# Stochastic approach to analyzing the uncertainties and possible changes in the availability of water in the future

3 based on scenarios of climate change.

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#### 10 Abstract

11 The objective of this study was to analyze the changes and uncertainties related to water 12 availability in the future (for purposes of this study the period between 2011 and 2040 was adopted), using a stochastic approach, taking as reference a climate projection from the 13 14 climate model Eta CPTEC/HadCM3. The study was applied to the Ijuí river basin in the south of Brazil. The set of methods adopted involved, among others, correcting the climatic 15 16 variables projected for the future, hydrological simulation using Artificial Neural Networks to 17 define a number of monthly flows and stochastic modeling to generate 1,000 hydrological series with equal probability of occurrence. A multiplicative type stochastic model was 18 19 developed in which monthly flow is the result of the product of four components: i) long term 20 trend component; ii) cyclic or seasonal component; iii) time dependency component; iv) 21 random component. In general the results showed a trend to increased flows. The mean flow 22 for a long period, for instance, presented an alteration from 141.6 m<sup>3</sup>/s (1961-1990) to 200.3 23  $m^{3}/s$  (2011-2040). An increment in mean flow and in the monthly standard deviation was also 24 observed between the months of January and October. Between the months of February and 25 June, the percentage of mean monthly flow increase was more marked, surpassing the 100% 26 index. Considering the confidence intervals in the flow estimates for the future, it can be 27 concluded that there is a tendency to increase the hydrological variability during the period 28 between 2011-2040, which indicates the possibility of occurrence of time series with more 29 marked periods of droughts and floods.

#### 1 1 Introduction

Discussions concerning variability and climate changes have intensified in the last few
decades. Many studies have proved significant alterations in the composition of the
atmosphere and in the concentration of gases which have implications on thermal energy,
changing climate-related variables. On this topic the Intergovernmental Panel on Climate
Changes (IPCC) should be highlighted.

7 The IPCC was established in 1988 by the World Meteorological Organization (WMO) and by 8 the United Nations Environment Program (UNEP). The objective is to supply scientific 9 information in order to gain a better understanding of changes in the global climate, so as to 10 evaluate their impact on society and on nature, and propose alternatives for adaptation and 11 mitigation.

According to IPCC (2013), it is already clear that the Earth is warming since the beginning of the industrial period, as proved by the rise in the mean temperatures of the air and the oceans. Consequently, negative impacts have being observed, as the increase in the mean level of the seas and the acceleration of ice melt in mountain or polar climate regions. Studies developed on a global scale have shown that several natural systems are already under the impact of climate changes.

18 Changes in temperature and precipitation will lead to an increased frequency of extreme 19 meteorological events, such as severe floods and droughts, which will inevitably affect the 20 availability of water for human consumption, irrigation, industries and other uses (IPCC, 21 2013). Some research studies on the sensitivity of agricultural crops to climate changes show 22 that there may be a strong negative effect on crop growth, increasing the risk of losses in 23 harvests worldwide (Mearns et al., 1996; Richter and Semenov, 2005; Zhang and Liu, 2005; 24 Rasmussen et al., 2012).

The climate scenario projections are performed using Global Climate Models (GCMs) and Regional Climate Models (RCMs). The resolution of RCMs is between 10 and 50 km, which allow applying them in scenarios of climate changes in medium and small basins. Using these models, together with the GCMs, enables detailing the climate processes at the local level, detecting the variations and specificities of a given region, and thus improving the understanding of impacts in small basins (Marengo et al., 2009; 2012). The Eta Model was developed at Belgrade University and operationally implemented by the National Centers for Environmental Prediction (Black, 1994). The vertical coordinate system used in this model is recommended for use over South America due to the presence of the Andes mountain range (Marengo et al., 2012). Recently, a new version of Eta Model, Eta CPTEC, was developed independently by the National Institute for Space Research (INPE).

6 The regional Eta Model was configured over South America and applied to downscale 7 HadCM3 members of the Perturbed Physics Ensemble (PPE) experiment for the baseline 8 (1961-1990). The dynamic downscaling method was used to generate the climate scenarios 9 (Chou et al., 2012). According to Mujumdar and Kumar (2013), the main advantage of 10 dynamical downscaling over the statistical downscale method is its ability to capture the mesoscale non-linear effects. Furthermore, the dynamical downscaling provide information 11 12 for many climate variables, while ensuring internal consistency with respect to the physical principles in meteorology, simulating satisfactorily some regional climatic conditions. 13

14 The Eta CPTEC Model includes the increase in CO<sub>2</sub> concentration levels according to the 15 scenario of emission and daily variation of the state of vegetation during the year. This model 16 reproduces scenario A1B of IPCC SRES, supplied by the global coupled ocean-atmosphere 17 HadCM3, in four members (versions) of disturbance in the global model - (no disturbance -18 CNTRL; low sensitivity - LOW; medium sensitivity - MID; high sensitivity - HIGH), which represent the uncertainty of boundary conditions, to produce variants of the same model 19 20 (Chou et al., 2012; Marengo et al., 2012). The regional model was integrated into the 21 horizontal resolution of 40 km, for the period between 1961 and 1990, and the future 22 scenarios were generated in three 30-year periods (from 2011 to 2040, from 2041 to 2070, 23 from 2071 to 2100) (Chou et al., 2012).

24 The study of Marengo et al. (2012) details the scenarios generated for South America using 25 the Eta CPTEC/HadCM3 Model. According to this study, the model is configured with 38 vertical layers with the top of the model at 25 hPa. The Mellor-Yamada level 2.5 procedure 26 27 (Mellor and Yamada, 1974) was used for the treatment of turbulence. The radiation package 28 was developed by the Geophysical Fluid Dynamics Laboratory, based in studies of Fels and 29 Schwarzkopf (1975) and Lacis and Hansen (1974). The Eta Model uses the Betts-Miller 30 (Betts and Miller, 1986) scheme modified by Janjic (1994) to parameterize deep and shallow 31 cumulus convection, and the Zhao scheme (Zhao et al., 1997) to parameterize cloud microphysics. This model uses also the NOAH scheme (Ek et al., 2003) to parameterize the
land-surface transfer processes (Marengo et al., 2012).

Pesquero (2009), Chou et al. (2012) and Marengo et al. (2012) used Model Eta CPTEC. In the first two studies, the model was used to reproduce the present climate on South America and certify the quality of the model. A smooth tendency was observed to underestimate precipitation over the Amazon in the rainy season and the central region of Brazil, in the Brazilian Savanna. In the last study (Marengo et al., 2012), model Eta CPTEC was used to study the climate changes in the Amazon, São Francisco and Paraná river basins between 2011 and 2100.

Currently, in the scientific literature, there are several studies that analyze the effects of 10 11 climate changes on water availability (examples: Kleinn et al., 2005; Hughes et al., 2011; 12 Gunawardhana and Kazama, 2012). On a continental or global scale, normally, the outputs of 13 the GCMs are used in combination with the empirical macroscale hydrological models which 14 perform the water balance (for instance, Arnell, 1999; Nijssen et al., 2001; Arnell, 2004; Milly et al., 2005; Nohara et al., 2006). The studies on water availability in smaller river 15 16 basins normally use the climate projections for the RCMs, associated with empirical or 17 physically-based hydrological models, in a deterministic approach, offering only a single 18 result in the hydrological sphere for each climate scenario. Examples of this are the studies by Middelkoop et al. (2001), Menzel and Bürger (2002) and Kleinn et al. (2005). 19

However, because of the randomness of hydrometeorological processes, the uncertainties related to climate modeling and future water availability favor the use of probabilistic methods based on stochastic time series, as in the studies by Wilks (1992), Semenov and Barrow (1997) and Booij (2005). The stochastic approach broadens the possibility of analyzing water availability and the climatic uncertainties in the future, offering a great number of scenarios for analysis. Thus, it is possible to identify the confidence intervals in the projection and to estimate the random component of the climatic and hydrological dynamics.

However, when generating hundreds or thousands of stochastic climate series, it is necessary to repeat the hydrological simulation often, rendering the modeling process very onerous from the computational standpoint. Moreover, in this approach the hydrological scenarios produced become even more sensitive to any imprecision in estimating the parameters of the rainfallflow transformation model. In order to minimize the processing time, this methodology will cover the randomness of the processes and the climate dynamics directly in the flow series, using a stochastic model appropriate for monthly flows. Thus, based on a single climate scenario, a flow series is generated by hydrological deterministic simulation and then the stochastic process is performed.

6 The objective of this study is to analyze the possible scenarios and uncertainties related to 7 water availability in future, using a stochastic approach based on a climatic change scenario 8 originating in the Eta CPTEC/HadCM3 climate model. This study will be applied to the Ijuí 9 river basin, in Rio Grande do Sul (RS), Brazil.

#### 10 2 Methodology

The set of methods adopted in this study comprised the use of observed and simulated hydrometeorological data, to analyze the uncertainties and possible scenarios of water availability in the future, based on the scenario A1B of IPCC SRES, generated by the regional climate model Eta CPTEC/HadCM3.

15 First, it is important to emphasize that the selection of climate change scenario was made at 16 the beginning of a research project (2010-2014). At that time, the new IPCC scenarios, for the 17 AR5, were not yet available. Furthermore, all the data from the climate model Eta CPTEC were provided by the National Institute for Space Research (INPE). This agency has 18 19 recommended the use the A1B scenario in four versions with different sensitivities. These versions were already being examined in large areas of the South American continent 20 21 (example: Marengo et al., 2012). Therefore, given this context, the impacts of climate changes 22 in medium and small river basins of Brazil were evaluated in more detail with the use of the 23 A1B scenario.

For this study, considering the availability of the climatic data derived from the regional climate model Eta CPTEC/HadCM3, the years between 1961 and 1990 were considered as the base period, and the years between 2011 and 2040 as the "future" period.

27 Simplifying, the methodological procedure covered: i) spatial interpolation of the 28 meteorological variables; ii) selection of the climatic scenario and correction of the climate 29 variables; iii) estimation of the potential evapotranspiration; iv) hydrological simulation 30 using Artificial Neural Networks (ANNs); v) stochastic modeling of monthly flows to 31 generate possible hydrological series in the future.

#### 1 2.1 Study Area

This study was applied in the Ijuí River Basin, in the Santo Ângelo stream gauging section, in the northwest of RS, Brazil. The basin area is 5,414 km<sup>2</sup> and it is located between the following geographic coordinates: latitudes 27.98°S to 28.74°S and longitudes 53.21°W and 54.28°W (Fig. 1). At this stream gauging station between 1941 and 2005, the mean flow ( $\bar{Q}$ ) was 138 m<sup>3</sup>/s, and the dry and high flow periods were the months of March ( $\bar{Q}$ = 72 m<sup>3</sup>/s) and October ( $\bar{Q}$ = 211 m<sup>3</sup>/s), respectively.

8 The area of the study was chosen because the region depends to a great extent on agricultural 9 activities and may suffer serious socioeconomic impacts from the climate changes. According 10 to the State Coordinator of Civil Defense of RS, during the period between 1982 and 2011 11 there were at least six severe dry periods in the basin region. These dry periods caused great 12 losses to the agricultural and cattle activities, mainly those involving soy beans and maize.

Considering the daily weather observations of the Cruz Alta station, operated by INMET (National Institute of Meteorology), the winter and spring months (from June to December) are the rainiest. According to Rossato (2011), the annual rainfall is 1,750 mm which occurs within 110 days during the year. The annual mean temperature oscillates between 17 and 20°C. The coldest months are June and July, with a mean of around 14°C, and the warmest months are January and February, with a mean of around 24°C.

#### 19 **2.2 Data**

20 The following materials were used in this study:

i) daily historical series of precipitations provided by the HidroWeb site of the National Water
Agency (ANA), during the period between 1961 and 1990, at 77 rain gauging stations within
the radius of coverage of 100 km of the basins boundaries (Fig. 2);

ii) daily historical series of precipitation provided by IPH (Castro et al., 1999), in the years
1989 and 1990, at 22 rain gauging stations (Fig. 2);

iii) daily historical series of precipitation, temperature, wind speed, solar radiation,
atmospheric pressure and relative humidity of the air provided through the portal of BDMEP
(Bank of Meteorological Data for Teaching and Research) of INMET, during the period
between 1961 and 1990, at five meteorological stations (Fig. 2);

iv) daily historical series of flows from the Santo Ângelo station, located at coordinates
 28.36°S and 54.27°W, provided through the HidroWeb site, during the period between 1961
 and 1990;

v) daily data simulated by the regional climate model Eta CPTEC, conducted by four
members of the global climate model HadCM3, with different levels of sensitivity (CNTRL,
LOW, MID and HIGH), during the periods of 1961-1990 (base) and 2011-2040 ("future").
The variables simulated were: precipitation, temperature, wind speed, relative humidity of the
air, atmospheric pressure and solar radiation.

#### 9 2.3 Spatial Interpolation

10 The first stage consisted of the spatial interpolation of the five daily climate variables 11 (temperature, wind speed, relative humidity of the air, atmospheric pressure and solar 12 radiation), and daily precipitation in the periods between 1961-1990 (observed and simulated 13 data) and 2011-2040 (data simulated by the Eta model). The interpolation grid was generated 14 with a spatial resolution of 5 km (Fig. 2), totalizing 264 nodes in the basin area. The interpolation procedure was performed for all data sets: i) series observed at 104 rain gauging 15 or meteorological stations; ii) series simulated using model Eta CPTEC/HadCM3 in four 16 scenarios of climate sensitivity (CNTRL, LOW, MID and HIGH). 17

The use of so many stations in a 100 km radius, to begin the interpolation process consists of a safety margin, since many of these stations present short series, with many gaps. Thus, only on a few days when the stations closest to the interpolation grid present gaps, the method can select rainfall data from stations located slightly further away, in this way avoiding failures in estimating precipitation during the interpolation process. It can be said that for each day, in every node of the interpolation grid, only the closest stations with rainfall data were used, usually within the basin and immediate surroundings

The interpolation method used was that of the natural neighbor (Sibson, 1981). This interpolation method obtained the best results in the study presented by Silva et al. (2013), with precipitation series similar to those used in the present study, also in the Ijuí river basin. In the study mentioned, the following methods were also tested: closest neighbor, linear triangulation and inverse distance weighting.

30 The natural neighbor method is based on the concept of area of influence of the sampling 31 points determined by Voronoi polygons. These polygons are obtained from the Delaunay triangulation. For each point on the interpolation grid, the weight of each sampling point is calculated because of the area of influence. The daily value of each variable in the basin was obtained from the mean of the values interpolated in all nodes of the regular grid.

Still at this stage, the daily mean value of the five climate variables and of precipitation in the Ijuí river basin was calculated, considering the data observed and the data simulated by the ETA model. Finally, the monthly accumulated precipitation for the observed series and for scenarios simulated by the Eta model in the periods of 1961-1990 (base) and 2011-2040 (future) were calculated.

#### 9 **2.4** Selection of climate scenario and correction of climate variables

The outputs of climate models should not be used directly to estimate future water availability (Graham, 2000). The climate models may not represent perfectly the current climate due mainly to the influence of the spatial discretization of the models. It is observed (Lenderink et al., 2007) that the outputs may present systematic errors. The correction of climate variables is intended to prevent that the errors intrinsic to the output of the climate models are propagated to the subsequent hydrologic modeling.

16 Recently several techniques to correct the climate variables resulting from the GCMs and 17 RCMs were developed and compared (Themeßl et al., 2012). The use of disturbances (Delta 18 Change Approach) in climate variables is a commonly used strategy to simulate the impacts 19 of climate changes, obtained via global or regional climate models on water resources (Graham, 2004; Lenderink et al., 2007). The technique consists of using only the seasonal 20 21 change foreseen between the current and future scenario, obtained with the climate model. 22 This change is represented by the difference between the current climatic conditions and those 23 foreseen for the future, both conditions obtained by the climate model. The change foreseen is 24 incorporated to the historical series of precipitations and temperature to generate the series in 25 the future. Thus the error associated with climate modeling is eliminated from the current 26 conditions, and becomes limited to the uncertainties associated with the forecast of climate changes for the future. Examples of applying this methodology are the studies by Kaczmarek 27 et al. (1996), Lettenmaier et al. (1999), Graham (2000), Bergström et al. (2001). 28

However, as mentioned by the authors themselves (Graham, 2000; Bergström et al., 2001), and supported by Lenderink et al. (2007), applying the forecast changes in temperature or in precipitation directly on the series observed implies considerable simplifications which may compromise the analysis of the projections in future. In this approach, for instance, probable changes in the number of rainy days, in dispersion (variance) of rains or in the extreme values of temperature are not considered. This occurs because the series itself observed in the past consists of the base of forecasts for the future, and only the seasonal mean variations are taken into account. In this case, there is a risk of considering that the same anomalies recorded in the past will be observed in the future with small changes in the monthly magnitude of climate variables, according to time of the year.

8 Thus, Lenderink et al. (2007) discuss and analyze how the output of a regional climate model 9 should be corrected to obtain more realistic flows for the current climate and, consequently, 10 for the future climate. According to the authors, the development of regional climate model, with some corrections in the output, allows the Direct Approach in using projections of 11 12 temperature and rainfall for the future. This method, instead of adding the changes forecast in the series observed performs a different procedure: i) it detects the differences between the 13 current climatic conditions, i.e., between the conditions observed using meteorological 14 15 stations and the conditions simulated by the regional climate model; ii) it applies these 16 differences in the series forecast for the future.

17 Other more sophisticated methods have been tested and compared, with applications at daily 18 or monthly time intervals, as can be seen in Wood et al. (2004), Maurer and Hidalgo (2008), 19 Boé et al. (2007), Piani et al. (2010) and Bárdossy and Pegram (2011). In a recent study, 20 Themeßl et al. (2011) compared a few correction methods and concluded that the Quantile-Based Mapping technique (Panofsky and Brier, 1968) is the most effective one to remove the 21 22 errors in the precipitation data. This method is applied with small adaptations in the studies 23 listed above. Essentially, the method is based on the differences between the accumulated 24 probability curves (simulated and observed) of daily or monthly precipitations.

25 In the study by Oliveira et al. (2015a), whose objective was to evaluate the climatic 26 conditions simulated using model Eta CPTEC/HadCM3, emphasizing the study of water 27 availability in the Ijuí river basin, four methods to correct the climate variables were tested; i) 28 Delta Change Approach, ii) Direct Approach, iii) Monthly Quantile-Based, iv) Quarterly Ouantile-Based. The control period in which the corrections were applied and the 29 30 hydrological model calibrated, was defined between 1961 and 1975. The evaluation period, in which the results of the climate scenarios and water availability were found, was 1976 and 31 32 1990. For both periods data were available that had been observed at rain gauging stations and meteorological stations and data simulated by the regional climate model Eta CPTEC,
 conducted by four members of the global climate model HadCM3, with different levels of
 sensitivity.

4 The main results obtained in Oliveira et al. (2015a) were: i) only Eta HIGH did not prove 5 satisfactory in most of the aspects analyzed regarding precipitation, evapotranspiration, and 6 flow; ii) in evaluating the flows resulting from the hydrologic modeling process, the Eta LOW 7 member was outstanding, especially as regards to the mean monthly flows (mean error of 8 22.6%), to the annual flow permanence curves (mean error 12.6%) and to the quarterly flow 9 permanence curves (mean error 27.3%); iii) with Eta LOW a good adjustment can be seen, 10 both to the low flows (permanence greater than 90%) and to the high flows (permanence less 11 than 10%); iv) the outstanding climate scenario was Eta LOW, applying the Direct Approach 12 correction method, especially as to the curve of permanence of the flows; v) finally, it was pointed out that in the case of the precipitations and flows the difference between simulated 13 14 values, based on the Eta model and the values observed was greater than those of 15 evapotranspiration, resulting in errors that were sometimes greater than 20%. One should, 16 therefore, consider that these uncertainties will be reproduced in future scenarios (for the coming decades of the 21<sup>st</sup> century). 17

18 Since the present study focuses on a stochastic approach that takes into account the 19 uncertainties associated with the various stages that comprise the modeling of water 20 availability in future, it was necessary to adopt a climate scenario to test the methodology. 21 Thus, taking into account the results obtained in Oliveira et al. (2015a), the use of the Eta 22 LOW member was defined, applying the Direct Approach correction method.

In the Direct Approach method used by Lenderink et al. (2007), the precipitation that is corrected in the future period (2011-2040), in month k, in year j, is equal to the precipitation simulated during the same period, month and year, multiplied by a correction factor. The correction factor in this method is the ratio between the mean precipitation observed during the base period (1961-1990), in month k, and the mean precipitation simulated in the same period and month (Eq. 1).

29 
$$Pcor(fut)_{k/j} = Psim(fut)_{k/j} * \left[ \overline{Pobs(base)_k} / \overline{Psim(base)_k} \right]$$
 (1)

30 Where:  $Psim(fut)_{k/j}$  is the precipitation simulated during the future period, in month k and 31 year j;  $\overline{Pobs(base)_k}$  is the mean of the precipitation observed during the base period for 1 month k;  $\overline{Psim(base)_k}$  is the mean of precipitation simulated during the base period for 2 month k.

The other five climatic variables (temperature, wind speed, relative humidity of the air,
atmospheric pressure and solar radiation) were corrected in the daily time interval, using
Direct Approach, as shown in Equation 2.

6 
$$C_Z cor(fut)_{i/k/j} = C_Z sim(fut)_{i/k/j} * \left[\overline{C_Z obs(base)_k} / \overline{C_Z sim(base)_k}\right]$$
 (2)

7 Where:  $C_z cor(fut)_{i/k/j}$  is the correct value of climate variable z in the future period, in day 8 *i*, month *k*, and year *j*;  $C_z sim(fut)_{i/k/j}$  is the value simulated of climate variable z during the 9 future period, in day *i*, month *k*, and year *j*;  $\overline{C_z obs(base)_k}$  is the mean value observed of the 10 climate variable z during the base period for month *k*;  $\overline{C_z sim(base)_k}$  is the mean value 11 simulated of the climate variable z during the base period for month *k*.

#### 12 **2.5** Estimation of reference evapotranspiration

In the third stage the (daily) reference evapotranspiration was calculated for the simulated and 13 14 corrected climate scenario and for the observed series in the base (1961-1990) and future 15 (2011-2040) periods. The reference evapotranspiration was calculated using the Penman-16 Monteith method (Penman, 1948; Monteith, 1965), which has been considered the most 17 reliable method by some authors and was adopted as the standard method by the United National Food and Agriculture Organization (FAO) (Allen et al., 1998). This method is 18 parameterized for an area completely covered with 12 cm high grass, considering the 19 aerodynamic resistance of the surface of 70 s.m<sup>-1</sup> and albedo of 0.23, in soil without a water 20 21 deficit.

After calculating the daily evapotranspiration, these values were converted to the monthly time interval, rendering it compatible with the monthly accumulated precipitation series for hydrological modeling.

#### 25 **2.6** Hydrological simulation using Artificial Neural Networks (ANNs)

Recently, several studies have obtained excellent results applying ANNs in the field of water
resources and hydrology, especially in the development of models for simulation, forecasting
and classification (Bowden et al., 2005; Jain and Kumar, 2007; Leahy et al., 2008).

1 The methodology adopted in this study comprised the use of a hydrological model based on 2 ANNs, consisting of transformations of the meteorological and pluviometric variables. The 3 program for the necessary implementation was developed in the MATLAB R2010a 4 environment, consisting mainly of a generalized model, constituted by linear transformations 5 of inputs and outputs from a neural network with a hidden layer (Eq. 3).

$$6 \qquad \frac{(y_t - bo)}{ao} = ANN\left(\frac{(x_t - bi)}{ai}\right) \tag{3}$$

7 Where:  $x_t$  and  $y_t$  are the input and output variables, respectively; *ao* and *bo* are the 8 parameters of scale and position of the model outputs; *ai* and *bi* are the parameters of scale 9 and position of the model inputs; *ANN* is the Artificial Neural Network.

10 The choice of a three-layer architecture was based on the Kolmogorov mapping neural 11 network existence theorem (Hecht-Nielsen, 1987), which stated that any continuous function 12 with n inputs can be implemented exactly by a three-layer feedforward neural network with 13 2n+1 processing elements in the intermediate layer.

14 The ANN is the model core and is represented by Equation 4:

15 
$$y = f_o(\sum_h w_o f_h(\sum_i w_h x + b_h) + b_o) + e_o$$
 (4)

16 Where: x and y are the matrices with inputs (i) and outputs (o), respectively;  $w_h$ ,  $b_h$ ,  $w_o$  and 17  $b_o$  are the synaptic weight and the tendencies of the hidden layer (h) and the output layer (o), 18 respectively;  $f_h$  and  $f_o$  are the activation functions, respectively of the hidden and output 19 layers;  $e_o$  is the expected error at the output layer.

The activation function used, both for the hidden layer and for the external layer was the unpolar sigmoid, with outputs at the interval [0,1], whose derivate can be calculated only as a function of the output, and they are represented by Equations 5 and 6.

23 
$$a = f(n) = \frac{1}{1+e^{-n}}$$
 (5)

24 
$$f'_{(n)} = a(1-a)$$
 (6)

25 Where: a is the output of the activation function; n is the input value.

The network training was performed through the backpropagation algorithm with crossed validation. This algorithm was proposed by Rumelhart et al. (1986), and consists of a method of searching for the synaptic weights to minimize errors, using the so called Delta Rule 1 (Widrow and Hoff, 1960), Equation 7, which was formulated initially for one-layer neural
2 networks.

3 
$$W_{k+1} = W_k + (\tau e_k \delta_k P_k)$$
(7)

4 Where:  $W_k$  are the current synaptic weights;  $\tau$  is the learning rate;  $e_k$  are the errors of outputs 5 from the layers;  $\delta_k$  is the derivate of the activation functions; and  $P_k$  are the inputs into the 6 layer itself, in iteration k.

In order to apply this method to neural networks with more layers, Equation 8 is used to
estimate the errors in the hidden layers (h), which depend only on the errors and properties of
the subsequent layers (s):

$$10 e_h = \sum (W_s \, e_s \, \delta_s) (8)$$

11 Where:  $e_h$  is the error in the hidden layer;  $W_s$  are the synaptic weights in the subsequent layer; 12  $e_s$  are the errors in the subsequent layer, and  $\delta_s$  are the derivates of the activation function in 13 the subsequent layer.

The ANN hydrological model used was performed in the study of Oliveira et al. (2014), and resulted of the application of an algorithm for simplification of the neural network (Oliveira et al. (2015b). The reduction of input variables and neurons in the internal layer was performed using an algorithm that looks at the model performance after the imposition of small disturbances in the ANN input data.

19 The initial ANN model was composed of ten input variables, which included precipitation and 20 evapotranspiration values at times t and t-1, mean values of precipitation and evapotranspiration in the previous two months, water balance (difference between 21 22 precipitation and evapotranspiration) and transformed values by applying an exponential 23 decay filter. After the simplification process it was selected a monthly model for the study 24 area, which presents only three input variables, with four neurons in the hidden layer, 25 totalizing 16 synaptic weights. The inputs are: i) mean water balance at times t and t-l; ii) 26 weighted mean of the past values of precipitation by applying an exponential decay filter 27 (Hunter, 1986), according to Equation 9; iii) weighted mean of the past values of the water 28 balance by applying an exponential decay filter (Eq. 10).

29 
$$fP_t = (1 - \alpha) \cdot fP_{t-1} + \alpha \cdot P_t$$
 (9)

1 Where:  $fP_t$  and  $fP_{t-1}$  are the values transformed by applying the exponential decay filter to 2 precipitation, at times t and t-1, respectively;  $P_t$  is precipitation in time t;  $\alpha$  is a coefficient 3 that was calibrated by trials, in order to increase the linear correlation (r) between the filtered 4 variable and the observed flow. In the series used in this study a value equal to 0.52 was 5 obtained for this coefficient.

6 
$$fS_t = (1 - \beta) \cdot fS_{t-1} + \beta \cdot S_t$$
 (10)

7 Where:  $fS_t$  and  $fS_{t-1}$  are the values transformed by applying the exponential decay filter to 8 the water balance at times t and t-1, respectively;  $S_t$  is the water balance at time t;  $\beta$  is a 9 coefficient that was calibrated, similarly to Equation 9. In the series used in this study, a value 10 equal to 0.41 was obtained for this coefficient.

#### 11 **2.7** Stochastic modeling of monthly flows

12 According to Salas et al. (1980), if a variable cannot be predicted with certainty, it can be considered a random variable, ruled by the laws of probability. A model can be defined as 13 stochastic when at least one of the variables involved presents random behavior. According to 14 15 Salas et al. (1980) the climatic and hydrological variables can be considered random and thus 16 modeled stochastically. In the scientific literature there are numerous references involving the 17 development of stochastic models to generate synthetic climatic and hydrological series 18 (Gabriel and Neumann, 1962; Thomas and Fiering, 1962; Bailey, 1964; Richardson, 1981; Semenov and Barrow, 1997). 19

20 In this study a multiplicative type stochastic model was developed to generate monthly flow 21 series. A preliminary analysis of monthly hydrological series was performed to examine the 22 stationarity, seasonality and the temporal dependence. Based on this analysis, for this model, 23 it was adopted the assumption that flow may be estimated by the result of the product of four 24 components that must be estimated in the following sequence: i) component of long period 25 tendency (C1), that depends on the position in time, month (m) and year (y); ii) cyclic or 26 seasonal component (C2), that depends only on the month (m); iii) time dependence 27 component (C3); iv) random component (C4). In this model the first three components are modeled deterministically, while the random component (C4), being ruled by probability 28 29 laws, depends only of the adjustment to any probability distribution function.

1 The product of the four components (Eq. 11), during all the time intervals of modeling, results 2 in a stochastic sequence of monthly flows  $(Q_{m/y})$ .

3 
$$Q_{m/y} = C1_{m/y} * C2_m * C3 * C4$$
 (11)

In this way, initially, the stochastic modeling process to generate monthly flow series comprised an analysis to look at the stationarity of the observed or simulated series and isolate the tendencies of a long period (C1) This process is necessary to be able to isolate the other components (C2 to C4), both in the base period (1961-1990) and in the future period (2011-2040).

9 In order to isolate and remove the tendency observed in the series of monthly mean flows 10 during the base period (1961-1990) a linear tendency function was adjusted, represented by 11 Equation 12, that calculates flow based only on the time interval (axis x, in months, ranging from 1 to 360). Next, the flow calculated by the linear function (*Otend*) is divided by the 12 13 observed long period mean flow (LPMF), to obtain a correction factor that represents the first 14 component of the model, with a long period tendency (C1), according to Equation 13. Finally, to obtain a stationary flow (*Qst*), Equation 14 was applied, in which the observed flow (*Qobs*) 15 16 is divided by component C1.

17 
$$Qtend = 0.2459x + 98.633$$
 (12)

$$18 \quad C1 = \frac{Qtend}{LPMF} \tag{13}$$

$$19 \quad Qst = \frac{Qobs}{C1} \tag{14}$$

In the series of mean monthly flows simulated during the future period (2011-2040) a linear tendency function was also adjusted, represented by Equation 15 which calculates flow based only on the time interval (axis x, in months, ranging from 1 to 360), to remove the tendency found. Then Equations 13 and 14 were applied to obtain the stationary series of monthly flows in the future period (2011-2040).

25 
$$Qtend = 0.3105x + 143.38$$
 (15)

After defining the long period tendency (C1) for both series (base and future), the other components of the model were estimated based on the stationary series. The cyclic or seasonal component  $(C2_m)$  was calculated as the mean of flows in each month (Table 1), in the base (1961-1990) and future periods (2011-2040). Then the time dependency component was modeled (C3), which represents the influence of the stream values of the *p* months before the flow that occurs in the current time. At this stage, the correlation of flow in the current time (t) was analyzed in relation to the previous (t-1, t-2, ..., t-12), for each month, in the two stationary series (base and future) in which one can find, in general, a significant time dependency up to time t-3, characterizing a model of the third order.

7 In the multiplicative model, component C3 is a non-dimensional factor, with a mean equal to 8 1 along the hydrological series, obtained by the ratio between observed flow (stationary), in 9 month m, year y, and mean flow in month  $m(C2_m)$ , as shown by Equation 16. This equation 10 can only be used when one has observed data. In the case of stochastic modeling, it is 11 assumed that this non-dimensional factor depends only on the value of C3 in the p previous months, thus allowing modeling component C3 at some time interval. The behavior of this 12 component can be modeled by a multiple regression (Equation 17) or even by a more 13 14 complex structure, like an ANN with three input variables (Equation 18).

15 
$$C3 = \frac{Qst_{m/y}}{C2_m}$$
 (16)

16 
$$C3_t = f(C3_{t-1}, C3_{t-2}, C3_{t-3})$$
 (17)

17 
$$C3_t = RNA(C3_{t-1}, C3_{t-2}, C3_{t-3})$$
 (18)

In this study it was chosen to use a model based on ANNs applying the same algorithm detailed in the hydrological modeling stage (Equations from 3 to 8). But, in this case, it was used as input variables the values of C3 at times t-1, t-2 and t-3, and as the expected output the value of C3 at time t. After a few tests and analyses of the results, a neural network was chosen with three neurons in the hidden layer, totalizing 12 synaptic weights.

The random component (C4) is defined as the part that is not explained by the three other deterministic components, i.e., that represents the changes in hydrological behavior provoked by extreme events that occurred in the month. This part of the monthly flow is represented by the ratio between stationary flow (month, m, year *y*) and the product of the components C2 (month *m*) and C3, as shown by Equation 19. As in component C3, the values of C4 tend to a mean value close to 1.

29 
$$C4 = \frac{Qest_{m/y}}{C2_m * C3}$$
 (19)

Next, aiming at the generation of synthetic series, first of all it was checked whether 1 2 component C4 presented any pattern related to the deterministic portion of the model. Considering the stationary series of the base period (1961-1990), it was found that the value 3 of C4 presented two slightly distinct patterns: i) when the value of C3 is greater than 1, 4 5 resulting in flow values higher than the monthly mean in the deterministic parcel of the model (high flow periods), the tendency of the random component C4 is to present less dispersed 6 7 values, varying from 0.33 to 2.83, with a slightly lower mean (0.97); ii) when the value of C3 8 is lower than 1, resulting in flows lower than the monthly mean in the deterministic portion of 9 the model (low flow periods), the tendency of C4 is to present greater dispersion, varying from 0.21 to 7.24, with a slightly higher mean (1,03). 10

11 The most marked oscillations (inflections or impulses) in the monthly hydrogram which 12 depend on the random component C4, occur predominantly in dry periods, when the flow is 13 below the mean observed for the month. This pattern observed in the historical series explains 14 the smooth tendency found in the values of this component.

Also, considering the stationary series of the future period (2011-2040), when the value of C3 was higher than 1 (high flow periods), the random component C4 presented less dispersed values, ranging from 0.24 to 2.46, with a slightly lower mean (0.99). On the other hand, when the value of C3 was less than 1 (dry periods), component C4 oscillated between 0.13 and 4.98, with a mean of 1.06.

Once the probability curves observed in both periods (base and future) had been observed, a few statistical distributions were adjusted (Gamma, Log-Normal, Weibull, among others) to the values of the random component C4. After the Kolmogorov-Smirnov adherence test was performed, it was found that the Gamma probabilities distribution with three parameters presented the best adjustment to the component modeled. The Gamma distribution with three parameters ( $\vartheta$ ,  $\eta$ ,  $\beta$ ) is represented by the function given by Equation 20.

26 
$$f_X(x) = \frac{\zeta^{\eta - 1} e^{-\zeta}}{\vartheta \Gamma(\eta)}$$
, com  $\zeta = \frac{x - \beta}{\vartheta}$ , para  $x, \vartheta \in \eta > 0$  (20)

27 Where:  $\vartheta$  is a parameter of scale, with the dimension *x*;  $\beta$  is is a parameter of position, where 28  $\beta < x < \infty$ , representing the smallest value of *x*;  $\eta$  is a shape parameter;  $\Gamma(\eta)$  is the Gamma 29 function, normally solved by numerical integration.

30 After the adjustment of the four components, the stochastic series for both periods were 31 generated, referenced to the parameters calculated based on the two monthly flow series (observed between 1961 and 1990, and simulated between 2011 and 2040). One thousand
 series with an equal probability of occurrence were generated for each period.

The stochastic modeling process was evaluated comparing the series generated and the series simulated in the future period, from the following aspects: i) mean monthly flows; ii) long period mean flow and volume discharged; iii) standard deviation of monthly flows; iv) permanence curves.

The changes and uncertainties in water behavior were evaluated by comparing the stochastic series generated for the future period (2011-2040) and the series generated for the base period (1961-1990), considering central values and limits of confidence, looking at the following aspects: i) mean monthly flows; ii) standard deviation of monthly flows; iii) long period mean flow and volume discharged; iv) permanence curves.

12

#### 13 3 Results and discussions

This section will present the results and the discussions held concerning the analysis of stochastic modeling of monthly flows and the changes and uncertainties in water availability in the future period, between 2011 and 2040.

#### 17 3.1 Stochastic modeling of monthly flows

The stochastic series generated preserved several characteristics of the original series, 18 19 simulated for the period between 2011 and 2040. Considering the mean of the 1,000 series 20 generated for the future period, the long period mean flow (LPMF) was 200.3 m<sup>3</sup>/s, only 1.1 21  $m^{3}/s$  (0.5%) more than the simulated LPMF (original series). Figure 3 shows that this result 22 was also reflected in the accumulated curve of the volume discharged. The mean difference 23 between the simulated curve (original series) and the central tendency of the 1,000 curves generated (stochastic series) was only 4.8%. Furthermore it can be seen that the smooth 24 25 tendency of a long period was also preserved, and the values grew more markedly in the final half of the period. 26

- 27 Another characteristic maintained from the original series was the mean monthly flow. Table
- 28 2 shows that the mean absolute difference was only 0.52%, considering the mean of the 1,000
- 29 series generated for the period between 2011 and 2040. The greatest absolute difference
- 30 between mean flows occurred in October, with an overestimation of  $1.6 \text{ m}^3/\text{s}$ .

Table 2 also shows that the monthly standard deviation was reasonably preserved, with a mean percentage absolute difference of 13.9% between the original series and the central tendency of the 1,000 series generated. The smallest difference was found in the month of October, and the greatest difference as to the monthly standard deviation was found in the month of May.

6 Figure 4 illustrates the permanence curves of the mean monthly flow in the future period 7 (2011-2040), in which the similarity between the original series and the central tendency 8 observed in the stochastic series generated becomes clear. The greatest differences were 9 observed in the extremely high flows, with a permanence of less than 2%. In the rest of the 10 permanence intervals the original curve was always located at the 90% confidence interval 11 defined by the red lines on the graph.

#### 12 **3.2** Changes and uncertainties in water availability

In Ijuí River Basin, according to the climate scenario used, the annual accumulated rainfall
will increase 12.3% between 2011 and 2040. This growth in volume of rainfall is mainly due
to an increasing trend in rainfall between the months of January and June.

16 On the other hand, the the evapotranspiration will reduce by 5.4%, based on the annual 17 average. According to the climate scenario used, this reduction in the evapotranspiration 18 should be observed in almost every months, even though the average temperature is higher in 19 the period 2011-2040. The reason for this reduction is associated with the increase of relative 20 humidity and the decrease of solar radiation, probably associated with the increase of 21 cloudiness. This statement was confirmed by analyzing the changes related to the five 22 climatic variables used to calculate evapotranspiration.

23 The first aspect analyzed as to changes and uncertainties regarding water availability in the 24 future refers to the long period mean flow (LPMF) and to the volume discharged over the 25 period of 30 years. On average, considering the stochastic series in the base period (1961-26 1990) the LPMF was 141.6 m<sup>3</sup>/s. The confidence interval of LPMF in the period, with a 27 significance level of 0.1, was between 123.7 and 162.3 m<sup>3</sup>/s (range 38.6 m<sup>3</sup>/s). On the other 28 hand, in the future period (2011-2040), the projected LPMF was 200.3 m<sup>3</sup>/s, considering the 29 mean value found in the series. This value represents a mean increase of 41.4% in the LPMF. The confidence interval of LPMF in the future, considering the same level of significance, 30

will be between 165.1 and 233.6 m<sup>3</sup>/s. Thus, the range of the interval will increase from 38.6
m<sup>3</sup>/s to 68.6 m<sup>3</sup>/s.

The change of LPMF according to the projection for the future is also reflected by the mean of the total volume discharged over a 30-year period. Figure 5 shows that, between the years of 1961 and 1990, the mean of the total volume discharged was 132,566 Hm<sup>3</sup>. On the other hand, in the future period (2011-2040) the mean total volume discharged was 185,869 Hm<sup>3</sup>.

Considering the stochastic series in the future period, at a 0.1 level of significance, Figure 5
shows that the total volume discharged at the end of 30 years is at the interval between
154,014 Hm<sup>3</sup> and 218,002 Hm<sup>3</sup>. This interval is broader and presents values much superior to
those observed in the base period.

The second aspect analyzed refers to mean monthly flows. Figure 6 presents the mean and the 90% confidence interval for the mean monthly flows, considering the 1,000 stochastic series generated during the base and future periods.

14 The mean monthly flow will increase between the months of January and October, during the 15 period between 2011 and 2040, compared to the base period, with percentages that vary from 16 15% (August) to 118% (March). Besides the month of March, at least four other months will 17 present a significant increase of mean flow: i) February (113%); ii) May (110%); iii) April 18 (101%); iv) June (74%). Considering a simple difference between the values obtained in the 19 two periods, the months of May and June presented the greatest changes, with an increased 20 mean monthly flow of 130 m<sup>3</sup>/s and 118 m<sup>3</sup>/s, respectively. The reduction in mean monthly 21 flow with percentages of 24% and 21%, respectively, will only occur in the months of 22 November and December.

Considering a statistical analysis of the 1,000 stochastic series generated for the two periods analyzed (base and future), at a 0.1 level of significance the confidence interval can be estimated which comprises the mean flow of each month. The greater the range of this interval, the greater also the uncertainty related to the mean monthly flow.

Figure 6 shows that the range of the 90% confidence interval for the mean monthly flows will only be reduced in the months of November and December, thus following the tendency observed in the mean monthly values. In November, for instance, the range of mean flow in the base period considering the series generated was 75 m<sup>3</sup>/s. On the other hand, in the future period, the range of mean flows in this month was 64 m<sup>3</sup>/s (reduction of 16% in the interval). In December, the range of the 90% confidence interval for mean flow was reduced by 14%,
 considering the two periods.

In all other months, the range of the confidence interval increased in the future, particularly between the months of February and June, with a greater percentage than 100%, indicating greater variability between the stochastic series generated and, consequently, greater uncertainties in estimating mean flow. The month of May presented the greatest change in this sense. The mean flow during the base period, considering a 90% confidence interval was between 95 and 145 m<sup>3</sup>/s (range of 50 m<sup>3</sup>/s). On the other hand, in the future period, the mean flow in May is inserted into the interval between 187 and 314 m<sup>3</sup>/s (range of 127 m<sup>3</sup>/s).

All the results of mean monthly flows presented indicate a significant change in the hydrological behavior of the Ijuí river basin, considering the climatic projection of the Eta model, between the months of February and June. Between the months of February and June, the confidence intervals do not present any overlap, i.e., the upper limit of the interval found in the base period is smaller than the lower limit of the interval found in the future.

The third aspect analyzed in the hydrological comparison between the base (1961-1990) and future period (2011-2040) was the standard deviation of mean monthly flows. As in the case of the averages of the flows in each month, considering the central tendency of the 1,000 series generated in the two periods, Table 3 illustrates that the standard deviation should increase between the months of January and October.

The period of the year between the months of February and July is that one where the greatest change occurs in the dispersion of the flow values. Table 3 shows that in May, for instance, the standard deviation increases 155% for the future. On the other hand, the month of November presents a smooth tendency to reduction in the flows with a 121 m<sup>3</sup>/s reduction to  $104 \text{ m}^3/\text{s}$  (-14%).

When dividing the monthly standard deviation by the mean monthly flow, the coefficients of variation (CV) were obtained for both series, for each month. It can be seen that during the base period (1961-1990), the CV oscillated between 0.7 (February) and 0.72 (November), while in the future period (2011-2040), the same index varied between 0.8 (April) and 0.85 (May). These results indicate a real increase of the monthly variability of flows, with greater fluctuations of monthly flows in the future. Another aspect analyzed as to changes in hydrological behavior in the future refers to permanence curves of mean monthly flows. Figures 7 and 8 respectively illustrate the mean value and confidence interval of 90% for the permanence curves of monthly flow, considering the 1,000 stochastic series generated in the base (1961-1990) and future periods (2011-2040).

5 Flows with probability of exceedance equal to or over than 90% (Q90, Q95, Q99) are 6 important in hydrology for sizing public water supply projects, since they indicates the 7 volume of water that can be guaranteed with the corresponding proportion. As for the flows 8 with probability of exceedance of the order of 50% (Q50), they are important to estimate the 9 maximum possible flow to be regularized. Flow rates with probability of exceedance equal to 10 or less than 10% (Q10, Q5) are used in studies related to extreme flood events.

The flow will be reduced in the future period only at permanence intervals greater than 91%, i.e., in the portion of lower flows which characterize dry periods. For flows with a permanence equal to or less than Q90 (intermediate and high flow), the tendency is toward increase in the flow values (Fig. 7). As to the range of the 90% confidence interval for the permanence curve of monthly flows (Fig. 8), the tendency is to increase in the future period, even the lower flows portion. This result illustrates an increase in the uncertainties associated with the estimate of the permanence curve in the future.

On average, considering all the series generated during the base and future periods, flow with a probability of exceedance equal to or less than 99% of the months (Q99) was 18 m<sup>3</sup>/s and 15 m<sup>3</sup>/s, respectively. This indicates a mean reduction of 16% in Q99 for the future period. Considering a statistical analysis of the stochastic series, at a 0.1 level of significance, we can say that Q99, during the period 1961-1990, is located at the interval between 13.7 and 22.3 m<sup>3</sup>/s (range of 8.6 m<sup>3</sup>/s). On the other hand in the future period this interval changes to values between 10.4 and 20.4 m<sup>3</sup>/s (range of 9.9 m<sup>3</sup>/s).

On average, considering all the series generated during the base (1961-1990) and future periods (2011-2040), flow with a probability of exceedance equal to or less than 95% of the months (Q95) was 30 and 28.5 m<sup>3</sup>/s, respectively. This indicates a mean reduction of 5% in Q95 for the future period. This percentage is lower than that observed in Q99, illustrating the tendency to inversion in the permanence curves for larger flows. As for the confidence interval of 90% of Q95, during the base period, the range was 10.4 m<sup>3</sup>/s. On the other hand in the future period the range was 13.4 m<sup>3</sup>/s. In the base and future periods, the mean flow with a probability of exceedance equal to or less than 90% of the months (Q90) was 39.6 and 40.5 m<sup>3</sup>/s, respectively. This shows a mean increase of 2% in Q90 for the future period. As to the confidence interval of 90% of Q90 in the base period, the range was 12.4 m<sup>3</sup>/s. On the other hand in the future period the range was 18.4 m<sup>3</sup>/s.

6 In the base period (1961-1990), on average, the flow with a probability of exceedance equal to 7 or less than 50% of the months (O50) was 108 m<sup>3</sup>/s. At a 0.1 significance level, it can be said 8 that Q50 in this period shows a range of 28 m<sup>3</sup>/s. On the other hand in the future period, on 9 average, the O50 was much higher, with a values of 145 m<sup>3</sup>/s, indicating a mean increase of 10 34% for the future. As to the confidence interval, it can be said that the Q50, in future period, 11 will be between 116 and 178 m<sup>3</sup>/s (range of 62 m<sup>3</sup>/s). Thus, the differences between the 12 confidence intervals of Q50 in the two periods indicate a significant increase in the uncertainties associated with the permanence of flows in future. These results also illustrate a 13 14 tendency to an increase in the differences between the flows of the base period and the future 15 period inversely proportional to permanence.

In the portion of flows with permanence between 5% (Q5) and 30% (Q30), the confidence intervals (0.1 significance) of the two periods do not overlap, i.e., the upper limit of the interval during the base period is smaller than the lower limit of the interval in a future period. In the other portions of flows, even if significant differences have been found between the confidence intervals estimated in the base and future periods, they present an overlapping area.

22 Finally, the changes in the permanence curves of the mean monthly flows in the future, 23 individually, for each month were analyzed. Comparing the permanence curves in the two 24 periods - base (1961-1990) and future (2011-2040) - it can be seen that the smallest changes 25 observed occurred between August and January. In December the absolute mean difference 26 between the permanence curves was 26 m<sup>3</sup>/s, and this is the lowest value observed. On the 27 other hand, in May more drastic changes were observed, with a mean absolute difference of 28 130 m<sup>3</sup>/s between the permanence curves. Other months that call attention due to the great 29 change in the permanence curves for the future are June (118 m<sup>3</sup>/s), April (99 m<sup>3</sup>/s), February 30  $(94 \text{ m}^3\text{/s})$ , March  $(84 \text{ m}^3\text{/s})$  and July  $(79 \text{ m}^3\text{/s})$ .

Figures 9, 10 and 11 illustrate the mean behavior and the 90% confidence interval for the permanence curves of the monthly flows from January to December, for both periods (base and future). In general, it can be said, based on the results obtained, that between the months of January and October there is tendency for the value of the flows with low permanence to increase – the high flow portion. Regarding this aspect, the main outstanding month is May, in which the mean flow with permanence equal to or less than 10% (Q10) was 240 m<sup>3</sup>/s (base period) for 573 m<sup>3</sup>/s (future), which means an increase of 333 m<sup>3</sup>/s (138%) in Q10. Next, the other months between February and July are also outstanding, with an increase in the value of Q10 close to or higher than 200 m<sup>3</sup>/s, as shown in Table 4.

8 Table 4 shows that between the months of February and June, also the flows with a high 9 permanence (portion of the lower flows) presented higher values in the future period 10 compared to the base period, indicating a tendency to a more generalized increase in the flows 11 for these months. In this case, in percentage terms, the month of March is outstanding, in 12 which the mean flow with a probability of exceedance equal to or inferior to 90% (Q90) was

13 22.8 m<sup>3</sup>/s (base period) to 34.5 m<sup>3</sup>/s (future), representing an increase of 52% in Q90.

On the other hand, in the months of January, July, August, September and October, even lower flow values were observed, with high permanence (low flows), indicating that in those months there was a tendency to amplify the extreme values: dry periods and more intense floods in the future period (2011-2040) than those observed in the base period (1961-1990). In January, for instance, the results indicate a mean increase of 47% in Q10 and an 8% reduction in Q90 (Table 4).

In the months of November and December, the tendency observed is for a reduction in the flow values in general, both in the high flow portion and in the low flow portion. As shown in Table 4, in November the results indicate a mean reduction of 18% (-61 m<sup>3</sup>/s) in Q10 and 48% (-26 m<sup>3</sup>/s) in Q90. On the other hand, in December for the future period, the mean reduction in Q10 and Q90 was 14% (-35 m<sup>3</sup>/s) and 46% (-18 m<sup>3</sup>/s), respectively.

Tables 5 and 6 show the limits and ranges of the confidence interval of flows during the base and future periods, with a permanence of 10% (Q10) and 90% (Q90), respectively. In the case of the ranges of the confidence interval, each month, considering the 1,000 stochastic series generated in both periods, there is a clear significant increase in the uncertainties related to the estimate of Q10 and Q90 between the months of January and October. The interval will only be reduced in the months of November and December.

In May, for instance, considering a level of significance of 0.1, the Q10 in the period between 2011 and 2040 is between 388.9 and 804 m<sup>3</sup>/s, while during the period between 1961 and 1990 the limits were 172.2 m<sup>3</sup>/s and 335.8 m<sup>3</sup>/s. This considerable change in hydrological
 behavior can be observed also in the other months, especially between February and July,
 with growth rates greater than 100% in range of the 90% confidence interval for Q10.

In general, the uncertainties regarding the hydrological behavior in the future (2011-2040), taking a single climate scenario as reference, were greater than during the base period (1961-1990). This increase was reflected mainly between the months of January and October, as shown by the results of the comparative analysis between the permanence curves and the mean month flows and their confidence intervals.

9 The primary source of uncertainties is in the original hydrological series itself, used in the 10 stochastic modeling process. By formulating the stochastic model it is expected that the 11 greater the mean monthly flow (seasonal component, C2), the greater also will be the 12 possibility of obtaining extremely high flows. This occurs because the seasonal component is 13 multiplied by the random component (C4) and the time dependence on (C3) which, although 14 they have mean volumes close to 1, may possibly present extreme values.

This becomes clear when compared to the mean monthly flow of each month (in the input series to the stochastic model), with the range of the confidence interval for the mean monthly flow obtained after modeling, as shown in Figure 12. There is a visible linear tendency to increased uncertainty as the mean monthly flow increases. However, a clear difference is also observed between the two straight lines that characterize the base and future periods. For a same mean monthly flow, the confidence interval range is higher in the future period series.

After a sensitivity analysis in the models based on ANN to determine the C3 component (time dependency) in both periods, it was found that in the future (2011-2040) the flow in time t is more sensitive because of the antecedent flows (*t*-1, *t*-2 and *t*-3). To illustrate this result, Figure 13 shows the variation of the value of C3(*t*) because of the value of C3(*t*-1), and the variables C3(*t*-2) and C3(*t*-3) are maintained equal to 1, for both periods.

In the base period, between 1961 and 1990, even when an extremely low flow occurs in the previous month resulting in a value close to 0 for variable C3(t-1), the calculated value of C3(t) is almost never less than 0.5, i.e., half the mean monthly flow. On the other hand, in the period between 2011 and 2040, due to the time sequence of the series and its characteristics, the value of C3(t) can be less than 0.3, giving rise to a value well below the mean of that

31 month (Fig. 13).

The same pattern was observed for the portion of the extremely high flows. During the period between 1961 and 1990, when there is a high flow in the previous months, resulting in a value of C3(t-1), for instance, five times higher than the monthly mean, the calculated value of C3(t) does not reach 2.2. In turn, during the future period, the value of C3(t) may be higher than 3.1, giving rise to a flow that is much higher than the monthly mean (Fig. 13).

In this way the component C3 presents greater fluctuations in the series between 2011 and
2040. This result helps explain the greater variability found between the stochastic series of
the future period in relation to the base period.

9 Figures 14 and 15 illustrate the adjustment in the distribution of Gamma probabilities for
10 modeling the random component (C4) in situations of low flow (C3, in time *t*, less than 1) and
11 high flow (C3, in time *t*, higher than 1), respectively.

In general, it is possible to observe that the behavior found in the base series (1961-1990) of the random component C4 was maintained in the series of the future period (2011-2040), especially as regards the months in which the value of C3 was less than 1 (Fig. 14), resulting in a flow lower than the monthly mean. In this case, considering the base and future periods, the chance of the value drawn for C4 being greater than 1 was 42% and 43.5%, respectively.

17 On the other hand, in the months when the time dependence component (C3) surpassed the 18 value of 1, the difference between the series was slightly higher (Fig. 15). The probability of a 19 value higher than 1 being drawn for component C4 was 38% and 43%, respectively, for the 20 base and future series.

21 These results indicate that the sensitivity of the model to the variable of time dependence also 22 contributed to increasing the uncertainties in the future period, between 2011 and 2040. In the 23 original series simulated for the future period, besides the mean and the dispersion of the data 24 being greater than in the base period, provoking more abrupt fluctuations in the flow values, the correlation coefficient between the flows at times t and t-1 was quite high (mean 0.66), 25 26 with values between 0.5 (November) and 0.78 (March). Already in the period between 1961 27 and 1990, the correlation coefficient values were between 0.26 (June) and 0.75 (February) 28 with a mean equal to 0.54.

29 Considering the methodology adopted to model flows in future and to generate the stochastic 30 series, it can therefore be said that there is a certain tendency to an increased hydrological 31 variability during the period between 2011-2040, with a greater dispersion of values in relation to the monthly mean. This finding implies greater uncertainty regarding the
 availability of water in the future, with the possible occurrence of time series that are very
 different from each other.

4

#### 5 4 Conclusions

6 This study analyzed the possible changes and uncertainties related to water availability in the 7 future using a stochastic approach, based on the climate change scenario originating in the 8 LOW member of Eta CPTEC/HadCM3 climate model, for the period between 2011 and 2040. 9 The study was applied to the Ijuí river basin, in the south of Brazil. The methodology 10 involved the correction of the climate variables projected for the future, the hydrological 11 simulation to define a series of monthly flows, and stochastic modeling to generate 1,000 12 hydrological series with an equal probability of occurrence.

13 As to the stochastic model to generate monthly flow series, several characteristics of the 14 original series were preserved, simulated for the period between 2011 and 2040. Outstanding 15 among them are LPMF and the mean monthly flows, both with differences of only 0.5%. The 16 monthly standard deviation was reasonably preserved, with a mean percentage absolute 17 difference of 13.9% between the original series and the central tendency of the 1,000 series 18 generated. The similarities between the permanence curves of the monthly flows of the 19 original series and the central tendency observed in the stochastic series generated also 20 became clear. Based on all these results it can be concluded that the stochastic model proposed is adequate to generate monthly flow series. 21

22 Various results showed a tendency to increased flows in a general context. The LPMF, for 23 instance, presented an alteration from 141.6 m<sup>3</sup>/s (1961-1990) to 200.3 m<sup>3</sup>/s (2011-2040), 24 which is a mean increase of 41.4% in LPMF. Comparing the two periods, the difference 25 between the total volume discharged in 30 years was 53,303 Hm<sup>3</sup>. It could also be seen that 26 the mean flow and the monthly standard deviation increased between the months of January 27 and October, in the period between 2011 and 2040. Between the months of February and June 28 the percentage increase of the mean monthly flow was greater than 100%. It was also found 29 that, in the base period (1961-1990), the coefficient of variation (CV) oscillated between 30 0.698 (February) and 0.716 (November), while for the future period (2011-2040), the same 31 index varied from 0.801 (April) to 0.848 (May), indicating a real increment in the monthly 32 variability of flows, with greater fluctuations in future flows.

Based on the comparison of the permanence curves of monthly flow between the base and future periods, it is concluded that the flow presented lower values (-5.1%) only at permanence intervals greater than 90% in the dry months. For flows with a permanence equal to or less than Q90 (intermediate and high flow), there is a tendency for the flow values to increase. For instance, in the base period (1961-1990), on average, the flow with permanence equal to or less than 50% of the months (Q50) was 108.2 m<sup>3</sup>/s. On the other hand, in the future period Q50 was much higher, with a value of 145.1 m<sup>3</sup>/s (34.2% increase).

8 It can also be observed that the smaller changes in flow permanence occurred between the 9 months of August and January. On the other hand, in the other months, the changes were 10 drastic. In May, for instance, an absolute mean difference was found of 130.5 m<sup>3</sup>/s between 11 the permanence curves.

12 In general it is concluded, based on the results obtained that between the months of January 13 and October there is tendency for the flood flows to increase. Between the months of 14 February and June, also the flows with high permanence (minimum flows) presented higher 15 values in the future compared to the base period. On the other hand, in the months of January, 16 July, August, September and October, even lower minimum flow values were observed, 17 indicating that in these months there is tendency to amplify the extreme values. Finally, in the 18 months of November and December the tendency observed is for the reduction of flow 19 values, in general, both in the high flow and in the low flow portions.

As to the uncertainties concerning the hydrological behavior and, consequently, water availability for the future, having as reference the results and discussions presented, it is concluded that uncertainties regarding hydrological behavior between 2011 and 2040 were greater than in the base period. The main factor that contributed to this result was the increase of the mean itself and of the standard deviation of monthly flows. Besides these, in the future, time dependency will present a more marked contribution to the composition of monthly flow, making the model more sensitive to abrupt variation in flow the previous month.

In this way, considering the stochastic series generated for the future, it can be said that there is a certain tendency for the increase hydrological variability during the period between 2011-2040. This finding means greater uncertainty regarding water availability in the future, with the possibility that time series may occur with marked differences as to the occurrence of drought and flood periods.

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- 9

1	Table 1. Cyclic (sease	onal) component in t	he base and future	periods: mean i	nonthly flow in
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Moreth	Mean Monthly flow (m <sup>3</sup> /s)				
Month	Base (1961-1990)	Future (2011-2040)			
January	99.7	126.4			
February	85.8	181.1			
March	74.7	159.7			
April	95.8	200.5			
May	114.9	249.1			
June	160.0	280.3			
July	180.8	263.6			
August	199.0	212.8			
September	228.5	264.1			
October	187.2	228.4			
November	166.7	128.2			
December	126.3	95.8			

the Ijuí river basin, Santo Ângelo station. 

- 1 Table 2. Difference between the original series (simulated) and the 1,000 stochastic series
- 2 generated mean flow and monthly standard deviation in the period between 2011 and 2040,
- 3 Santo Ângelo gauge station.

	Mea	n Monthly Flow (m	1 <sup>3</sup> /s)	Monthly Standard Deviation (m <sup>3</sup> /s)			
Month	Original Series	Mean of Series Generated	Percentage Difference	Original Series	Mean of Series Generated	Percentage Difference	
January	128.7	127.6	-0.83%	118.0	102.5	-13.13%	
February	178.5	177.2	-0.74%	179.3	142.2	-20.67%	
March	155.4	154.5	-0.62%	143.7	124.0	-13.73%	
April	199.2	198.2	-0.48%	163.2	158.8	-2.66%	
May	249.6	249.2	-0.16%	168.0	211.2	25.70%	
June	276.5	276.5	0.01%	195.0	224.0	14.87%	
July	262.5	262.9	0.16%	183.5	213.4	16.29%	
August	217.7	218.3	0.27%	149.4	175.5	17.44%	
September	268.1	269.5	0.52%	205.2	216.6	5.54%	
October	228.9	230.6	0.72%	183.9	188.0	2.24%	
November	126.9	128.0	0.86%	83.8	104.2	24.43%	
December	97.0	97.9	0.93%	87.8	78.9	-10.11%	

- . .

- 1 Table 3. Mean of standard deviation of monthly flows during the base and future periods –
- 2 Santo Ângelo gauge station.

Month	Mean Monthly Standard Deviation (m³/s)					
	Base (1961-1990)	Future (2011-2040)				
January	67.7	102.5				
February	58.0	142.2				
March	49.9	124.0				
April	69.2	158.8				
May	82.9	211.2				
June	112.1	224.0				
July	131.9	213.4				
August	134.9	175.5				
September	161.0	216.6				
October	131.0	188.0				
November	121.0	104.2				
December	88.6	78.9				

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1 Table 4. Mean flows  $(m^3/s)$  with 10% (Q10) and 90% (Q90) permanence during the base

Month	Base (1961-1990)		Future (2011-2040)		Changes	Changes
wonth	Q10	Q90	Q10	Q90	Q10	Q90
January	195.2	30.5	286.0	28.1	90.8	-2.4
February	168.6	26.8	393.5	39.2	224.9	12.4
March	145.3	22.8	343.0	34.5	197.7	11.7
April	201.4	31.7	438.1	44.4	236.7	12.7
May	240.4	38.7	573.0	50.2	332.6	11.5
June	324.0	51.5	610.5	61.6	286.5	10.1
July	376.5	58.9	582.9	58.2	206.4	-0.7
August	390.2	61.0	486.4	48.5	96.3	-12.5
September	463.6	72.6	594.5	60.9	130.9	-11.7
October	376.4	59.4	517.2	50.6	140.8	-8.8
November	345.7	54.1	284.6	28.1	-61.1	-26.0
December	252.2	39.8	217.1	21.6	-35.1	-18.2

2 period (1961-1990) and future period (2011-2040) – Santo Ângelo gauge station.

- Table 5. Limits and ranges of the confidence interval of flows  $(m^{3}/s)$  with a permanence of
- 10% (Q10) during the base period (1961-1990) and future period (2011-2040), Santo Ângelo
- gauge station.

	Base (1961-1990)			Future (2011-2040)		
Month	Lower limit	Upper limit	Range	Lower limit	Upper limit	Range
January	136.3	275.3	139.0	196.3	395.4	199.0
February	117.6	232.6	115.0	272.3	548.0	275.7
March	102.5	201.3	98.9	236.7	474.6	238.0
April	141.3	285.0	143.8	305.4	620.1	314.7
May	172.2	335.8	163.6	388.9	804.0	415.1
June	229.6	451.4	221.8	420.1	855.8	435.7
July	261.9	526.2	264.3	403.8	790.2	386.4
August	274.1	541.5	267.4	333.8	679.1	345.3
September	325.1	648.0	323.0	400.7	828.7	428.0
October	269.3	515.8	246.5	352.6	723.1	370.5
November	245.4	485.3	239.9	197.1	400.5	203.3
December	174.3	352.6	178.3	151.6	297.3	145.7

- Table 6. Limits and ranges of the confidence interval of flows (m<sup>3</sup>/s) with 90% permanence
- (Q90) in the base period (1961-1990) and future period (2011-2040), Santo Ângelo gauge
- station.

	Base (1961-1990)			Future (2011-2040)		
Month	Lower limit	Upper limit	Range	Lower limit	Upper limit	Range
January	20.4	42.9	22.5	16.6	43.3	26.7
February	17.9	38.0	20.1	23.8	60.0	36.2
March	15.2	32.0	16.8	20.5	53.6	33.0
April	21.5	43.7	22.2	26.1	68.9	42.8
May	25.9	53.3	27.4	29.8	80.1	50.3
June	34.6	71.3	36.7	35.7	95.3	59.6
July	38.9	83.4	44.4	34.4	87.7	53.3
August	41.4	85.3	43.9	28.7	72.9	44.2
September	48.9	100.1	51.2	37.0	93.9	56.9
October	39.6	82.5	42.8	30.9	77.4	46.6
November	35.8	76.3	40.4	16.6	43.6	27.1
December	26.5	55.9	29.4	12.9	32.5	19.6



Figure 1. Location of the Ijuí river basin, section upstream from the Santo Ângelo gauging station (5,414 km<sup>2</sup>), RS, Brazil.



Figure 2. Location of the stations with hydrologic and climate data used in a radius of coverage of 100 km in relation to the Ijuí river basin.





Figure 3. Curves of volume discharged in the future (2011-2040) – difference between the
original series (simulated) and the 1,000 stochastic series generated – Ijuí river basin, Santo
Ângelo gauge station.

- 1 7





Figure 4. Permanence curves for the mean monthly flow between 2011 and 2040 - difference
between the original series (simulated) and the 1,000 stochastic series generated- Santo
Ângelo gauge station.



Figure 5. Mean and 90% confidence interval for the volume discharged, based on the
stochastic series generated, base period (1961-1990) and future period (2011-2040), Santo
Ângelo gauge station.



Figure 6. Mean and 90% confidence interval for the mean monthly flows based on the
stochastic series generated, during the base (1961-1990) and future periods (2011-2040), at
Santo Ângelo gauge station.

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Figure 7. Mean value of permanence curves of monthly flow according to the stochastic series
generated during the base (1961-1990) and future periods (2011-2040), at Santo Angelo
gauge station.

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Figure 9. Mean and 90% confidence interval for the monthly flow permanence curves at Santo Ângelo gauge station, according to the stochastic series generated in the base period (1961-1990) and future period (2011-2040): January, February, March and April.



Figure 10. Mean and 90% confidence interval for the monthly flow permanence curves at Santo Ângelo gauge station, according to the stochastic series generated in the base period (1961-1990) and future period (2011-2040): May, June, July and August.



Figure 11. Mean and 90% confidence interval for the monthly flow permanence curves at Santo Ângelo gauge station, according to the stochastic series generated in the base period (1961-1990) and future period (2011-2040): September, October, November and December.



Figure 12. Relationship between the mean of the monthly flow and the range of the
confidence interval for the periods between 1961 and 1990 (base) and between 2011 and 2040
(future).

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Figure 13. Variation of the value of C3(t), based on the value of C3(t-1), in modeling the time
dependence component using ANN, keeping the other variables, C3(t-2) and C3(t-3), equal to
1 for the periods between 1961 and 1990 (base) and between 2011 and 2040 (future).

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3 Figure 15. Adjustment of the Gamma distribution for the modeling of the random component

4 in a high flow situation, C3(t) higher than 1, for the base and future periods.