ_v}{dt} = IN(t) \quad (2)$$

$$CW = Q_o + Q_v \quad (3)$$

(Q_o : Water stored at the overland plume (mm^3); Q_v : Water stored at the upper vadose zone (mm^3); W : Water inflow ($\text{mm}^3 \text{s}^{-1}$); $IN(t)$: Total infiltration flow ($\text{mm}^3 \text{s}^{-1}$); CW : Cumulative water inflow (mm^3); $O(t)$: overland flow ($\text{mm}^3 \text{s}^{-1}$)).

Note that evaporation was disregarded due to the short duration of the field experiments. While W is time-constant for each single plot experiment, $IN_t - O_t$ can be expected to change along time depending on the effects of soil micro-relief and the physical properties of the surface soil. Further, the overland flow plume contains storage areas where free water accumulates during the water inflow period, and other parts where the water inflow is not enough to maintain free water at the soil surface. Accordingly, $IN(t)$ may be expected to result from the composition of flows under both saturated and unsaturated conditions. Saturated conditions occur at DS areas, while both saturated and/or unsaturated infiltration flows can occur at other areas depending on their water content:

$$IN(t) = IN_{\text{sat}}(t) + IN_{\text{unsat}}(t) \quad (4)$$

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($IN_{\text{sat}}(t)$): transient, saturated infiltration flow from DS areas ($\text{mm}^3 \text{s}^{-1}$); $IN_{\text{unsat}}(t)$: transient, saturated and/or non-saturated infiltration flow from non-DS areas ($\text{mm}^3 \text{s}^{-1}$).

The IN_{sat} flow was modelled through a Green-Ampt's concept (Green and Ampt, 1911; Muñoz-Cárpena and Gowdsh, 2005) where infiltration was assumed to occur under saturated conditions depending on the gradient of water head between the soil surface and a point at the infiltrating wetting front:

$$IN_{\text{sat}} = K_s \times \left[\Psi_f \times \left(\frac{\theta_s - \theta(t)}{F} \right) + 1 \right] \times A_{\text{DS}} \quad (5)$$

$$F = z_f(t) \times (\theta_s - \theta(t)) \quad (6)$$

(K_s): saturated hydraulic conductivity (mm s^{-1}); $\Psi_f(\theta)$: suction at the wetting front ($\text{mm H}_2\text{O}$); θ_s , $\theta(t)$: saturation and instantaneous volumetric water content at the saturated wetting front respectively (dimensionless); F : accumulated infiltration (mm); A_{DS} : DS area (mm^2); $z_f(t)$: instantaneous water infiltration depth (mm)

In areas of the overland plume where no DS occurs, water infiltration was assumed to occur under unsaturated conditions. This component of the infiltration flow was estimated by replacing in Eqs. (5, 6) through corresponding terms:

$$IN_{\text{unsat}}(t) = K_h(\theta(t)) \times \left[\Psi_f(\theta(t)) \times \left(\frac{\theta(t) - \theta_i}{F} \right) + 1 \right] \times (A(t) - A_{\text{DS}}) \quad (7)$$

($K_h(\theta(t))$): variable hydraulic conductivity (mm s^{-1}); θ_i : Antecedent water content of the soil; $A(t)$: Wet area of overland plume at time t (mm^2)

$K_h(\theta(t))$ and, $\Psi_f(\theta(t))$ were estimated through the solution (van Genuchten, 1980) of Mualem's (Mualem, 1976) formulation to predict the relative hydraulic conductivity from knowledge of the soil-water retention curve.

The water instantaneously stored at the overland flow was estimated as:

$$Q_o = A(t) \times d(t) \quad (8)$$

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$d(t)$: average depth of the overland flowing water volume at time t (mm)

flowing in the x - y space, with varying speed depending on the local altitude gradient along its border, such that $d(t)$ is inversely related to the slope of the underlying soil surface. This relation would follow Darcy's general form of frictional law (Dingman, 2007; Mügler et al., 2011):

$$d(t) = \left(\frac{W}{C}\right)^2 / S(t, t + dt) \quad (9)$$

(C : Proportionality constant; $S(t, t + dt)$: Average terrain height drop around the overland plume during the interval $t, t + dt$ (mm))

where the Darcy's velocity term is replaced in this case with the water inflow rate W , and $S(t, t - dt)$ was obtained from successive video images of the overland flow overlaid on the plot DEMs. The frictional term C was found through inverse modelling of the continuity Eqs. (1)–(3), with the boundary conditions explained in Sect. 3.1.

Equations (1)–(3) were solved through numerical integration with a fourth-order Runge-Kutta approximation, at solution time intervals $0.06 \leq \Delta t \leq 0.3$ s, and an accepted mass balance error $e \leq 1 \times 10^{-7} \text{ mm}^3$.

Modelling parameters and variables: convergence and validation

Table 1 summarizes the parameters and variables measured and modelled in relation with the field plot experiments. Model estimates were validated both at the plot and inter-plot scales through simultaneous convergence to measured values defining the experimental setting, the morphology of the plot surface soil and the hydrological properties of the upper vadose zone. The criterion at intra-plot scale in the case of parameters 1–8 was strict convergence to measured data values which were used as model input. Convergence to parameter 9 was required within the confidence interval of the ANN estimation process as reported in Shaap and Leij (1998). Variable 10

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was required to converge to data as estimated through a Nash-Sutcliffe's Efficiency Coefficient ($EC \geq 0.99$) (Nash and Sutcliffe, 1970). Values of 11–16 were required to converge to:

$$\sum_{j=1,n} (x_j - x_{\text{data}})^2 = \min \quad (10)$$

where x_j is the model estimate and x_{data} is the corresponding experimental-measured value. 11–16 were further tested for convergence to data at the inter-plot scale such that their correlation with measured values would be significant at $P < 0.001$ over the set of 10 plots studied. Variables 12–14 are average values of the corresponding variable over the simulation time.

Several procedures were used for model validation. Unit consistency and ranges of K_s , K_h ($\theta(t)$), ψ_f ($\theta(t)$), were independently checked through comparison with estimates obtained with CHEMFLO-2000 (Nofziger and Wu, 2003), an application that solves Richards equations to simulate transient water (and chemical) transport in soils. Confidence intervals ($P < 0.05$) of the correlation coefficient r of measured-modelled values 9–16 were built by bootstrapping paired comparisons such that randomly selected five plots were used for model calibration and the rest for model validation. z_f and θ_{end} values observed in the field were additionally tested for consistency with CHEMFLO-2000 estimates. Since this latter application solves the Richards equation in one dimension ($-z$), the comparison of results obtained with the model here presented was limited to the soil core obtained at the water inflow spot.

Additionally, the following variables were defined based on model outputs:

$$\sigma_S = (\sum_{t=1,n-1} (S(t) - S^*)^2 / (n - 1))^{0.5}, \quad (11)$$

(S^* : $S(t)$ average at the end of the water inflow period, mm)

the standard deviation of $S(t)$, as a measure of soil surface roughness independent of plot slope effects,

$$\gamma_{A/W}^{-1} = a - b / (A^* / CW)^{0.5}, \quad (12)$$

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($V_{A^*/CW}$: exponential ratio of the total wet surface soil area A^* to the accumulated water inflow volume at the end of the water inflow period, mm^{-1} ; $a = 2.42 \times 10^{-4}$; $b = 3.31 \times 10^{-4}$)

$$\zeta = \int_{t=0,n} \left(\frac{dQ_o}{dt} - \frac{dQ_v}{dt} \right) \quad (13)$$

(ζ : run-off coefficient, dimensionless)

$$d^* = (\sum_{t=1,n} d(t)) / n \quad (14)$$

(d^* : mean $d(t)$ (mm) (see Fig. 2) at the end of the $t = n$ water inflow period)

$$v^* = (A^*)^{0.5} / t_{\text{end}} \quad (15)$$

(v^* : average overland flow velocity (mms^{-1}) at the end of the water inflow period)

Several parameters of the experimental plots which are usually estimated in overland flow studies (Smith et al., 2007) were also computed to ease comparisons of conditions in this study with reported data. As long as basic assumptions underlying their rationale are granted, the Froude number (F) relates the velocity of the overland laminar flow to the expected velocity of a gravitational wave, or the ratio of the laminar flow inertia to gravitational forces. Analogously, the dimensionless Reynolds number (Re) gives a measure of the ratio of inertial forces to viscous forces for given flow conditions. High Reynolds numbers characterize turbulent flow.

Statistical relations among measured and theoretical hydrodynamic variables (Eqs. 16–19) were identified through stepwise backward regressions analyses (SPSS 15.01, IBM Corporation, New York, USA).

4 Results

Figure 2 shows a summary of the calibration results obtained with the CRS-GSM procedure in a control plot where a DEM was also obtained through a regular

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optical-level/staff procedure. A detailed analysis of these results is presented elsewhere (Rossi and Ares, 2012). It is here relevant to consider that local errors in the estimation of z are in the sub-millimetre range and although they are in cases related to singularities in the local micro-relief (small mounds, depressions), they are not systematic in this or in some other sense that could bias the results here presented.

Table 2 summarizes results obtained from the plot experiments. The upper vadose zone ($z_f \leq 55$ mm) of all plots showed various combinations of sand, silt and clay fractions within the boundaries of Sandy-Loam and Loamy-Sand USDA textural classes (Parameters 1–3) as well as their basic hydraulic properties (parameters 4–5, 9). The values observed in parameter 6 corresponded to very dry conditions of the upper soil due to the fact that all the plot experiments were performed during the dry season of the local climate regime. The range of water input flows ($225\text{--}4500\text{ mm}^3\text{ s}^{-1}$, parameter 7) produced a set of slow to fast overland flow and water infiltration conditions. The model estimates of K_s at the upper vadose zone were significantly correlated to the ANN estimate based on textural data ($x_{\text{model}} = a + b \times x_{\text{data}}$; $H_0: a = 0, b = 1$). The modelled time series of overland flow areas $A(t)$ (variable 10) were in all cases significantly correlated with the observed areas as indicated by the Nash-Sutcliffe's EC (see also Fig. 5). At inter-plot level, parameters 11–16 were also significantly correlated with data values. $H_0: a = 0, b = 1$ was rejected in the case of parameter 16 ($z_{f\text{ model}} = -8.41 + 0.955 \times z_{f\text{ data}}$, $P = 1.42 \times 10^{-6}$). This is to be expected due to the fact that $z_{f\text{ data}}$ were estimated from soil cores along the main overland flow axes, while $z_{f\text{ model}}$ expresses the average infiltration depth corresponding to the whole wet area. The model overall error in mass balance was in all cases $\leq 1 \times 10^{-7}\text{ mm}^3$.

No significant correlations were found between the extent of DS areas and other variables at the inter-plot scale. At intra-plot scale, DS was time-serially (negatively) correlated with the run-off coefficient ζ (Eq. 13) at plots under high water inflow (P1: -0.764^{**} ; P2: -0.767^{**} ; P3: -0.31 ; P4: -0.28) and in most cases was positively correlated with it at plots with low water inflow (P5: 0.243^* ; P6: 0.635^{**} ; P7: -0.799^{**} ; P8: 0.763^{**} ; P9: 0.656^{**} ; P10: 0.15).

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Figure 3 shows four examples of the distribution of upper vadose zone (0–30 mm) moisture ($\theta_{\text{end}, 0-30 \text{ mm}}$) at four plots in this study. Depending on the position of the water inflow and the physical characteristics of the upper vadose zone, some areas contain moisture at near saturation levels while various levels of unsaturated conditions prevail at some other areas.

Figure 4 shows the comparison of z_f and θ_{end} values as obtained from the CHEMFLO-2000 simulation. These are compared with estimates from the model in Sect. 3; photographs of soil cores at the water inflow spot are also shown for comparison.

Table 3 summarizes theoretical hydrodynamic overland flow parameters of the plot experiments and the parameter C obtained through inverse modelling of the system of Eqs. (1)–(3). Figure 6 shows statistical relations linking the variables in Table 3 including relations between C - F and d^* - Re . It is observed that C is significantly related to F and d^* is significantly related to Re , although their values are independent of the variations of flow intensity across the plot series.

The analyses of the (standardized variables) results of all experiments further identify some significant statistical relations among plot characteristics:

$$C = (1.048 \times \gamma_{A/W} - 0.156 \times \sigma_s + 0.114 \times S^*)(r = 0.994^{***}) \quad (16)$$

$$\zeta = (-0.439 \times C + 0.601 \times d^* + 0.794 \times W - 0.491 \times \log_{10} K_s)(r = 0.958^{***}) \quad (17)$$

$$d^* = (-0.461 \times Re + 1.178 \times W)(r = 0.946^{***}) \quad (18)$$

$$v^* = (1.039 \times W - 0.421 \times d^*)(r = 0.995^{***}) \quad (19)$$

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5 Discussion

5.1 Effect of spatial variability of micro-topography at plot scale on the infiltration-overland flow intensities

The consideration of the soil as a heterogeneous system at physical non-equilibrium (PNE) has been applied to a number of hydrological models (see Köhne et al., 2009 for an extensive review of various forms of published PNE models). In most cases, the emphasis has been focused on the effects of sub-surface soil heterogeneity on infiltration flows. In this study, attention is given to surface soil heterogeneity in the x - y plane and it is hypothesized that infiltration must occur under saturated conditions whenever DS occurs, while saturated-unsaturated infiltration could occur at non-DS areas. In considering this hypothesis, we introduce a complementary alternative to Antoine's et al. (2011) geometric interpretation of the DS concept by considering that depending on the intensity of overland flow, unsaturated infiltration occurs at some areas of the soil where the water inflow is insufficient to maintain a free water film on the soil surface. This would hold whenever water infiltration rates in DS areas are high enough to keep pace with the local water supply rate. This interpretation is analogous to observations on composite overland flow by Abraham et al. (1990) on runoff plots in arid Southern Arizona (USA) where overland flow occurred as shallow sheets of water with threads of deeper, faster flow diverging and converging around surface protuberances, rocks, and vegetation.

The results here presented imply that the effect of DS areas on the overland flow speed would be more complex than expected if those would behave like passive water storages. DS areas would be important as places where water that would otherwise contribute to overland flow infiltrates at high rates under saturated conditions. This view is supported by the fact that the time serial correlation $DS-\zeta$ is negative or positive at plots with varying intensity of water inflow. At plots P1–P4, (high water input rates during short times), the saturated infiltration rate is negligible in comparison with the water inflow. In these cases, DS areas become filled before the underlying top soil

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reaches saturation and their behaviour would approach that of a passive impervious storage. This would result in runoff delay (negative DS- ζ correlation). In most of the cases where the water input occurs at low rates during longer periods (Plots P5–P10) DS areas become filled at a lower pace while the water input is nearly balanced by the high saturated conductivity at DS areas. After this period, infiltration in DS areas diminishes reaching a low, steady rate (Mao et al., 2008) and the runoff flow increases positively correlated with DS because no significant infiltration at DS areas occurs. This would result in the observed predominantly positive DS- ζ correlations.

The results from the experimental plots are also consistent with those of Esteves et al. (2000) and Parsons et al. (1990) on the occurrence of composite flows during runoff-infiltration events. In the modelling experimentation during this study, it was not possible to simultaneously reproduce the observed values of overland flow areas $A(t)$ and the observed $\theta_{\text{end}} - z_f$ values unless saturated infiltration conditions were assumed in DS areas. Assuming saturated infiltration conditions overall the laminar flow area resulted in complete depletion of the overland water stock in all cases. This was contrary to the observed laminar flow expansions. Conversely, assuming unsaturated conditions overall the wetted soil area would not account for the observed $\theta_{\text{end}} - z_f$ values. A balance of saturated-unsaturated flows apportioned between DS and other areas as estimated in this study yields concordant results with observations, as shown in Table 2.

5.2 Estimation of overland flow parameters

Equations (16)–(19) show a set of statistical relations between characteristics of the infiltration-overland flow processes and measurable parameters in the context of plot experiments. The Darcy's formulation of a friction law in the form of Eq. (10) allows the estimation of the average overland flow depth d^* (Fig. 2) once the frictional term C is estimated through inverse modelling of the continuity (Eqs. 1–3). Values $d(t) - d^*$ obtained in this study are in a similar scale as those reported by Esteves et al. (2000) and instrumentally measured by Dunkerley (2001) in laboratory conditions.

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The analysis of Eq. (16) indicates that the meaning of C exceeds that of the strictly frictional term (σ_S) but also depends on the interaction of the ratio $\gamma_{A/W}$ and S^* . The former is a measure of the infiltration rate at the plot scale and the second is a measure of the effect of gravitational forces. Since infiltration introduces inertial drag on overland flow, Eq. (16) is (in statistics sense) an empirical estimate of the ratio of inertial to gravity forces on the overland flow. This is further confirmed through the high correlation observed between C and the Froude number (Table 3b, Fig. 6a). No significant correlations between C or Re and the Darcy-Weisbach's frictional coefficient f which is usually employed in overland flow modelling were observed. Abrahams et al. (1990) working on sparsely vegetated desert hill slopes similar to those at our plots, observed that the relation of this coefficient with the Reynolds number varied during the inundation of roughness elements and the variations in overland flow depth.

Equations (17)–(19) allow computing the runoff coefficient ζ , the depth d^* and the overland flow velocity v^* from field and modelling data. The relation in Eq. (18) – Fig. 6b deserves further attention, since the water kinematic viscosity involved in the computation of the Reynolds number changes by about 300% within inter-seasonal temperature ranges observable at the soil surface in the Patagonian Monte and similar areas with semi-arid/arid regime. This implies that estimates of d^* and resulting overland flows usual in hydrological practice in this type of environments should take account of seasonal temperature conditions at the site. This may probably apply to any modelling effort of overland flow processes that would need estimates of overland flow depth.

6 Conclusions

This study gives the following findings about the simultaneous occurrence of water infiltration and overland flow in conditions of arid regime at the Patagonian Monte: (1) overland flow velocities as well as infiltration-overland flow mass balances are consistently modelled by considering variable infiltration rates corresponding to depression storage and/or non-ponded areas. (2) Several statistical relations are presented which

can be used to estimate relevant parameters describing overland flow in arid-semiarid environments. These estimates allow the computation of theoretical hydrodynamics parameters (Chezy's frictional C , average overland flow depth d^*) through measurable characteristics of the surface soil and overland flow. (3) A protocol of field experiments and coupled time-distributed modelling to 1–2 above is described.

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Table 1. Parameters and variables of soil surface and upper vadose and convergence criteria used in validating a model of the field plot experiments. ANN CI: Confidence interval of the ANN estimation process; r : correlation coefficient, linear regression model-data values ($x_{\text{model}} = a + b \times x_{\text{data}}$; $H_0: a = 0, b = 1$) over all plot experiments. ($A(t)$): convergence evaluated at 10–15 times during the water inflow period.

Parameter	Convergence criteria	Compartment
1 Sand (%)	Data value	Soil
2 Silt (%)	Data value	Soil
3 Clay (%)	Data value	Soil
4 θ_{sat}	Data value	Soil
5 θ_r	Data value	Soil
6 $\theta_{\text{antecedent}}$	Data value	Soil
7 Water inflow W ($\text{mm}^3 \text{s}^{-1}$)	Data value	Surf. soil
8 Water inflow period (s)	Data value	Surf. soil
9 K_s (mm s^{-1})	ANN CI – r^{**}	Soil
10 Wet Area ($A(t)$) (mm^2)	$\text{EC} \geq 0.99^a$	Surf. soil
11 Total Wet Area (A^*) (mm^2)	r^{***}	Surf. soil
12 Average runoff speed (mm s^{-1})	r^{***}	Surf. soil
13 Average S (mm)	r^{***}	Surf. soil
14 Average DS area (%)	r^{***}	Surf. soil
15 θ_{end}	r^{***}	Soil
16 z_f (mm)	r^{***}	Soil
17 Water mass balance error (mm^3)	$e < 1 \times 10^{-6} \text{mm}^3$	Soil

^a Nash and Sutcliffe, 1970.



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Table 2. Experimental results and model parameter values.

Parameter, variable	P1	P2	P3	P4	Plots						<i>r</i>	CI ^a
					P5	P6	P7	P8	P9	P10		
1 Sand (%)	80.7	80.8	74.2	71.2	68.4	76.2	70.4	65.5	73.6	76.8		
2 Silt (%)	12.0	10.3	13.8	14.4	14.9	17.3	15.6	20.1	10.4	10.4		
3 Clay (%)	7.3	9.2	12.0	14.4	16.7	6.5	14.0	14.4	16.0	12.8		
4 θ_{sat}	0.38	0.38	0.39	0.38	0.38	0.39	0.38	0.38	0.38	0.38		
5 θ_r	0.04	0.05	0.05	0.05	0.04	0.04	0.05	0.05	0.05	0.05		
6 $\theta_{\text{antecedent}}$	0.06	0.05	0.06	0.11	0.05	0.04	0.05	0.05	0.06	0.06		
7 Water inflow W (mm ³ s ⁻¹)	4500	4500	4500	4500	308	442	450	408	500	225		
8 Water inflow period (s)	72	96	48	72	1128	1026	714	660	792	852		
9 K_s (mm s ⁻¹)	1.1E-02	1.1E-02	8.2E-03	5.8E-03	5.3E-03	9.2E-03	1.2E-03	2.9E-03	4.5E-03	6.5E-03	0.865	9.0E-02
	1.1E-02	9.7E-03	5.7E-03	4.7E-03	6.2E-03	8.2E-03	4.2E-03	3.6E-03	4.4E-03	5.9E-03		
10 Wet Area (<i>A(t)</i>) (mm ²)	0.996^b	0.994	0.996	0.997	0.990	0.990	0.994	0.995	0.999	0.998		
11 Total Wet Area (<i>A'</i>) (mm ²)	1.6E+05	3.7E+04	5.2E+04	1.4E+05	5.3E+04	5.4E+04	1.1E+05	7.4E+04	7.3E+04	3.2E+04	0.999	2.5E-04
	1.6E+05	3.6E+04	5.1E+04	1.4E+05	5.5E+04	5.3E+04	1.1E+05	7.5E+04	7.3E+04	3.2E+04		
12 Average run of speed (mm s ⁻¹)	5.55	1.96	4.77	5.24	0.20	0.23	0.47	0.41	0.34	0.21	0.999	1.7E-04
	5.56	1.98	4.73	5.25	0.21	0.22	0.47	0.42	0.34	0.21		
13 Average <i>S</i> (mm)	0.3	1.8	4.5	1.8	1.2	2.7	0.5	1.0	2.1	0.3	0.971	2.5E-02
	0.2	1.6	4.6	1.5	1.5	3.4	0.7	1.0	1.8	0.3		
14 Average DS area (%)	9.1	2.7	2.9	2.7	4.7	7.6	7.3	1.0	10.1	14.7	0.941	2.2E-02
	11.1	1.4	2.9	1.5	5.2	6.4	9.3	1.5	10.0	12.3		
15 θ_{end}	0.19	0.21	0.19	0.2	0.18	0.2	0.13	0.13	0.19	0.17	0.985	2.2E-02
	0.19	0.21	0.19	0.2	0.18	0.20	0.13	0.12	0.20	0.17		
16 z_f (mm)	14	12	13.5	12	36	43	24	35	34	42	0.968	1.6E-02
	23	21	24	27	47	53	28	42	45	55		
17 Water mass balance error (mm ³)	6.E-11	-1.E-09	2.E-10	2.E-10	-1.E-07	6.E-10	6.E-10	2.E-10	-3.E-10	-3.E-10		

All model estimates in bold.

^a Confidence interval (**) of the mean of 5 random samples out of 10 values.

^b Nash-Sutcliffe's Efficiency Coefficient (see also Fig. 5).

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Table 3. Froude (F), Reynolds (Re), Friction (Darcy-Weisbach's f) numbers and C , d^* parameter values of the field plot experiments.

	P1	P2	P3	P4	P5	P6	P7	P8	P9	P10
1 F^a	0.102	0.006	0.033	0.064	0.001	0.001	0.004	0.006	0.003	0.002
2 Re^b	7.407	101.775	45.758	16.217	2.186	3.715	2.582	0.840	1.726	1.098
3 f^c	4.96E-01	2.85E+03	1.48E+02	8.95E+00	2.40E+04	6.55E+04	7.11E+02	9.10E+02	6.19E+03	3.10E+03
4 C	1.05E+05	4.80E+03	7.60E+03	2.90E+04	1.00E+03	8.60E+02	3.80E+03	3.50E+03	2.00E+03	2.70E+03
5 d^*	0.300	11.400	2.160	0.690	2.410	3.700	1.250	0.480	1.150	1.170

^a $F = v / (g \times d^*)^{0.5}$; $g = 9806.65 \text{ mms}^{-2}$

^b $Re = 4 \times v^* \times d^* / \nu$; $\nu = 0.9 \text{ mm}^2 \text{ s}^{-1}$

^c $f = 8 \times g \times d^* / S^*$

Correlation table					
	F	Re	f	C	d^*
F	1.00				
Re	-0.01	1.00			
f	-0.33	-0.21	1.00		
C	0.93	-0.09	-0.25	1.00	
d^*	-0.31	0.88	0.15	-0.27	1.00

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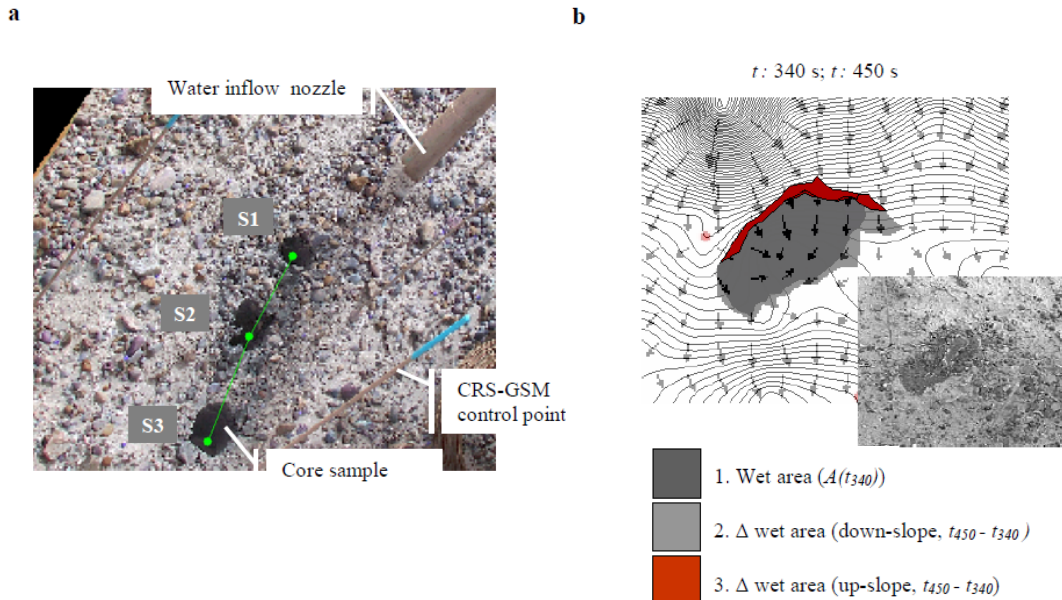


Fig. 1. (a) Overhead view of field plot at the end of the water inflow period. Soil cores along the main axis of the overland flow area are already extracted. Flow occurred from S1 (the irrigated spot) to S3. Spikes with control marks for CRS-GSM are shown. **(b)** Example (Plot #10) of the procedure used to estimate the progression of Depression Storage (DS) areas. Video scenes (lower-right inset) obtained at times 340 s and 450 s after water inflow start were overlaid on the plot DEM (contour interval: 1 mm) and flow arrows (intensity of flow proportional to arrow length). Sectors where the overland flow advances against local up-slope are considered DS areas.

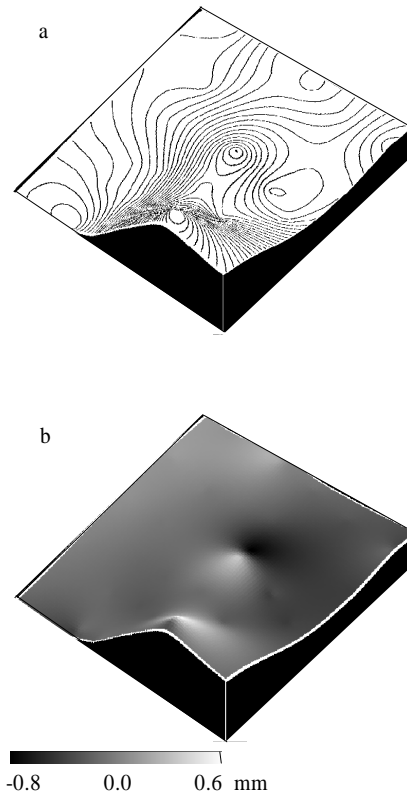


Fig. 2. (a) 3-D view of a DEM of 800 × 800 mm calibration plot with draped height contours (interval: 1.5 mm, maximum height $z = 77$ mm). (b) 3-D view of the same DEM with draped image of the spatial distribution of the standard error in z estimates, respect the value obtained with optical level-topography staff method.

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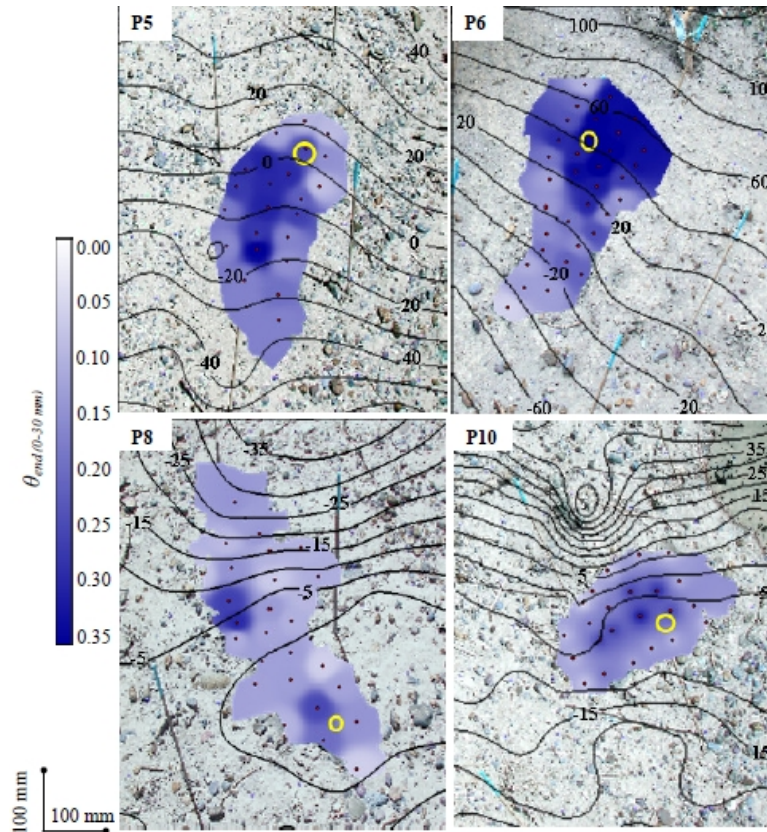


Fig. 3. Upper soil moisture (θ_{end} , 0–30 mm) maps of the overland plumes based on TDR probing (red dots) at the end of the water inflow periods at four plots in this study. Yellow circles indicate the water inflow areas. Maps overlay rectified plot photos and corresponding level (mm) contour lines.

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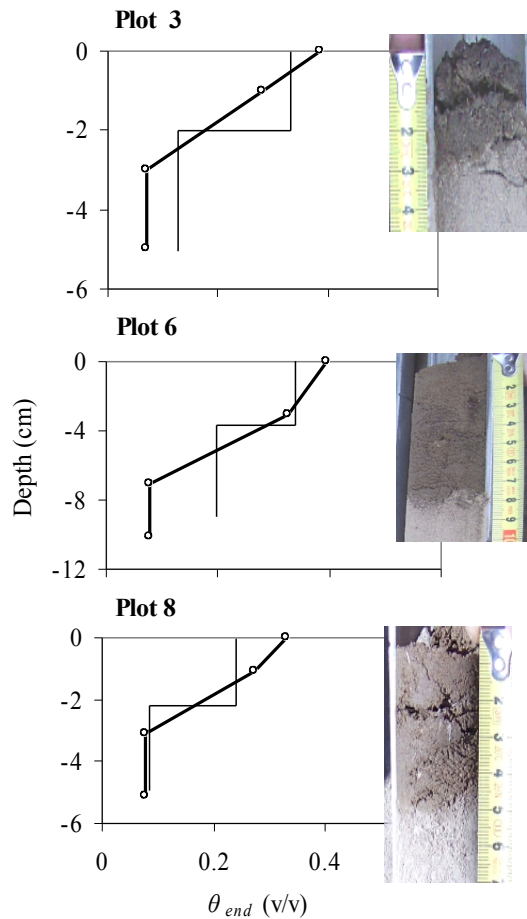


Fig. 4. Comparison of z_f (photo insets) and θ_{end} field soil core data (thin lines) used to validate the model in this study and estimates through CHEMFLO-2000 (thick lines).

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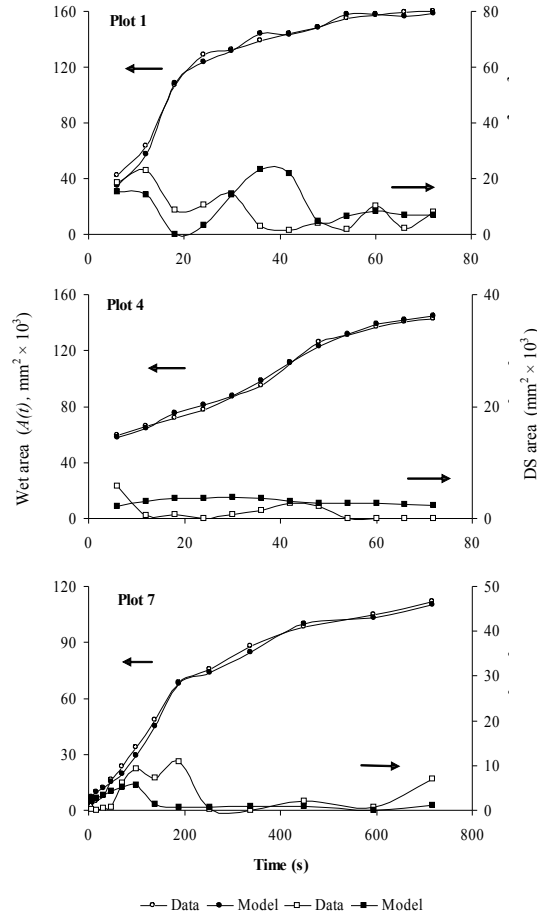


Fig. 5. The progression of overland flow areas $A(t)$ and DS during the water inflow periods in three experimental field plots as estimated from rectified video imagery and the model used in this study.

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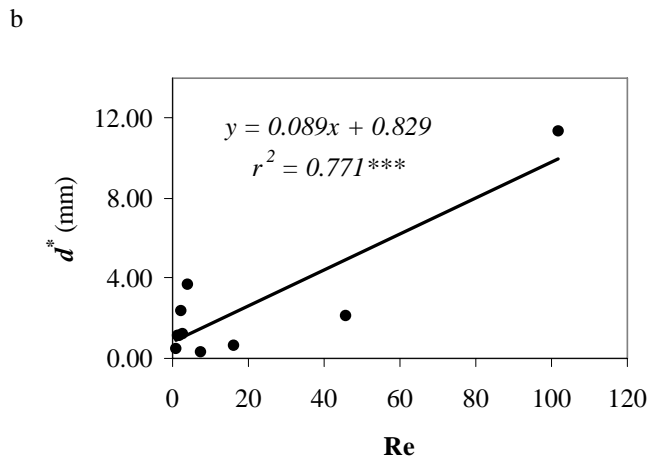
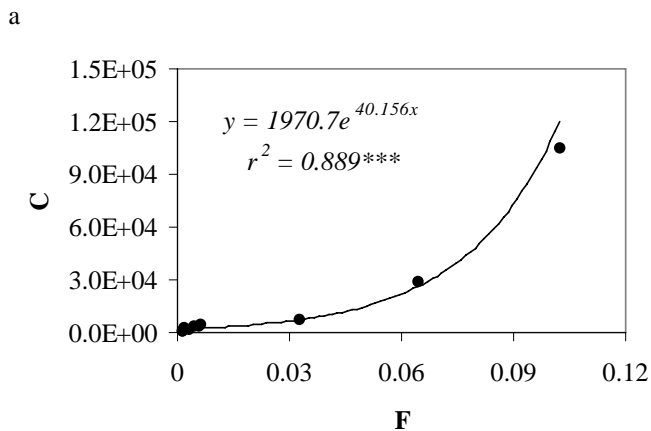


Fig. 6. Inter-plot statistical relations between parameters C - F and d^* - Re .

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