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**Staged cost  
optimization of urban  
storm drainage  
systems**

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# Staged cost optimization of urban storm drainage systems based on hydraulic performance in a changing environment

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Received: 17 April 2008 – Accepted: 21 April 2008 – Published: 17 June 2008

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Published by Copernicus Publications on behalf of the European Geosciences Union.

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## Abstract

Urban flooding causes large economic losses, property damage and loss of lives. The impact of environmental changes mainly, the urbanization and the climatic change leads to increased runoff and increased peak flows which the drainage system must be able to cope with to overcome possible damage and inconveniences caused by the induced flooding. Allowing for detention storage to compliment the capacity of the drainage system network is one of the approaches to reduce urban floods. The traditional practice was to design systems against stationary environmental forcings – including design rainfall, landuse, etc. Due to the rapid change in climate-environment, this approach is no longer economically viable and safe, and explicit consideration of changes that gradually take place during the life-time of the drainage system is warranted. In this paper, a staged cost optimization tool based on the hydraulic performance of the drainage system is presented. A one dimensional hydraulic model is used for hydraulic evaluation of the network together with a genetic algorithm based optimization tool to determine optimal intervention timings and amounts throughout the lifespan of the drainage network. The model was applied in a case study area in the city of Porto Alegre, Brazil. It was concluded that considerable financial savings and/or additional level of flood-safety can be achieved by approaching the design problem as a staged plan rather than one-off scheme.

## 20 1 Introduction

Floods are a constant threat to life and property. An increasing trend in extreme flood events can be observed in many countries around the world. Annual statistics of natural disasters over the past 30 years show that the number of floods is rapidly increasing around the world in comparison with all other disasters (Dutta et al., 2003). In 2007, more than 16 thousand people were killed, over 234 million others were affected and almost 75 US\$ billion worth of assets were damaged of which floods and windstorms

HESSD

5, 1479–1509, 2008

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remain the most important source. They accounted for more than 86% of the overall disaster mortality and hydro-meteorological disasters accounted for more than 98% of total affected (Sapir, 2008).

With urbanization, the impermeable surfaces increase whereas the natural water courses change to drainage systems. As a result, the runoff increases which yields more frequent and faster floods with greater peak flows. This leads to the requirement of proper drainage solutions to avoid flooding and prevent the possible occurrence of inconvenience, damage and adverse health impact.

With global climate changes, extreme climate events have been predicted in different regions and as consequences of such extreme events, large floods are predicted (Houghton et al., 2001). In addition to that, the increased imperviousness due to urbanization and changes in land use has increased the exposure and vulnerability to flash floods (Gruntfest and Rippis, 2000). Scientists and engineers have been constantly experimenting with various means to reduce this vulnerability and exposure of urban areas by various structural and non-structural measures. Apart from the traditional approaches that primarily involve improvements in conveyance capacity, on-site measures such as provision of detention ponds, infiltration ponds, rain water collection and reuse etc., can be used to reduce the runoff peaks and reduce the downstream flooding by providing the flood attenuation during the rainfall events (Woods-Ballard et al., 2007). Further they often can be parts of integrated urban water management solutions that provide many other positive outcomes than flood control.

The term “Whole Life Costing” (WLC) has been around for two decades being born out of the recognition that initial capital costs often represent a only small portion of the lifetime costs of an infrastructure system. Whole life costing approaches have been shown to provide an ideal platform for the evaluation of sewer system by taking account of system behaviour, performance and regulation within a sensible economic and engineering framework (Shepherd et al., 2004). Thus, the term “whole life costing” includes all the cost components of the asset from planning, designing, construction, repair and maintenance, rehabilitation and the salvage value, evaluated in terms of net

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present value. However, the use of the term “whole life cost” in this study covers only the two major costs: flood cost and intervention cost.

This paper presents a whole life (staged) cost optimization tool that can be used to determine the planning of detention delivery based on the hydraulic performance of the storm water drainage system for flood control and provides a basis for proactive decision making in a changing environment.

We implement a hydraulic analysis based genetic algorithm optimizer as an aid to determine staged intervention strategies for urban storm water drainage systems subjected to constantly changing external forcings like climate change, demographic and landuse changes, etc. Applying the tool to an urban system in Brazil, we argue that the staged approaches are not only more flexible as they allow the decision makers to keep options open in the backdrop of large degree of uncertainty, but also show that they are more economically efficient without sacrificing the degree of safety provided. The following section explains the model implementation, followed by data used for the model with major assumptions made. Then we present the case study application in Porto Alegre, Brazil, and show that the staged approach is indeed a better design paradigm compared to traditional “implement-once and operate” schemes. There is a significant degree of possible variability in input variables used in this study, due to the obvious fact of large uncertainty involved in future scenarios. In order to ensure that the qualitative conclusions drawn are not affected by this, we conducted an analysis of the sensitivity of each input variable to the model prediction. Selected sensitivity results are presented. Finally there is a discussion on the results obtained, together with general conclusions.

## 2 Whole life cost optimization model

The decision making tool developed consists of a dynamic hydraulic model coupled with a lumped-physically based hydrological model, that computes runoff from urban catchments and flow in conveyance elements. Flooding is considered only in volumet-

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ric terms (as overflow from sewers), not considering the inundation effects explicitly. We use EPA-SWMM 5.0 (EPA, 2008) as the hydrologic/hydraulic simulator in the tool. The EPA Storm Water Management Model (SWMM) is a dynamic rainfall-runoff simulation model used for single event or long-term (continuous) simulation of runoff quantity and quality from primarily urban areas. It is a physically based discrete-time simulation model which employs principle of conservation of mass, energy and momentum wherever appropriate. It solves the numerical equations explicitly and has ability to automatically adjust the time step as needed to maintain the numerical stability (Rossman, 2004). Apart from these important features, SWMM is also available in free open source form which makes it convenient to be adopted for the present purpose.

The hydrology was implemented as physically based lumped model based on single hillslope analogy, using the SCS curve number based runoff computation (Gupta, 2001). The reason for the adoption of curve number method was historical as the available data for Porto Alegre are based on this approach. The hydraulic network model was developed using the full one dimensional Saint Venants equation. The hydraulic performance of the network model is analysed to find out the specific locations where flooding occurs for 1 h 50 years design storm. While there is no quantitative data on flooding history, the qualitative input from the authorities in Porto Alegre was used to validate these. Public open spaces were selected to provide detention ponds.

We assumed that the detention storage is provided in a staged fashion, interventions done once at regular period (10 years in the example). For example, it is possible for a location to have no detention storage until 20th year, then  $2000\text{ m}^3$  storage can be build at that stage and introducing a further  $5000\text{ m}^3$  storage at the end of 40th year. The advantage of this scheme is the possibility to defer investment until the necessary moment arrives (due to increasing tendencies in environmental forcings like rainfall, landuse change etc.) and thereby achieving cost-savings in net present value terms. The detention schedule is optimized using a total cost function which comprises of cost of interventions and the residual flood damage, as objective function.

The flood damage is computed in terms of expected annual damage that can be

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defined as:

$$E(D) = \int_{f=0}^{\infty} D(z)df \quad (1)$$

Where  $E(D)$  is the expected annual damage,  $D(z)$  is the flood damage caused by flood depth  $z$ ,  $f$  is frequency of occurrence (inverse of return period) and  $z$  is a function of flood depth and extent. In this study we use a major assumption that the flood water volume is linearly related to the  $D(z)$  above, which is not strictly valid for complex urban terrain. Further instead of the continues integral above, we use a difference approximation that involves a set of return periods (1 year, 5 year, 20 year and 50 year). Equation (2) shows this computation.

Equation (3) shows the computation of net present value of total intervention (in this case construction of detention storage facilities at different stages) costs.

$$TPFC = \sum_{j=1}^{n \text{ steps}} \left[ \left\{ \sum_{i=1}^{n \text{ return } p} \left( \frac{V_{i+1} + V_i}{2} \times C_f \times \left( \frac{1}{R_i} - \frac{1}{R_{i+1}} \right) \right) \right\}_j \times FD_{fj} \right] \quad (2)$$

$$TPIC = \sum_{j=1}^{n \text{ steps}} \left[ \left\{ \sum_{i=1}^{n \text{ interv}} (SA_i \times SD_{\max(i)} \times C_d) \right\}_j \times ID_{fj} \right] \quad (3)$$

Where, TPFC is the total present flood cost,  $R_i$  is the  $i$ -th return period,  $V_i$  is the volume flooded due to  $i$ -th return period,  $C_f$  is the unit cost of flooding per unit volume,  $R_j$  is the return period, TPIC is the total present intervention cost,  $n$  steps is the number of time steps,  $n$  interv is the number of intervention places,  $SA_i$  is the storage area at intervention location,  $SD_{\max}$  is the storage depth,  $C_d$  is the unit cost of detention per unit volume,  $FD_f$  is the flood discount factor and  $ID_f$  is the intervention discount factor

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for each time step  $j$  calculated using the Eqs. (4) and (5) respectively.

$$FD_{fj} = \sum_{i=0}^{\Delta t-1} \frac{1}{(1 + \frac{r}{100})^{tstep * \Delta t + i}} \quad (4)$$

$$ID_{fj} = \frac{1}{(1 + \frac{r}{100})^{tstep * \Delta t + \frac{\Delta t}{2}}} \quad (5)$$

Where,  $\Delta t$  is time duration of each time step and  $r$  is discount rate in percentage;  $j$  is the corresponding time step (tstep).

The optimized cost among the number of generations is thus given by the Eq. (6).

$$\text{Optimized cost} = \min(\text{TPFC} + \text{TPIC}) \quad (6)$$

There are numerous approaches to implement the optimization scheme for this problem. We selected genetic algorithms (Fraser, 1957; Goldberg, 1989), largely due to their robustness in handling complex problems that display large variability and intermittency in input parameters and large-degree of nonlinearity in functional relationships (Mitchell, 1997). Genetic algorithms are implemented as a computer simulation in which a population of abstract representations called chromosomes are implemented as vectors of bits (0 or 1), integers or real numbers. We used real-number based chromosomes in this study. Initially many individual solutions are randomly generated to form an initial population. Then the process of “evolution” is mimicked by a large number of steps of generation. During each successive generation, a proportion of the existing population is selected as ‘parents’ to breed a new generation. Individual solutions are selected through a fitness-based process, where fitter solutions are typically more likely to be selected. Due to this “selection pressure”, statistically the population evolves in a direction that increases the fitness of its members. Compared to calculus based optimization schemes that are used in simpler problems, one drawback of genetic algorithms is the inability to guarantee that a reached solution represents the

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true global optimum of the domain, because it can be trapped in local optima or even temporarily stranded in a mild gradient sector. It is possible to overcome this dilemma at least to some degree by extensive repetition of trials with random initializations. This is quite practical with fast computers available today, particularly with the ease with 5 which the genetic algorithms render themselves to distributed parallel computing.

The development of the numerical optimization scheme is done using a genetic algorithm optimizer with minimization of whole life cost as objective function. The integration of whole life cost component is carried out with numerical optimization and hydraulic simulator (SWMM) with interfacing code written in the C language.

10 Figure 1 gives the conceptual diagram of the model.

### 3 Data used for the cost optimization model

A variety of data types are needed as input for the optimization model. Most of these, in the present case are based on major simplifications. Most of these simplifications are directly a consequence of uncertainty and unpredictability of future change both 15 local and global.

#### 3.1 Land-use and climate change related data

Climate change and urbanization, the two basic phenomena of the changing environment are parameterized to take into account their effects on urban drainage. The change in precipitation data with time is estimated based on the climate studies, which 20 provides the rate of rainfall increase. The current rainfall time series data is multiplied by the rainfall increase rate of corresponding time step to get the time series data for that time step.

An indirect method of evaluating the impact of urbanization on drainage is used relating population density with the extent of impermeable cover, which in its turn is 25 directly correlated with runoff coefficients. Figure 2 gives a typical relationship of the

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impermeable area with urban density (Campana and Tucci, 1994). For population densities below 20 hab/ha (beyond the curve in Fig. 2), a linear relation is used as shown in Eq. (7).

For density,  $D < 20$  hab/ha,  $\text{impf} = 0.403 * D$  (7)

5 Where,  $\text{Impf}$  is impervious area (%) and  $UD$  is urban population density.

The estimation of the impervious fraction thus depends on the population density; ultimately the population density is estimated at different times using a population growth rate for the area as represented in Fig. 3, which is calculated from the available population data of Porto Alegre. (Source: Wikipedia, Porto Alegre).

## 10 3.2 Rainfall data

The rainfall data for this study is derived from an IDF relationship given by Eq. (8) (Bemfica, 1999). Using the relation, the cumulative rainfall is derived from which the time series data is prepared using the alternating block method (Fig. 4). Four return periods, 1 year, 5 years, 20 years and 50 years are considered for the study.

$$15 i = \frac{826.8 * R^{0.143}}{(t + 13.3)^{0.79}} \quad (8)$$

Where,  $i$  is intensity mm/hr,  $R$  is return period in years,  $t$  is duration of rainfall.

## 3.3 Soil data and estimation of corresponding model parameter

For the computation of the runoff in SWMM, the SCS curve number method is used. The mathematical relationship derived from the available data of impervious factor with 20 curve number is used to estimate the curve number of the different hydrological soil groups with changing impervious fraction with time. The common relation between CN and impervious fraction for different hydrological soil groups are derived Eqs. (9) to (13).

$$\text{For } A, \text{CN} = 0.58 \text{Impf} + 39.301 \quad (9)$$

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$$\text{For B, } \text{CN} = 0.3776 \text{ Impf} + 60.542$$

(10)

$$\text{For C, } \text{CN} = 0.2448 \text{ Impf} + 73.916$$

(11)

$$\text{For D, } \text{CN} = 0.188 \text{ Impf} + 80.205$$

(12)

### 3.4 Genetic algorithm parameters

5 The genetic algorithm starts working from the initialization of random population that corresponds to a series of trial detention schedules and selection based on fitness function and breeding is carried out. The process undergoes a number of repetitions to find the “best solution” meeting the objective function. The numerical implementation of the genetic algorithm requires the following parameters to be defined: the population size, stop criteria, length of the chromosome (should correspond to the number of values in the detention schedule), crossover probability and mutation probability. For this study, the population size 10, no of variables 20, cross over probability 0.8, mutation probability 0.5, tournament size 5, range of real mutation 0.01 are used. The steady state generation condition is adopted by defining the number of steady generations without any improvements to 10 numbers of constant values instead of defining the maximum generation size in order to allow the GA to search for the best optimal values.

## 4 Model application: case study of Porto Alegre, Brazil

20 The model has been applied to a part of storm water drainage network of Porto Alegre, Brazil. The case study area is the Arroio da Areia Basin of Porto Alegre municipality, the capital city of the Brazilian state of Rio Grande do Sul. The Arroio da Areia basin occupies an area of  $20.85 \text{ km}^2$ , of which approximately half ( $11.7 \text{ km}^2$ ) corresponds to the basin of Arroio da Areia, and the remaining belongs to Airport Polder. This study deals with the upstream part of basin Arroio da Areia.

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## 4.1 Original system description (sub catchment A)

The sub catchment A is the upstream part of the Areia basin and occupies a drainage area of 236.8 hectare and can be sub divided into five zones (A1 to A5) as shown in Fig. 5. The study area is drained through 2.412 km length of drainage networks at 5 macro level which are considered for this study.

The original system consists of a simple drainage network with 12 nodes, 11 links and 17 sub catchments. Figure 6 shows the system network with the four most flooded nodes when analyzed for a 1 h 50 years design storm at present situation and having space available for provision of detention storages.

### 10 4.2 Optimal solutions

The network is tested for 10 different trials for a flood unit cost of 100R\$/m<sup>3</sup> and a discount rate of 2%. The model output consists of the total optimized cost, maximum cost from which GA starts optimization, flood cost and intervention cost. The output results for the trials are studied in terms of the maximum values, minimum values, averages and standard deviations.

15 All ten trials show a convergence of the cost to minimum values as shown in Fig. 7 and the best optimal solutions at different trials compared with the do nothing case is shown in Fig. 8. The best optimal value among the trials is equal to R\$ 42 260 000. The number of generations observed in this trial is 927 and the cost is optimized by 20 R\$ 34 200 000 (45%) with respect to the do nothing scenario. The composition of the optimized cost consists of flood cost of R\$ 17 300 000 (41%) and intervention cost of R\$ 24 900 000 (59%).

#### 4.2.1 Detention volumes at different trials in optimal solutions

25 The corresponding detention volumes recommended for the optimal solutions in different trials vary exploring the possibilities of different alternatives. Figure 9 shows the

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variation of the detention volumes at storage S1 at different trials and in different time steps.

The recommended detention schedule from the best optimal trial (trial 8) in 1000 cubic meter units is given in Table 1. For example, 10 detention units are to be constructed in current year, then extended to 16 units in next 10 years and 17 units after 20 years, and 20 units after 30 years, after which, there is no need for further construction in the case of Storage S1.

#### 4.2.2 Sensitivity with respect to the input data parameters

There are numerous inputs in the present model that are based on quite broad assumptions, as was discussed in Sect. 3. Therefore it is important to examine the model's sensitivity to variations of these parameters. A number of sensitivity analyses were done in order to assure that the qualitative nature of the trends is not affected by the assumptions. Four of these tests are described below. The uppermost line shows the cost in do nothing case, the other solid lines show the costs in staged option and the dotted lines show the corresponding costs in static option in all the tests.

##### *Sensitivity to flood unit costs*

With a change in flood unit costs, the costs are increased with an increase in flood unit costs. The no intervention cost increases very rapidly compared to other costs with intervention cases as shown in Fig. 10. Thus, the effectiveness of provision of detention storage is higher in higher flood cast.

##### *Sensitivity to rainfall increase rate*

With a change of rainfall increase rate, a linear change of optimized cost is observed as shown in Fig. 11. It is observed that the total cost increases by 16% when the rainfall increase rate is increased by 100%.

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With change in population growth rate, the output data follow a linear trend.

5 The total cost is increased by 32% when the population growth rate is increased by 100% (Fig. 12).

*Sensitivity to discount rate*

10 The optimal costs follow a logarithmic trend with respect to discount rates. The smaller the discount rates, the higher the cost values. While the discount rate changed from 1% to 9%, the no intervention cost reduced to 21% where as the total optimal cost reduced to 33% as shown in Fig. 13. The reduction for smaller costs is also smaller.

15 **5 Conclusions**

An automated cost-optimization tool to compute the cost-benefit of staged intervention to reduce flood damage in urban drainage networks, based on hydraulic performance was implemented. The model takes into account the changes of the operating environment (both local and global) during the lifetime of the network in cost optimization

20 analysis.

The tool was applied to a case study network representing a part of the city of Porto Alegre, Brazil. The application showed that the model can be used as a tool for decision making on staged construction plans, in order to reduce the total cost that comprises the investment cost and the damage due to residual floods, in terms of net

25 present value. Under reasonable assumptions of population growth, rainfall change and inflation-adjusted discount rate, we showed that the option of staged intervention is significantly advantageous in financial terms.

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As an example, the crude calculation made based on the stage damage relationship given by (Nascimento et al., 2006) urban floods are typically in the depths of about 0.2 m to 0.6 m, the corresponding damage per cum flooded volume are approximately 201R\$ to 170R\$/m<sup>3</sup> and for the cost cases of 100 to 300, the total cost savings found to be 1% to 4%, consequently the flood cost is reduced by 7% to 25% more though the intervention costs are higher in staged option. For the optimized detention schedule shown in Table 1, thus, certain savings in total cost can be realized with reduced flood cost and at the same time more investments can be done as compared to the single stage implementation. This itself is a significant financial saving; however, this is only a part of the advantage of a staged construction policy. The staged approach inherently allows for flexibility in decision making that makes it possible for the decision maker to revise some decisions made today, in light of better information on a future date. For example the level of uncertainty on the rainfall during the last ten years of a project with 50 year life-span often will be much less at 25th year than it is at the inception of the project. Thus, even the proof that the staged approach is not costlier than a built-once approach is adequate evidence in favor of the staged approach. However, at the same time, it should be borne in mind that such staged approaches are possible only for certain components of the whole urban drainage picture. For example staged increase of size of conveyance is often prohibitively expensive and thus makes little sense. However, other components like detention storage, infiltration ponds, plot-scale interventions, etc, render themselves very well to this approach.

By provision of the detention storages, the flooding volume can be reduced by 85% due to 1 year storm, 80% due to 5 year storm, 72% due to 20 year storm and 49% due to 50 years storm when the flood unit cost is 100R\$/m<sup>3</sup>. Thus, it helps better manage the flood volume using the existing drainage system (network) with provision of optional detention storages.

The contribution of the rainfall with smaller return periods is higher as compared to the higher return periods in terms of annual expected flood damage. However, in planning it is often necessary to consider issues beyond mere financial cost. For

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example while large floods may cause less damage in terms of expected annual cost due to their low frequency, but when occurred they can be catastrophic, causing loss of human life. However, such floods are often related to rivers and are not caused by small urban watersheds.

5 Though the model works to search the optimal solution in a random probabilistic basis and the chances of getting local optimized value still exists (this is an inherent problem with genetic algorithms). It provides a contingency plan of detention storages based on the optimized cost values to be implemented for the network, which specifies the volume of detention storage to be provided and the intervention timing. This helps 10 in decision making for the allocation of the resources as well as provides an implementation plan for flood management in the specified time frame. Also, the detention plan is best suited to the changing environment as the possible impacts of urbanization as well as climate change have already been considered. Still, the reliability depends on the estimation of the changes in the environment.

15 The model is developed considering flood cost as penalty cost from nodal floods and the intervention costs only include the construction cost of the detention storages. Apart from these, other costs like maintenance cost of detention storages, repair and maintenance of network costs, planning and designing costs, initial construction costs etc also affect the whole life cost. Thus it is better if all those costs can be incorporated 20 in assessing the whole life cost.

The major contribution of the present approach is to encourage the planner to take a different philosophical direction in urban drainage planning and design, namely, to depart from a static, inception-stage implementation to a staged approach in urban flood defense. This is particularly useful in today's environment of highly dynamic changes in 25 local and external forcings that makes the design process quite uncertain. The staged approach provides the added advantage of flexibility to allow for change during the life time of a project. This approach can also help in resource limited situations that third world countries often face. It is usually more practical both in terms of financing and capacity to adopt a staged approach. We have shown that such an approach is

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**Table 1.** Recommended detention units for construction at different time and storages.

| Storage | Time steps |    |    |    |    |
|---------|------------|----|----|----|----|
|         | 0          | 1  | 2  | 3  | 4  |
| S1      | 10         | 16 | 17 | 20 | 20 |
| S2      | 0          | 0  | 4  | 4  | 4  |
| S3      | 5          | 7  | 7  | 10 | 10 |
| S4      | 0          | 0  | 1  | 1  | 1  |

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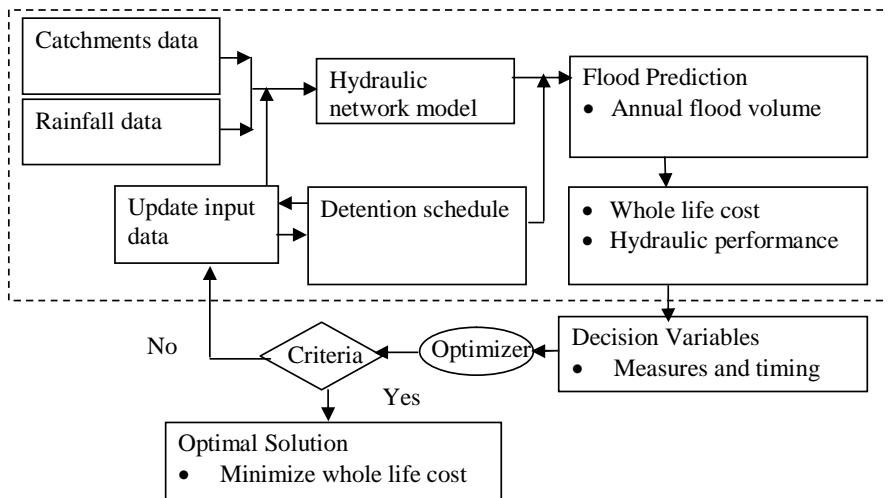
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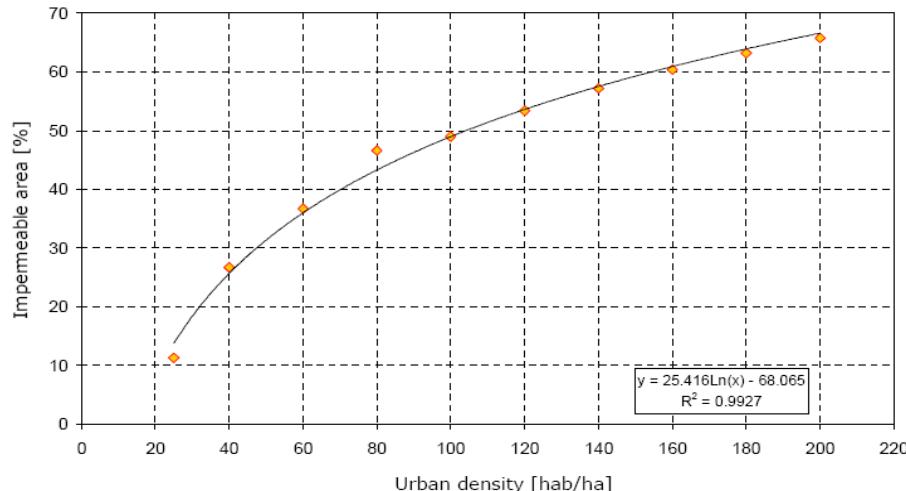


**Fig. 1.** Conceptual diagram of whole life cost optimization model.



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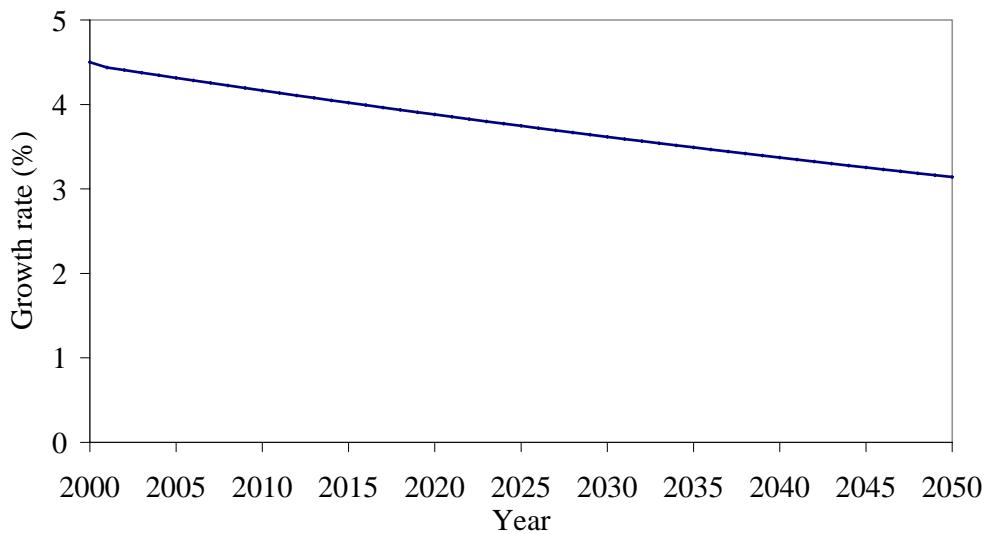


**Fig. 2.** Relations of urban density with impervious percentage.

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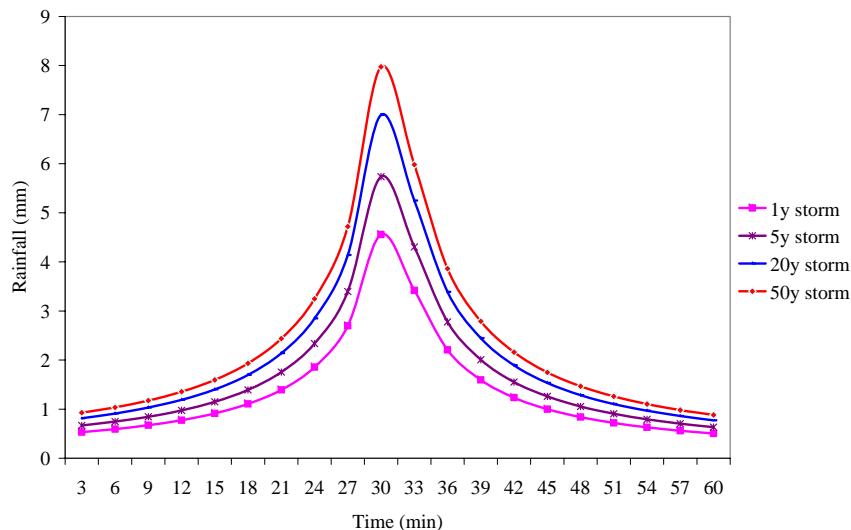


**Fig. 3.** Forecasted population growth rate for Porto Alegre, Brazil.

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**Fig. 4.** Time series data for different return periods.

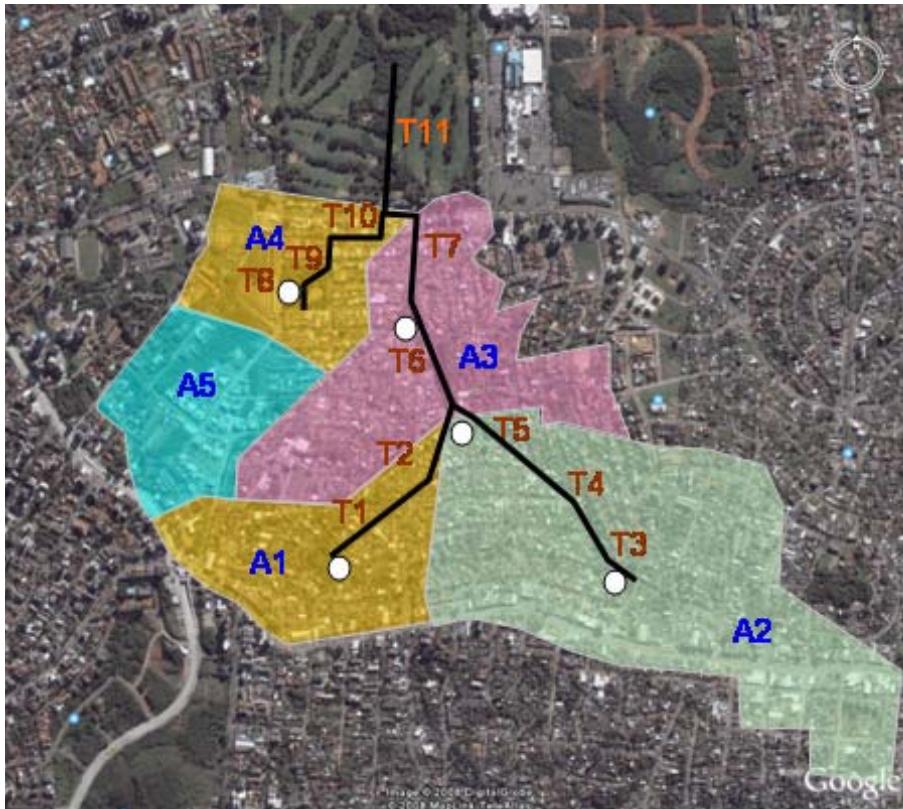
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**Fig. 5.** Sub catchment A of Porto Alegre (Arroio da Areia) basin.

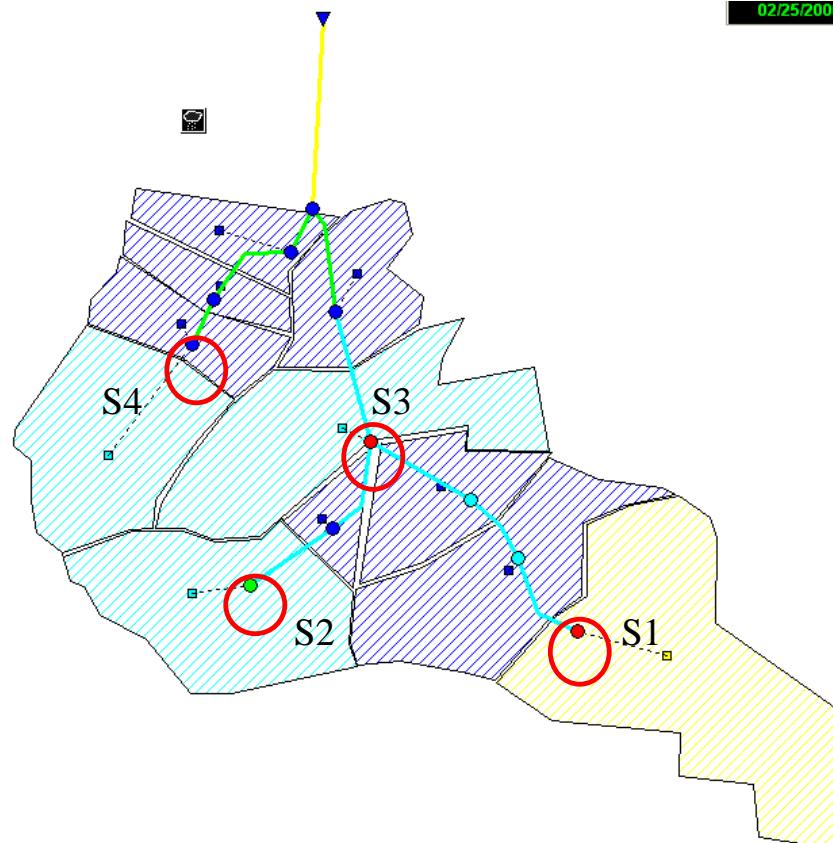
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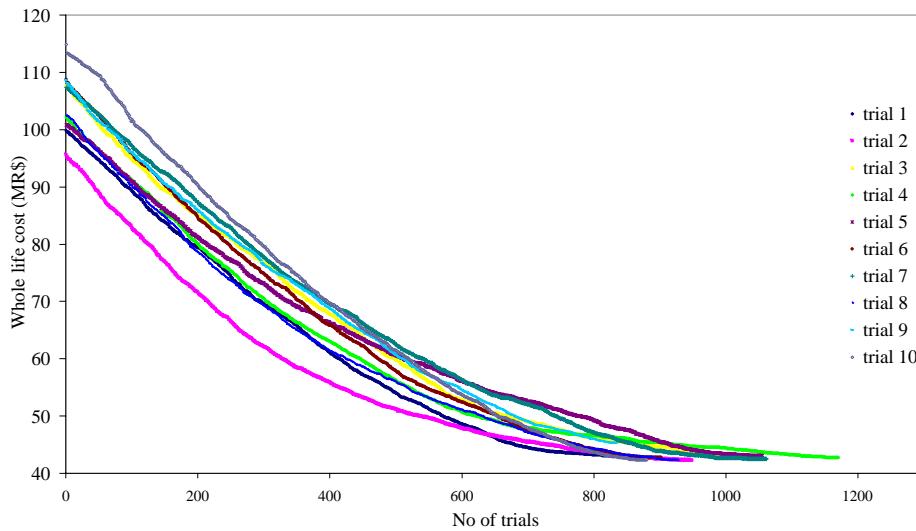
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**Fig. 6.** Existing drainage network with flooded nodes.

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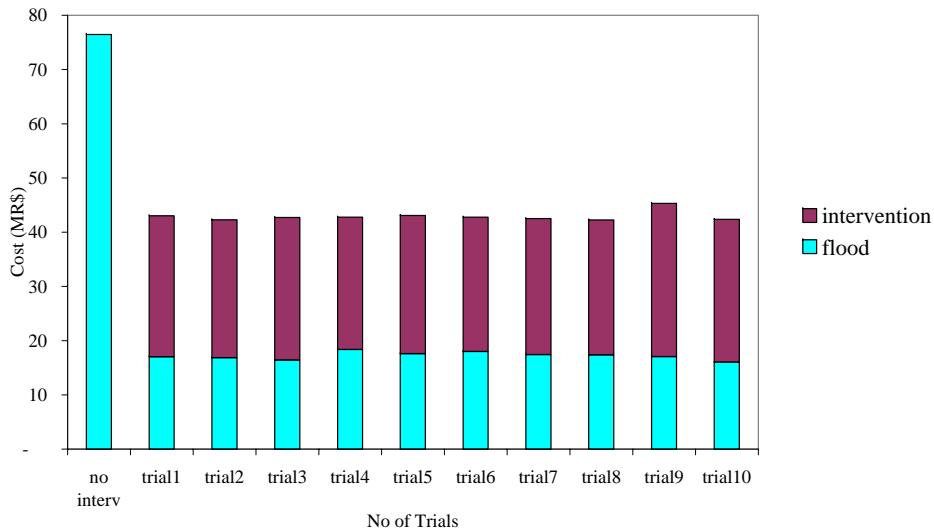


**Fig. 7.** Optimization patterns in different trials.

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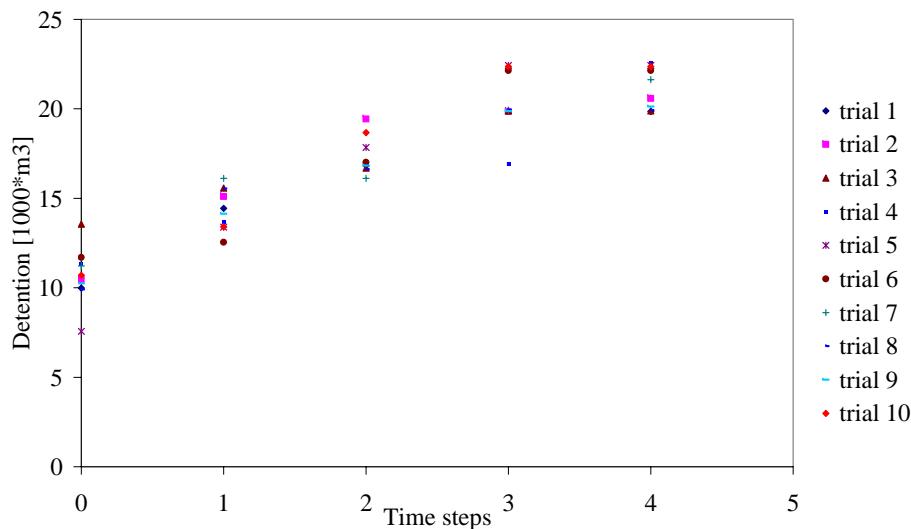


**Fig. 8.** Optimal costs scenarios in different trials.

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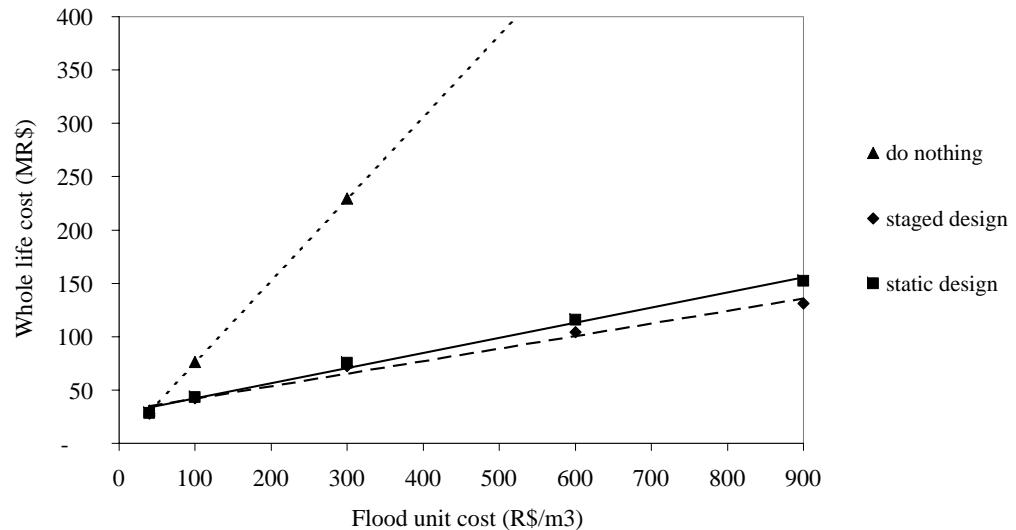
**Fig. 9.** Detention volumes at different trials for cost unit of 100R\$/m<sup>3</sup> case.

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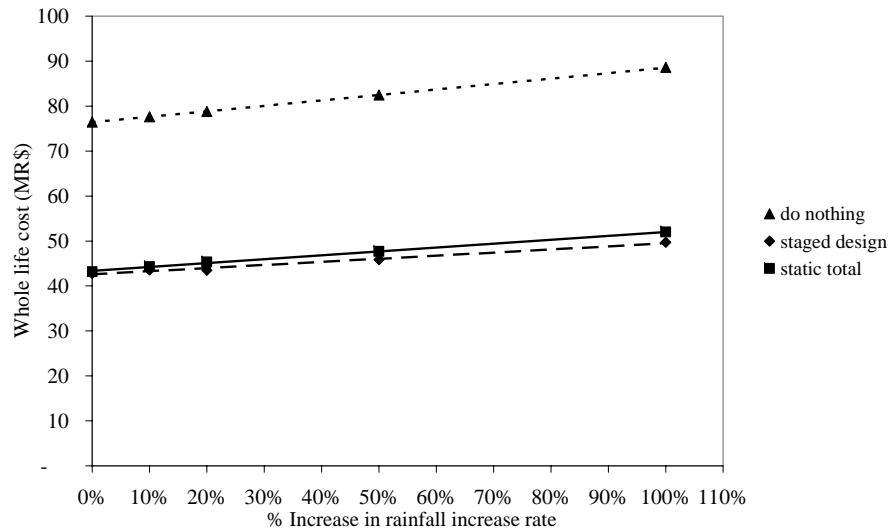


**Fig. 10.** Cost sensitivity with respect to the flood unit cost in staged and static options.

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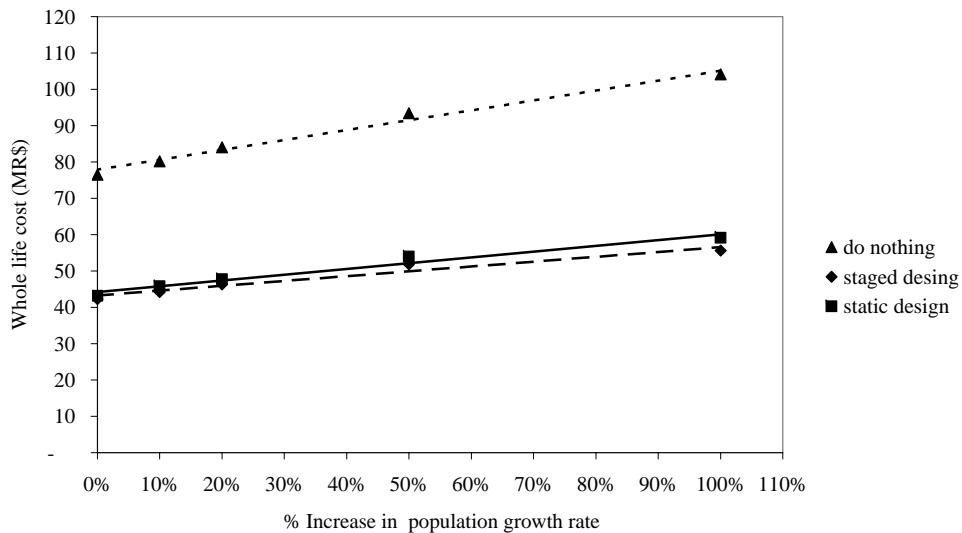


**Fig. 11.** Cost sensitivity with respect to rainfall increase rate in staged and static options.

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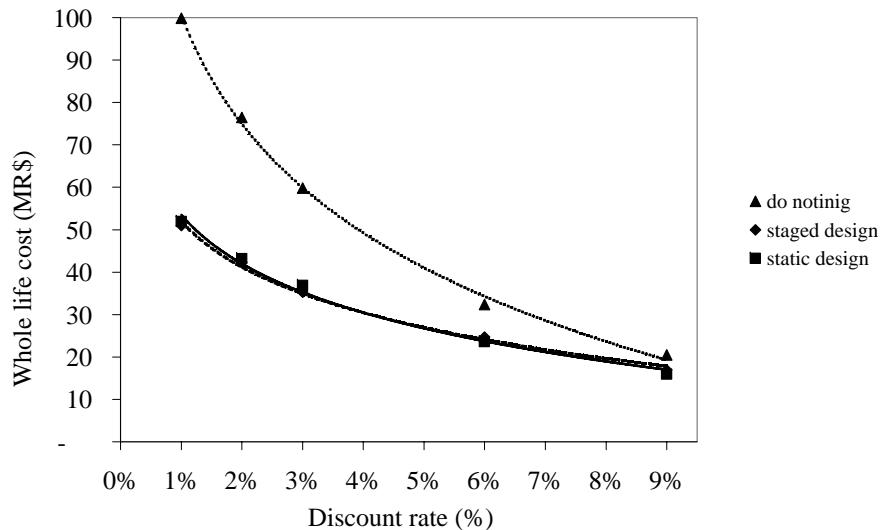


**Fig. 12.** Cost sensitivity with change in population growth rate (%) in staged and static options.

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**Fig. 13.** Cost sensitivity with respect to discount rates in staged and static options.

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