

# The Socio-ecohydrology of Rainwater Harvesting in India: Understanding Water Storage and Release Dynamics at Tank and Catchment Scales

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19 **Abstract**

20 Rainwater harvesting (RWH), the small-scale collection and storage of runoff for  
21 irrigated agriculture, is recognized as a sustainable strategy for ensuring food security,  
22 especially in monsoonal landscapes in the developing world. In south India, these  
23 strategies have been used for millennia to mitigate problems of water scarcity. However,  
24 in the past 100 years many traditional RWH systems have fallen into disrepair due to  
25 increasing dependence on groundwater. This dependence has contributed to accelerated  
26 decline in groundwater resources, which has in turn led to increased efforts at the state  
27 and national levels to revive older RWH systems. Critical to the success of such efforts is  
28 an improved understanding of how these ancient systems function in contemporary  
29 landscapes with extensive groundwater pumping and shifted climatic regimes.  
30 Knowledge is especially lacking regarding the water-exchange dynamics of these RWH  
31 “tanks” at tank and catchment scales, and how these exchanges regulate tank  
32 performance and catchment water balances. Here, we use fine-scale water-level variation  
33 to quantify daily fluxes of groundwater, evapotranspiration (ET), and sluice outflows in  
34 four tanks over the 2013 northeast monsoon season in a tank cascade that covers a  
35 catchment area of 28 km<sup>2</sup>. At the tank scale, our results indicate the groundwater recharge  
36 and irrigation outflows comprise the largest fractions of the tank water budget, with ET  
37 accounting for only 13-22% of the outflows. At the scale of the cascade, we observe a  
38 distinct spatial pattern in groundwater-exchange dynamics, with the frequency and  
39 magnitude of groundwater inflows increasing down the cascade of tanks. The significant  
40 magnitude of return flows along the tank cascade leads to the most downgradient tank in  
41 the cascade having an outflow-to capacity ratio greater than 2. The presence of tanks in  
42 the landscape dramatically alters the catchment water balance, with runoff decreasing by  
43 nearly 75%, and recharge increasing by more than 40%. Finally, while water from the  
44 tanks directly satisfies ~ 40% of the crop water requirement across the northeast monsoon  
45 season via surface water irrigation, a large fraction of the tank water is “wasted,” and  
46 more efficient management of sluice outflows could lead to tanks meeting a higher  
47 fraction of crop water requirements.

## 48 1 Introduction

49 Issues of water stress are now estimated to impact more than one-third of the global  
50 population, and it is predicted that this fraction will nearly double as the world reaches  
51 peak population (Wada et al., 2014). Such increases in water stress are driven not only  
52 by a growing population, changing patterns of food consumption, and climate-driven  
53 changes in water availability (Wiltshire et al., 2013), but also by spatial and temporal  
54 mismatches between water availability and water demand (Oki, 2006). From a spatial  
55 perspective, regional per capita water availability can vary drastically from more than  
56 50,000 m<sup>3</sup>/year to less than 500 m<sup>3</sup>/year (Parish et al., 2012; Wada et al., 2014), with  
57 levels of water stress in one basin having little impact on that in another. Similarly,  
58 temporal mismatches, particularly in areas with high seasonal rainfall variability, can  
59 create high rates of runoff leading to flood events and high short-term availability during  
60 wet seasons, followed by severe water stress during dry periods (Haile, 2005). Such  
61 temporal mismatches, paired with a shortage of surface-water storage, have been linked  
62 to both reduced incomes and a lack of food security (Gohar et al., 2013; Grey and Sadoff,  
63 2007).

64 Both spatial and temporal mismatches in water stress and availability characterize the  
65 climatic regime of India. The monsoon-driven climate common to semi-arid areas of  
66 India results in remarkable temporal variation where it is common for half of the year's  
67 total rainfall to fall over a period of only twenty hours (Keller et al., 2000). With such  
68 limited annual water availability and the extreme intra-annual rainfall variability, there  
69 have been ongoing efforts in India to increase storage capacity and additional water  
70 supplies for agricultural production and economic development (Grey and Sadoff, 2007).  
71 Over the last century, such efforts have focused primarily on large-scale projects  
72 designed to ensure higher levels of water storage and availability such as the building of  
73 large dams and canal systems (Cullet and Gupta, 2009; Mehta, 2001). For millennia,  
74 however, India has met the demand for seasonal water storage and increased water  
75 availability at the local level via the building of village-scale rainwater harvesting (RWH)  
76 structures, often referred to as tanks (Van Meter et al., 2014).

77 It is estimated that more than 39,000 of these RWH tanks are present in the southern  
78 Indian state of Tamil Nadu, which is the focus of the present study (Van Meter et al.,  
79 2014). These RWH tanks, which commonly take the form of earthen impoundments, 20-  
80 40 ha in size (Gunnell and Krishnamurthy, 2003), are built up from natural depressions in  
81 the landscape and have historically been designed to meet the water needs of subsistence-  
82 level farmers for rice production via managed sluice channels for irrigation (Farmer,  
83 1977). Tanks are often linked in a cascade with overflow from the upstream tanks  
84 spilling into surplus channels that lead to downstream tanks. The tank systems have fallen  
85 into decline in recent decades, primarily as a result of increasing reliance on groundwater  
86 pumping, and cheap access to electricity. This has led to declining groundwater levels,  
87 which coupled with a growing demand for increased agricultural production, have led to  
88 renewed interest in these traditional systems (Kumar et al., 2008; Shah, 2004). Although  
89 the majority of existing RWH tanks still remain in a state of disrepair (Anbumozhi et al.,  
90 2001), it is estimated that reviving RWH systems at an all-India scale could potentially  
91 add as much as 125 km<sup>3</sup> per year to the country's current water supply, making them  
92 critical in meeting the projected water shortfall of 300 km<sup>3</sup> per year by 2050 (Gupta and  
93 Deshpande, 2004). Consequently, in India's Groundwater Recharge Master Plan (2005),  
94 the need for renovation or new construction of RWH structures was highlighted at a cost  
95 of approximately \$6 billion, leading to high rates of revival of RWH structures across  
96 India (Agarwal and Narain, 1997; Shah et al 2009)

97 With the renewed and large-scale interest in the use of RWH structures, it is critically  
98 important to ask whether these ancient structures perform their intended purpose of  
99 significantly improving water availability in a basin. To do so requires quantifying the  
100 dominant tank inflows and outflows, specifically evapotranspiration (ET), groundwater  
101 recharge, and sluice outflows to irrigated fields. These water fluxes determine relative  
102 water allocation to aquifer supplies, irrigation needs, and atmospheric losses, and are  
103 influenced by a wide range of both natural and management controls, from climate and  
104 geology to the more direct anthropogenic controls (e.g., sluice outflow regulation). As  
105 such, a better understanding of tank fluxes and drivers of these fluxes is necessary when  
106 managing individual and cascades of tanks to meet both societal (irrigation demand) and

107 environmental (increasing rates of groundwater recharge) needs (Glendenning et al.,  
108 2012; Neumann et al., 2004; Ngigi, 2003).

109 Unfortunately, there is a lack of empirical studies that quantify tank hydrologic fluxes,  
110 especially at the scale of watersheds comprising of multiple tanks (Glendenning et al.,  
111 2012). One reason for the lack of information is that both groundwater recharge and ET  
112 are highly spatially variable, and thus difficult to accurately measure at the field scale  
113 (Glendenning et al., 2012). Most previous studies of RWH tanks estimate recharge as a  
114 residual term in the water-balance method (Glendenning et al., 2012); in arid  
115 environments, however, recharge magnitude is small compared to other fluxes (Bond,  
116 1998), making estimates from water balance residuals vulnerable to errors in other  
117 measured components. Furthermore, water-balance methods used in RWH tanks estimate  
118 recharge using modeled values of tank evapotranspiration, another rarely measured but  
119 critically important water flux in these arid environments (Sharda et al., 2006). While  
120 there is consensus regarding the value of direct measurements of temporal variations in  
121 recharge and evapotranspiration fluxes from RWH structures, such data are difficult to  
122 obtain due to the inherent complexities in making these measurements, especially under  
123 resource constraints (Glendenning et al., 2012).

124 Here, we propose an innovative use of the White (1932) method as a cost-effective means  
125 of obtaining spatially integrated, direct measurements of both ET and groundwater  
126 exchange in flooded RWH tanks. The White method, which was originally developed to  
127 estimate the magnitude of groundwater consumption by phreatophytes (Loheide, 2008;  
128 Loheide et al., 2005), has since been used to estimate ET and groundwater exchange in  
129 small, surface water systems (Carlson Mazur et al., 2014; Hill and Durchholz, 2015;  
130 McLaughlin et al., 2014; McLaughlin and Cohen, 2014). In these systems, diurnal  
131 variations in high-resolution surface water level data are used to decouple ET dynamics  
132 from groundwater exchange. In this paper, we demonstrate an application of this method  
133 to RWH structures, which are more complex than the systems studied thus far in that they  
134 have additional outflows (overflow and sluice outflow), and are much larger in spatial  
135 extent (~1 ha vs. 20-60 ha). Furthermore, while most studies of RWH systems have  
136 focused on individual tanks, we explore how groundwater-exchange dynamics change

137 along a tank cascade made up of four tanks, and scale up measured fluxes to estimate  
138 cumulative effects of tanks on catchment water balances. Our study has two linked  
139 objectives: (1) quantify temporal patterns in groundwater exchange, ET, and sluice  
140 outflows over the Northeast monsoon season; and (2) describe spatial patterns of  
141 measured fluxes from upstream to downstream tanks in a cascade. Using these estimates,  
142 we attempt to answer the following questions:

- 143 • At the local scale, how do tanks partition water, and what is the spatial  
144 variability in this partitioning behavior along a tank cascade?
- 145 • At the catchment scale, how do tanks alter the water balance in a basin?
- 146 • What percentage of the irrigation requirements do tanks meet, and can  
147 they be managed more efficiently to increase this fraction?

## 148 **2 Study Area**

### 149 **2.1 Site Description**

150 The study site is located in the South Indian state of Tamil Nadu, in the foothills of the  
151 Western Ghats mountain range (**Figure 1a**). The region surrounding the tank cascade is  
152 semi-arid, receiving a mean annual rainfall of 850 mm, with the Northeast (October to  
153 December) and Southwest (June to September) monsoons accounting for 42% and 14%  
154 of total rainfall, respectively (Government of Tamil Nadu, 2011; Vose et al., 1992). ET is  
155 greater than rainfall from January through July, while it is less than rainfall during the  
156 monsoon months (**Figure 1b**). For the year in which the field study was done (2013),  
157 rainfall over the northeast monsoon season (October – December) was 355 mm, which is  
158 close to the 70-year average of 363 mm.

159 The focus of the study is the Thirumal Samudram (TS) tank cascade, a hydrologically  
160 connected group of four rainwater harvesting tanks that encompass an overall catchment  
161 area of 28 km<sup>2</sup>, in the Madurai district of Tamil Nadu near the headwaters of the Gundar  
162 river basin (**Figure 1a**). All four tanks in the cascade have undergone renovation through  
163 a joint effort of local stakeholders and the Development of Humane Action (DHAN)  
164 Foundation, an NGO group leading tank rehabilitation efforts across South India (DHAN,  
165 2010), including regular desiltation, strengthening of tank bunds, repair of surplus and

166 sluice weirs. The four tanks provide irrigation water for three village revenue districts:  
167 Pappanaickenpatti (Tank 1), Kudipatti (Tanks 2 and 3), and Ketuvarpatti (Tank 4), from  
168 upstream to downstream. The population of the tank cascade area is 6,057 (Government  
169 of India, 2011), and 88% of the working population hold jobs either as farmers or  
170 agricultural laborers (**Table 1**).

171 The landscape surrounding the tank cascade has a gentle slope, ranging from 0.5%-1.0%,  
172 and is characterized by heavy, clay-rich red (alfisol) and black (vertisol) soils underlain  
173 by fractured rock of granitic origin (CGWB 2012; ICRISAT, 1987; Palaniappan et al.,  
174 2009) . Land use for the study area is primarily agricultural. Within the study cascade,  
175 81% of the land is devoted to agricultural use, with 42% of this total being irrigated  
176 (**Table 1**) (DHAN, 2010) . During the northeast monsoon season (October-January),  
177 paddy (rice) is the primary crop in the region, while during other periods of the year, a  
178 variety of other crops are cultivated, including cotton, groundnuts, and pulses  
179 (Government of Tamil Nadu, 2011).

## 180 **2.2 Rainwater Harvesting Structures**

181 Tanks in South India are created through the construction of an earthen dam (bund)  
182 across depressional areas in the landscape as a means of storing surface runoff (Van  
183 Meter et al. 2014) (**Figure 2**). During elevated water levels, flooding extends beyond the  
184 main depressional area and into flatter, often farmed areas (i.e., tank water spread area).  
185 The bunds are constructed using locally available materials, usually a combination of  
186 amassed earth and stones, supported by the roots of trees and bushes growing along the  
187 bunds (Weiz 2005). Sluices (typically sliding gates) are constructed within the tank bund  
188 and are used to control the release of water into irrigation channels, which then transport  
189 the stored water to agricultural fields in the tank command area (i.e., tank-supported  
190 irrigated fields). During heavy monsoon rains, water may spill over the tank's overflow  
191 weir into surplus channels leading to downstream tanks or to nearby waterways (Van  
192 Meter et al. 2014). Tanks are often linked through these surplus channels in chains, or  
193 cascades, that can range in size from several to more than a hundred tanks, forming a  
194 dense hydrological network across this intensively managed agricultural landscape.

195 Tank storage capacities vary across sites and time, with the latter due to siltation and  
196 desiltation cycles (Weiz, 2005). Historical data regarding maximum tank area and  
197 storage volumes for the four study tanks, obtained by the Public Works Department in  
198 India in approximately 1900, are summarized in **Table 2** (DHAN, 2010). Information  
199 regarding the tank irrigated area, also known as the command area or “ayacut” (Weiz,  
200 2005), is also provided. Although the maximum water depths of the four tanks are  
201 similar, ranging from 3-4 m at maximum fill, the historical data show that the tank areas  
202 vary significantly, ranging from 19.3 ha (Tank 3) to 58.7 ha (Tank 2). The ratio of  
203 command area to tank area historically ranged between 0.77 – 1.25 (**Table 2**), which is  
204 characteristic of tank systems found in this area (von Oppen & Subba Rao, 1987; Weiz,  
205 2005). **Table 2** also includes measurements made in the present study for comparison  
206 (discussed later).

### 207 **3 Methods**

#### 208 **3.1 Field Methods: Sensor Installation and Bathymetric Survey**

209 Tank water levels were continuously measured during and in the months immediately  
210 following the 2013 Northeast Monsoon season (October 2013-February 2014) using total  
211 pressure transducers (Solinst Levellogger Edge, accuracy =  $\pm 0.3$  cm, resolution = 0.01  
212 cm; Solinst Canada, Georgetown, Ontario, Canada) installed in wells at the deepest point  
213 of each tank. Wells, which were constructed of 5-cm-diameter 10 gage PVC, were  
214 installed to a belowground depth of 70 cm and were screened above and below the  
215 ground surface. The pressure transducers measured total pressure (m H<sub>2</sub>O) at 5-min  
216 intervals, and these measurements were corrected for variations in barometric pressure  
217 based on measurements collected at the same intervals with barometric pressure  
218 transducers (Solinst Barologger, accuracy =  $\pm 0.5$ cm ( $\pm 0.05$  kPa), resolution = 0.001 cm  
219 (.0001 kPa)). The barometric pressure transducers were installed in dry wells open to  
220 atmospheric pressure but below ground to ~~buffer~~ avoid changes in temperature and  
221 known temperature sensitivities (McLaughlin and Cohen 2011). The corrected tank stage  
222 data were verified based on frequent direct stage measurements made at the study site.  
223 Pressure transducers were installed on September 26<sup>th</sup> before the start of the rainy season,  
224 and retrieved on January 20<sup>th</sup> for Tanks 1 and 2, and March 7<sup>th</sup> for Tanks 3 and 4



225 generally when wells became dry. Continuous precipitation was measured using Onset  
226 RG3-M automatic tipping bucket rain gages (Onset Computer Corporation, Bourne, MA)  
227 installed near each of the four tanks.

228 Bathymetric surveys were conducted using a combination of measured water depths in  
229 flooded areas (i.e., ground elevations relative to water surface) and a Trimble ProXRT2  
230 GPS receiver paired with a Juno handheld computer for absolute ground elevations in  
231 exposed areas. Since Tank 4 had a large number of acacia trees that interfered with the  
232 accuracy of the Trimble, a Sokkia Total Station was used for ground elevation surveys.  
233 Sixteen to twenty-four transects at a grid-spacing of 40 m were taken in each tank, and all  
234 surveyed elevations were converted to ground elevations relative to the tank base (lowest  
235 point), which was defined as zero. The bathymetric data were used to create stage-  
236 volume and area-volume relationships for each tank, and estimate current tank capacities.  
237 The capacities estimated by this method led to reasonable values, with current capacities  
238 ranging between 62 – 92 % of the historical capacities (**Table 2**).

### 239 **3.2 Sluice and Overflow Weir Outflow Estimates**

240 There are six sluices in the study area, two in Tank 1, two in Tank 2 and one each in  
241 Tanks 3 and 4. Water release from the sluices is controlled by a sluice gate that can be  
242 opened to different degrees by a sluice rod. For our study tanks, the degree of sluice  
243 openness remained primarily unchanged during the period of study, and thus the major  
244 factor that controlled sluice discharge was found to be the tank water level. To  
245 understand this relationship, sluice discharge was estimated at different tank water levels.  
246 Discharge was estimated by measuring the velocity and cross-sectional area over a  
247 chosen section of each outflow channel just downstream from the sluice outlet. This  
248 section was selected based on width uniformity and channel straightness. Approximately  
249 20-40 measurements were made during each discharge measurement to obtain a reliable  
250 velocity estimate. Stage-discharge relationships developed for each sluice were used to  
251 estimate volumetric daily sluice outflow rates; these rates were then converted to area-  
252 normalized rates ( $S_o$ , cm/day) based on tank stage-area relationships (Section 3.1).

253 As described in Section 2.2, in addition to water loss via sluice outflow, water may also  
254 flow out of the tank by spillage through the overflow weir into surplus channels during  
255 large storm events. Overflow was observed during the study period only in the case of  
256 Tank 4 on 10/20, during the first major rains of the monsoon season. For this event, the  
257 surplus flow volume was estimated based on the observed drop in water levels between  
258 10/20 and 10/21.

### 259 **3.3 Estimation of Groundwater Recharge and Evapotranspiration (ET)**



260 The White (1932) method was used to calculate daily ET and net groundwater exchange  
261 from high-resolution stage data on days with no rainfall (**Figure 3**). The White method is  
262 based on two central assumptions: (1) ET (cm/d) fluxes are negligible at night, enabling  
263 groundwater flows to be estimated from nighttime stage changes, and (2) there is no  
264 diurnal variation in the groundwater exchange (GE; cm/d). Here, the White method was  
265 modified to account for sluice outflow ( $S_o$ ; cm/d) that occurred both during night and day  
266 in our study. ET and GE (cm/d; positive values indicate tank outflow, or recharge) were  
267 estimated using the following equations:


$$268 \quad ET = S_y \times (s - 24h) \quad (1)$$

$$GE = S_y \times 24 h - S_o \quad (2)$$

269 where  $S_y$  is the specific yield (dimensionless),  $s$  (cm) is the 24-hour stage change  
270 (positive values indicate net stage decline), and  $h$  (cm/h) is the linear slope of the  
271 nighttime decline between 0:00 and 5:00 hours. Specific yield ( $S_y$ ) is defined as the  
272 volume of water released from or added to storage in porous media divided by the total  
273 volume of the system (Healy and Cook, 2002). On a per unit area basis,  $S_y$  represents the  
274 input (rain) or output (ET) depth divided by the observed change in the water level. In  
275 our study,  $S_y$  was set to 1.0 following the common assumption for flooded areas (Mitsch  
276 and Gosselink, 2007); however, see Section 4.3 and McLaughlin and Cohen (2014) for  
277 important caveats regarding this assumption.

### 278 **3.4 Tank and Catchment Water Balances**

279 Volumetric water balance calculations were carried out at both the individual tank and  
280 the tank catchment scales across the Northeast monsoon season to answer questions  
281 regarding the partitioning of rainfall into the various outflow components (e.g.  $S_o$ , ET,  
282 GE). For individual tank water balances, we utilized daily data for water levels, rainfall,  
283  $S_o$ , ET, and GE. For non-rainfall days, ET and GE values were calculated using the White  
284 method. For rainfall days, ~~however~~, ET and GE could not be calculated directly via the  
285 White method, as the method necessarily assumes a constant groundwater flow and  
286 therefore cannot account for rainfall-related inputs (McLaughlin & Cohen 2013). This  
287 disruption in the continuity of the data set, without correction, would lead to gaps in the  
288 daily water balance and an underestimation of both ET and groundwater exchange across  
289 the monsoon season. To eliminate these gaps, we estimated ET values on rainfall days  
290 via interpolation between White method-estimated ET rates on days without rain. GE on  
291 rainfall days was estimated based on the residuals of the daily water balance, using the  
292 measured 24-hour change in  water levels, estimated ET rates, measured  
293 precipitation, and **estimated runoff** (McLaughlin and Cohen, 2013). Runoff was estimated  
294 using the Strange method (Shanmugham and Kanagavalli, 2013), an empirical method  
295  developed to predict runoff from catchments with irrigation tanks and small  
296 **reservoirs** and that is widely used throughout India by government departments dealing  
297 with irrigation (Latha et al., 2012). Stage-to-area relationships (Section 3.1) were used to  
298 convert daily stage change and estimated fluxes (ET, GE, and  $S_o$ ) into volumes, which  
299 were calculated for each tank. Note that the water balances for all tanks are calculated for  
300 the period from October 17, 2013-January 13<sup>th</sup>, 2014, a period that spans the entire  
301 monsoon season and for which water-level data is available for all four tanks.

302 Water balances were also calculated at the catchment scale using a nested catchment  
303 design for four catchments: 1) Catchment 1 (C1): Tank 1 (T1), and its contributing  
304 catchment; 2) Catchment 2 (C2): Tank 2 (T2) and its contributing catchment which  
305 includes Tank 1 and its catchment area and command area; 3)  ent 3 (C3): Tank 3  
306 (T3) and its contributing catchment which includes tanks 1 and 2, **and their catchment**  
307 **and command areas**; and 4) Catchment 4 (C4): Tank 4 (T4) and its contributing  
308 catchment which includes tanks 1, 2 and 3, and their catchment and command areas.

309 This nested catchment design enabled us to explore the effect of varying catchment sizes  
310 and tank to catchment ratios on the water partitioning.

311 Further, in order to understand the impact of the tanks at the catchment-scale, we  
312 explored two scenarios for each of the four catchments scales (i.e., C1 - C4): (1) a with-  
313 tank (WT) scenario to represent current conditions within the catchment (i.e., four  
314 existing tanks); and (2) a no-tank (NT) scenario, with all other conditions (e.g., rainfall,  
315 ET on the catchment area) being the same. For the NT case, catchment-scale runoff was  
316 calculated using the Strange method (Shanmugham and Kanagavalli, 2013) and daily  
317 rainfall over the monsoon season. Remaining rainfall was assumed to exit the system  
318 through ET and groundwater recharge. For the WT case, we assumed the sluice outflow  
319 from the most downstream tank in the catchment (T1 for C1, T2 for C2, T3 for C3 and  
320 T4 for C4) to represent the Q value for the catchment. For T4 a surplus overflow event  
321 occurred at the start of the season, the volume of which was estimated based on stage-  
322 volume relationships; this volume was added to the sluice outflow to estimate the Q for  
323 C4. The Q values for the NT and WT scenarios were compared for all four catchments to  
324 understand the effect of tanks on the catchment runoff.

325 To understand the effect of tanks on groundwater recharge, we assumed the mean  
326 recharge to be 17% of the mean annual rainfall for the NT case following Anurag et al.  
327 (2006). For the WT case, the landscape was assumed to include three different domains,  
328 with separate recharge fractions being assumed for each domain: (1) tank bed area: GE  
329 (Section 3.2) was used, (2) tank command area: 50% of the sum of rainfall and sluice  
330 outflow (based on typical values for paddy fields (Hundertmark and Facon, 2003)), and  
331 (3) the rest of the watershed: 17% of rainfall (Anurag et al., 2006). The command area  
332 and the tank bed area estimates for the four tanks are provided in Table 2.

#### 333 **4.0 Results and Discussion**

334 The current section is divided into two broad subsections. In the first, we report  
335 measurements of tank water levels, and fluxes (ET and GE), and use these data as a basis  
336 for discussing tank water level dynamics across the monsoon season. In the second, we  
337 provide analysis of these and complementary data to answer questions regarding controls

338 on the tank and catchment water balances and the ability of tank rainwater harvesting  
339 systems to meet irrigation water demand.

## 340 **4.1 Tank Water-Exchange Dynamics**

### 341 **4.1.1 Tank Water levels over the Northeast Monsoon Season**

342 Water levels in the tanks rose sharply in mid-October following the monsoon rains, and  
343 then dropped over the next 3 months as water left the tanks through ET, sluice outflow,  
344 and groundwater recharge (**Figure 4**). Note that although the Northeast Monsoon rains  
345 began in early September, the tanks started filling only in mid-October. This time lag is  
346 likely due to a threshold effect, where runoff to the tanks occurs after cumulative rain  
347 volumes begin to exceed catchment infiltration capacity. Two distinct fill events can be  
348 observed, one on October 16<sup>th</sup> and the second on Nov 17<sup>th</sup> for all tanks except Tank 1, for  
349 which the second fill event is not as apparent. Between Oct 16<sup>th</sup> and Nov 17<sup>th</sup>, the  
350 trajectories of tanks 1 and 3 parallel each other, while those of tanks 2 and 4 are similar.  
351 Towards the later part of the season, the water level trajectories of the four tanks  
352 approximately parallel each other. Tank 1 loses its water the earliest and is mostly dry by  
353 January, while the other three tanks retain some water till February. In the following  
354 sections, we explore how the outflow fluxes in the four tanks vary over the course of the  
355 monsoon season.

### 356 **4.1.3 Estimation of Evapotranspiration**

357 Evapotranspiration (ET) fluxes estimated with Equation 1 for the four tanks are shown in  
358 **Figure 5** ET rates derived with the White method are reasonable for the region and  
359 season (potential ET (PET) ca. 3 – 12 mm/day for Madurai (Rao et al., 2012), ranging  
360 from 5.5± 1.0 for Tank 1 to 10.1± 0.8 mm/day for Tank 3 during periods when the tank  
361 inundated area is greater than 25 % of maximum area. Below this 25% threshold (shown  
362 in **Figure 5** with dashed line), ET estimates for the tanks exceed PET rates by factors of  
363 2-3.

13-1

13-2

364 Two mechanisms can explain this effect of smaller inundated area on ET rates. First,  
365 small areas of flooding surrounded by comparatively extensive areas of exposed soils can  
366 create an oasis effect (Drexler et al., 2004, Paraskevas et al., 2013), particularly in arid  
367 regions where advection of dry air from exposed areas can increase ET rates in flooded  
368 areas beyond typical values (and PET). Second, the White method requires a known  $S_y$   
369 (see equation 1) to determine ET and groundwater exchange from diurnal fluctuations of  
370 water levels.  $S_y$  can be considered as the ratio of input (rain, discharge) or output (ET,  
371 recharge) depth relative to the induced water level change (Healy and Cook, 2002). Open  
372 water  $S_y$  values of 1.0 are typically assumed for flooded areas (Mitsch and Gosselink,  
373 2007), and this value was used here. In contrast, soil  $S_y$  values range from 0.1 to 0.35  
374 (Loheide et al., 2005), meaning that belowground water levels experience a greater  
375 decline compared to flooded areas for an equal ET flux. As such, a hydraulic gradient for  
376 water subsidy from a flooded area to adjacent exposed areas can occur, and any rapid  
377 equilibration means that daytime decline from the flooded area includes subsidy to  
378 adjacent exposed areas (McLaughlin and Cohen, 2014). Accordingly, ET estimated with  
379 the White method for small flooded areas includes both ET from standing water plus any  
380 daytime flux to adjacent exposed areas to equilibrate greater ET-induced declines in  
381 belowground water levels. McLaughlin and Cohen (2014) measured ET rates using the  
382 White method (and a  $S_y = 1$ ) that exceeded PET by a factor of 5 or more when flooded  
383 areas were small, compared to  $ET/PET \approx 1.0$  at moderate to maximum flooded area.

#### 384 **4.1.4 Estimation of Groundwater Exchange**

385 The temporal pattern of net groundwater exchange, estimated using equation 2, is  
386 presented in **Figure 6** together with trends in tank water levels and daily precipitation.  
387 GE rates across the monsoon season appear to be driven by a combination of both tank  
388 water levels and the occurrence and magnitude of rainfall events. Tank 2, for example,  
389 has relatively lower recharge rates (positive values in Figure 6) in the earlier part of the  
390 season, with values decreasing with the occurrence of each major rainfall event, and then  
391 increasing incrementally over time until the next rainfall. The last period of significant  
392 rainfall occurs in mid-December, and shortly after this time, recharge magnitudes for  
393 Tank 2 reach a peak, and then slowly decrease with decreasing tank water levels. A

394 similar pattern can be seen for Tank 4, where the peak recharge value occurs during the  
395 mid-December period, followed by a steady decline in recharge magnitudes as tank water  
396 levels decrease. In contrast, Tanks 1 and 3 appear to be less impacted by rainfall events;  
397 for these tanks, recharge magnitudes begin to decrease with decreases in tank water levels  
398 much earlier in the season, after the last major rainfall (64 mm) on November 17<sup>th</sup>. In the  
399 last few weeks of the monsoon season, Tanks 2-4 all switch over to a groundwater inflow  
400 regime (negative GE values). Lower recharge rates as well as these switches to  
401 groundwater inflow towards the end of the season may be due to tank water levels  
402 consistently having greater declines compared to the surrounding aquifer, resulting in  
403 decreases and potential reversals of hydraulic head gradients. This period is also,  
404 however, punctuated by some distinct, very high groundwater outflow events that may  
405 correspond to observed groundwater pumping in the vicinity, highlighting a potential  
406 direct human influence to tank recharge rates.

407 To better characterize the dominant drivers for the magnitude and direction of GE, with  
408 the overall goal of generalizing these observations to larger scales, we plotted GE as a  
409 function of days since last rainfall for all four tanks (**Figure 7a**). For Tanks 2 and 4, there  
410 is a threshold value of days since rain (14 days for Tank 2 and 16 days for Tank 4) that  
411 separates rainfall-GE relationships. That is, there is significant scatter in the rainfall-GE  
412 relationship at values less than this threshold, but strong negative relationships emerge  
413 between the two variables at higher values of day since rain (**Figure 7a**). In contrast,  
414 Tank 1 and Tank 3 have much lower threshold values of only 1 and 3 days, respectively.

415 This pattern of decreasing recharge with days since last rainfall is reasonable, as water  
416 levels in the tank steadily decrease over time, leading to decreased hydraulic head and  
417 thus lower rates of recharge. In contrast, immediately following a rain event, the system  
418 becomes more dynamic, and recharge is a function of not only tank water levels but also  
419 the short-term response of the local surrounding aquifer. When plotted for all tanks, GE  
420 was also found to respond linearly to tank water levels for most days throughout the  
421 monsoon season, except in the hydrologically dynamic periods after rain events, when the  
422 behavior was more erratic (**Figure 7b**).

423 In addition to these patterns of groundwater exchange across the monsoon season,  
424 differences can also be seen along the tank cascade, from top (Tank 1) to bottom (Tank  
425 4). First, while recharge, as represented by the positive GE values in **Figure 6**, can be  
426 seen to dominate the exchange dynamics of Tanks 1-3, Tank 4 is more discharge-driven.  
427 As shown in **Figure 8a**, close to 90% of all days throughout the monsoon show net  
428 recharge behavior for Tanks 1-3, while Tank 4 is split almost equally between net  
429 recharge and net discharge days. From a volume perspective, the discharge-to-recharge  
430 ratio for the tanks shows a general trend from smaller (0.3 in Tank 1) to larger (1.2 in  
431 Tank 4) across the tank cascade (**Figure 8b**), with Tank 4 demonstrating net discharge  
432 behavior. Tank 4 is the most down-gradient tank, suggesting the possibility that aquifer  
433 levels adjacent to Tank 4 are higher (possibly due to upstream tanks' recharge) for a  
434 longer period of time than the other three tanks, leading to more frequent groundwater  
435 inflow.

436 Our finding of a distinct spatial pattern in groundwater exchange and sluice outflow  
437 dynamics across the tank cascade is a novel contribution of the present study. Most  
438 studies that have explored the recharge/discharge functions of tanks (Glendenning et al.,  
439 2012) have focused on individual tanks, with no consideration of the position of the tank  
440 in a cascade as an important control on its functioning. Our results indicate that in order  
441 to upscale tank-scale information to understand catchment and regional scale impact of  
442 tanks, more studies should focus on exploring the spatial arrangement of tanks in the  
443 landscape.

## 444 **4.2 Exploring biophysical vs. management controls on tank water** 445 **balance at the tank and catchment scales**

446 Three questions were posed in the introduction regarding the partitioning of water within  
447 a tank cascade, the ways in which tanks alter the catchment water balance, and the ability  
448 of tanks to meet irrigation requirements in the semi-arid landscapes of South India.  
449 Below, we use our measured data to provide answers to these questions in the context of  
450 a discussion of physical versus management controls on tank functionality.

### 451 **4.2.1 Water balance at the tank scale**



452 The first question we asked was how tanks partition incoming water (direct rainfall on  
453 tank and surface runoff from tank catchment) into various outflow components, namely  
454 evapotranspiration, groundwater outflow/inflow, and sluice outflow to the fields in the  
455 tank command area. The flow volumes corresponding to these components for each tank  
456 over the duration of the Northeast monsoon season are plotted by week in **Figure 9a** and  
457 are summarized in **Table 3**. **Notably, recharge to groundwater is a significant component**  
458 **of tank outflows**. Although the primary function of tanks in South India has historically  
459 been to provide surface water for irrigation, and despite the high clay content of soils in  
460 the area, groundwater recharge is the primary outflow mechanism in Tanks 1-3 (from 46-  
461 59% of total outflows). For Tank 4, however, which is dominated by discharge behavior,  
462 the primary outflow mechanism is sluice outflow, which directly provides irrigation  
463 water to the tank command area. As seen in **Figure 9a**, sluice outflows and recharge are  
464 the greatest early in the season, when tank levels are at their highest, and then decrease  
465 over time, ceasing entirely by mid-December for all four tanks.

466 Although the volume of water lost to ET is substantial (0.48 – 1.64 million cubic meter  
467 over the 83-day study period), it is a relatively small fraction of the overall water budget.  
468 On a cumulative scale (Table 3), ET values range from 13% of total outflows for Tank 1  
469 to 22% for Tanks 2 and 3. These relatively small percentages contradict the established  
470 view of tanks losing a significant fraction of their water through ET (Kumar et al., 2006).  
471 In addition, although the tanks have been constructed in soils with a high clay content, all  
472 but Tank 4, which has a high discharge-recharge ratio, have high relative rates of  
473 groundwater recharge. For Tanks 2 and 3, recharge is the largest outflow component (57-  
474 59%) and is more than double the values for sluice outflow and evapotranspiration. For  
475 Tank 1, recharge is also the largest outflow component (47%), although it is similar in  
476 magnitude to sluice outflows (41%). The differences in flow partitioning between the  
477 four tanks can be attributed to differences in both natural (e.g., topographical position of  
478 the tank along the cascade) and human (e.g., sluice management) factors.

479 Interestingly, a trend can be seen in the relationship between total tank outflows over the  
480 monsoon season and the maximum tank capacity (Figure 9b). As we move down the  
481 cascade of tanks, the outflow-to-capacity ratio increases, from 1.06 for Tank 1 to as high

482 as 2.25 for Tank 4. The outflow-to-capacity ratio is an indication of how many times a  
483 tank fills up during the season, and the increase in values along the cascade of tanks is a  
484 function of increasing return flows from upstream command areas entering the  
485 downstream tanks. For Tank 4 in particular, groundwater discharge provides a  
486 significant input of water into the tank (Figure 8). Accordingly, Tank 4 has relatively  
487 greater amounts of water available for surface water irrigation throughout the season,  
488 with sluice outflow alone accounting for 1.2 times the total tank capacity. This increase  
489 in the outflow-to capacity ratio along the cascade of tanks is an important feature of the  
490 tank cascade system, and highlights the need to study the tanks not in isolation, but in  
491 relation to their position along the cascade. Biophysical controls (for example weeds or  
492 sediments in tank beds of upgradient tanks) or management choices (for example,  
493 planting crops with lower or high water requirement ins upgradient tanks) can completely  
494 alter the water availability in a downstream tank. Thus, rehabilitation efforts and tank  
495 management should focus on maximizing benefits at the cascade scale instead of only at  
496 the individual tank scale.

#### 497 **4.2.2 Water balance at the catchment scale**

498 The second question we asked was how tanks alter the partitioning of rainfall into runoff  
499 at the catchment outlet (Q) and recharge within the catchment. Water balance  
500 calculations were done at the tank and catchment scales for the four nested catchment  
501 scenarios described in Section 3.4. Further, we simulated scenarios both with and without  
502 tanks to understand the contribution of tanks towards altering catchment scale water  
503 partitioning.

504 Our results show a dramatic difference between the with-tank and no-tank scenarios, and  
505 a distinct spatial pattern of response in the four nested catchments. We found a  
506 significant decrease in Q at the four nested scales, from 22% of rainfall in the no-tank  
507 scenario to 5-9% of rainfall with tanks (**Table 4**). At the largest catchment scale (C4), the  
508 runoff decreased from approximately 2.29 million cubic meter (MCM) in the NT  
509 scenario to only 0.69 MCM in the presence of tanks (**Table 4**). This approximately 70%  
510 decrease is consistent with other work showing large decreases in runoff due to the


511 presence of tanks (Kumar et al., 2008). Conversely, catchment-scale net recharge was  
512 observed to increase from 17% of rainfall without tanks to 24-27% with tanks (**Table 4**),  
513 which corresponds to an overall increase in net groundwater recharge of 40%,  
514 highlighting the potential beneficial role tanks may play in augmenting groundwater  
515 resources.


516 Despite this strong link between the presence of tanks and groundwater recharge, tank  
517 maintenance has declined across South India as farmers have become increasingly reliant  
518 on groundwater irrigation sources (Balasubramanian and Selvaraj, 2003). With tank-  
519 irrigated area across Tamil Nadu having decreased from 940,000 ha in 1960 to  
520 approximately 503,000 ha in 2010, some suggest that current tanks are operating at only  
521 30% of their potential capacity (Amarasinghe et al., 2009; Government of Tamil Nadu,  
522 2011; Palanisami and Meinzen-Dick, 2001). This degradation of tank functionality is  
523 eliminating or significantly degrading the primary mechanism for aquifer recharge in an  
524 area where, without rainwater harvesting, the majority of monsoon rainfall will leave a  
525 catchment as runoff within hours of falling. Our water balance calculations show that  
526 tanks provide a mean groundwater recharge benefit of 5,600 m<sup>3</sup> per hectare of tank  
527 waterspread area. At the scale of the Gundar basin, with its 2276 village-scale RWH  
528 tanks, each covering an area of approximately 40 ha (DHAN, 2010), these results suggest  
529 that fully functional tanks could provide a groundwater recharge benefit of 522 MCM.  
530 However, with the currently reduced tank functionality, the yearly recharge volume is  
531 likely closer to 157 MCM, a difference of 365 MCM. With a population of  
532 approximately 3,000,000, this difference translates to a difference in water availability  
533 throughout the Gundar Basin of 122 m<sup>3</sup> per capita. It is currently estimated that all of  
534 India is experiencing some degree of water stress, with per capita availability ranging  
535 from 1000-1700 m<sup>3</sup>/year (Amarasinghe et al., 2005). Accordingly, maintaining tanks at  
536 full functionality has the potential to increase per capita water availability in the Gundar  
537 by approximately 10%.

538 It should be noted that the recharge benefit suggested by the results in our tank cascade is  
539 significantly larger than that reported for a watershed in Gujarat a state in Western India,  
540 where it was shown that the construction of new rainwater harvesting structures would

541 lead to a 60% decrease in catchment runoff, but only a 5% increase in recharge (Sharma  
542 and Thakur, 2007). In the Gujarat catchment, however, annual rainfall is approximately  
543 half that in our South India catchment, and ET rates are estimated at more than 50  
544 mm/day, suggesting that variations in climate can strongly impact the contribution of  
545 rainwater harvesting structures to groundwater recharge.

### 546 **4.2.3 Management controls on irrigation efficiency**



547 While the first two questions focused on the physical controls on tank water dynamics,  
548 our third question focused on understanding how tank water management affects water  
549 balances and, in doing so, contributes to meeting the irrigation requirements of the tank  
550 command areas. To answer this question we have plotted supply-and-demand curves over  
551 the growing season (**Figure 10**). The supply curves are the sluice outflow volumes from  
552 the four tanks. The demand curve in this case is the crop water requirement in mm/day ,  
20-1 553 which is adjusted by the available rainfall to get the Irrigation Water Demand (IWD =  
554 Crop Water Requirement – Rai ). The crop water requirement data in mm/day were  
555 obtained from (Brouwer et al., 1989) for the four growing stages of paddy. Paddy planting  
556 dates, which differed dramatically between the four tanks (10/17, 10/17, 9/25, and 9/13  
557 for Tanks 1, 2, 3, and 4), are based on field observations. The earlier planting dates in the  
558 command areas of Tanks 3 and 4 were most likely due to the availability of borewell  
559 water for those areas. As can be seen in **Figure 11**, the difference in planting dates leads  
560 to different demand curves for the four tanks.

561 The supply-and-demand curves assess the ability of the tanks to meet paddy water  
562 demand by comparing IWDs to sluice outflows. The darker red areas in **Figure 11** denote  
563 sluice water used to meet the IWD, while the lighter red areas represent sluice water that  
564 is “wasted,” as it is flowing out at a time when crops are not requiring that water. The  
565 grey areas in the figure represent the IWD unmet by sluice outflow. Notably, large  
566 quantities of surplus sluice water leave the tank soon after it fills. These surplus sluice  
20-2 567 outflows are not needed by the crops at the time they leave the tank and will ultimately  
568 leave the catchment  evaporation or as downstream runoff. Because the sluices are for  
569 the most part not actively managed or appropriately maintained, there is substantial  
570 wastage through sluice outflow in these systems, with the sluices remaining perpetually

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571 open and outflows being purely a function of water levels in the tank. As reported in  
572 **Table 5**, it was found that anywhere from 31-79% of IWD within the study cascade  
573 remains unmet, while approximately 15-50% of available sluice outflows leave the tank  
574 cascade unutilized. This remaining irrigation water demand would in many cases be met  
575 by farmers using groundwater pumping to supplement tank water, and would in other  
576 cases remain unmet, leading to reduced yields or crop failure. In the case of groundwater  
577 pumping, it should be noted that a significant portion of the tank water does leave the  
578 tanks as groundwater outflow, and is subsequently extracted by groundwater wells for  
579 irrigation, thus helping to meet the crop water requirements by a non-direct route. The  
580 magnitude of this contribution of tank outflows to the crop water budget, however, is  
581 difficult to ascertain, and thus has not been included herein.

582 The timing of planting also has a significant impact on the ability of the tanks to meet  
583 crop water requirements (Figure 10), with the later planting dates in Tanks 1 and 2  
584 leading to more than 70% of the IWD being unmet by sluice outflows (**Table 5**).

585 Conversely, Tank 4, with its much earlier planting time (9/13), more effectively meets  
586 crop water requirements with sluice outflow.  the early planting time leads to the  
587 lowest total IWD of all the tanks (752 mm), as **more of the crop water requirements can**  
588 **be met by rainfall.** In addition, there is a better temporal match for Tank 4 between the  
589 unregulated sluice outflows at high tank water levels (Figure 11) and the crop water  
590 needs of the plants. Accordingly, more than 500 mm of the IWD is met by sluice  
591 outflows, and only 31% of the overall demand remains unmet. These results suggest that,  
592 to optimize tank operations and to maximize the  provisioning capabilities of the  
593 tanks, earlier planting times could be utilized by **farmers.** Such a change in management,  
594 however, would be dependent on both groundwater availability and the economics of  
595 groundwater pumping.

## 596 **5.0 Conclusion**

597 In recent decades there has been growing interest in the revival and expanded use of  
598 rainwater harvesting tanks across the agricultural landscapes of India and other semi-arid  
599 regions to address issues of water scarcity and aquifer depletion. While it is well  
600 established that these tanks can increase local water availability, leading to higher crop

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601 yields and direct socioeconomic benefits (Palanisami et al., 2010), the impact of  
602 widespread use of small, distributed storage reservoirs on the catchment-scale  
603 partitioning of water resources is still an open question. Furthermore, while significant  
604 resources are being used to rehabilitate tanks, there is a lack of understanding regarding  
605 how these ancient structures function in a modern landscape, under current  
606 socioeconomic and environmental pressures. The hydrology of these tanks is so  
607 intricately tied with the social system in which they are embedded that only a systems  
608 approach, accounting for interactions between natural and human systems, can allow us  
609 to fully understand and manage these systems. In the decade of Panta Rhei, in which we  
610 attempt to reach a better understanding of processes governing the water cycle in the face  
611 of rapidly changing human systems (Montanari et al., 2013), any full analysis of tank  
612 water dynamics must be carried out within the domain of the emerging science of  
613 sociohydrology (Sivapalan et al., 2012).

614 In this paper we have used high-resolution monitoring of tank water levels to help  
615 quantify daily fluxes of evapotranspiration, groundwater recharge and sluice outflows  
616 from the tanks, and have coupled this information with village level data on planting  
617 dates and irrigated areas, to further our understanding of both the natural and human  
22-1 618 controls on water partitioning at both tank and catchment scales. At the tank scale,  
619 groundwater recharge and ~~sluice~~ outflow were observed to be the largest components of  
620 the tank water budget, with ~~ET~~ ET accounting for only 13-22% of the outflows. At the  
621 catchment scale, our results demonstrate that the presence of tanks within the catchment  
622 decreases runoff by approximately 70%, increases recharge by 40%, and directly satisfies  
623 approximately 40% of crop water requirements across the Northeast monsoon season via  
624 surface water irrigation. These findings suggest that village-scale rainwater harvesting  
625 tanks can dramatically increase water availability at a local or village scale, but also that  
626 they may have negative impacts on downstream users due to large decreases in catchment  
22-2 627 runoff. Our results also highlight that a large fraction of the tank water is “wasted”  
628 because, despite ongoing ~~sluice~~ efforts toward tank rehabilitation and maintenance in our  
629 study cascade, the sluices ~~leak~~ leak continuously, thus providing surplus water at times of  
630 lower demand. Thus, a more efficient management of sluice outflows, and better

631 maintenance of the sluices themselves, could lead to the tanks meeting a higher fraction  
632 of crop water requirements.

633 An interesting and novel attribute of our study is the exploration of biophysical and social  
634 controls on tank water dynamics as a function of the location of the tank along a cascade,  
635 in a four-tank cascade system. We observe a distinct spatial pattern in groundwater-  
636 exchange dynamics with the most down-gradient tank being mostly driven by  
637 groundwater inflow, while the other tanks are more outflow-driven. Consequently the  
638 most down-gradient tank has a much greater outflow-to-capacity ratio, and is able to  
639 provide a much larger volume of sluice outflow compared to its capacity. The ability of  
640 the most downgradient tank to provide more irrigation water is a function of the return  
641 flow from the command areas of the upstream tanks, and highlights the need to study  
642 tanks, not in isolation, but as a part of a cascade. There is also a distinct pattern in the  
643 crop planting dates in the four tanks, with the more down-gradient tanks having earlier  
644 planting dates that eventually lead to a more efficient use of the tank water. Interactions  
645 with the villagers revealed that the earlier planting dates in the downgradient tanks could  
646 be attributed to the greater availability of groundwater in that region, which enables the  
647 farmers to plant before the monsoons have arrived. This dynamic highlights the  
648 feedbacks between the natural and human systems, where a greater availability of water  
649 at the catchment outlet leads to farmers deciding on earlier planting dates, which in turn  
650 leads to a more efficient use of the available water.

651 In conclusion, our results demonstrate the significant role that tanks can play in  
652 addressing challenges of limited water availability, by both increasing groundwater  
653 recharge as well as the water available for irrigation. However, they also draw attention  
654 to the detrimental environmental impacts of tanks with respect to reducing downstream  
655 flows. These findings highlight the need to understand the spatio-temporal patterns in  
656 tank water dynamics at the basin scale, especially within the framework of a coupled  
657 natural and human systems approach that allow us a more complete understanding of  
658 how tanks alter the sociohydrological dynamics of water stressed landscapes. Thus,  
659 ongoing rehabilitation efforts of tanks need to be complemented with more studies that  
660 quantify the functioning of these rehabilitated tanks and their impacts in altering basin

661 scale water dynamics, with the overall goal of appropriately managing the tradeoffs  
662 between socioeconomic benefits and environmental costs.  
663



664 **Author Contributions**

665 The field study was carried out by M.S. under the guidance of D.L.M. Data analysis was  
666 carried out by K.V.M. and M.S. Drafting of the manuscript was led by K.V.M. and  
667 N.B.B. with contributions by D.L.M. N.B.B. conceived of the project and was  
668 instrumental to the basic experimental design.

669 **Acknowledgements**

670 This research is financially supported by the U.S. National Science Foundation  
671 (1211968), Dynamics of Coupled Natural-Human Systems and by University of  
672 Waterloo start-up funds. We thank the DHAN Foundation for generously sharing  
673 their experiences, data, and hospitality during the two field visits that allowed us to  
674 carry out this project. We would also like to thank Dr. K. Palanisami for his help  
675 with establishing connections for the research team within India and with his  
676 guidance regarding field site selection.  
677

678 **Figure Captions**

679 **Figure 1.** (a) Location of the Thirumal Samudram cascade within Tamil Nadu. The  
680 dotted lines indicate flowpaths calculated based on a digital elevation map (DEM) for the  
681 area; (b) Average rainfall and Potential Evapotranspiration (PET) (1900-1970) measured  
682 at Peraiyur weather station, 10 km from the study cascade.

683 **Figure 2.** (a) Aerial view of a Tank 4 in the TS cascade; (b) plan view of typical tank  
684 along with catchment and command area; (c) cross section of tank water budget  
685 components.

686 **Figure 3.** The White Method for estimating ET and groundwater exchange using diurnal  
687 water level fluctuations. Gray bars denote nighttime.

688 **Figure 4.** Tank water level and daily rainfall for the four tanks over the North East  
689 monsoon season. Tank water level is measured from the deepest point of the tank.

690 **Figure 5.** The temporal variation in daily ET over the monsoon season, shown as green  
691 bars. There are data gaps in the figure since estimates were made using the White method  
692 only on non-rainfall days. ET increases towards the later part of the season, coincident  
693 with decreases in tank surface area (shown as the grey shaded area). ET rates are  
694 reasonable for the region and season when the inundated area is greater than 25 % of  
695 maximum area, as indicated by the dashed line.

696 **Figure 6.** (a) Relationship between groundwater exchange and days since last rainfall,  
697 shown separately for the four tanks. The threshold line (dashed orange) separates the  
698 more erratic rainfall-driven groundwater exchange behavior following rain events (shown  
699 as light-blue diamonds) from the more predictable behavior typical of drier periods  
700 (shown as dark blue diamonds), when GE is driven primarily by hydraulic head values  
701 determined by tank water levels. (b) Relationship between tank water levels and

702 groundwater exchange shown for all four tanks combined. Lighter blue diamonds  
703 correspond to the rainfall values below the threshold shown above in 7a.

704 **Figure 8:** (a) The frequency of daily recharge (outflow) and discharge (inflow) events  
705 over the Northeast Monsoon season, and (b) the ratios of cumulative discharge to  
706 cumulative recharge magnitudes. The results for the four tanks indicate that all tanks  
707 function as both recharge and discharge systems, but that Tank 4 is much more  
708 dominated by discharge behavior based on both frequency and overall magnitudes.

709 **Figure 9:** (a) Tank outflow dynamics (ET in green, sluice outflow in red and GE in blue)  
710 shown as weekly integrated volumes for all four tanks. These are stacked bar graphs with  
711 the areas shown in the different colors representing the subcomponents of the outflow. (b)  
712 Tank water outflows as a fraction of the tank capacity, with total outflows calculated as  
713 the sum of ET,  $S_0$  and groundwater recharge. The outflow-to-capacity ratios increase  
714 down the cascade, such that total outflows for Tank 4 over the study period are more than  
715 double the total tank capacity.

716 **Figure 10:** Water supply-and-demand portraits in our tank cascade. The grey area  
717 represents the Irrigation Water Demand (IWD), which is calculated as the difference  
718 between crop water requirements and rainfall (Brouwer et al., 1989). Planting dates were  
719 10/17, 10/17, 9/25, and 9/13 for Tanks 1, 2, 3, and 4, respectively. The darker red area  
720 corresponds to the portion of sluice outflow that is utilized to meet the irrigation water  
721 demand, while the light red area corresponds to the portion of sluice outflow that is  
722 “wasted.”

723

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- 883



884 Table 1

885 Population and land-use data for the study cascade.

Tank #	Village Revenue District	Population				Land Use			
		Total Population	Workforce	Farmers & Agricultural Laborers	% of Total	Agriculture	Forest	Settlements	Other
Tank 1	Pappinaickenpatti	3313	1986	1724	87%	73%	16%	2%	9%
Tank 2	Kudipatti	2122	1300	1172	87%	74%	13%	3%	11%
Tank 3						91%	-	5%	4%
Tank 4	Ketuvarpatti	622	356	316	89%	99%	-	1%	-
Cascade		6057	3642	3212	88%	81%	9%	3%	7%

886

887 Table 2: Summary of tank attributes based on historical tank data (made available by  
 888 DHAN Foundation) and the current study.

Tank #	Soil Type	Maximum Depth (m)	Maximum Tank Surface Area (ha)	Tank Command Area (ha)	Command Area/Surface Area Ratio	Tank Capacity (m <sup>3</sup> )		Current Capacity/Historical Capacity
						Historical	Current	
Tank 1	Alfisol	3.2	15	27	0.96	357,700	276,405	0.77
Tank 2	Vertisol	3.4	51	45	0.77	656,500	407,513	0.62
Tank 3	Vertisol	4.0	14	19	0.93	237,000	217,633	0.92
Tank 4	Vertisol	3.3	21	24	1.25	168,000	139,270	0.83

889

34-1

890 **Table 3** Partitioning of tank outflows across the Northeast Monsoon season.

	Tank 1	Tank 2	Tank 3	Tank 4
Total Outflows (m <sup>3</sup> )	376,794	762,483	352,934	377,257*
Evapotranspiration				
<i>Total (m<sup>3</sup>)</i>	48,291	164,423	78,745	64,358
<i>Percent of Total Outflows</i>	13%	22%	22%	17%
Sluice Outflow				
<i>Total (m<sup>3</sup>)</i>	153,038	146,612	72,279	207,636
<i>Percent of Total Outflows</i>	41%	19%	20%	55%
Recharge				
<i>Total (m<sup>3</sup>)</i>	175,465	451,448	201,910	105,263
<i>Percent of Total Outflows</i>	47%	59%	57%	28%

891 \*Note that the total outflow volume given here for Tank 4 does not include the 10/20 overflow event at the  
892 start of the monsoon season. As water exiting the tank via the overflow weir passes directly out of the tank  
893 catchment, bypassing the tank command area and thus not remaining as a source for irrigation or  
894 groundwater exchange within the tank cascade, we considered it separately from other flows.

895 **Table 4:** Water Balance Summary at the Tank Catchment scale

	Catchment 1	Catchment 2	Catchment 3	Catchment 4
Area (km <sup>2</sup> )	5.0	16.2	22.5	28.4
Precipitation P (MCM)	1.8	5.8	8.1	10.2
Runoff, Q (MCM)				
<i>with tanks</i>	0.15	0.30	0.37	0.69
<i>without tanks</i>	0.40	1.31	1.81	2.29
Recharge, R (MCM)				
<i>with tanks</i>	0.48	1.44	1.97	2.42
<i>without tanks</i>	0.31	0.99	1.37	1.73
Q/P				
<i>with tanks</i>	0.09	0.05	0.05	0.07
<i>without tanks</i>	0.22	0.22	0.22	0.22
R/P				
<i>with tanks</i>	0.27	0.25	0.24	0.24
<i>without tanks</i>	0.17	0.17	0.17	0.17

896

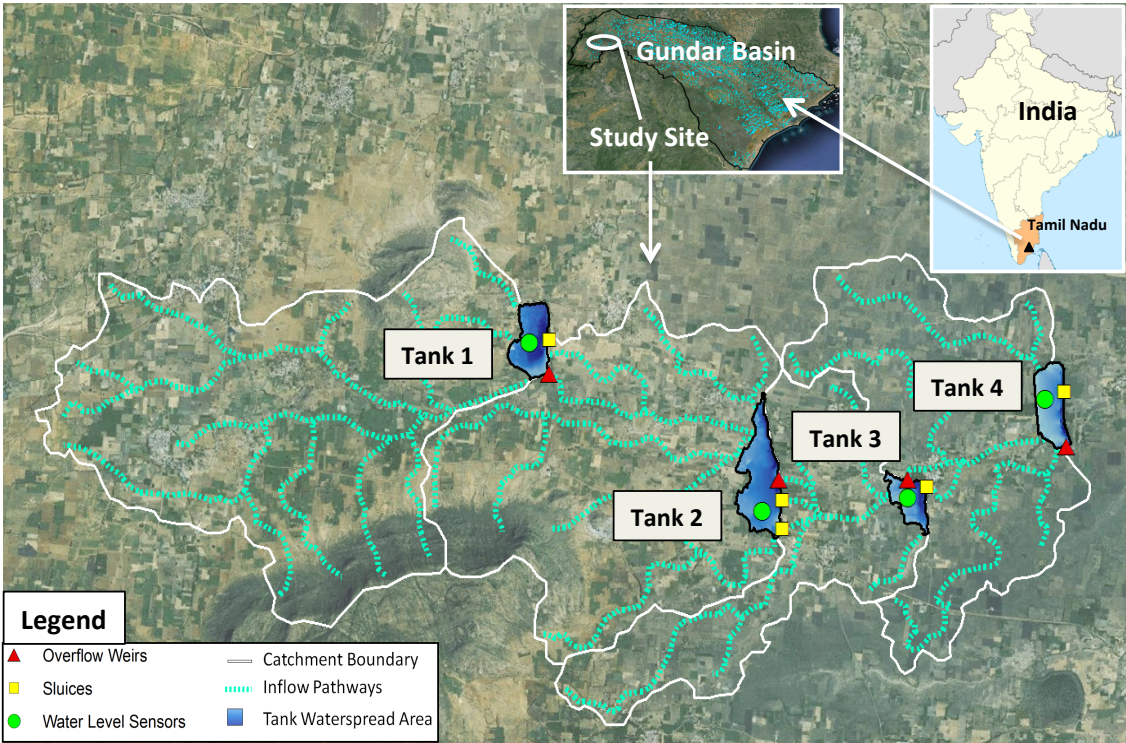
897 **Table 5:** Sluice outflows and irrigation water demand (IWD).

	Tank 1	Tank 2	Tank 3	Tank 4
Planting Date	10/17	10/17	9/25	9/13
Sluice Water				
<i>Total (mm)</i>	570	326	391	861
<i>Utilized (mm)</i>	283	210	333	516
<i>Surplus (mm)</i>	287	116	58	345
<i>Percent Surplus</i>	50%	36%	15%	40%
Irrigation Water Demand				
<i>Total (mm)</i>	996	996	872	752
<i>Unmet Demand (mm)</i>	713	786	540	235
<i>Percent Unmet</i>	72%	79%	62%	31%

898

Figure 1

a)



b)

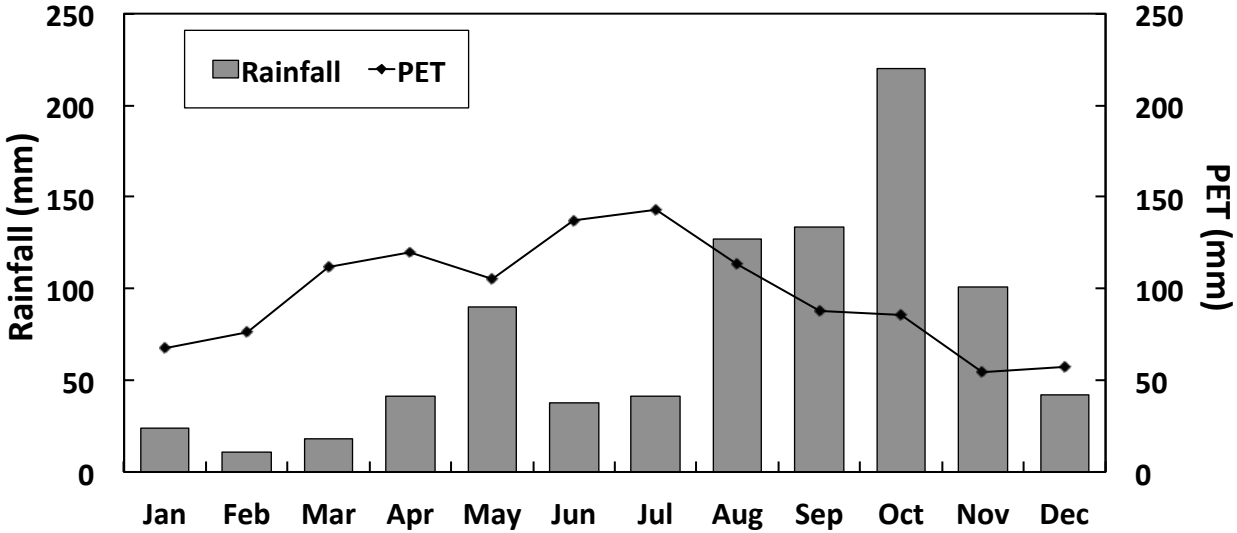
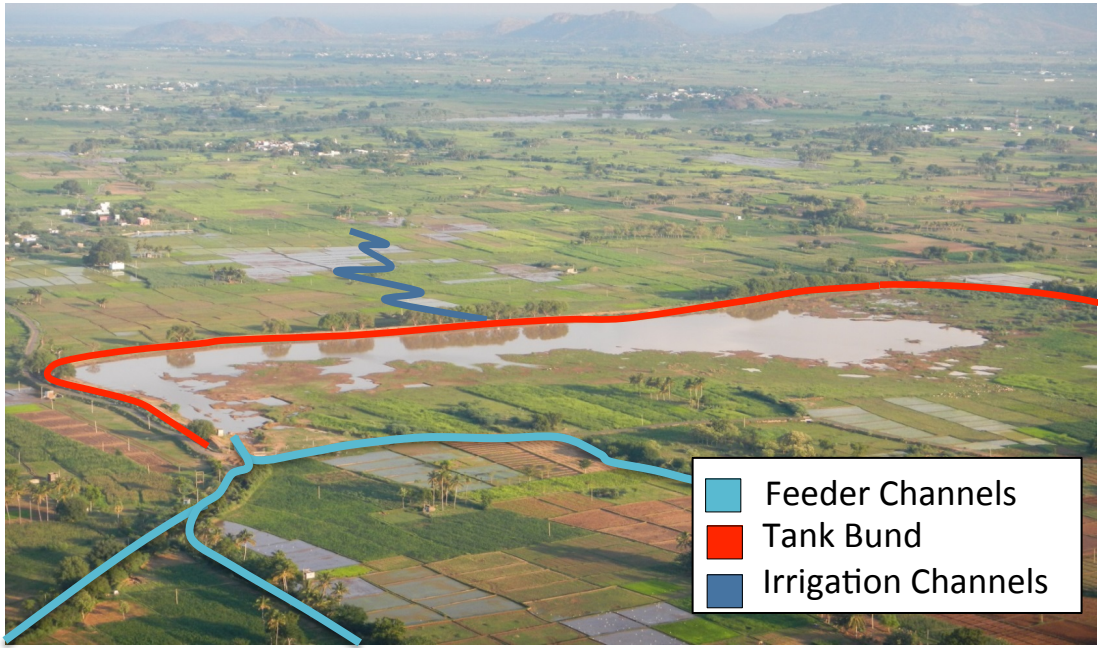
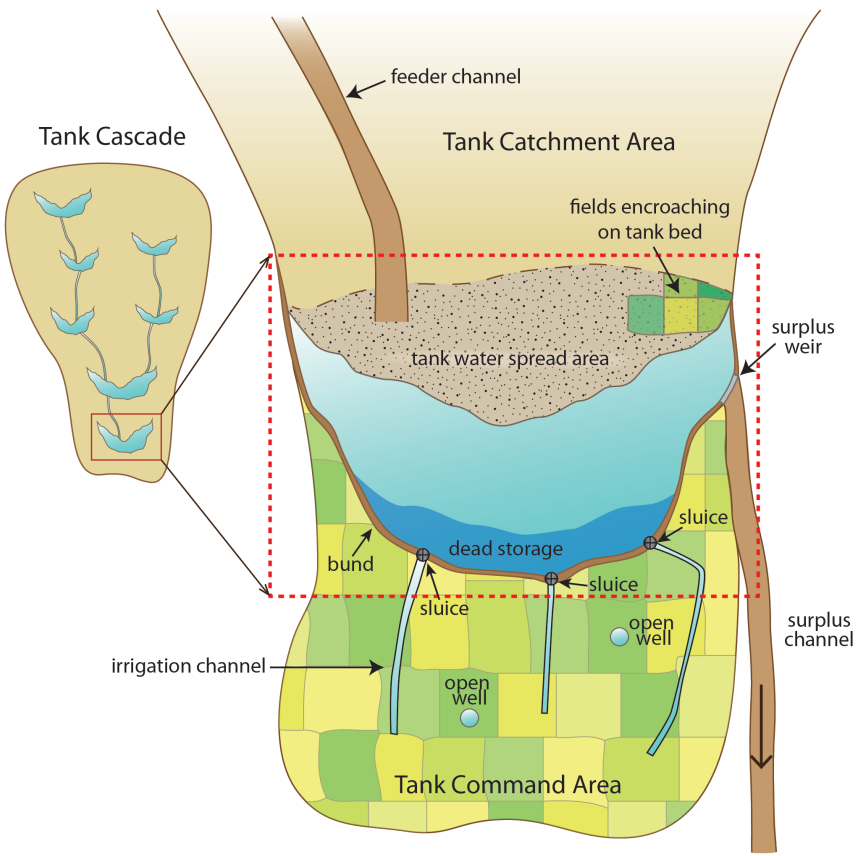


Figure 2

a)



b)



c)

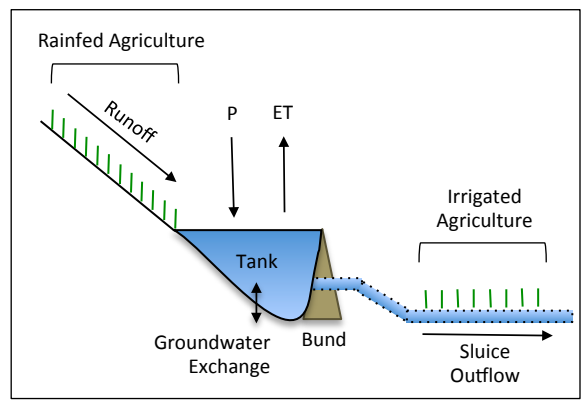
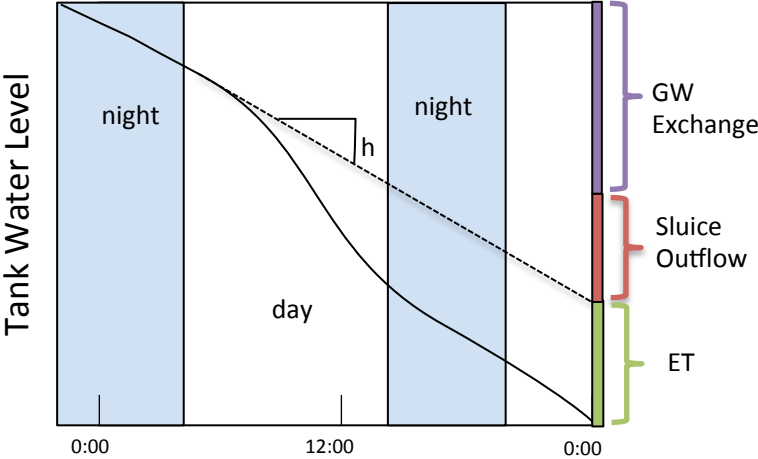


Figure 3



$$ET = S_y \cdot (s - 24h)$$

$$GE = S_y \cdot 24h - S_o$$



Figure 4

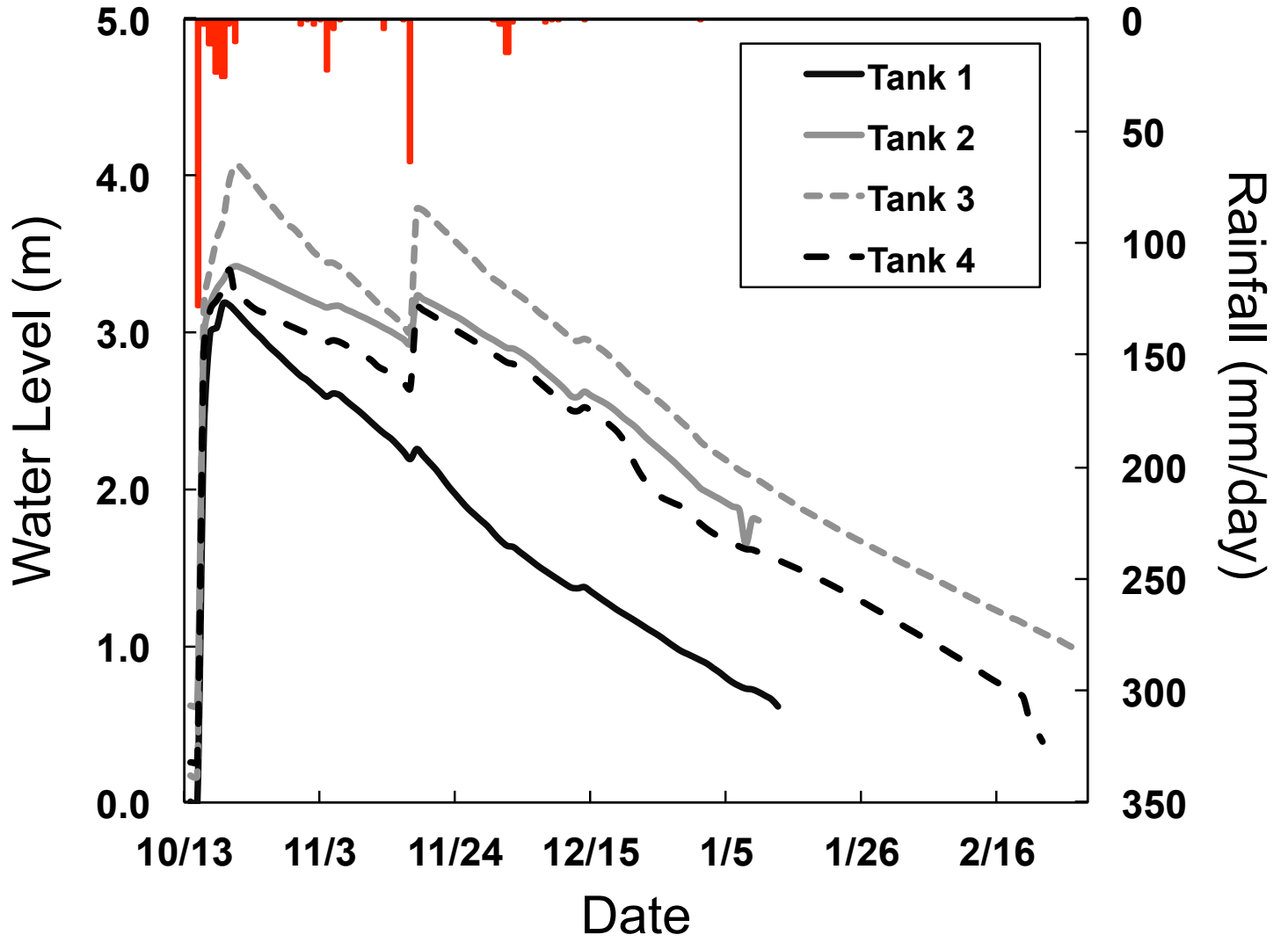


Figure 5

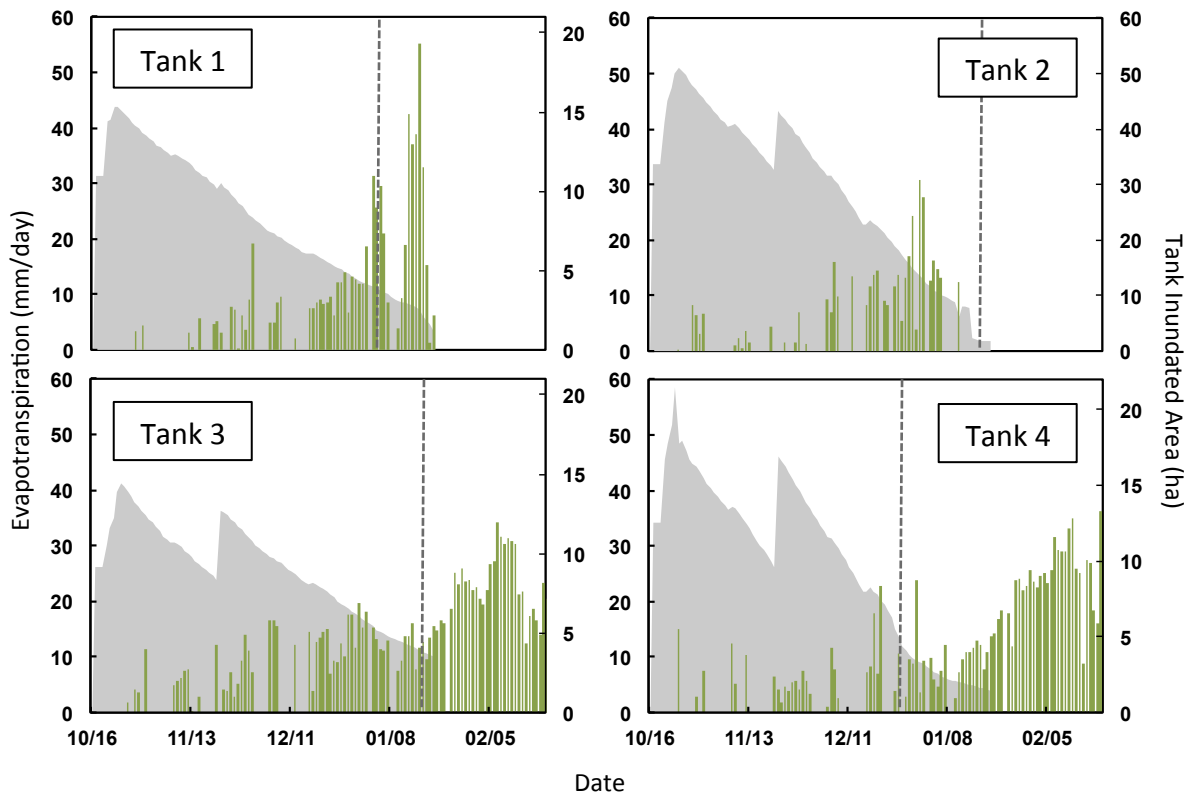


Figure 6

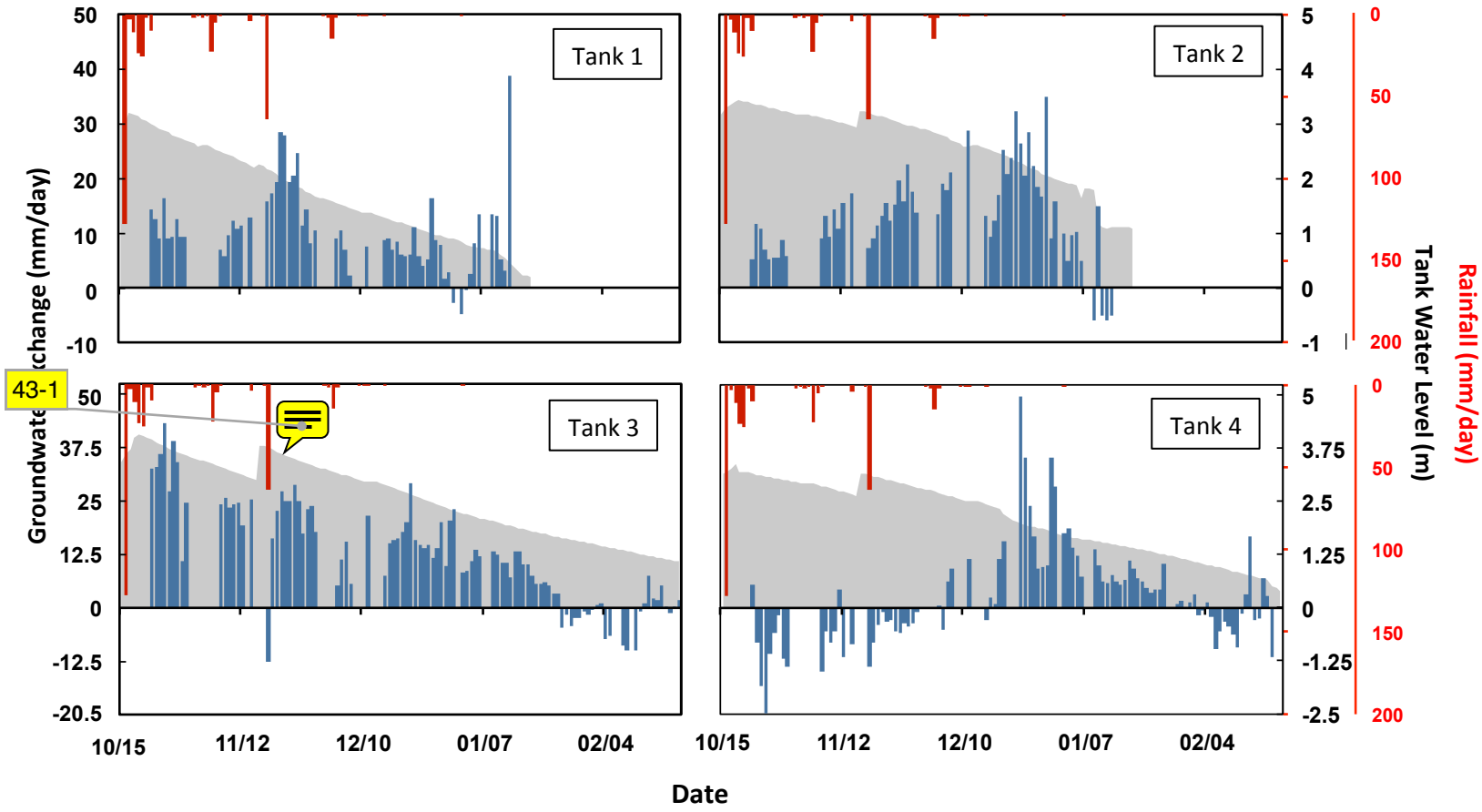


Figure 7

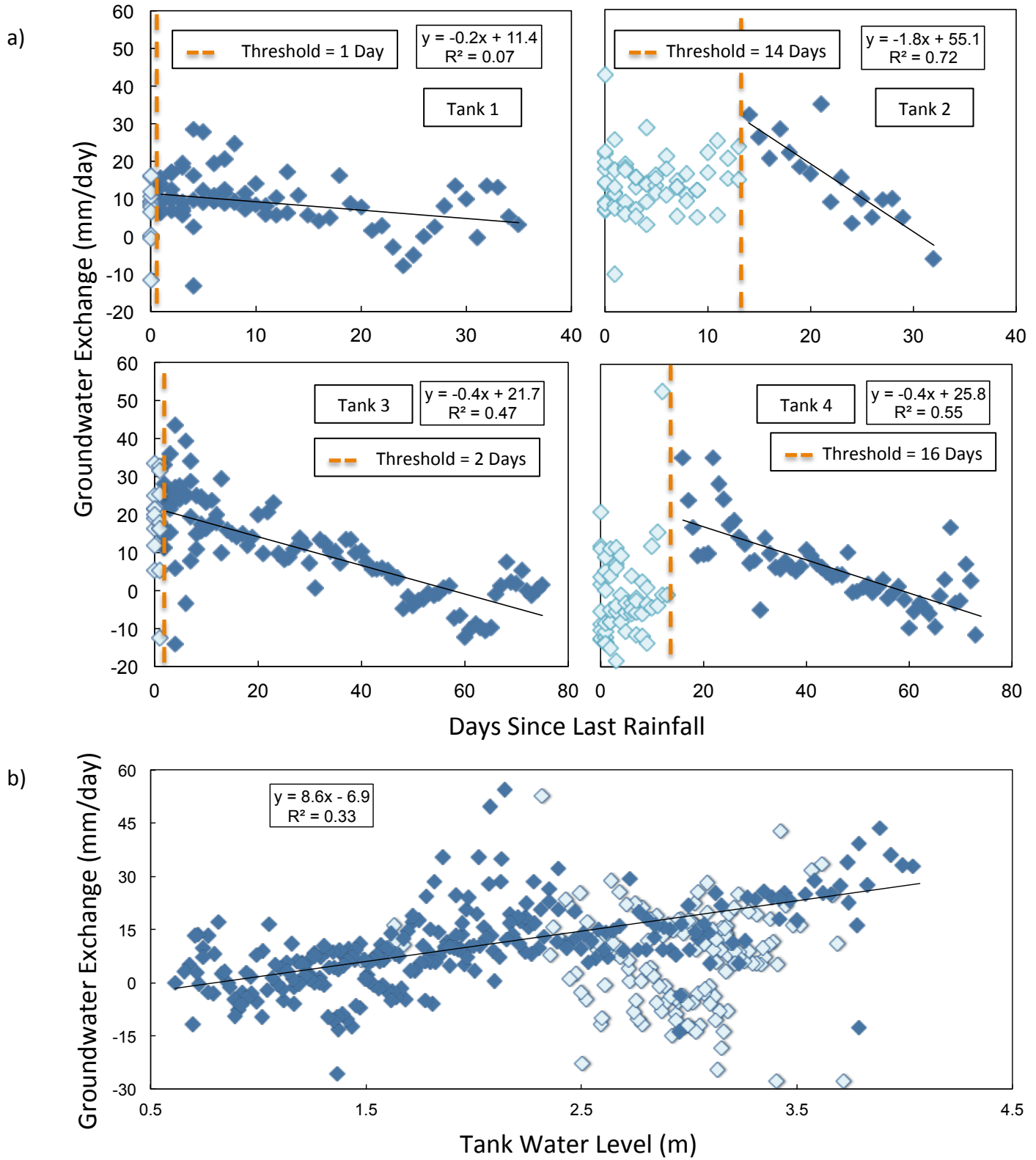
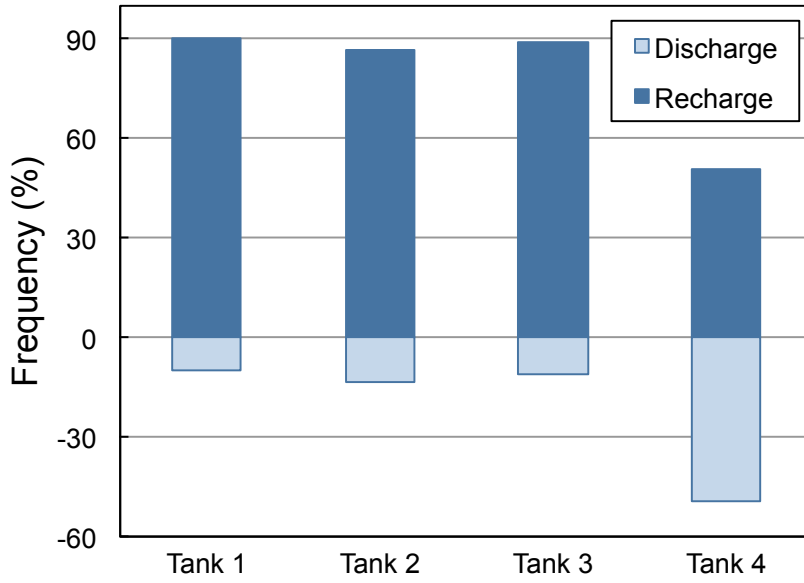


Figure 8

a)



b)

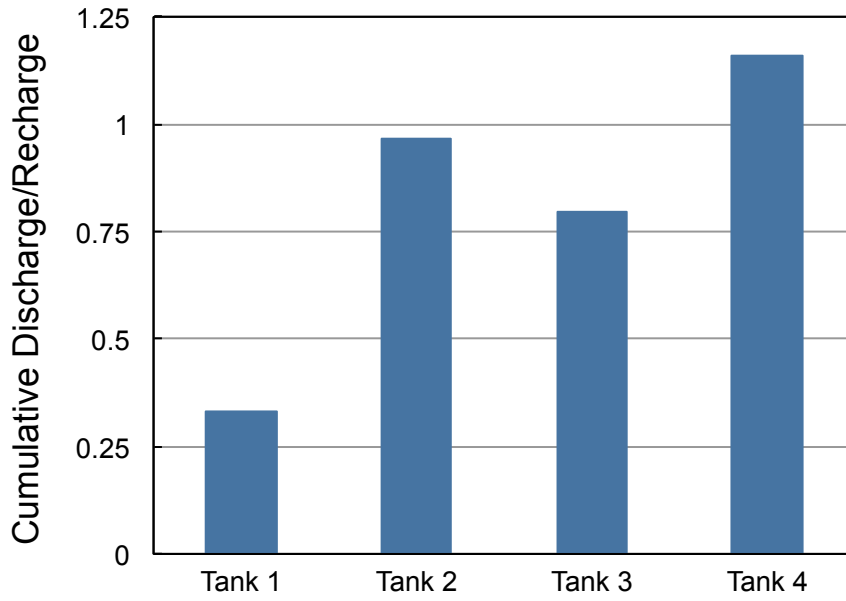


Figure 9

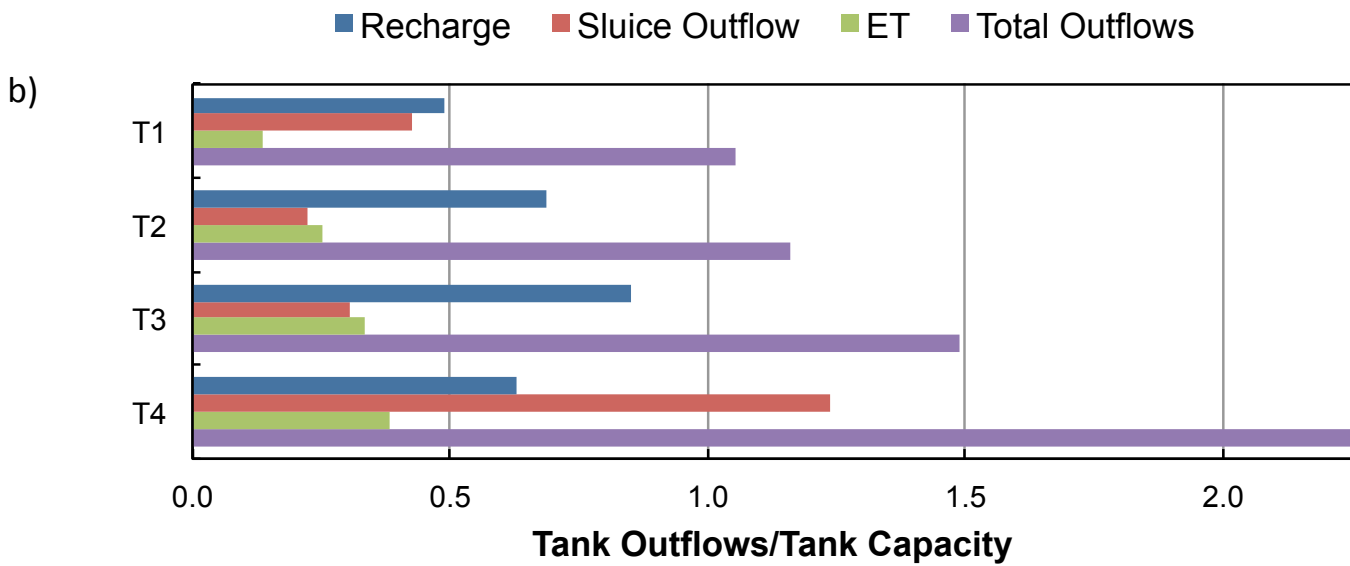
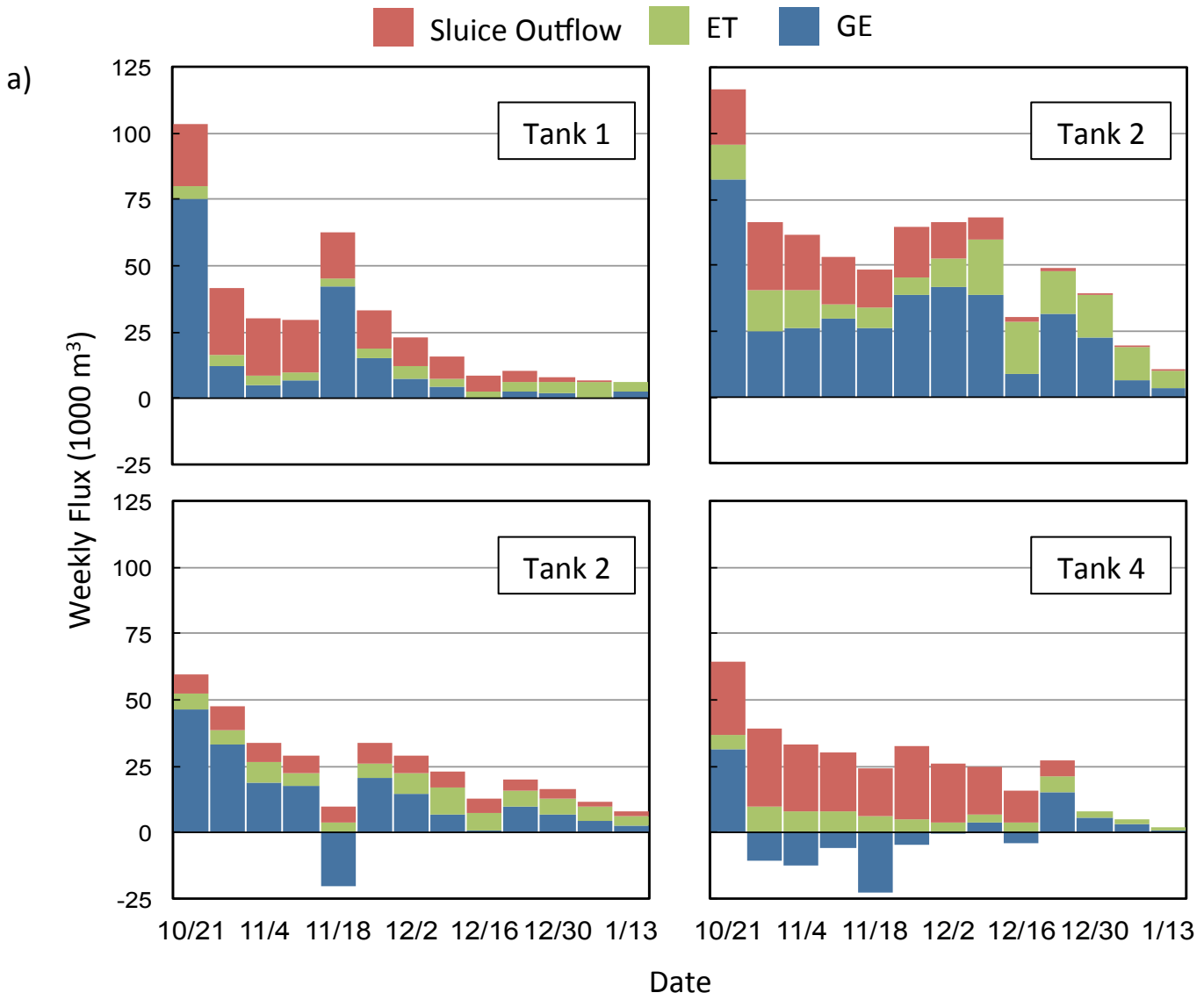
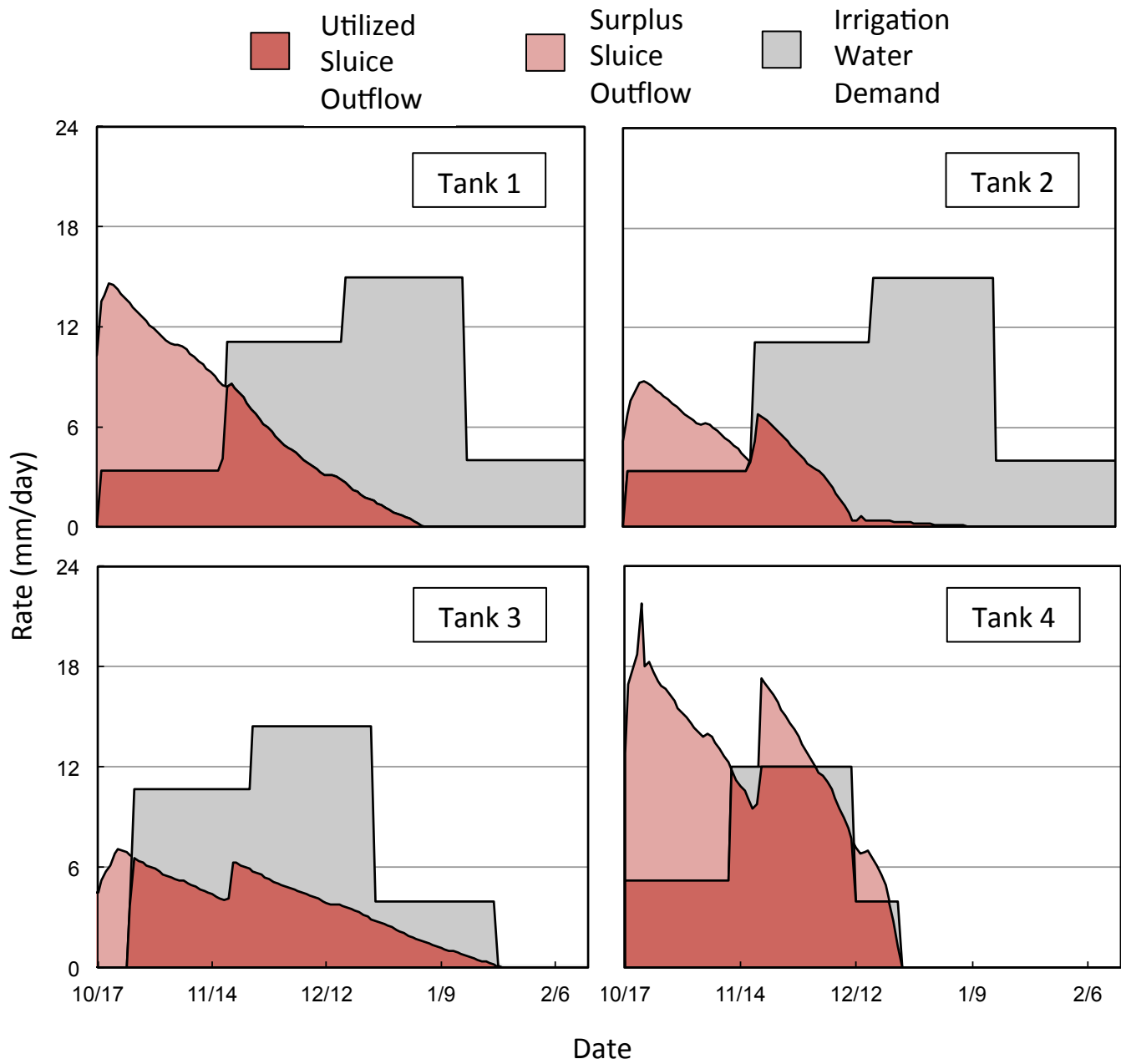


Figure 10



6-1 Dec 15, 2015, 11:42  
The other 44% falls when?

11-1 Dec 15, 2015, 11:42  
What is used as input? Tank water level? In stream water stage?

11-2 Dec 15, 2015, 11:42  
Into the tank? Downstream of the tank?

11-3 Dec 15, 2015, 11:42  
Since you measured overflow from the tanks 1 and 2, do you still need to calculate runoff contributions from their watersheds when calculating runoff into tank 3? Or is that part of the strange method?

13-1 Dec 25, 2015, 06:43  
To each other, and different than 1 and 3?

13-2 Dec 25, 2015, 06:43  
latter?

16-1 Dec 25, 2015, 06:43  
Good, helpful comment.

19-1 Dec 25, 2015, 06:43  
Because of sedimentation? Reduced capacity due to dam breaches! Functionality could include things like maintenance of irrigation canals, which shouldn't impact recharge. Can you clarify the mechanism linking maintenance to recharge, and how you calculated the resection in recharge due to poor maintenance?

20-1 Dec 25, 2015, 06:43  
(1989)

20-2 Dec 25, 2015, 06:43  
Interesting. Is that historically the case, or only recent due to neglect?

20-3 Dec 25, 2015, 06:43  
Or could recharge groundwater through channel infiltration? Could that result in less "waste" of the excess sluice water?

21-1 Dec 25, 2015, 06:43  
Though your iwd here doesn't include soil moisture storage at the start of planting....would inclusion of soil moisture in the iwd equation change your estimate of unmet demand? A detailed calculation isn't necessary.

21-2 Dec 25, 2015, 06:43  
With supplemental irrigation from groundwater until the tanks fill?

22-1 Dec 25, 2015, 06:43  
Including open water evaporation and et of plants in the tank bed itself (to clarify that it doesn't include et of crops irrigated by the tanks)

22-2 Dec 25, 2015, 06:43  
Discharge. Leak could mean loss through canal infiltration..?

34-1 Dec 15, 2015, 11:42  
 $27/15=0.96$ ? I don't think I understand how this ratio was calculated, or there is an error in the table.



43-1

Dec 25, 2015, 06:43

It looks like water level rise happens before rainfall. Is the an artifact of series alignment in the graph? It would be nice to have it make sense visually.