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# Self-potential investigations of a gravel bar in a restored river corridor

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## Abstract

Self-potentials (SP) are sensitive to water fluxes and concentration gradients in both saturated and unsaturated geological media, but quantitative interpretations of SP field data are often hindered by the superposition of different source contributions and time-varying electrode potentials. Self-potential mapping and close to two months of SP monitoring on a gravel bar were performed to investigate the origins of SP signals at a restored river section of the Thur River in northeastern Switzerland. The SP mapping and subsequent inversion of the data indicate that the SP sources are mainly located in the upper few meters in regions of soil cover rather than bare gravel. Wavelet analyses of the time-series indicate a strong, but non-linear influence of water table and water content variations, as well as rainfall intensity on the recorded SP signals. Modeling of the SP response with respect to an increase in the water table elevation and precipitation indicate that the distribution of soil properties in the vadose zone has a very strong influence. We conclude that the observed SP responses on the gravel bar are more complicated than previously proposed relationships between SP signals and hydraulic head or the thickness of the vadose zone. We suggest that future SP monitoring in restored river corridors should either focus on quantifying vadose zone processes by installing vertical profiles of closely spaced SP electrodes or by installing the electrodes within the river to avoid signals arising from vadose zone processes and time-varying electrochemical conditions in the vicinity of the electrodes.

## 1 Introduction

The self-potential (SP) method is a passive geophysical method, in which natural spatial and temporal variations in the electrical potential field are measured on the surface of the earth or in boreholes. The resulting SP maps and monitoring data are sensitive to flow processes in the sub-surface (e.g., Doussan et al., 2002; Rizzo et al., 2004; Suski et al., 2006), but interpretation of field measurements is complicated. First, accurate

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modeling of SP responses (to given source currents) can only be achieved when detailed knowledge about the electrical conductivity distribution is available (see Chapter 4 in Minsley, 2007). Second, the self-potential method is a potential field method and the inverse problem of retrieving the time-evolving source-current distribution in the subsurface is plagued by non-uniqueness. Even if the electrical conductivity distribution of the sub-surface is known, there exist an infinite number of source current distributions that can explain the data equally well. Third, many different processes can create SP signals and it is often unclear which source types that will dominate the response at a given site. This leads to a situation in which the SP method is useful on a case-by-case basis and that its applicability at a certain site can often only be reliably assessed after the data have been acquired. Interpretation is further complicated by electrode responses that are affected by temperature variations and non-linear drift terms that are related to electrode design and age, as well as changing electrochemical conditions in the vicinity of the electrodes.

Despite the limitations mentioned above, the SP method continues to receive considerable interest in hydrogeology as SP data are sensitive to contaminant transport (e.g., Maineult et al., 2004, 2005; Revil et al., 2009), redox processes (e.g., Linde and Revil, 2007), flow in saturated (e.g., Bolèvre et al., 2009) and unsaturated porous media (e.g., Thony et al., 1997; Doussan et al., 2002; Linde et al., 2007a), flow in fractures (e.g., Wishart et al., 2006), the water table elevation (e.g., Fournier, 1989; Revil et al., 2003), or the thickness of the vadose zone (Aubert and Yéné Atangana, 1996), etc. For example, the SP method might potentially be used to estimate water fluxes in the vadose zone. Such fluxes are difficult to measure in the field and are commonly inferred indirectly by differencing water content measurements over time (e.g., Vereecken et al., 2008). This richness of application areas mirrors the main limitation of the SP method; many different processes contribute to the measured response.

Several research groups have recently made significant investments in SP monitoring networks in river environments, for example, to study fluctuations in the water table in the vicinity of Columbia River, Washington (Timothy Johnson, personal

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communication, 2010) and to optimize pumping rates for bank filtration at the Russian River, California by combining SP data with multiphase flow modeling (Gasperikova et al., 2008).

To decrease non-uniqueness in the interpretation of SP data, a promising approach is to treat the data as being dependent on the state of hydrogeological and geochemical variables. This makes it possible to use SP data with other data (e.g., time-series of hydrological head and tracer concentrations) to constrain hydrological boundary conditions, permeability structure, or vadose zone flow properties within a hydrogeological inverse modeling framework. This is only possible if the source contributions of different hydrological processes are accurately modeled, which highlights the importance of having access to different types of hydrological and geophysical data.

Self-potential source generation and the modeling of the resulting SP field are well understood under saturated conditions (Sill, 1983; Sheffer and Oldenburg, 2007). A better understanding for multiphase conditions has developed in recent years (e.g., Linde et al., 2007a; Revil et al., 2007), but there is still room for improvements. Nonetheless, it is quite clear that no general linear relationships exist between unsaturated water flux and SP gradients as suggested by Thony et al. (1997) or between the local hydraulic pressure gradient and SP gradient as suggested by Darnet and Marquis (2004).

A good understanding of physical processes at a given site, together with advanced signal processing and modeling, appears to be the only way to reliably assess the origins of the dominant contributions to the measured signals. To assess the sensitivity of SP data to hydrological processes at a restored river corridor, we performed SP mapping and monitoring on a gravel bar within a restored reach of the Thur River in northeastern Switzerland. Our first results illustrate the SP mapping and subsequent inversion of these data. We then perform an exploratory analysis of SP and hydrological time-series using wavelet analysis. For suitable periods, we compare SP time-series with those of water table fluctuations, precipitation events, moisture content, and temperature. We then model, for a simplified geological model, the expected

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response related to the two candidates that are the most likely source of the observed SP variability, namely (1) fluctuations in the water table and (2) infiltration following precipitation events.

Understanding the origins of SP signals at this site is important as one could make inferences about the hydraulic diffusivity of the aquifer if the water table effect dominates or one could obtain information about soil properties if infiltration processes dominate. SP data could then be used to evaluate the effect of river restoration on hydrological subsurface processes. Another more general motivation is that better understanding of near-surface SP sources can help to remove SP signals of shallow origin when investigating deeper phenomena such as volcanic activity (Friedel et al., 2004), earthquake precursors (Park, 1983), or when processing magnetotelluric data (Perrier and Morat, 2000).

## 2 Methods

### 2.1 Thur River field site

The Thur River in northeastern Switzerland (see Fig. 1) is the largest Swiss river without natural or artificial reservoirs. It is a peri-alpine tributary of the River Rhine with a catchment area of about 1750 km<sup>2</sup>. River hydrology shows a typical nivo-pluvial regime. Water level and discharge variations in the Thur are characteristic of unregulated alpine rivers where neither lakes nor reservoirs attenuate the discharge. The Thur aquifer consists mainly of glacio-fluvial sandy gravels (5–7 m thick in our study area; average hydraulic conductivity  $5 \times 10^{-3}$  ms<sup>-1</sup>) overlaying lacustrine clays. The top of the aquifer is formed by fine alluvial sediments (fine-sand with silt). Like many other rivers, the meandering Thur was channelized at the end of the 19th century for flood protection and to gain arable land (Lacey, 1930; Brookes, 1988). In an attempt to combine flood protection with ecological objectives, a more natural river environment was restored at a 5 km long reach of the Thur at Neunforn, starting in 2002. The

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effects of these restoration efforts are currently being investigated within the RECORD project (for details see <http://www.cces.ethz.ch/projects/nature/Record>; Schneider et al., 2010).

While the channelized river was practically flowing straight prior to restoration, the riverbed morphology has changed dramatically since the removal of the northern bank stabilization and overbanks in 2002 (Trush et al., 2000; Soar and Thorne, 2001; Pasquale et al., 2010). This large widening forced the river to deposit its sediments in a typical alternate bar pattern (Tubino and Seminara, 1990). By 2005, one of those gravel bars (see Fig. 2a) had developed on the northern shore of the river with a surface exposure that depends strongly on the varying river discharge  $Q$ . At low flow conditions ( $Q \sim 20 \text{ m}^3 \text{ s}^{-1}$ ) a low-lying part with bare gravel at the surface is exposed, at intermediate flow conditions ( $Q \sim 100 \text{ m}^3 \text{ s}^{-1}$ ) this part is flooded and the exposed surface of the gravel bar consists mainly of gravel covered by up to 1 m thick recent deposits of sandy loam colonized mainly by canary reed grass (*phalaris arundinacea*), while the entire gravel bar is flooded at high-flow conditions ( $Q > 200 \text{ m}^3 \text{ s}^{-1}$ ). This gravel bar is the focus of the SP experiments presented here.

Ten piezometers instrumented with loggers (temperature, electrical conductivity, and pressure with a sampling rate of 15 min) were installed on the gravel bar to investigate river-groundwater interactions (Schneider et al., 2010; Vogt et al., 2010a,b). Sensors for water content (Decagon EC-5 and EC-TM) and temperature (Decagon EC-TM) were installed at different locations at 0.1, 0.5 and 1 m depth in the soil. Data were acquired with a sample rate of 30 min using a Decagon EM50 data logger (Decagon Devices Inc., Pullman, WA, USA). Raw values of soil dielectric permittivity were converted to volumetric water content using specific calibration curves for the soils at the site. A complete meteorological station (Campbell Scientific) was installed to monitor micrometeorological variables (Pasquale et al., 2010). The meteorological station includes two sensors for air temperature and relative humidity with a 10 min sampling rate (at 2.5 m and 8 m above soil level), a complete solar radiation device, two wind flow meters and an atmospheric pressure sensor. A pluviometer (OTT) with a sampling rate

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of 1 min completes the station. Ground penetrating radar (GPR) and electrical resistivity tomography (ERT) geophysical data have also been acquired on the gravel bar to delineate the subsurface aquifer structure (Doetsch et al., 2010).

## 2.2 Governing SP equations

5 The total electrical current density  $j$  ( $\text{A m}^{-2}$ ) is given by Sill (1983)

$$j = \sigma E + j_s, \quad (1)$$

$$\nabla \cdot j = 0, \quad (2)$$

where  $\sigma$  ( $\text{S m}^{-1}$ ) is the electrical conductivity,  $E$  ( $\text{V m}^{-1}$ ) is the electric field  $E = -\nabla\varphi$ ,  $\varphi$  (V) is the electrical potential, and  $j_s$  ( $\text{A m}^{-2}$ ) is the source current density. Equation (1) is a generalized Ohm's law and Eq. (2) is the conservation equation in the low-frequency limit of Maxwell's equations. These two equations can be combined for any electrical conductivity distribution and boundary conditions to solve for the distribution of  $\varphi$  given knowledge of  $j_s$ . The total source current density for the three dominating SP sources in hydrological applications can be described by Revil and Linde (2006) and Arora et al. (2007)

$$j_s = \bar{Q}_v \mathbf{u} - k_b T \sum_{i=1}^N \left( \frac{t_i \sigma}{q_i} \right) \nabla \ln \{i\} - \sigma \nabla E_h, \quad (3)$$

where  $\bar{Q}_v$  ( $\text{C m}^{-3}$ ) is the effective charge per unit volume that can be dragged by the flow of the pore water;  $\mathbf{u}$  is the Darcy velocity ( $\text{m s}^{-1}$ ),  $k_b$  ( $1.381 \times 10^{-23} \text{ J K}^{-1}$ ) is the Boltzmann constant,  $T$  (K) is the temperature,  $q_i$  (C) is the charge of ionic species  $i$  dissolved in the water,  $t_i$  (–) is the corresponding microscopic Hittorf number (i.e., the fraction of electrical current carried by species  $i$  in the water phase),  $\{i\}$  is the corresponding activity, and  $E_h$  (V) is the redox potential.

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The first contribution is associated with the drag of excess charge, in the diffuse Gouy-Chapman layer within the electrical double layer, by the movement of the pore water. This contribution forms the streaming current and the formulation is valid under both saturated and unsaturated conditions. The second contribution is related to chemical gradients in the pore water that gives rise to diffusion currents. The third contribution is related to redox processes.

Revil and Leroy (2004) relate  $\bar{Q}_v$  at saturation to the voltage coupling coefficient at saturation  $C_{\text{sat}}$  ( $\text{V Pa}^{-1}$ ) through

$$C_{\text{sat}} = -\frac{\bar{Q}_v k}{\mu_w \sigma_w}, \quad (4)$$

where  $k$  is the permeability ( $\text{m}^2$ ),  $\mu_w$  is the dynamic water viscosity ( $\text{Pa s}$ ), and  $\sigma_w$  ( $\text{S m}^{-1}$ ) is the electrical conductivity of the pore water. Experimental data suggests that  $C_{\text{sat}}$  is largely controlled by  $\sigma_w$  (Revil et al., 2003) and laboratory measurements of aquifer material are straight-forward (Suski et al., 2006). Jardani et al. (2007) present experimental data that suggest that  $\bar{Q}_v$  decreases with increasing hydraulic conductivity. Linde et al. (2007a) suggested that  $\bar{Q}_v$  under unsaturated conditions should to a first-order scale inversely with the water saturation  $S_w$  as

$$\bar{Q}_v (S_w) = \frac{\bar{Q}_v (S_w = 1)}{S_w}. \quad (5)$$

This parameterization is similar to the one of Waxman and Smits (1968) for a closely related parameter. This relationship is also discussed in Revil et al. (2007) and has been used by Jougnot et al. (2010).

SP measurements are performed with respect to the electrical potential of a reference electrode. The measured SP data  $\varphi_i^{\text{meas}}(t)$  of electrode  $i$  at time  $t$  is given by

$$\varphi_i^{\text{meas}}(t) = (\varphi_i(t) + \varphi_i^{\text{elec}}(t)) - (\varphi_{\text{ref}}(t) + \varphi_{\text{ref}}^{\text{elec}}(t)), \quad (6)$$

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where  $\varphi_i(t)$  and  $\varphi_{\text{ref}}(t)$  are SP responses related to the hydraulic and/or geochemical forcing defined in Eqs. (1–3). The measurements will also be affected by temporal variations in electrode coupling, electrode temperature, electrode age, and geochemistry and geology in the intermediate vicinity of each electrode. These behaviors are

described by  $\varphi_i^{\text{elec}}(t)$  and  $\varphi_{\text{ref}}^{\text{elec}}(t)$ . Despite important improvements in electrode design (Petiau, 2000), these drift terms often contaminate the long-term behavior of SP monitoring signals. Common practice is to make a linear drift correction based on SP measurements performed with the two electrodes in contact with each other in the beginning and end of the monitoring period.

## 10 2.3 SP inversion

The SP inverse problem consists of determining the position and magnitude of SP sources that can explain the observed data within the measurement errors for a given electrical conductivity distribution, while honoring any constraints on the source distribution. The SP field can be calculated by inserting Eq. (1) in Eq. (2) to get

$$15 \quad \nabla \cdot \sigma \nabla \varphi = s, \quad (7)$$

where  $s$  ( $\text{A m}^{-3}$ ) is a source distribution term given by  $\nabla \cdot j_s$ . For a given conductivity distribution and boundary conditions, Eq. (7) can be expressed as

$$K \varphi = s, \quad (8)$$

where  $K$  is a sparse linear matrix operator that contains all information about the dimensions of the model, the electrical conductivity distribution, and boundary conditions, while  $\varphi$  is the electrical potential and  $s$  the source term at all model cells.

The inversion recovers a source model that minimizes the error between the measured  $\varphi^{\text{obs}}$  and predicted SP response in a least-squares sense. The objective function

$$41 \quad \psi(s) = \left| \varphi^{\text{obs}} - P K^\dagger s \right|_2 + \lambda |W s|_2, \quad (9)$$

5 consists of a data misfit term and a model regularization term, where  $\mathbf{P}$  is a selector matrix that picks out the rows of  $\mathbf{K}^\dagger$  (i.e., the inverse of  $\mathbf{K}$ ) that correspond to a SP measurement location where a potential was measured. The regularization weight  $\lambda$  controls the trade-off between the data misfit and the source current distribution, here quantified by the norm of the regularization operator  $\mathbf{W}$  acting on the source model

10 The operator  $\mathbf{W}$  incorporates inverse sensitivity scaling information that accounts for rapidly decaying sensitivities with distance from the measurement locations. Additionally,  $\mathbf{W}$  promotes source solutions that are spatially sparse (Portniaguine and Zhdanov, 1999; Minsley et al., 2007); that is, it favors solutions with the fewest number of non-  
15 zero source amplitudes while still fitting the data. This sparsity constraint is non-linear and requires an iteratively re-weighted least-squares (IRLS) inversion approach. The first iteration of the IRLS inversion corresponds to the sensitivity-weighted minimum length solution, which results in models that display very smoothly varying source current distributions even if the true source locations have a limited volumetric extent. In subsequent iterations,  $\mathbf{W}$  is updated with weights that favor models that occupy a small volume instead of being spatially smooth. We refer to Minsley et al. (2007) for more details about SP source current inversion,

## 2.4 Wavelets

20 Exploratory analysis of SP field data should consider that there are many different processes that can create measurable SP signals at different temporal and spatial scales. A very useful and widely used approach to analyze non-stationary geophysical time-series is wavelet transforms (e.g., Kumar and Foufoula-Georgiou, 1997; Torrence and Compo, 1998; Grinsted et al., 2004; Henderson et al., 2009). In contrast to Fourier transforms that focus on the frequency content of a given time-series, wavelet transforms 25 describe the temporally varying frequency content of a signal. They can also be used to evaluate how the frequency components of two different signals relate to each other over time. It appears thus that wavelet analyses could be useful to disentangle the relative contributions of the different source currents to the observed SP signals. In

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one of the rare wavelet applications to SP monitoring data, Friedel et al. (2004) analyzed data acquired at Merapi Volcano, Indonesia and found that many of the observed anomalies were associated with precipitation events.

A short description of wavelet theory is given below. We refer to Torrence and Compo (1998) and Kumar and Foufoula-Georgiou (1997) for a more detailed treatment and primary references. The wavelet used in this work is the commonly used complex valued Morlet wavelet

$$\psi_0(\eta) = \pi^{-1/4} e^{-i\omega_0\eta} e^{-\eta^2/2} \quad \omega_0 \geq 5, \quad (10)$$

where  $\eta$  is a non-dimensional time and  $\omega_0$  is the non-dimensional frequency. We use  $\omega_0 = 6$  as it provides a good balance between time and frequency localization (Grinsted et al., 2004). The continuous wavelet transform (CWT) of a discrete sequence  $x_n$  with a uniform time-sampling  $\delta t$  is defined as the convolution of  $x_n$  with a scaled and translated version of  $\psi_0(\eta)$  as

$$W_n^X(s) = \sum_{n'=0}^{N-1} x_{n'} \psi^* \left[ \frac{(n' - n) \delta t}{s} \right], \quad (11)$$

where  $(^*)$  indicates the complex conjugate. By varying the wavelet scale  $s$  and translating along the localized time index  $n$  it is possible to construct an image of the amplitude as a function of scale (when  $s$  increases the wavelet becomes more spread out and takes only long-term behavior of  $x_n$  into account) and of how this amplitude varies with time. The wavelet in Eq. (11) is normalized at each scale to have unit energy (Torrence and Compo, 1998). The convolution of Eq. (11) is in practice solved using Fast Fourier Transforms. The CWT has edge artifacts and it is therefore useful to define a cone of influence that identifies the regions that are sensitive to such artifacts. To assess the reliability of any features in the CWT one needs to perform a statistical test with respect to the CWT of a random process, typically red noise that can be modeled with a first order regressive process. It is then possible to test at a 5% significance level if the null

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hypothesis that the observed signal constitute red noise holds. The resolved regions are then highlighted in the displayed CWT.

The wavelet transform offers also the possibility to create a filtered time-series  $x_n^{\text{filt}}$  between two scales  $j_1$  and  $j_2$  by

$$x_n^{\text{filt}} = \frac{\delta j \sqrt{\delta t}}{C_\delta \psi_0(0)} \sum_{j=j_1}^{j=j_2} \frac{\text{Re} \{W_n(s_j)\}}{\sqrt{s_j}}, \quad (12)$$

where  $C_\delta = 0.776$  is a reconstruction factor that is specific to the choice of wavelet,  $\psi_0(0) = \pi/4$ , and  $\delta j$  is the scale sampling (12 samples for each decade is used in this study).

As  $W_n^X(s)$  is complex valued, it is easier to display the real-valued wavelet power spectrum  $|W_n^X(s)|^2$ . To compare wavelet transforms of different time-series  $x_n$  and  $y_n$  one can calculate the cross-wavelet spectrum (WPS) as  $W_n^{XY}(s) = W_n^X(s)W_n^{Y*}(s)$ , where  $W_n^{Y*}(s)$  is the complex conjugate of  $W_n^Y(s)$  and  $|W_n^{XY}(s)|$  is the corresponding cross-wavelet power. The wavelet coherence is defined as

$$\Gamma_n^{XY}(s) = \frac{|W_n^{XY}|}{\sqrt{|W_n^X(s) W_n^Y(s)|}}, \quad (13)$$

which can be seen as a localized correlation coefficient that varies between 0 and 1 in time-frequency space. Equation (13) is always 1, which can be avoided when calculating the coherency by first smoothing the different contributions in time and space according to Torrence and Webster (1998). This modified coherence is the preferred measure for significance testing compared with the cross-wavelet power, which can display high values due to changes in one of the time-series only (Maraun and Kurths, 2004).

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### 3 Results

#### 3.1 Self-potential mapping

The SP mapping survey was carried out on 7 March 2008 using Pb-PbCl<sub>2</sub>-NaCl electrodes, so-called Petiau electrodes (Petiau, 2000). Figure 2a indicates the area that was surveyed. Measurement profiles were oriented perpendicular to the river shore with measurements every 3 m and a profile spacing of approximately 5 m (see dots in Fig. 2b). Five measurements were acquired in the vicinity of each measurement point and the median value was chosen for later processing. The measurements were performed at approximately 5 cm depth. The total data set consisted of 246 measurement points with respect to a reference electrode located in a loamy ditch. The data were corrected for a linear drift of 2.8 mV over time using five drift measurements (i.e., the SP signal is recorded when the measurement electrode is located close to the reference electrode) acquired during the day. For further interpretation, the reference (0 V) was assigned to the position indicated in Fig. 2b.

The data were detrended in the direction of water flow and interpolated using ordinary kriging (Deutsch and Journel, 1998). The kriging was performed with a spherical model that fitted the detrended experimental semi-variogram with a nugget of 3 (mV)<sup>2</sup>, an effective range of 18 m and a sill of 20 (mV)<sup>2</sup> (see Linde et al., 2007b for a more detailed description of kriging of SP data). Figure 2b displays the kriged map. The vegetated region with sandy loam soils shown in Fig. 2a have values in the range of –8 to –13 mV, whereas the regions in the lower-lying bare gravel have values close to 0 mV. One possibility is that the source regions are located within the finer and thicker soils instead of the exposed gravel, though another possibility is that the SP sources originate from groundwater flow and aquifer heterogeneity in the gravel aquifer itself that happen to coincide with the soil boundaries seen on the surface.

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## 3.2 SP inversion

We use the 3-D SP source current inversion method of Minsley (2007) to invert our SP mapping data to determine the depths and locations of the source currents (see Sect. 2.3). The inversion domain ( $112 \times 69 \times 14 \text{ m}^3$ ) neglected topography. The top 5 m were assumed to consist of saturated gravel with a resistivity of  $250 \Omega \text{ m}$ , whereas the underlying clay was assumed to have a resistivity of  $25 \Omega \text{ m}$  in accordance with results from ERT (Doetsch et al., 2010). The discretization was 1 m in the x- and y-direction, whereas it was 0.5 and 1.0 m in the vertical direction for the gravel and clay layers, respectively. The inversion was first carried out until the model reached a 10 data misfit of 1.3 mV before an additional iteration was carried out with compactness constraints.

Figure 2c and d display the magnitudes of the source currents between 1.5–2 m and 3.5–4 m depth, respectively. They clearly indicate zones of negative source currents that approximately cover the outer part of the soil-covered region of the gravel bar. The 15 maximum amplitudes of the source currents are found at 3 m depth. The magnitude of the source current decreases with deeper depths and is negligible at 5.5–6 m depth (Fig. 2e). The inversion results thus indicate that the source currents are found in the shallow subsurface at locations that mainly correspond to the soil-covered part of the gravel bar. Note that the SP inversion cannot resolve the accurate depth of the 20 sources due to the inherent non-uniqueness of the inverse problem and the simplicity of the resistivity model.

## 3.3 SP time-series and wavelet analysis

The self-potential monitoring was performed from 13 February until 11 April 2009 in the region indicated in Fig. 2a. The locations of the SP monitoring electrodes, piezometers, and soil monitoring sites (temperature and water content) are shown in Fig. 3. The 25 16 Petiau electrodes were installed at 20 cm depth from the surface. A thin layer of fine sediments was added to the electrode-soil interface to improve the electrical contact

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and the region around the electrode was wetted before the monitoring started. The reference electrode was located in a thick loam layer. We used a Campbell Scientific Inc. CR1000 data logger that measured and stored the voltage difference between the measuring electrodes and the reference electrode every 5 s. A 12 V battery was used to power the CR1000. The data were processed by median filtering and sampled every 15 min for subsequent analysis.

Figure 4a shows time-series of rainfall intensity during the monitoring period, in which two periods of more significant rainfall occurred between days 60–70 (first rainy period) and 82–87 (second rainy period). Figure 4b shows time-series of water content at 10 cm depth (see Fig. 3) with increases in water content corresponding to the rainfall periods. The hydraulic head data of Fig. 4c have a delayed response to the rainfall and display some uncorrelated events that are attributed to snowmelt and rainfall upstream. The soil temperature data at 10 cm depth (Fig. 4d) shows that the ground was partly frozen until day 57 followed by a successive warming of the soil with daily fluctuations of a few degrees. Selected SP data (Fig. 4e) show that the main periods of SP variability correspond to the periods of rainfall. One can also observe substantial long-term variations in SP11, which is most likely related to electrode drift. No SP data were acquired in the beginning of the second rainy period as interfering ERT geophysical measurements were acquired during this period.

The WPS of the precipitation data (Fig. 5a) display significant energy covering the shortest period of 30 min to a period of approximately two days. The WPS of the water content (Fig. 5b) shows a similar pattern, but less high-frequency content and limited response to the rainfall between days 45 and 55, possibly due to frozen ground conditions. The WPS of the hydraulic head (Fig. 5c) displays energy during the two main periods of rainfall, but there is also significant energy with periods longer than a day corresponding to variations in the hydrological conditions in the upstream region of the catchment. The WPS of the soil temperature data (Fig. 5d) shows only well-defined variability associated with daily fluctuations. Four different SP electrodes were chosen with WPSs that appear related to those discussed above. The WPSs in Fig. 5

of electrodes (Fig. 5e) SP5, (Fig. 5f) SP3, and (Fig. 5g) SP11 show that most of the higher frequency energy is found during the two rainy periods, whereas (Fig. 5h) SP2 has a different behavior and shows a rather significant daily variation.

The wavelet coherencies were calculated for the (Fig. 5i) rainfall intensity and SP5, (Fig. 5j) the water content and SP3, (Fig. 5k) hydraulic head and SP11, and (Fig. 5l) temperature and SP2 following Grinsted et al. (2004). The coherent events indicate in most cases time-varying phase relations. The more significant coherent periods with consistent phase relations occur for the precipitation and water content data for periods of 1 to 4 days during the two more rainy periods.

A close up of the second rainy period is shown in Fig. 6. The long-period contributions of more than two weeks have been removed using Eq. (12) to focus on short-term variability without superimposed long-term trends or drifts. The rainfall intensity, detrended water content, and detrended hydraulic head variations are shown in Fig. 6a–c. Detrended SP time-series are shown in Fig. 6d–i. It appears that (Fig. 6d) SP11 and (Fig. 6g) SP6 are very sensitive to the rainfall intensity, such that even short periods of rainfall within the rainy periods have clearly defined SP peaks. The variability in (Fig. 6e) SP3 shows a rather close resemblance with variations in water content, whereas (Fig. 6h) SP1 shows more of a gradual buildup of the SP signal over time with an abrupt decrease after the end of the rainy period. Finally, both (Fig. 6f) SP8 and (Fig. 6i) SP12 appear to be mainly correlated with the hydraulic head. These examples indicate that the dominating SP source-generating processes can vary dramatically within a rather small monitoring area.

### 3.4 SP modeling

An idealized geological model was created to investigate how SP signals are expected to vary with respect to a rising water table and precipitation (Fig. 7). Two simulations of each type are investigated. Tests 1 and 2 simulate a water table increase with a rate of  $10 \text{ cm h}^{-1}$  during one hour from an initial level  $H_0$  of 0.5 and 1.5 m depth, respectively. Tests 3 and 4 simulate two rainfall events with an intensity of  $0.3 \text{ cm h}^{-1}$  during 1 h with

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a constant water table at 0.5 m and 1.5 m depth, respectively. The simulation time is 2 h for all tests, where the hydrological events take place during the first hour and the relaxation of the SP signal is investigated in the second hour (Fig. 7b–d).

The model geometry (Fig. 7a) consists of an upper 1 m thick layer with gravel on the left side in which we investigate variations of the SP signal, and clay on the right side, in which we have placed our reference (i.e., the electrical potential is zero). The reference is located in the clay because it is the most stable location over time and because it corresponds to the field situation in which the reference was placed in the finest sediments that constitute loam. A uniform gravel aquifer is located between 5 1 and 9 m depth followed by a 20 m thick clay aquitard. The boundary conditions for the electrical problem are electrical insulation at all boundaries. Hydrological boundary conditions are no flow boundaries at the sides and imposed pressure at the clay-gravel aquifer interface for all simulations (Fig. 7c–d). A no flow boundary at the top of the gravel is defined for the increasing water table simulations, while a prescribed flux into 10 the gravel is prescribed for the precipitation experiment as displayed in Fig. 7b. No flow boundaries are imposed on top of the clay as it is assumed that infiltration is negligible 15 in the clay for the examples and time scales considered here.

The modeling is performed in 2-D using finite element calculations in COMSOL Multiphysics 3.5 with a mesh consisting of close to 10 000 triangular elements with specific 20 mesh refinement in the unsaturated zone. The electrical resistivity of the saturated gravel and clay is 300 and 25  $\Omega\text{-m}$ , respectively. Variations of the electrical resistivity are modeled by Archie's second law with the electrical resistivity increasing according to  $S_w^{-n}$  ( $n = 2$  in our simulations) with respect to saturated conditions. This is an inappropriate model for clay, but sensitivity tests show that it does not affect the simulation 25 results as the clay stays close to saturation for all simulations. We used the parameterization of van Genuchten (1980) for the relative permeability  $k_r$  and capillary pressure  $P_c$  functions as

$$k_r = \sqrt{S_e} \left[ 1 - \left( 1 - S_e^{1/m} \right)^2 \right], \quad (14)$$

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$$P_c = \frac{1}{\alpha} \left( S_e^{-1/m} - 1 \right)^{1-m}, \quad (15)$$

$$S_e = \frac{S_w - S_{wr}}{1 - S_{wr}}, \quad (16)$$

where  $S_w$  is the water saturation,  $S_e$  is the effective and  $S_{wr}$  is the residual water saturation, respectively, and  $m$  and  $\alpha$  ( $m^{-1}$ ) are soil-specific parameters. We used typical parameters for gravel and clay (e.g., Carsel and Parrish, 1988) as outlined in Table 1. The hydrological problem is solved with Richard's equation. It is assumed that  $\bar{Q}_v(S_w = 1)$  is  $0.48 \text{ C m}^{-3}$  for the gravel and  $28 \text{ C m}^{-3}$  for the clay following the experimental relationship of Jardani et al. (2007) for a saturated medium. We use Eq. (5) to model  $\bar{Q}_v(S_w)$ .

The SP signal for test 1 (Fig. 8a) displays a positive SP signal over time that is at its maximum after 1 h and then decreases slowly as a certain upward flow continue to occur in the vadose zone to reach hydrostatic equilibrium (Fig. 8b). At a given time, the SP signal is constant with depth throughout the part of the vadose zone in which no changes in water content occurs. No SP signals (Fig. 8c) occur in this region for test 2 for which the source currents at the water table are the same throughout the model. This effect occurs as the water table (Fig. 8d) is located in a region of uniform geological media. These results indicate that SP signals in the unsaturated zone only occur when there are lateral contrasts in the sediments in which the water table rises or where the water content changes with time. It appears thus that any empirical relationship between water table dynamics and SP signals will vary between electrode positions and that it might be highly nonlinear (e.g., no sensitivity at all until a contrast is reached). Heterogeneity will thus play a key role in determining the SP signals associated with water table fluctuations both in terms of determining the location and magnitude of source currents, but also in determining the electrical conductivity distribution that also strongly affects the SP magnitudes (see Eq. 7).

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Self-potential mapping makes it possible to investigate the overall SP field on the surface at high spatial resolution, but the interpretation needs to consider that contributions of SP sources located in the vadose zone might vary over short spatial and time scales. We suggest that SP mapping for hydrogeological purposes should be performed following a relatively long period (e.g., a week) of stable meteorological and hydrological conditions to minimize the effects of sources in the vadose zone.

SP monitoring can be useful to monitor hydrological processes and to better understand the interrelations between different processes and mechanisms of SP source generation. The monitoring is however severely affected by long-term electrode drifts and what appears to be a high sensitivity to electrochemical conditions in the close vicinity of the electrodes and possibly wetting conditions at the electrode tip. Repeated (e.g., weekly) SP mapping with measurements in the vicinity of the monitoring electrodes might be useful to remove some of the problems with monitoring data. Even so, the data acquired in this study raise doubts about the usefulness of slowly varying (e.g., periods of weeks) natural variations in SP signals when their amplitudes are in the range of some 10 mV and electrodes are placed in the vadose zone.

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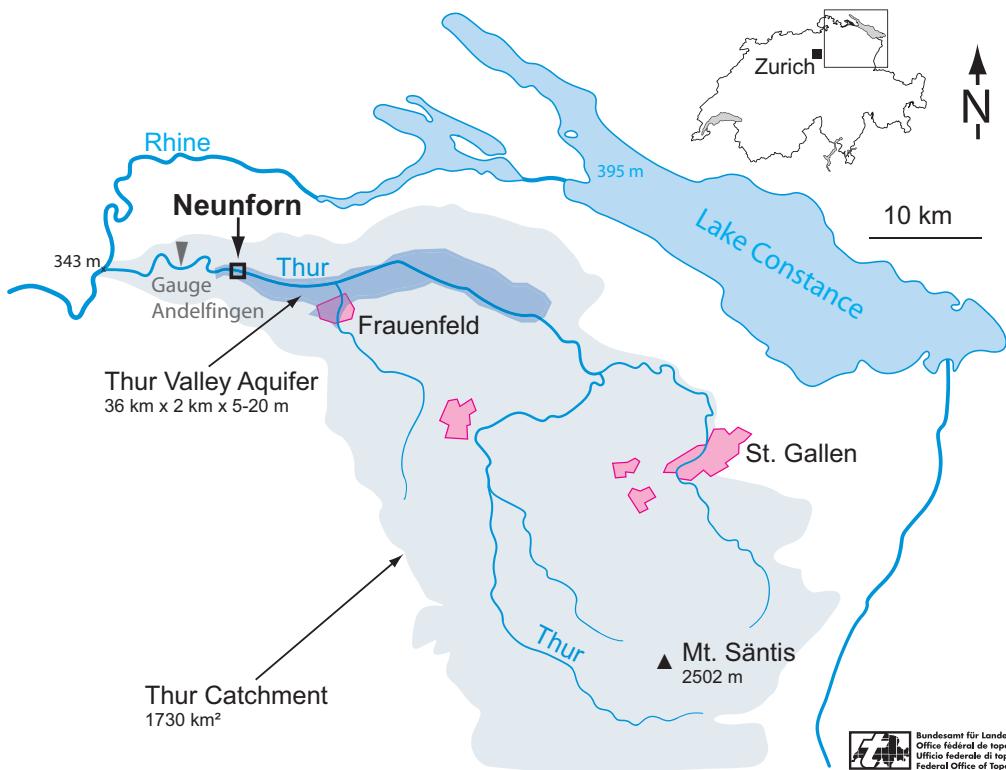
**Table 1.** Hydrological parameters used for the SP modeling.

	Saturated hydraulic conductivity, $K_s$ [ $\text{m s}^{-1}$ ]	Residual saturation, $S_{\text{wr}}$ [-]	Van Genuchten parameters	
			$\alpha$ [ $\text{m}^{-1}$ ]	$m$ [-]
Gravel	$10^{-3}$	0.18	14.5	0.63
Clay	$10^{-6}$	0.18	0.8	0.083

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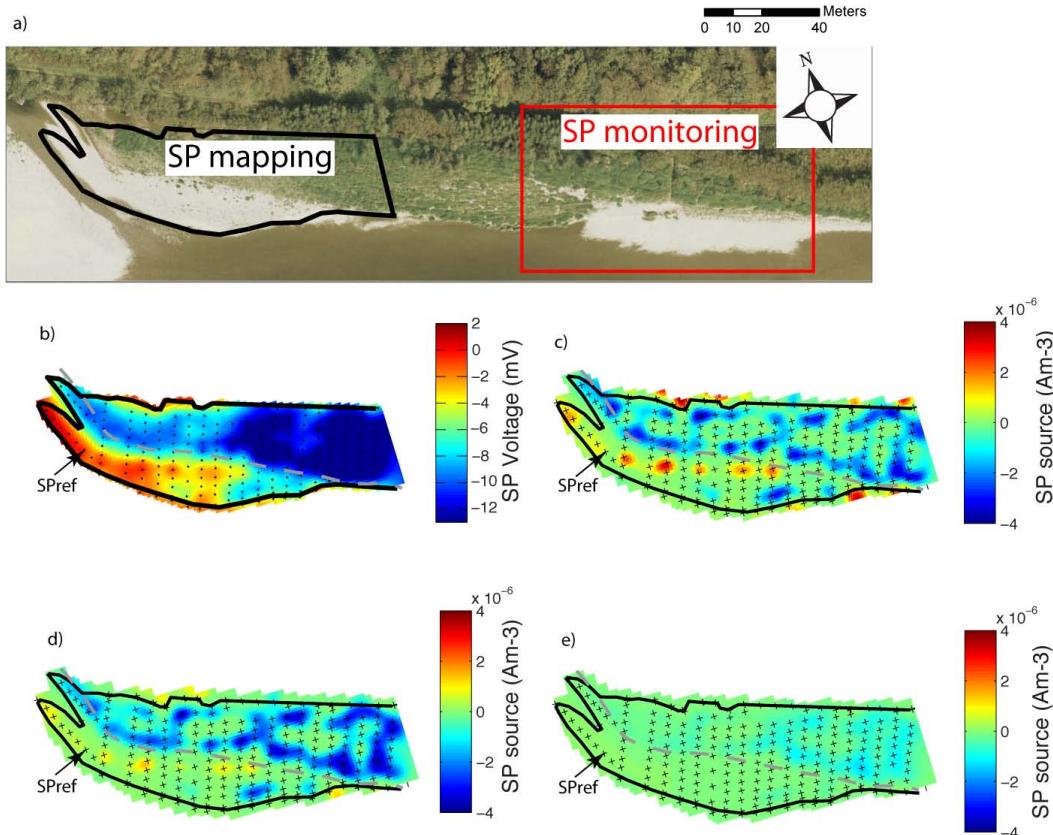
**Fig. 1.** Location of the Thur catchment, Thur valley aquifer, and Neunforn test site in north-eastern Switzerland. Modified from a figure prepared by Swisstopo (Swiss Federal Office of Topography).

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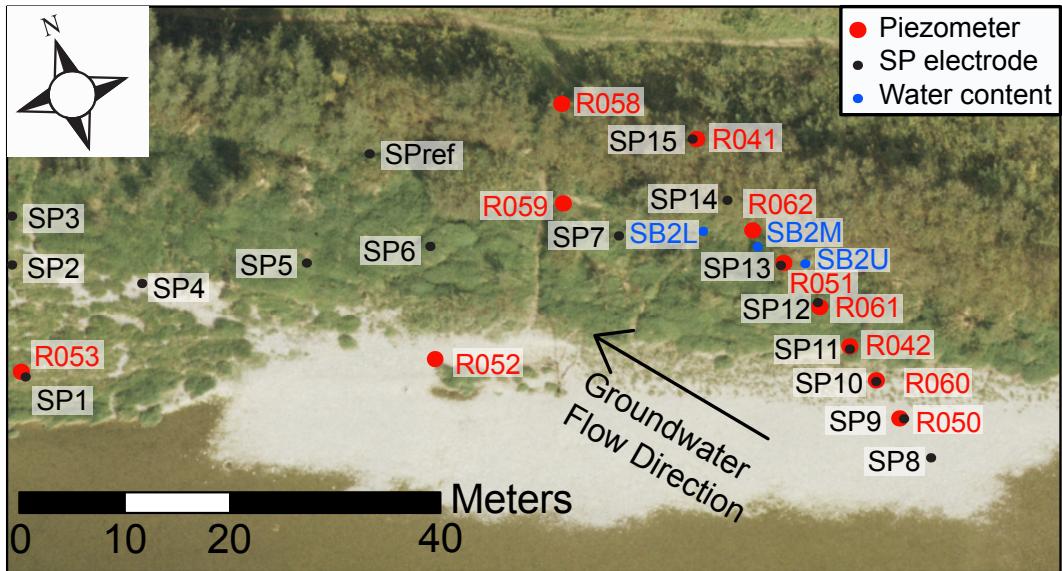
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**Fig. 2.** (a) The gravel bar at Neunforn on which SP mapping and SP monitoring (see Fig. 3) was carried out. (b) The kriged SP map with measurement locations. (c–e) Source current distributions at 1.5–2 m, 3.5–4 m, and 5.5–6 m depth obtained from inversion. The boundaries of the gravel bar are indicated with a black solid line, whereas the gray dashed line indicate the transition between the grassy areas and the barren gravel surface.

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**Fig. 3.** SP monitoring area with SP monitoring (SP1-15) and reference (SPref) electrodes, piezometers, and soil monitoring stations. The tower on which rainfall intensity was measured is located approximately 100 m away from this area.

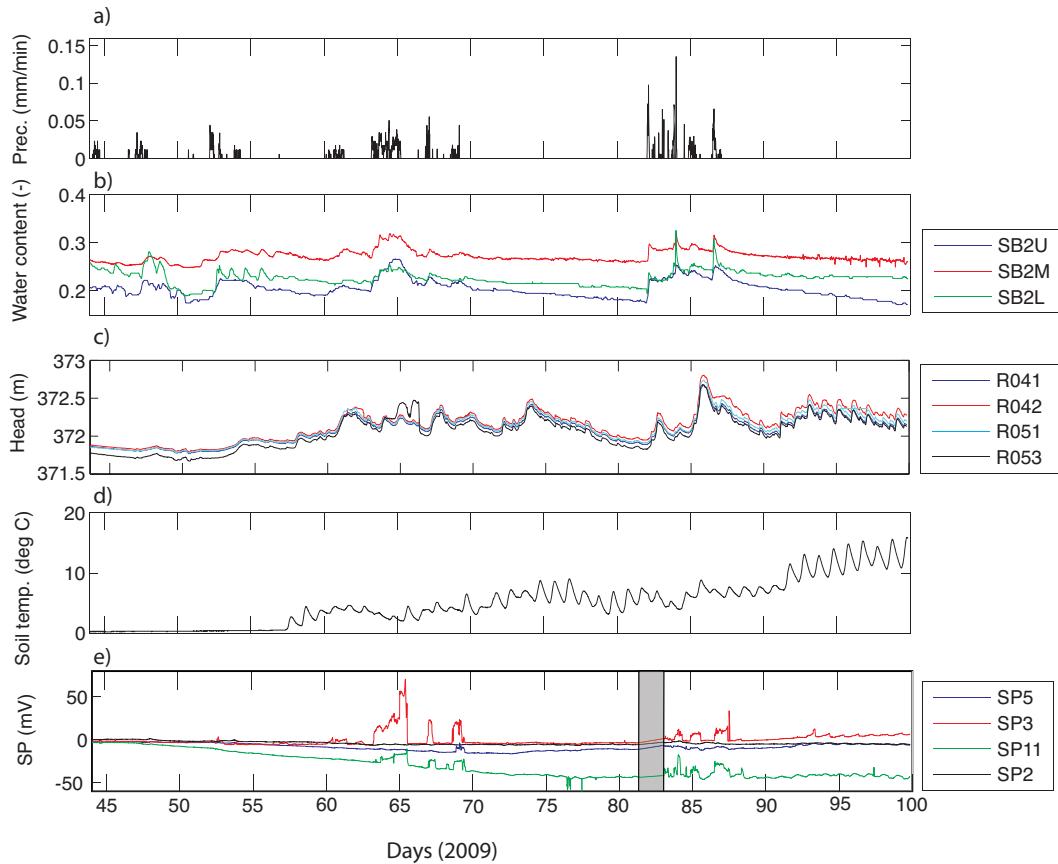
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**Fig. 4.** Time-series of **(a)** rainfall intensity, **(b)** water content at 10 cm depth (see Fig. 2 for location of sensors), **(c)** hydraulic head, **(d)** soil temperature at 10 cm depth at location SB2M, and **(e)** SP signals from four of the SP monitoring electrodes. The gray area in (e) indicates a period when no SP data are available.

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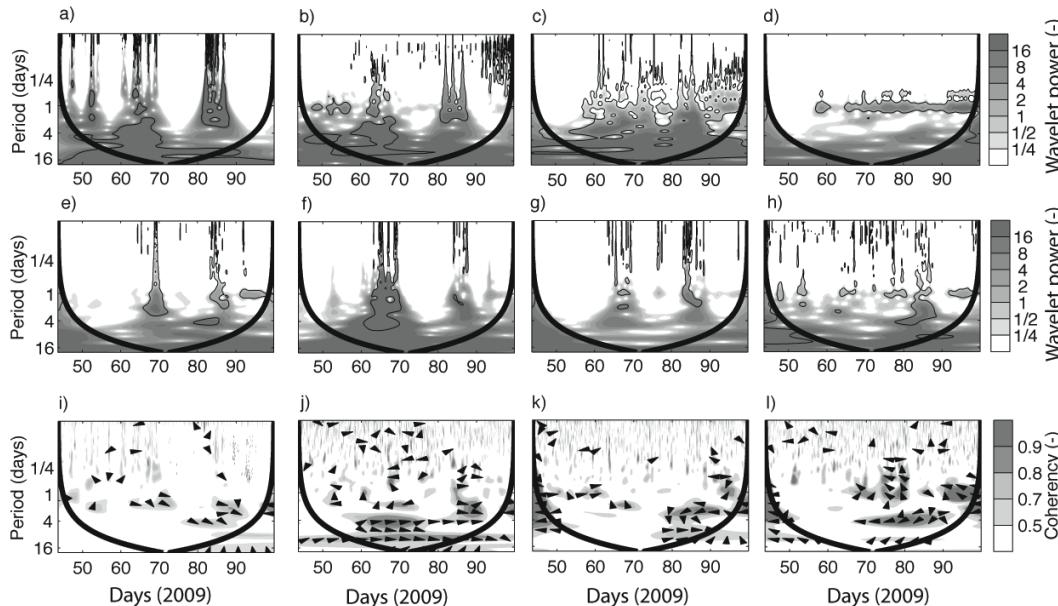
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**Fig. 5.** The wavelet power spectra for (a) rainfall intensity, (b) water content at 10 cm depth at station SB2M, (c) hydraulic head for piezometer R042, (d) soil temperature at 10 cm depth at station SB2M, (e) SP5, (f) SP3, (g) SP11, (h) SP2. The 5% significance level against red noise is shown as contour lines in (a)–(h). Regions outside the thick solid lines (cone of influence) are increasingly affected by edge effects. Wavelet coherency between (i) rainfall intensity and SP5, (j) water content and SP3, (k) hydraulic head and SP11, (l) soil temperature and SP2. The arrows indicate the phase relation between the two time-series when the wavelet coherency is above 0.5 (in-phase when pointing right; anti-phase when pointing left; series 1 leading series 2 by 90° when pointing down).

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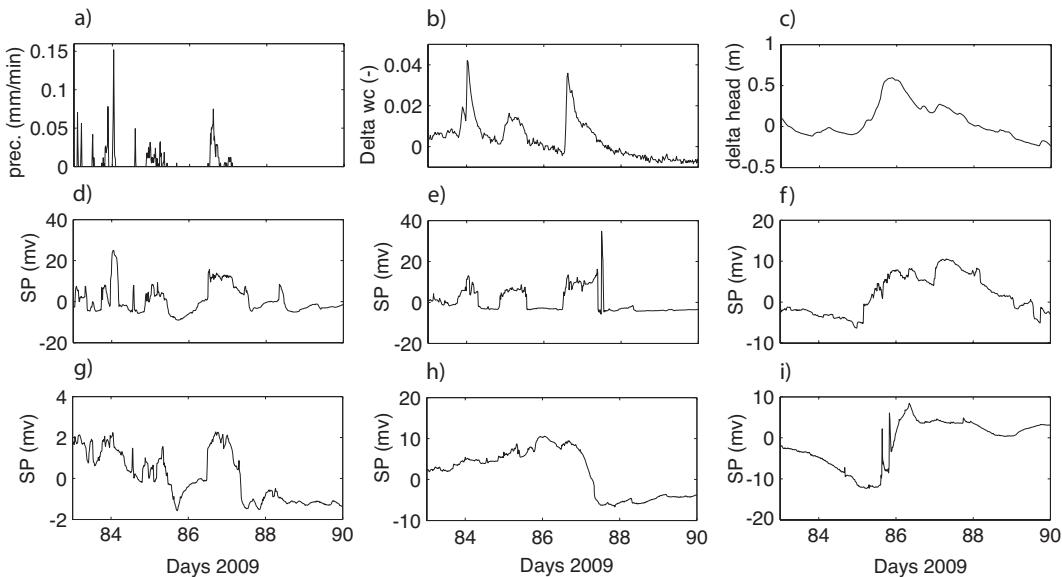
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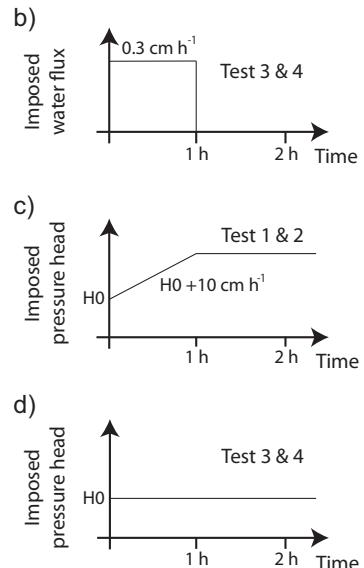
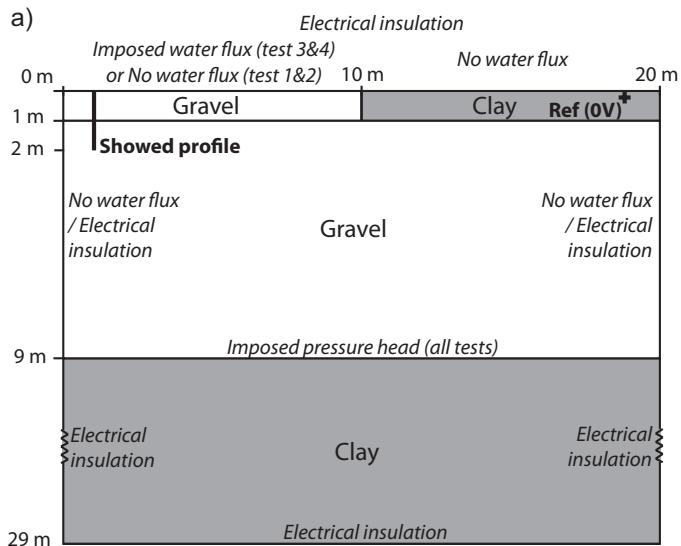
**Fig. 6.** Filtered time-series of **(a)** rainfall intensity, **(b)** water content at 10 cm depth for station SB2M and **(c)** hydraulic head for piezometer R042 for time-scales less than two weeks during the major rainy period. Corresponding relative changes in the filtered SP signals are shown for **(d)** SP11, **(e)** SP3, **(f)** SP8, **(g)** SP6, **(h)** SP1, and **(i)** SP12.

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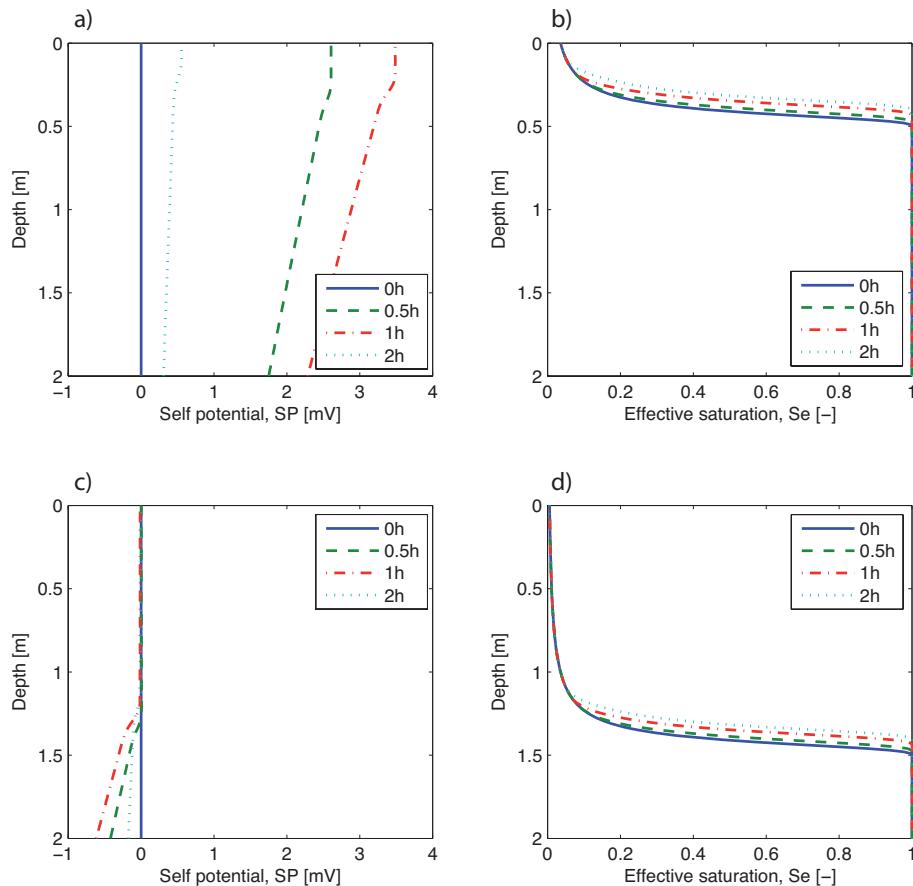
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**Fig. 7.** (a) Conceptual model used to investigate how SP signals vary with respect to variations in the water table (tests 1 and 2) and due to rainfall (tests 3 and 4) for an initial water table and 0.5 m (tests 1 and 3) and 1.5 m (tests 2 and 4) depth. (b) Imposed infiltration rate in the gravel during tests 2 and 4. Imposed groundwater level fluctuations tests (c) 1 and 2 and (d) 3 and 4, respectively.

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**Fig. 8.** (a) Variation of the modeled SP signal with depth at different times for a rising water table (10 cm/h during 1 h) located at (a) 0.5–0.4 m and (c) 1.5–1.4 m depth (see Fig. 7). (b) and (d) Corresponding values of the effective water saturation.

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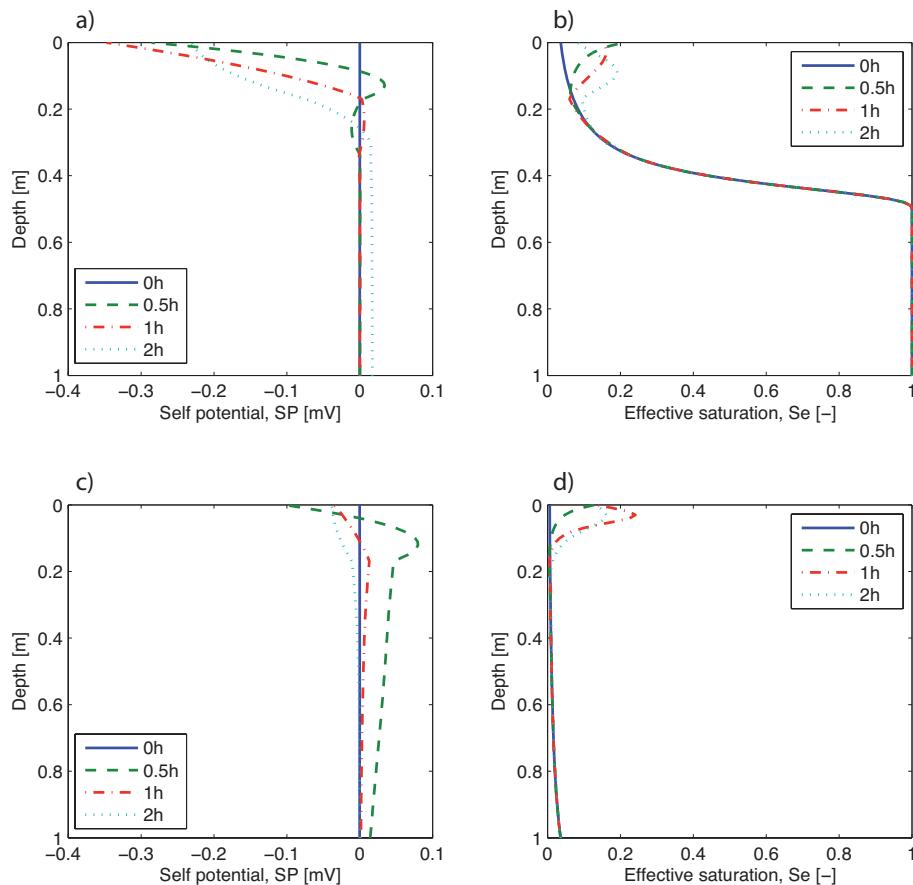
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**Fig. 9.** (a) Variation of the modeled SP signal with depth at different times for a rainfall event (3 mm/h during 1 h) with a constant water table located at (a) 0.5 m and (c) 1.5 m depth (see Fig. 7). (b) and (d) Corresponding values of the effective water saturation.

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