Hydrology and Earth System Sciences Discussions

Bi-criteria evaluation of MIKE SHE model for a forested watershed on South Carolina coastal plain

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Abstract

Hydrological models are important tools for effective management, conservation and restoration of forested wetlands. The objective of this study was to test a distributed hydrological model, MIKE SHE by using bi-criteria (two measurable variables, streamflow and water table depth) to describe the hydrological processes in a forested watershed that is characteristic of the lower Atlantic Coastal Plain. Simulations were compared against observations of both streamflow and water table depth measured on a first-order watershed (WS80) on the Santee Experimental Forest in South Carolina, USA. Model performance was evaluated using coefficient of determination (R^2) and Nash-Sutcliffe's model efficiency (E). The E and root mean squared error (RMSE) were chosen as objective functions for sensitivity analysis of parameters. The model calibration and validation results demonstrated that the streamflow and water table depth were sensitive to most of the model input parameters, especially to surface detention storage, drainage depth, soil hydraulic properties, plant rooting depth, and surface roughness. Furthermore, the bi-criteria used for distributed model calibration and validation was shown to be better than the single-criterion in obtaining optimum model input parameters, especially for those parameters that were only sensitive to some specific conditions. Model calibration using the bi-criteria approach should be advantageous for constructing the uncertainty bounds of model inputs to simulate the hydrology for this type of forested watersheds. R^2 varied from 0.60–0.99 for daily and monthly streamflow, and from 0.52–0.91 for daily water table depth. E changed from 0.53–0.96 for calibration and 0.51–0.98 for validation of daily and monthly streamflow, while E varied from 0.50–0.90 for calibration and 0.66–0.80 for validation of daily water table depth. This study showed that MIKE SHE was applicable for predicting the streamflow and water table depth in this coastal plain watershed.

1   Introduction

Computer models are effective tools for understanding and quantifying watershed hydrology, but may be limited by various constraints of different model types. Most hydrological models are lumped using spatially averaged conditions for the study sites (Singh et al., 1999). However, geological and hydrological conditions in a large catchment or watershed may exhibit considerable spatial and temporal variability such that it can be difficult to accurately describe their hydrology using lumped hydrological models. In contrast, distributed models consider spatial variability in watersheds and are widely used. Nevertheless, the distributed models may also have some disadvantages, such as equifinality due to over-parameterization (Beven, 2006) and uncertainties in model predictions due to variability in the large number of input parameters (Vrugt et al., 2007). Because of the high uncertainties, distributed models may perform poorly even if they are calibrated well using data from another time period (Kirchner, 2006); similar problem can also occur when the models are tested against the data from different study sites. In general, distributed models are most likely to perform better than lumped models because of their capability to utilize spatial and temporal characteristics of watersheds (Refsgaard, 1997).

Distributed models require appropriate calibration and validation, which has been recognized and emphasized by users and developers of hydrological models (Freer et al., 2003). Typically, hydrological model performance is evaluated by comparing the predicted values of discharge with the observed values. However, discharge is usually measured at the outlet of a sub-basin on a watershed due to limited resources for complex measuring equipment, time and personnel. Therefore, there is a considerable merit in evaluating hydrological model performance by including more variables (i.e., a multiple criteria approach) (Boyle et al., 2003; Meixner et al., 2003; Shrestha and Rode, 2008). Water table depth may be a good candidate of measurable variables to be used for distributed model calibration and validation (Lamb et al., 1998) because water table depth is a surrogate for soil moisture that affects hydrological processes

such as discharge and evapotranspiration and can vary largely in space and time in watersheds, especially in large river catchments. Model evaluation using the bi-criteria approach (i.e., one with two measurable variables, discharge and water table depth) would allow for closer examination of the internal consistency of the model (El-Nasr
5 et al., 2001) and for better determination of model biases, especially in watersheds associated with high spatial heterogeneity in geology, soils and vegetation.

Accurately predicting the hydrological conditions governing wetland ecology is necessary to assess the ramifications of land use change and climate change to the functions and services of the wetland ecosystems. Although hydrological models for the
10 wetland systems have been constructed in the past (Sun et al., 1998; Mansell et al., 2000; Martinez et al., 2008), other types of models describing wetland hydrology such as DRAINMOD (Amatya and Skaggs, 2001) and SWAT (Arnold et al., 2001) are also available. Most of those models either are primarily used for simulating field-scale hydrological processes (Amatya et al., 2003; El-Sadek, 2007; Liu et al., 2007) or have
15 not been thoroughly evaluated with spatial distributions of water table (Lu et al., 2009). The MIKE SHE model has been tested in recent years for multiple sites in USA (Sahoo et al., 2006; Lu et al., 2009) and around the world (Graham and Butts, 2005; Mernild et al., 2008; Vázquez et al., 2008; Staes et al., 2009). These testing results indicate that MIKE SHE is well suited for watersheds containing both uplands and wetlands.
20 However, this model was mostly calibrated and validated using a single variable (either discharge or water table) (Sahoo et al., 2006; Lu et al., 2006; Zhang et al., 2008). The objective of this study was to evaluate the ability of MIKE SHE to simulate the hydrology of a forested watershed containing both uplands and wetlands on Atlantic Coastal Plain in South Carolina by using a bi-criteria (streamflow and water table depth) calibration
25 and validation approach.

2 Method

2.1 Site description

We chose a first-order watershed (WS80) on Santee Experimental Forest of Forest Service (SEF) for this study because it contains both uplands and wetlands and is one of the long-term monitored watersheds. The 160 ha watershed is located at 33.15° N, 79.8° W, and 55 km northwest of Charleston, South Carolina (Fig. 1). WS80 serves as the control watershed for a paired watershed system (WS77 and WS80) within a second-order watershed (WS79, 500 ha) draining into the Huger River, a tributary of east Branch of the Cooper River. The site has gauging records since 1967. It is characteristic of the subtropical region of the Atlantic Coast with short, warm and humid winters and long and hot summers; the 30-year (1971–2000) average temperature is 18.7 °C, and the mean annual precipitation is 1350 mm (Amatya et al., 2003). The topography is planar, and the slope is less than 4%. The elevation is between 4–10 m above mean sea level. WS80 has a shallow water table, and about 23% of the watershed is classified as wetlands (Sun et al., 2000, Harder et al., 2007).

The soils developed by coastal plain sediments in this site are hydric (Federal Register, 1994, 2002), which is moderately well drained in the upland and poorly drained in the riparian zone (SCS, 1980; Fig. 2a). They are typified by loam to surface and clayey subsoil. The main soil type is loam, covering about 90% of the watershed. Clay content is ≤30% in topsoil (within 30 cm), 40–60% in subsoil (>30 cm) (SCS, 1980). Soil reaction is acidic; pH is between 4.5 and 6.5. The available water capacity is between 0.1 and 0.2 cm/cm.

WS80 has not been managed in more than five decades (Amatya et al., 2003; Harder et al., 2007). However, the forest was heavily impacted by Hurricane Hugo in 1989 (Hook et al., 1991). This site remained unmanaged after the hurricane, without biomass removal or salvage logging. Most of the trees regenerated naturally after the hurricane. The current forest cover (Fig. 2b) consists of bottomland hardwoods in the riparian zone and mixed pine-hardwoods elsewhere (Harder et al., 2007). The

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dominant trees are loblolly (*Pinus taeda* L.), sweetgum (*Liquidambar styraciflua*) and a variety of oak species (*Queercus* spp.) (Hook et al., 1991; Harder et al., 2007).

2.2 Field measurements and data collection

Precipitation and air temperature were measured on-site at hourly intervals. Additional meteorological data were collected at 30 min intervals at a weather station at the Santee Experimental Forest Headquarters (SEFH) about 3 km away from the study site. The measurements included precipitation, solar and net radiation, wind speed, wind direction, temperature, vapor pressure, and relative humidity, which were processed to estimate daily potential evapotranspiration (PET) using the Penman-Monteith method (Xu and Singh, 2005; Harder et al., 2007).

Water table depth was measured at 4 h intervals (2003–2007) by two automatic recording wells that were installed in an upland area (Global Water's WL-16; the well was 230 cm deep) and a lowland area (Remote Data Systems' WL-40; the well was 54 cm deep before March 2004 and 94 cm afterwards) to record water table elevation. Eight manual wells (≥ 2 m deep) were installed across the watershed with biweekly measurements (2003–2004) (Fig. 1). An automatic Teledyne ISCO-4210 flow meter measured stream gauge heights above a compound V-notch weir at 10 min intervals at the watershed outlet. The streamflow was calculated using a standard rating curve method developed for the compound weir. The 10 min values were integrated into daily and monthly streamflow in cubic meters per second, and then normalized to millimeters per day to be comparable to daily precipitation. The measured total annual streamflow and average water table depth were presented in Table 1.

The physical soil properties were obtained from SoildataMart of NRC (http://soildatamart.nrcs.usda.gov). Topography was obtained from a traditional topographic survey of WS80 done in 1982 with 15 cm contour and in the scale of 1:200. In addition to leaf area index (LAI) calculated from measured leaf biomass, LAI was also measured periodically through the year using a LI-2000 leaf area meter.

2.3 The MIKE SHE model

MIKE SHE is a GIS-based distributed model designed for applications in low-relief terrains (Graham and Butts, 2005). It is a spatially and temporally explicit, modularized modeling system. This model simulates the complete terrestrial water cycle, including saturated water movement in soils, 2-D water movement of overland flow, 1-D water movement in rivers/streams, unsaturated water movement and evapotranspiration (ET). Saturated water movement in soils is modeled using 3-D Finite Difference or Linear Reservoir. The 2-D water movement of overland flow is simulated using Finite Difference or Subcatchment-based method. The diffusive wave version of Saint Venant equations is used to simulate 1-D water movement in rivers/streams. The unsaturated water movement is simulated using either Richards equation or Gravity Flow or Two-Layer water balance method (DHI, 2005). Detailed descriptions of the model and algorithms can be found in many publications (Abbott et al., 1986a,b; DHI, 2005; Graham and Butts, 2005).

2.4 Model setup and parameterization

In this study, MIKE SHE was coupled with the flow routing model MIKE 11 (DHI, 2005; Sahoo et al., 2006), a one-dimensional river/channel water movement model, to simulate the full hydrological cycle of the watershed, including evapotranspiration, infiltration, unsaturated flow, saturated flow, overland flow and stream flow. The main inputs for the model included spatial data on topography, soils, vegetation, and drainage network; and temporal data on precipitation and potential evapotranspiration (PET) based on Penman-Monteith (Monteith, 1965; Xu and Singh, 2005; Harder et al., 2007). To apply MIKE SHE, this study site (Fig. 1) was divided into 675 (50 m by 50 m) cells.

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2.4.1 Unsaturated flow

The Two-Layer Water Balance model (Yan and Smith, 1994; DHI, 2005), which is de-signed for the areas with a shallow groundwater table, was used to simulate the un-saturated flow for this study. The model divides the unsaturated zone into a root-zone where ET can occur, and a below-root-zone where ET does not occur (Yan and Smith, 1994). The model inputs were given in Table 2, including vegetation characteristics (cover, LAI and plant rooting depth) and the physical soil properties (infiltration capac-ity, and the soil moisture contents at the wilting point, saturation and field capacity). The data for physical soil properties and vegetation characteristics (Fig. 2a,b) were spatially distributed to simulate unsaturated flow in space and time.

2.4.2 Saturated flow

The 3-D Finite Difference method (DHI, 2005) was used to simulate the saturated flow for this study. The inputs needed to simulate saturated flow were soil hydraulic properties, including horizontal and vertical hydraulic conductivities, specific yield, and storage coefficient. Horizontal hydraulic conductivity (K_x) significantly influences base, peak and subsurface flows. Overland flow, subsurface flow (lateral flow) and ground water table level are significantly affected by the values of vertical hydraulic conductivity (K_y). Both the horizontal and vertical hydraulic conductivities used in this study were spatially distributed based on the distribution of soils (Fig. 2a).

A drainage depth (from the phreatic surface to the level where the flow of drainage water can occur) and a drainage time constant are required for simulating the flow of drainage water using an empirical formula in MIKE SHE. Both these parameters are important for simulating subsurface flow. The drainage time constant affects stream-flow, with small values delaying subsurface flow to reach the stream. In contrast, the drainage depth can significantly influence both the streamflow and water table depth. When the water table rises above the elevation of the drainage depth, drainage starts and varies linearly with change in the difference between water table level and drainage

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depth. Therefore, lower drainage depths (below the ground surface) yield deeper water table level. The drainage depth was initialized to 50 cm due to a shallow water table in this study area.

2.4.3 Overland flow and stream flow

5 Overland flow was simulated using diffusive wave approximation. The inputs include initial water depth on the surface, surface detention storage, and Manning number (M). The measured surface water depth was used to initialize the water depth above the ground surface for the model to run. Surface detention storage largely affects routing water toward the stream and water table dynamics. Large values of surface detention
10 storage reduce the overland flow reaching the stream, but increase ponding water that may lead to a subsequent increase in water table level. Manning M significantly influences routing overland flow toward the stream and stream flow toward the outlet of the stream with higher values leading to faster water movement.

2.4.4 Evapotranspiration (ET)

15 In this study, daily ET was simulated using the Two-Layer Water Balance model (Yan and Smith, 1994; DHI, 2005). Actual daily evapotranspiration (AET) was estimated as:

$$AET = E_c + E_p + E_u + E_s \tag{1}$$

Where E_c is the daily evaporation from canopy; E_p is the daily evaporation from soil or ponded water; E_u is the daily ET from unsaturated zone extracted by plant; E_s is
20 the daily ET from saturated zone extracted by plants. The key parameters used to calculate AET include LAI, plant rooting depth, coefficient of canopy interception (C_{int}), ET surface depth, and surface detention storage.

2.5 Simulation time steps

MIKE SHE has the flexibility of using variable simulation time steps for different hydrological modeling components and flow characteristics (DHI, 2005; Zhang et al., 2008). In this study, maximum allowed time steps were set to 2 h for unsaturated flow, overland flow and ET, 4 h for saturated flow, and 10 min for channel flow. The time steps for outputs were 4 h for streamflow and 24 h for water table depth.

2.6 Boundaries of surface flow and subsurface flow

In this study, it was assumed that the boundary of subsurface ground water area was the same as that of the surface water flow of the watershed because the watershed is bordered by the Forest Service roads (except for its northeast section) that are well compacted to minimize lateral flows across the borders. Ground water and surface flow across the northeast border was assumed minimal as compared to the overall flows based on the water balance in this watershed analyzed by Harder et al. (2007). Typically, the deep seepage was considered as only a small fraction of total precipitation on Atlantic Coastal areas (Heath, 1975; Riekerk et al., 1979; Harder et al., 2007); thus, it was assumed negligible.

3 Model calibration and validation

Unlike most previous studies, this study used both streamflow and water table depth for model calibration and validation. Model calibration was conducted through sensitivity analysis using data observed in 2003, while model validation was performed with the data from 2004 to 2008. The observed data in this six-year study period (2003–2008) consisted of wet, dry and normal rainfall years. The wet year was 2003 with 1671 mm of precipitation, about 320 mm higher than the 30-year average (1350 mm during 1971–2000) (Amatya et al., 2003). The dry years were 2004 and 2007 with precipitation of

962 and 923 mm, about 400 mm lower than the 30-year average. The years of 2005, 2006, and 2008 were relatively normal with precipitation of 1540, 1255, and 1562 mm, respectively. The large variability in precipitation among those years yielded substantial differences in streamflow and water table depth in this area. These wide ranges of climatic and hydrological conditions were optimal for model testing to determine whether any model components were biased and whether the model could perform equally well under different conditions outside the calibration time period (Kirchner, 2006).

Several quantitative methods were used to evaluate model performance, including the model efficiency (E) (Nash and Sutcliffe, 1970), the root mean squared error (RMSE), and the coefficient of determination (R^2). The parameter sensitivity was evaluated by minimizing RMSE and maximizing E of both streamflow and water table depth as objective functions. The model performance was evaluated by calculating E for daily and monthly streamflow and water table depth (Moriasi et al., 2007).

MIKE SHE was initialized first with a group of baseline input parameters, most of which were empirical values (Table 2). For calibration, the model was rerun with alternative values for each of the input parameters until the maximum E and minimum RMSE for streamflow and water table depth were achieved. The range of alternative values for each input parameter was chosen to allow for adequate variability, with the minimum values for most of parameters set at about 10–25% of their empirical values and the maximum values at about 200% of their empirical values (Table 3). However, much larger ranges were needed for infiltration rate, hydraulic conductivities and the coefficient of canopy interception due to their low sensitivity in this watershed. The optimized parameter values from the calibration procedure were then used for the model validation. All simulations were carried out with one year warm up starting from 2002.

4   Results and discussion

4.1   Calibration

The calibration results indicated that surface detention storage was a critical calibration parameter with substantial influence on water table depth and streamflow in this site (Fig. 3a,b). For example, the water table depth increased from −0.74 to −0.38 m when surface detention storage was set, respectively, to 5 and 100 mm. Calibration for daily streamflow yielded an optimal value of surface detention storage at about 50 mm (given its range of 5–100 mm). However, calibration for water table depth seemed to suggest that detention storage may be likely to vary between 25 and 50 mm because the simulated average water table levels displayed different patterns as compared to the measured water table level (−0.58 m), showing higher values by 7 and 20 cm when detention storage was set to 50 and 100 mm but lower values by 16 and 4 cm when detention storage was set to 5 and 25 mm. Most importantly, distributed detention storage was needed because it provided more optimized simulation output (E of 0.70 for daily streamflow and 0.90 for water table depth) than uniform detention storage (E of 0.58 for daily streamflow and 0.49 for water table depth). As a result, distributed detention storage was determined to vary 11–180 mm in space, based on the topography and the observation of stromflow in this site (Harder et al., 2007), with an average of 36 mm. This average detention storage was comparable to the 40 mm value used by Harder et al. (2006) in their hydrological simulation using DRAINMOD model on the same site.

Plant rooting depth may be the second most critical calibration parameter, especially for water table depth. In the discussions of calibration with 2003 data hereafter, we wanted to distinguish a high precipitation period (1 January to 9 September; 1606 mm) from a low precipitation period (10 September to 31 December; 164 mm) because calibration parameters may respond to them differently. Plant rooting depth showed weak effects on streamflow (E of 0.42–0.64, RMSE of 0.21–0.27) during the low precipitation period, but no effects (E of 0.57, RMSE of 3.48–3.50) in the high precipitation period (Fig. 3a, Table 3). In contrast, plant rooting depth strongly affected water table depth

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(*E* of 0.05–0.54, RMSE of 0.17–0.24) (Fig. 3b, Table 3), especially during the low precipitation period (Fig. 4). The average simulated water table depth was 5.5 and 3.0 cm shallower than the observed data when plant rooting depth was set to 30 and 50 cm, but 5.0 and 9.2 cm deeper than the observed when plant rooting depth was set to 70

5   and 90 cm. Water table decreased with increase in plant rooting depth, similar to the pattern reported by Skaggs et al. (1991) in their simulation with DRAINMOD for an Atlantic Coastal watershed in North Carolina. However, sensitivity of water table level to plant rooting depth was reduced during the high precipitation period because of the abundant supply of water in the root zone.

10   Drainage depth, i.e., the depth from the average ground surface to the position where the flow of drainage water can occur, was another critical calibration parameter that required representation of distributed values. Drainage depth was determined to change between 0.05–0.95 m (average of 0.35 m) in space based on the variability in topography (slope) and the distance to streams (Table 4a,b). The calibration results showed

15   that streamflow (*E* of 0.50–0.64) was less sensitive to drainage depth than water table depth (*E* of −4.96–0.45) (Table 3, Fig. 5). The reason for this low sensitivity of streamflow was likely to be the dominance of the surface flow in WS80 with shallow water table and shallow stream (Harder et al., 2007).

Surface roughness (i.e., Manning *M*, which is the inverse of the commonly used Man-

20   ning's *n*) affected streamflow (*E* of 0.53–0.64, RMSE of 3.21–3.65; Fig. 3a), but showed little effect on water table depth (*E* of 0.46–0.47, RMSE of 0.182–0.183; Fig. 3b). Manning *M* was to be set at 35 based on streamflow responses and on physical conditions of WS80 (e.g., the planar topography, high litter content on the ground).

Horizontal hydraulic conductivity affected both streamflow and water table depth.

25   However, horizontal hydraulic conductivity displayed different optimal values in calibration for streamflow and water table depth: with 0.00008 m/s (weighted average using the area of soils) for streamflow (*E* of 0.77, RMSE of 2.53) and 0.00001 m/s for water table depth (*E* of 0.46, RMSE of 0.18). Because large spatial differences in soil properties may be the cause for the differences, distributed horizontal hydraulic conductivity

was used with a spatial range of 0.00001–0.0001 m/s based on the topography and the spatial distribution of soil type and texture in this site.

The coefficient of canopy interception (C_{int}) was insignificant in calibrating for either streamflow (E of 0.55–0.57, RMSE of 3.48–3.57) or water table depth (E of 0.54–0.55,
5   RMSE of 0.168–0.171). This low sensitivity of streamflow and water table depth to C_{int} may be due to the low proportion of the canopy storage in the precipitation (about 4%). However, C_{int} during the low precipitation period became significant when canopy storage fraction in the total precipitation increased to about 11%. The optimal value of C_{int} was determined to be 0.225 mm (given the optimum range of 0.05–0.35 mm). This
10 value of C_{int} corresponded to canopy storage capacity of 0.69 mm, which is similar to the value of 0.7 mm observed by Harder (2004) in the same watershed for the same period.

Vertical hydraulic conductivity and infiltration rate showed little effect on either streamflow or water table depth (Table 3). This lack of model sensitivity to these two
15 parameters was likely to be related to the shallow water table level and the boundaries of the surface water flow and subsurface flow on the watershed. WS80 was bound by the Forest Service roads that were well compacted to minimize lateral flow across the border. Vertical hydraulic conductivity and infiltration rate were defined by initial values in Table 2, changing with soil type and texture.
20 The results from sensitivity analysis suggest that model calibration using both streamflow and distributed water table depth may have great advantages over the single parameter approach in selecting optimal parameter values and constructing uncertainty bounds for model inputs. For example, the calibration results showed that, to simulate hydrology of the study area for 2003, plant rooting depth was not important to
25 streamflow, but was a significant factor with an optimal value to water table depth. Most importantly, distributed surface detention storage was needed to accommodate for the differences in optimal parameter values when calibrating for both streamflow and water table depth. Thus, the model calibration using bi-criteria should be required to assure prediction accuracy with distributed hydrology models. In addition, our results indicate

that the distributed parameter values should be expected in areas of high spatial heterogeneity, especially for those parameters that influence water table dynamics and for watersheds with a shallow water table and low-relief topography.

MIKE SHE was well calibrated based on the E and R^2 values for daily and monthly streamflow, and daily water table depth (i.e., E of 0.53, 0.96 and 0.90; R^2 of 0.53, 0.96 and 0.90, respectively; Table 1), most of which were within the "very good" rating range ($E > 0.75$) suggested by Moriasi et al. (2007). However, there were a few problems. For example, streamflow was over-predicted on 20 June, 2 and 3 July 2003 and the dry periods when the stream ran dry (and Fig. 6a). The over-prediction of daily streamflow on 2 and 3 July 2003 might be related to the measurement errors caused by beaver activities (Harder et al., 2007). However, a more likely reason would be that the over-prediction during the dry periods should result from an artifact of MIKE SHE that does not allow a river/stream to dry out (Lu et al., 2006; see additional discussion below). Nonetheless, despite the over-prediction of the streamflow during the dry periods, MIKE SHE was able to capture most of the dynamics of streamflow and water table depth in the watershed (Fig. 6a,b).

4.2 Model validation

The model was validated using streamflow and water table depth measured between 2004 and 2008. The predicted daily and monthly streamflow were in good agreement with the measurements (R^2 of 0.60–0.99 for both; Table 5). The model efficiency (E) values (0.84–0.98) for monthly streamflow were in the "very good" rating range ($E > 0.75$) prescribed by Moriasi et al. (2007). However, despite the good correlation between the observed and predicted values and the high E values, the modeled streamflow tended to be higher than the observations (Table 5), especially for 2004. The higher simulated streamflow may be attributed to over-predictions during dry periods (Fig. 7a), especially on days with a heavy precipitation, such as 29 August 2004 and 2 June 2005 (Fig. 7a). The over-prediction of streamflow occurred consistently when there were high intensity precipitation events during normally dry periods. This

type of response should be related to an artifact of MIKE SHE because it does not al-
low for no-flow periods (Lu et al., 2006). In the case of the stream draining, it is typical
to have no-flow periods, for example during the dry period from the fall of 2003 to the
summer of 2004 and the summer of 2007; however in these cases, MIKE SHE must
5 maintain a very low stream flow. This artifact in MIKE SHE was not reported before by
Lu et al. (2006), because most of the applications were in larger watersheds, but it is
an important consideration for drainage areas with intermittent streamflow.

There was good ($p<0.01$) correspondence between the predicted water table depth
and the measured values from two wells in the watershed (Fig. 7b). The simulated
10 temporal water table dynamic was also in good correspondence with the measured
data (Table 4a,b and Fig. 7c), with the E values of 0.66, 0.80 for the manual and
automatic wells, respectively. Lu et al. (2009) have also shown that MIKE SHE is
effective at predicting water table depth in the central Florida coastal plain.

4.3 Spatial pattern of water table levels

15 Although the surface topography of this watershed is planar (i.e., slope 0–3%), there is
considerable variation in water table depth (Table 4a,b). The simulated distribution of
water table depth and flow direction reflected a complex pattern (Fig. 8a). The variation
in water table depth was most pronounced during dry periods (Fig. 8b), as opposed to
a relatively uniform distribution when the site is near saturation (Fig. 8a). Studying the
20 same watershed, Harder et al. (2007) reported that stormflow was generated primarily
from saturated area, which was consistent with our findings on water table depth and
flow direction during raining periods. The water table in dry seasons was substantially
lower than wet seasons (Fig. 8b), over 40 cm below the surface in a large area of the
study site in very dry periods. The difference in water table depth between dry and wet
25 periods was 1–2 m depending on the location in the watershed. The magnitude of water
table rise during wet periods and fall during dry periods were related to topography, with
the very flat area having less variation as compared to areas with some relief.

5   Conclusions

The MIKE SHE simulations showed that streamflow from this lower coastal plain forested watershed was highly sensitive to surface detention storage, drainage depth, surface roughness and horizontal hydraulic conductivity. Water table dynamics were highly sensitive to plant rooting depth and the coefficient of canopy interception, which likely reflects the influence of vegetation through evapotranspiration. Having measurements of ET and/or canopy interception measurements would improve the model calibration. Modeling the spatial distribution of shallow groundwater table at watershed scales remains challenging. This exercise represents one of the few attempts to evaluate the MIKE SHE model using the bi-criteria approach (e.g., streamflow and water table depth). The results showed that calibration using the bi-criteria was better than a single-criterion approach to obtain optimum model input parameters, especially for those parameters that are sensitive to specific conditions. For example, plant rooting depth was influential on water table depth but did not directly affect streamflow in this first-order watershed. Model calibration using multi-criteria approaches should be advantageous for assuring prediction accuracy when applying distributed models to simulate hydrology for this type of forested watersheds. Qualitative and quantitative results from the calibration and validation procedures showed that MIKE SHE was capable of predicting the dynamic water table and streamflow for both daily and monthly time steps. However, the systematic over-prediction of streamflow during periods of no-flow for the ephemeral stream is an issue that must be considered when applying MIKE SHE to first order watershed with intermittent flow.

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Table 1. Measured vs. simulated streamflow, water table and ET[a]

Year	Rainfall (mm)	Streamflow (mm)				Water table Depth (m)[b]				ET (mm)	
		M	S	R^2	E	M	S	R^2	E	PET	AET
2003	1671	2.01	1.90	0.62	0.53	0.58	0.52	0.91	0.90	914	874
2004	962	0.28	0.37	0.60	0.56	0.81	0.78	0.65	0.65	1160	774
2005	1540	0.84	0.87	0.61	0.51	0.76	0.71	0.79	0.78	1217	1037
2006	1255	0.38	0.52	0.77	0.75	1.02	1.09	0.66	0.65	1170	965
2007	923	0.16	0.21	0.79	0.73	1.06	1.16	0.73	0.56	1199	822
2008	1562	0.87	0.95	0.68	0.64	0.53	0.47	0.71	0.66	1156	1064

[a]: M=measurement; S=simulation; R^2 is the coefficient of determination; E is Nash-Sutcliffe model efficiency; PET is potential evapotranspiration estimated based on the climate at the meteorological station at Santee Headquarters; AET is the estimated actual evapotranspiration from 10 points where the 10 wells are located.

[b]: The water table depth is the distance from the ground surface to the water table level below the ground surface. The unit of water table depth is meter.

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Table 2. Initial values of important parameters of MIKE SHE simulation model[a].

Parameter	Value
Plant rooting depth (mm)	500
Leaf area index (LAI) (m^2/m^2)	0.2–6.6 (2.8 on average)
Potential evapotranspiration (PET) (mm/d) (P-M)	0.0–7.5
Detention storage (mm)	40
Manning M ($m^{1/3}s^{-1}$)	40
Initial water depth (m)	0
Soil water content at saturated conditions (WCSC) (m^3/m^3)	0.4–0.496
Soil water content at field capacity (WCFC) (m^3/m^3)	0.3–0.458
Soil water content at wilting point (WCWP) (m^3/m^3))	0.2–0.38
Infiltration ($\times 10^{-6}$ m/s)	1–100
ET surface depth (m)	0.2
Horizontal hydraulic conductivity ($\times 10^{-6}$ m/s)	10–800
Vertical hydraulic conductivity ($\times 10^{-6}$ m/s)	1–80
Drainage depth (m)	0.5
Drainage time constant (s^{-1})	1e−07
C_{int} (mm)	0.10

[a]: C_{int} is the coefficient of canopy interception used in MIKE SHE (DHI, 2005); P-M=Penman-Monteith method.

Table 3. Root mean squared error (RMSE) and Nash and Sutcliffe's model efficiency (E) from calibration for water table and streamflow[a].

Parameter	Value (min-max)	Water table		Streamflow	
		RMSE	E	RMSE	E
C_{int} (mm)	0.05–0.80	0.168–0.171	0.54–0.55	3.48–3.57	0.55–0.57
Drainage depth (m)	0.0–1.0	0.19–0.61	−4.93–0.45	3.20–3.76	0.50–0.64
Detention storage (mm)	5–100	0.18–0.32	−0.59–0.49	3.44–3.98	0.43–0.58
Manning M (m$^{1/3}$ s^{-1})	10–70	0.182–0.183	0.46–0.47	3.21–3.65	0.53–0.64
HHC (×10^{-6} m/s)	1–5000	0.18–0.94	−13.2–0.46	2.53–3.61	0.54–0.77
VHC (×10^{-6} m/s)	0.5–100	0.185–0.186	0.45	3.35–3.40	0.58–0.60
Plant rooting depth (mm)	300–900	0.17–0.24	0.05–0.54	3.48–3.50	0.57
Infiltration rate (×10^{-6} m/s)[b]	1–1000	0.182–0.183	0.47	3.48–3.54	0.56–0.57
WCSC (m^3/m^3)[b]	0.40–0.66	0.16–0.20	0.38–0.59	3.48–3.53	0.56–0.57
WCFC (m^3/m^3)[b]	0.30–0.50	0.15–0.21	0.33–0.63	3.45–3.49	0.57–0.58
WCWP (m^3/m^3)[b]	0.25–0.45	0.19–0.20	0.32–0.45	3.50–3.53	0.56–0.57

[a]: HHC=horizontal hydraulic conductivity; VHC=vertical hydraulic conductivity; WCSC=soil water content at saturated conditions; WCFC=soil water content at field capacity; WCWP=soil water content at wilting point; C_{int}=coefficient of canopy interception.
[b]: the value is dependent on soil type.

Table 4a. Measured and simulated groundwater table in 2003–2004[a].

Well	ELE (m)	Measured		Simulated		R^2	Significance (P)
		Mean	STD	Mean	STD		
W1	9.2	−0.77	0.29	−0.82	0.37	0.50	<0.01
W2	8.2	−0.42	0.60	−0.52	0.40	0.59	<0.01
W3	9.1	−0.28	0.29	−0.31	0.39	0.78	<0.01
W4	8.8	−0.70	0.38	−0.71	0.37	0.55	<0.01
W5	9.6	−0.86	0.47	−0.77	0.35	0.66	<0.01
W6	8.7	−1.29	0.72	−1.45	0.42	0.66	<0.01
W7	8.1	−1.46	0.55	−1.49	0.40	0.74	<0.01
W8	5.6	−0.77	0.42	−0.74	0.39	0.78	<0.01
W9	5.5	−0.54	0.38	−0.52	0.38	0.78	<0.01
W10	8.6	−1.36	0.59	−1.28	0.64	0.88	<0.01

[a]: The unit of water table depth is meter and negative value means water table below ground surface; STD is standard deviation; W1–W10 are the numbers of wells (Fig. 1).

Table 4b. Measured and simulated water table for automatic wells in 2005–2007[a].

| | W3 | | | | | W7 | | | | |
| | Measured | | Simulated | | | Measured | | Simulated | | |
year	Mean	STD	Mean	STD	R^2	Mean	STD	Mean	STD	R^2
2005	−0.40	0.37	−0.48	0.47	0.89	−1.14	0.63	−1.13	0.51	0.66
2006	−0.50	0.33	−0.60	0.48	0.89	−1.45	0.73	−1.43	0.61	0.46
2007	−0.64	0.32	−0.76	0.43	0.85	−1.52	0.73	−1.66	0.52	0.54

[a]: R^2 is the coefficient of determination; There wasn't measured water table data from 2005–2007 for the manual wells; STD is standard deviation.

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Table 5. Measured and simulated daily and monthly streamflow in 2004–2008[a].

Year	R^2	E	MM	MS	CV-M	CV-S
2004 daily	0.60	0.56	0.28	0.37	4.34	3.11
2004 monthly	0.84	0.84	8.6	11.4	1.54	0.99
2005 daily	0.61	0.51	0.84	0.87	2.50	2.74
2005 monthly	0.99	0.98	25.5	26.3	0.97	0.87
2006 daily	0.77	0.75	0.38	0.52	2.99	1.71
2006 monthly	0.95	0.84	11.5	16.1	1.18	0.88
2007 daily	0.79	0.73	0.16	0.21	3.49	1.75
2007 monthly	0.97	0.90	4.9	6.4	2.10	1.21
2008 daily	0.68	0.64	0.87	0.95	4.69	4.48
2008 monthly	0.94	0.91	29.5	33.5	1.42	1.06
2004–2008 daily	0.67	0.62	0.51	0.59	4.35	3.91
2004–2008 monthly	0.94	0.93	16.0	18.7	1.61	1.23

[a]: Daily streamflow is mm per day; monthly streamflow is mm per month; R^2 is coefficient of determination; E is model efficiency; MM is the mean of measurements; MS is the mean of the simulations; CV-M is the observed coefficient of variation; CV-S is the simulated coefficient of variation.

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Fig. 1. Watershed 80 on Santee Experimental Forest, South Carolina, USA.
(W1–W10 are the names (number) of the wells. WS79 (500 ha) is divided into three parts; they are WS77 (155 ha), WS80 (160 ha) and the part (WS79b) between WS77 and WS80, respectively.)
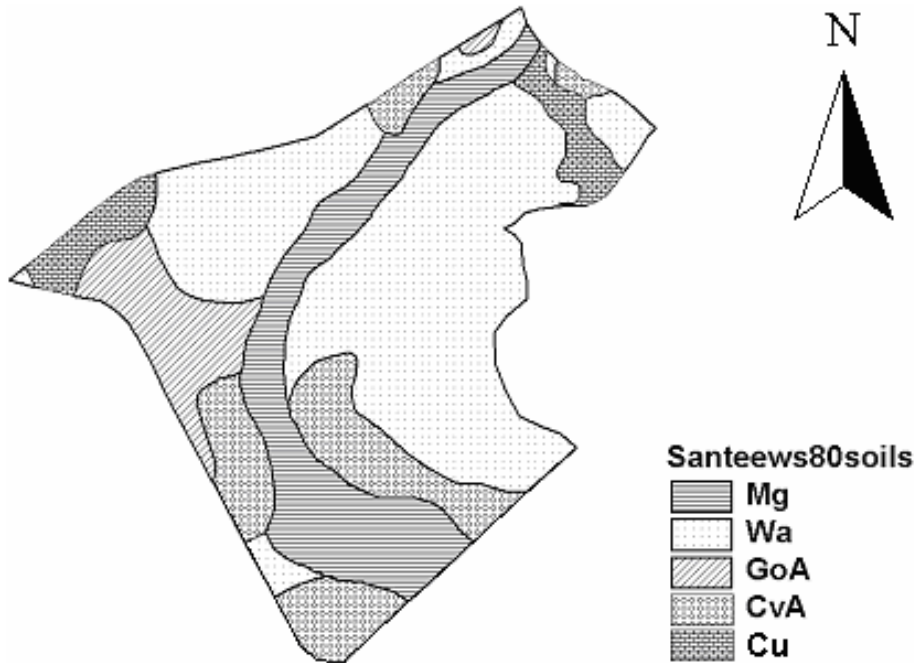
Fig. 2a. Soil distribution in watershed WS80.
(Mg=Meggett (loam); Wa=Wahee (loam); GoA=Goldsboro (sandy loam); CvA=Craven (loam);
Cu=Coxville (fine sandy loam).)
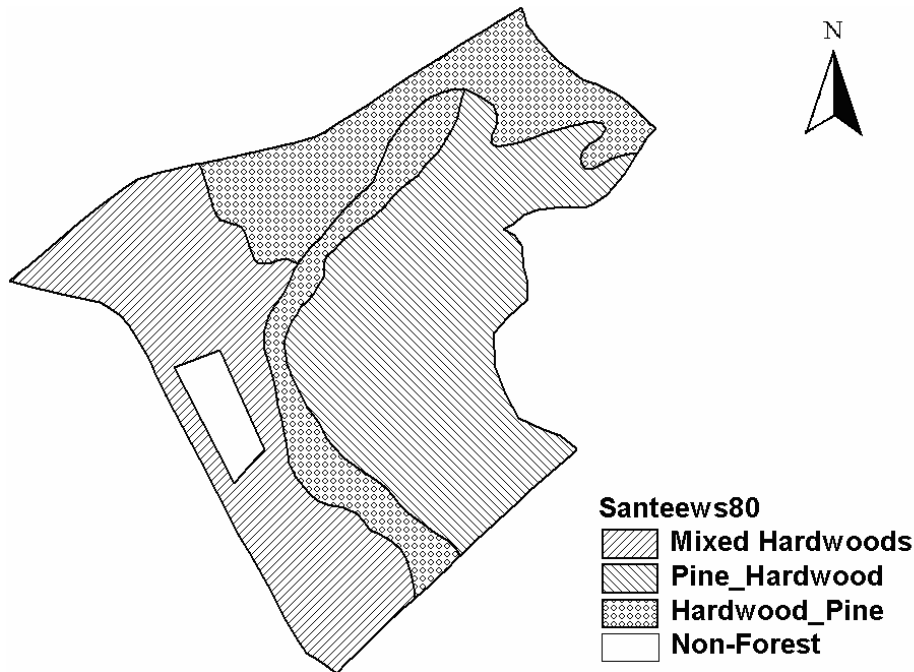
Fig. 2b. Vegetation types in watershed WS80.

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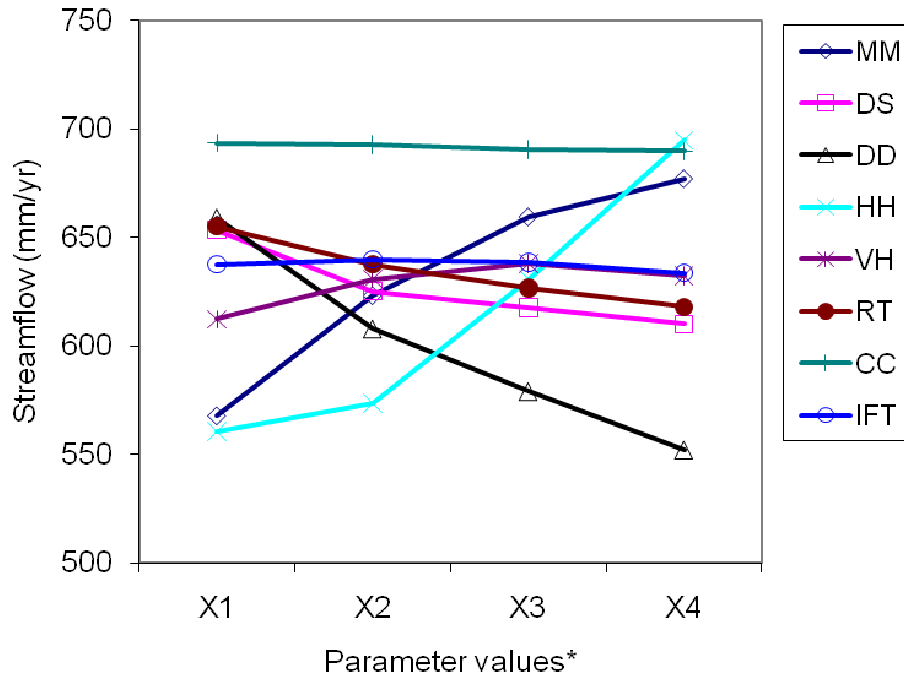
Fig. 3a. Sensitivity of streamflow to inputs.
(*: $X1$=10, $X2$=30, $X3$=50 and $X4$=70 m$^{1/3}$ s^{-1} for Manning M (MM); 5, 25, 50 and 100 mm for detention storage (DS); 5, 25, 50 and 100 cm for Drainage depth (DD); 1×10^{-6}, 4×10^{-5}, 4×10^{-4} and 5×10^{-3} m/s for horizontal hydraulic conductivity (HH); 5×10^{-7}, 5×10^{-6}, 5×10^{-5} and 1×10^{-4} m/s for vertical hydraulic conductivity (VH); 30, 50, 70 and 90 cm for plant rooting depth (RT); 0.05, 0.2, 0.4 and 0.8 mm for coefficient of canopy interception (CC); 8×10^{-6}, 8×10^{-5}, 8×10^{-4} and 1×10^{-3} m/s for infiltration rate (IFT).)

Fig. 3b. Sensitivity of water table to parameter inputs.
(*: $X1$=10, $X2$=30, $X3$=50 and $X4$=70 m$^{1/3}$ s^{-1} for Manning M (MM); 5, 25, 50 and 100 mm for detention storage (DS); 5, 25, 50 and 100 cm for Drainage depth (DD); 1×10^{-6}, 4×10^{-5}, 4×10^{-4} and 5×10^{-3} m/s for horizontal hydraulic conductivity (HH); 5×10^{-7}, 5×10^{-6}, 5×10^{-5} and 1×10^{-4} m/s for vertical hydraulic conductivity (VH); 30, 50, 70 and 90 cm for plant rooting depth (RT); 0.05, 0.2, 0.4 and 0.8 mm for coefficient of canopy interception (CC); 8×10^{-6}, 8×10^{-5}, 8×10^{-4} and 1×10^{-3} m/s for infiltration rate (IFT).)

Fig. 4. Effect of plant rooting depth on water table in watershed WS80.
(RT30, RT50, RT70 and RT90 represent 30, 50, 70 and 90 cm plant rooting depth, respectively.)
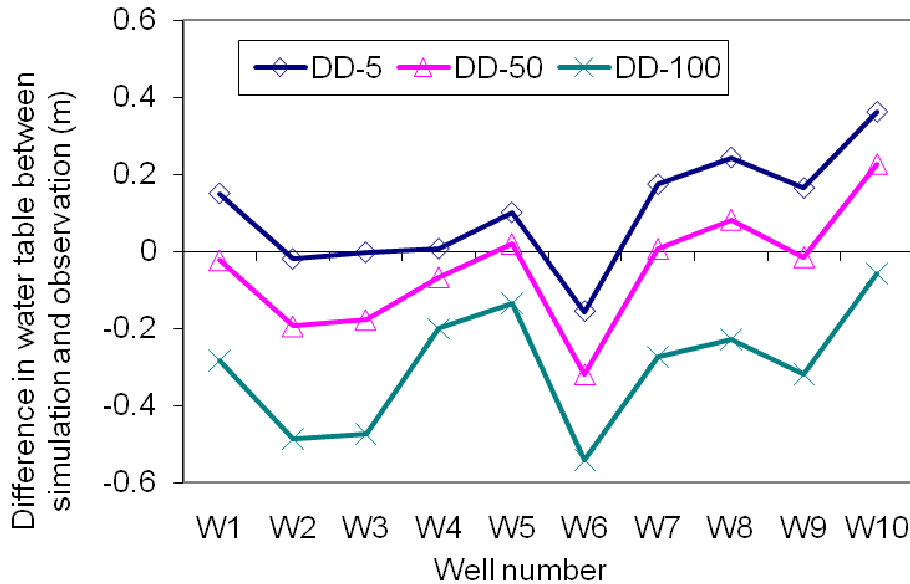
Fig. 5. Difference in water table depth between observed and simulated with different drainage depth from calibration.
(DD-5, DD-50 and DD-100 represent 5, 50 and 100 cm of drainage depths below the ground surface.)
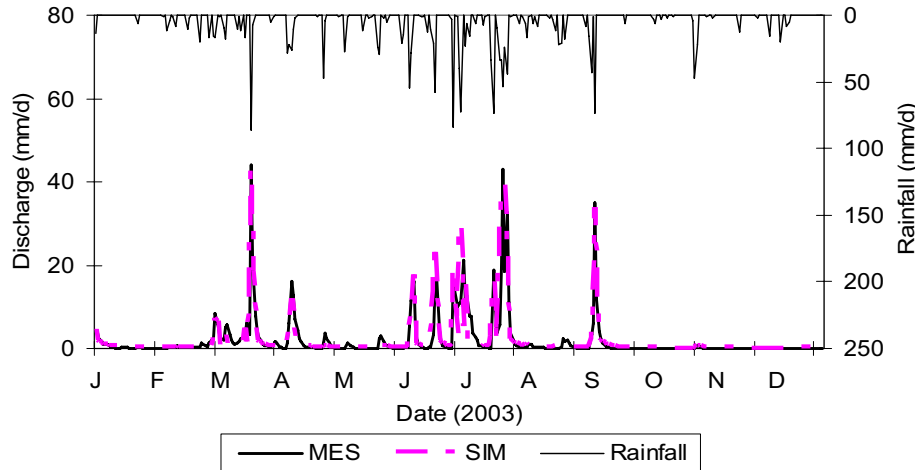
STD-MES = 5.3 mm/d   STD-SIM = 5.7 mm/d   ME = 0.03 mm/d   MAE = 1.05 mm/d

Fig. 6a. Measured vs. simulated daily streamflow in 2003.
(MES=measurement, SIM=simulation, STD=standard deviation, ME=mean error between simulation and observation, MAE=mean absolute error between simulation and observation.)

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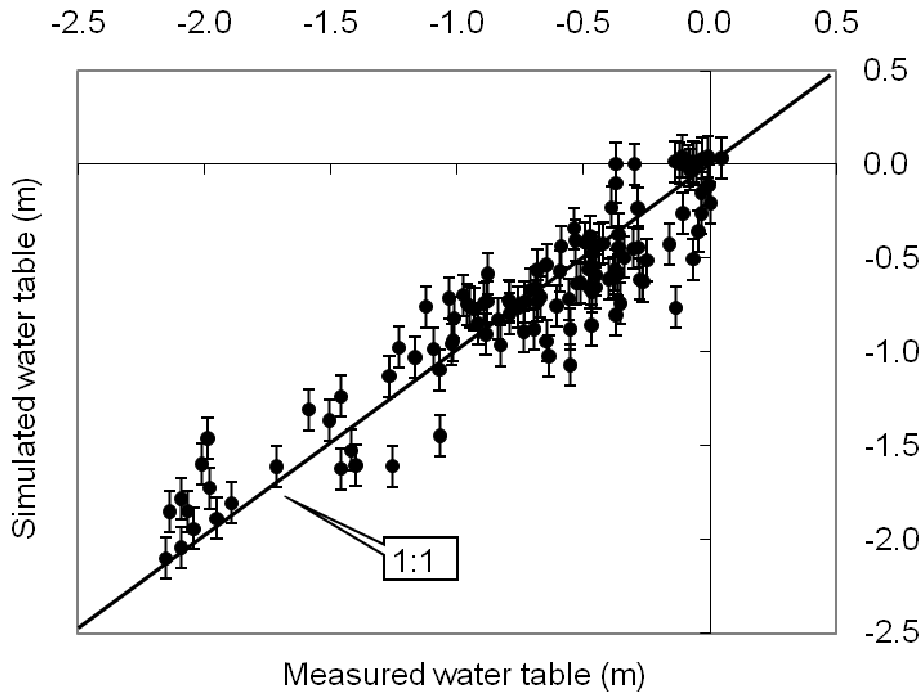
Fig. 6b. Water table at the 10 wells with synchronized observation in 2003. (Bar is the mean absolute error between observation and simulation.)

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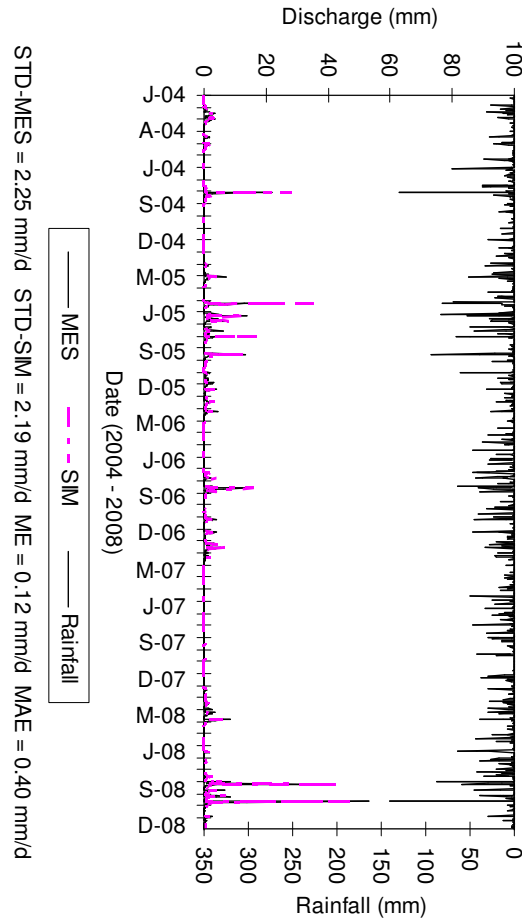
Fig. 7a. Daily streamflow simulated vs. observed in 2004–2008.
(MES=observation; SIM=simulation, STD=standard deviation, ME=mean error between sim-
ulation and observation, MAE=mean absolute error between simulation and observation.)
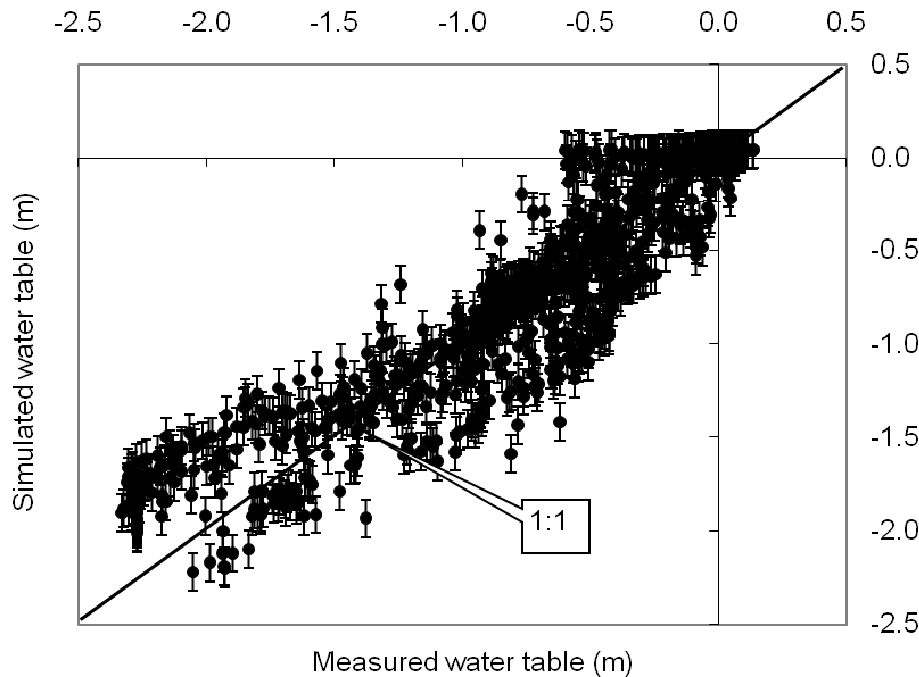
Fig. 7b. Water table depth observed vs. simulated at well W3 and W7 in 2004–2005. (Bar is the mean absolute error between observation and simulation.)
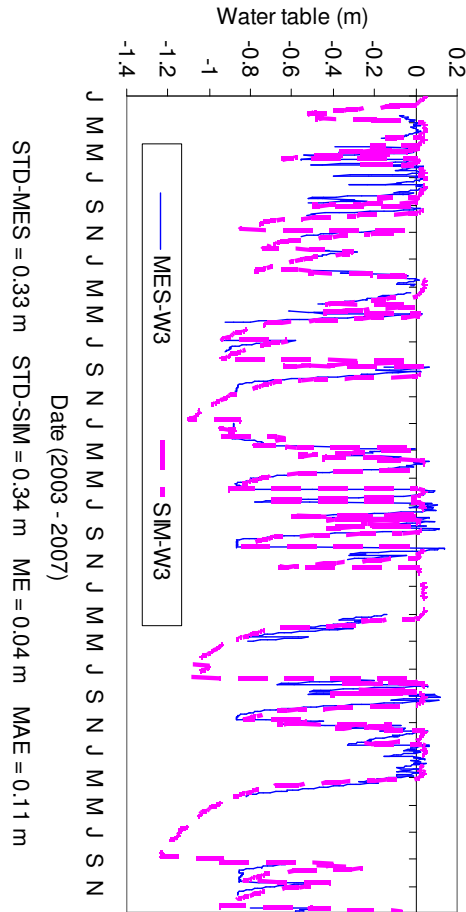
Fig. 7c. Temporal variation of groundwater table depth at well W3.
(This is a shallower well, and the depth was 54 cm below the ground surface before March of 2004, 94 cm after (Harder et al., 2007).)
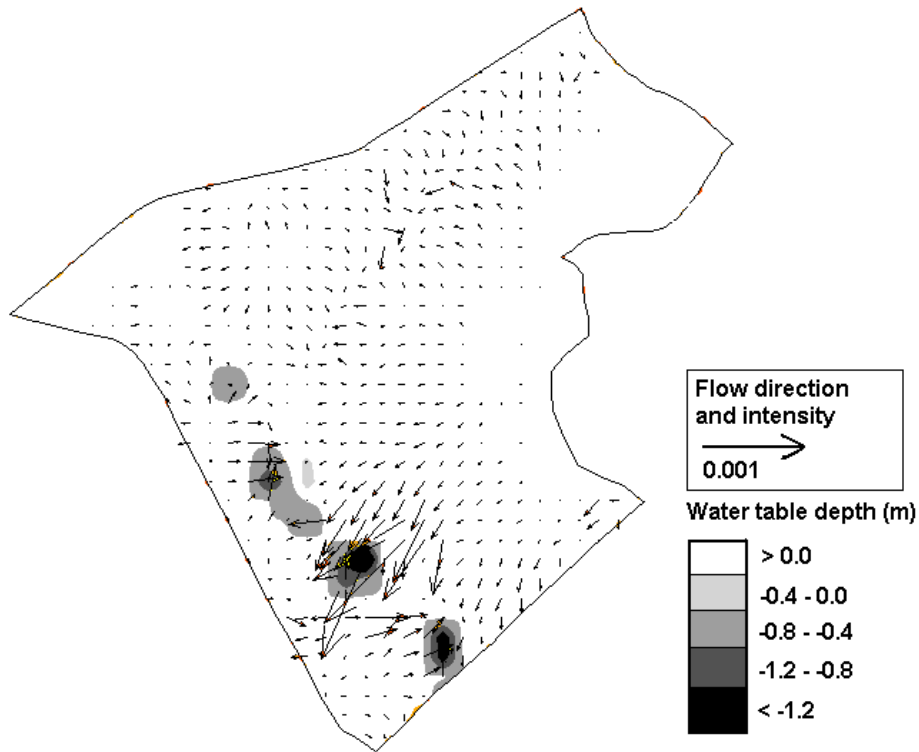
Fig. 8a. Spatial distribution of water table depth and flow direction after an intense storm.
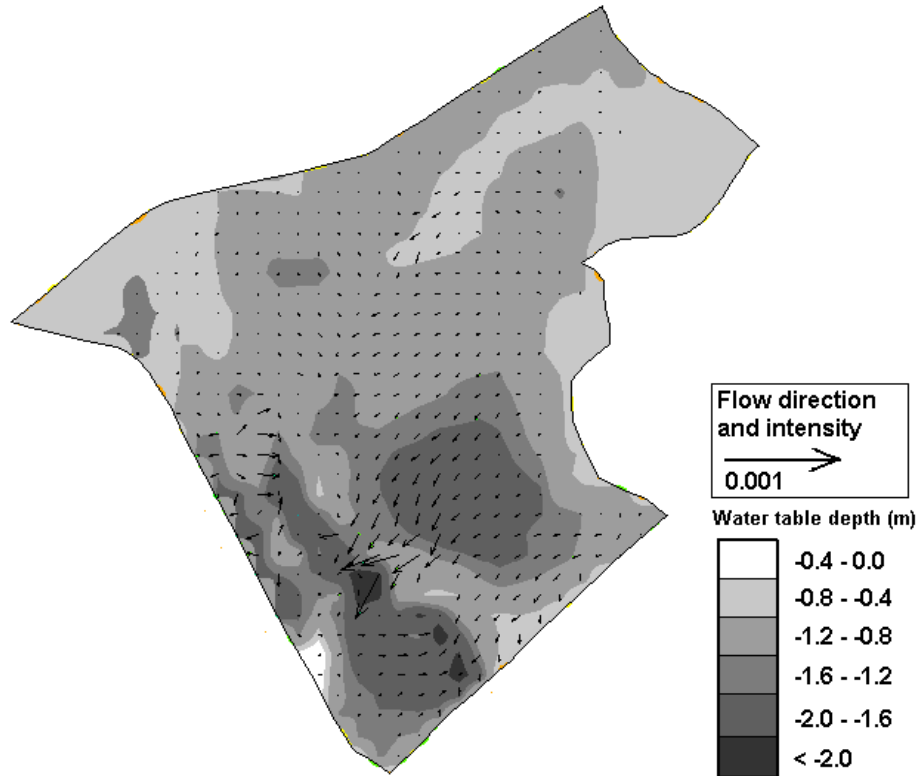
Fig. 8b. Spatial distribution of water table depth and flow direction in the duration of low precipitation.