

1 **Stochastic rainfall analysis for storm tank performance**
2 **evaluation**

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8 **Response to Referee Comment RC-C804 – Remko Uijlenhoet (Referee)**

9 On behalf of co-authors, I thank gratefully Remko Uijlenhoet for his constructive and useful
10 comments. Then, here are the responses for specific referred issues. This response completes
11 the one given by co-author A. Montanari to the general remark of the interactive comment.

12

13 SPECIFIC REMARKS

14 **P.1855, 1.21-22 / P.1855, 1.22-23.** This sentence will be reformulated in the revised version of
15 the paper taken into account these remarks.

16 **P.1859, Eq(9).** It is true that an assumption incorporated in this expression is that rainfall is
17 spatially uniform over the considered urban catchment.

18 Historical record series at Valencia show some spatial variability, especially along the
19 orthogonal direction to the seafront. But with respect to the application of the model to storm
20 tank sizing, it should be noticed that catchments managed by them should not be very large to
21 achieve an optimal management. The case study presented in the paper corresponds to a 500
22 m x 1400 m area, approximately rectangular shaped (68.8 ha), where the assumption of no
23 spatial variability is very reasonable.

24

25 **P.1862, 1.7-14.** As different selection criteria are used, they do not fit the optimal value
26 simultaneously. A plausible range for s_{crit} is thus identified and finally, a value in its lower
27 bound selected in order to increase the sample size of rainfall events.

1 The critical interevent time s_{crit} allows identifying and defining statistically independent
2 rainfall events; indeed, s_{crit} has not in fact a physical interpretation, it is only a statistical
3 criterion.

4 One of the main purposes of the paper is to show that with a relatively simple model, good
5 accuracy is achieved while results from the analytical model are compared against continuous
6 simulation ones.

7

8 **P.1862, I.22.** As indicated in the paper, some authors (see, for instance, Adams and Papa,
9 2000 – referred in the original manuscript) dealt with this issue, concluding that values similar
10 to those obtained by the authors are not “appreciable” enough to consider a bivaried model.

11 Moreover, the dependence between v and d is not really significant for the results achieved in
12 the paper since only a volumetric analysis based on V is performed. Results correspond to the
13 case $Q_v=0$, i.e., no flow is derived to the plant during the event, which corresponds to the
14 most precautionary situation in order to obtain the tank efficiencies. In this case, the event
15 duration d do not play any role in the derivation of the tank efficiencies.

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17 **P.1863, I.3.** The analytical development presented is based on the choice of the Pareto model
18 for rainfall depth description. As a consequence, if another pdf fits better this variable, the
19 resulting analytical expressions would be different.

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21 **P.1864, I.11-17.** The variability of expected values of runoff and overflow volumes is not
22 presented in the paper, as the purpose is to evaluate long term efficiencies based on the
23 average response of the storm tank through the year. Nevertheless, the obtained analytical
24 expressions could also allow exploring this possibility.

25

26 **P.1864, I.18.** The data set used to perform the continuous simulation is the same as the one
27 used to estimate the analytical model parameters because the aim of the procedure is to
28 validate the accuracy of this latter towards the system response. As indicated in the paper and
29 in previous interactive comments, the hydraulic and hydrological parameters of the model
30 were previously calibrated at Valencia.

1 EDITORIAL REMARKS

2 All the editorial remarks will be considered for the revised manuscript version.