



The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

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The cost of ending groundwater overdraft on the North China Plain

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Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



Abstract

Over-exploitation of groundwater reserves is a major environmental problem around the world. In many river basins, groundwater and surface water are used conjunctively and joint optimization strategies are required. A hydroeconomic modelling approach is used to find cost-optimal sustainable surface water and groundwater allocation strategies for a river basin, given an arbitrary initial groundwater level in the aquifer. A simplified management problem with conjunctive use of scarce surface water and groundwater under inflow and recharge uncertainty is presented. Because of head-dependent groundwater pumping costs the optimization problem is non-linear and non-convex, and a genetic algorithm is used to solve the 1-step-ahead sub-problems with the objective of minimizing the sum of immediate and expected future costs. A real-world application in the Ziya River Basin in northern China is used to demonstrate the model capabilities. Persistent overdraft from the groundwater aquifers on the North China Plain has caused declining groundwater tables, salinization and infiltration of wastewater. The model maps the opportunity cost of water in different scenarios, and the cost of ending groundwater overdraft in the basin is estimated to be 5.47 billion CNY yr⁻¹. The model can be used to guide decision makers to ensure long-term sustainability of groundwater and surface water resources management in the basin in an economically optimal way.

1 Introduction

Groundwater aquifers are of high economic importance around the world and often act as buffers in the water supply system during droughts (Tsur and Graham-Tomasi, 1991; Tsur, 1990). On the North China Plain, persistent groundwater overexploitation over the past decades has caused decline of the shallow and deep groundwater tables (Liu et al., 2001). The immediate benefits of satisfying the water demands greatly exceed the costs of pumping, which highlights the problem of the present self-regulating

HESSD

12, 5931–5966, 2015

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

[Title Page](#)

[Abstract](#)

[Introduction](#)

[Conclusions](#)

[References](#)

[Tables](#)

[Figures](#)

[⏪](#)

[⏩](#)

[◀](#)

[▶](#)

[Back](#)

[Close](#)

[Full Screen / Esc](#)

[Printer-friendly Version](#)

[Interactive Discussion](#)



The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

[Title Page](#)[Abstract](#)[Introduction](#)[Conclusions](#)[References](#)[Tables](#)[Figures](#)[⏪](#)[⏩](#)[◀](#)[▶](#)[Back](#)[Close](#)[Full Screen / Esc](#)[Printer-friendly Version](#)[Interactive Discussion](#)

25 million people (data from 2007; Bright et al., 2008). A hydroeconomic study of the Ziya River by Davidsen et al. (2015) focused primarily on optimal management of the surface water resources. Five major reservoirs with a combined storage capacity of 3.5 km^3 are located in the basin. In this study, it is assumed that the full storage capacity can be managed flexibly without consideration of storage reserved for flood protection or existing management rules.

In Davidsen et al. (2015), the groundwater resource was included as a simple monthly upper allocation constraint, which prevents analysis of dynamic interactions between the groundwater and surface water resources and limits the decision space. In the present model setup, the groundwater is included as a simple dynamic aquifer box model with an upper storage capacity of 275 km^3 . This allows for more flexible management with larger abstractions in dry years and increased recharge in wet years. The groundwater aquifer can thereby be used to bridge longer drought periods.

A conceptual sketch of the management problem is shown in Fig. 2. The water users are divided into groups of economic activities; irrigation agriculture, industrial and domestic water users. Each water user group is characterized by constant water demands (m^3) and constant curtailment costs of not meeting the demand (see Table 1), as also applied by Davidsen et al. (2015). The water demands are assumed to be deterministic and decoupled from the stochastic runoff. This is a reasonable assumption because the rainfall on the NCP normally occurs in the summer months, while irrigation water demands are concentrated in the dry spring. The irrigation schedule is centrally planned and typically the same every year. The water users upstream the surface water reservoir (u) have access to the runoff and a monthly limited volume of groundwater, X_{gw} . The water users located downstream the reservoir (d) have access to reservoir releases, water delivered through the South-to-North Water Transfer Project (SNWTP) and groundwater from the dynamic aquifer.

2.2 Optimization model formulation

An SDP formulation is used to find the expected value of storing an incremental amount of surface water or groundwater, given the month of the year, the available storage in surface and groundwater reservoirs and the inflow scenarios. The backward recursive equation calculates the sum of immediate and expected future costs for all combinations of discrete reservoir storage levels (states) and monthly time steps (stages). The immediate management costs (IC) arise from water supply and water curtailment, whereas the expected future costs (EFC) are the optimal value function in $t + 1$ weighed by the corresponding transition probabilities. In the present setup, we decided to weighted the IC and EFC equally, but inclusion of discount rates other than zero is possible. Because of the head and rate dependent groundwater pumping costs, which will be described in detail later, the immediate cost depends non-linearly on the decision variables. The objective is to minimize the total costs over the planning period, given by the optimal value function $F_t^* (V_{gw,t}, V_{sw,t}, Q_{sw,t}^k)$ based on the classical Bellman formulation:

$$F_t^* (V_{gw,t}, V_{sw,t}, Q_{sw,t}^k) = \min \left(IC (V_{gw,t}, V_{sw,t}, Q_{sw,t}^k) + \sum_{l=1}^L (p_{kl} F_{t+1}^* (V_{gw,t+1}, V_{sw,t+1}, Q_{sw,t+1}^l)) \right) \quad (1)$$

with IC being the immediate costs:

$$IC (V_{gw,t}, V_{sw,t}, Q_{sw,t}^k) = \sum_{m=1}^M (c_{sw} X_{sw} + c_{gw} X_{gw} + c_{SNWTP} X_{SNWTP} + c_{ct} X_{ct})_{m,t} - r_{sw,t} b_{hp} \quad (2)$$

subject to:

$$X_{sw,t,m} + X_{gw,t,m} + X_{SNWTP,t,m} + X_{ct,t,m} = dm_{m,t} \quad (3)$$

HESSD

12, 5931–5966, 2015

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



$$V_{sw,t} + Q_{sw,t} - \sum_{u=1}^U x_{sw,u,t} - r_{sw,t} - s_{gw,t} = V_{gw,t+1} \quad (4)$$

$$r_{sw,t} + s_{sw,t} = \sum_{d=1}^D x_{sw,d,t} + q_{E,t} \quad (5)$$

$$V_{gw,t} + Q_{gw,t} - \sum_{d=1}^D x_{gw,d,t} - s_{gw,t} = V_{gw,t+1} \quad (6)$$

$$\sum_{u=1}^U x_{sw,u,t} \leq Q_{sw,t} \quad (7)$$

$$\sum_{u=1}^U x_{gw,u,t} \leq X_{gw,t} \quad (8)$$

$$r_t \leq R, x_{sw,Bei} + x_{SNWTP,Bei} \leq Q_{SNWTP}, q_{E,t} \geq Q_E, V_{sw,t} \leq V_{max,sw}, V_{gw,t} \leq V_{max,gw} \quad (9)$$

$$C_{gw} = f \left(V_{gw}, \sum_{d=1}^D x_{sw,d} \right) \quad (10)$$

(see Table 3).

Equation (3) is the water demand fulfillment constraint, i.e. the sum of water allocation and water curtailments equals the water demand of each user. Eq. (4) is the water balance of the combined surface water reservoir, while Eq. (5) is the water balance of the reservoir releases. A similar water balance for the dynamic groundwater aquifer follows in Eq. (6). The upstream surface water allocations are constrained to the upstream runoff (Eq. 7), while the upstream groundwater allocations are constrained to a fixed sustainable monthly average (Eq. 8). In Eq. (9), the upper and lower hard constraints on the decision variables are shown. Last, Eq. (10) is the marginal groundwater

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



with unknown future runoff. In this study, the available historic runoff time series are used to demonstrate how the derived water value tables should be used in real time operation. The simulation will be initialized from different initial groundwater aquifer storage levels thereby demonstrating which pricing policy should be used to bring the NCP back into a sustainable state.

2.3 Dynamic groundwater aquifer

The groundwater aquifer is represented as a simple box model (see Fig. 2) with recharge and groundwater pumping determining the change in the stored volume of the aquifer (Eq. 6). The pumping is associated with a pumping cost determined by the energy needed to lift the water from the groundwater table to the land surface (Eq. 10):

$$P = (\rho g \Delta h) / \varepsilon \quad (11)$$

where P is the specific pump energy (J m^{-3}), ρ is the density of water (kg m^{-3}), g is the gravitational acceleration (m s^{-2}), Δh is the head difference between groundwater table and land surface (m) and ε is the pump efficiency (-). The marginal pumping cost c_{gw} (CNY m^{-3}) is found from the average electricity price c_{el} (CNY Wh^{-1}) in Northern China:

$$c_{\text{gw}} = c_{\text{el}} P \quad (12)$$

Hence this cost will vary with the stored volume in the groundwater aquifer. The present electricity price structure in China is quite complex, with the users typically paying between 0.4 and 1 CNY kWh^{-1} depending on power source, province and consumer type (Li, 2012; Yu, 2011). In this study a fixed electricity price of 1 CNY kWh^{-1} is used. The immediate costs of supplying groundwater to a single user follow:

$$c_{\text{gw},t} X_{\text{gw},t} = \rho g \Delta h \varepsilon^{-1} c_{\text{el}} X_{\text{gw},t} \quad (13)$$

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

Title Page

Abstract Introduction

Conclusions References

Tables Figures

⏪ ⏩

◀ ▶

Back Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



where Δh is found as the mean depth from the land surface to the groundwater table (see Fig. 2) between t and $t + 1$:

$$\Delta h = \Delta h_{\text{top}} + \left(V_{\text{max,gw}} - \frac{V_{\text{gw},t} + V_{\text{gw},t+1}}{2} \right) S_y^{-1} A^{-1} \quad (14)$$

where Δh_{top} is the distance from the land surface to the top of the aquifer at full storage (m), S_y is the specific yield (–) of the aquifer and A is the area of the aquifer (m²). Here $V_{\text{gw},t+1}$ is a decision variable and once substituted into Eq. (13), it is clear that the problem becomes non-linear.

In Eq. (14) the drawdown is assumed uniform over the entire aquifer. This simplification might be problematic as the local cone of depression around each well could contribute significantly to the pumping cost and thereby the optimal policy. Therefore, the steady state Thiem drawdown (Thiem, 1906) solution is used to estimate local drawdown at the pumping wells. Local drawdown is then added to Eq. (15) to estimate total required lift:

$$\Delta h_{\text{Thiem}} = \frac{Q_w}{2\pi T} \ln \left(\frac{r_{\text{in}}}{r_w} \right) \quad (15)$$

where Q_w is the pumping rate of each well (m³ month⁻¹), T is the transmissivity (m² month⁻¹), r_{in} is the radius of influence (m) and r_w is the distance from origin to the point of interest (m), here the radius of the well. The transmissivity is based on a hydraulic conductivity of 1.3×10^{-6} m² month⁻¹ for silty loam (Qin et al., 2013) which was tested to be realistic in a MIKE SHE model of the Ziya River Basin (Marker, 2013). Field interviews revealed that the wells typically reach no deeper than 200 m below surface, which results in a specific yield of 5%. The thickness of the aquifer Q_w is defined as the total allocated groundwater within the stage (m³ month⁻¹), distributed evenly to

HESSD

12, 5931–5966, 2015

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



the number of wells in the catchment:

$$Q_w = \frac{\sum_{d=1}^D X_{gw,d,t}}{n_w} = \frac{V_{gw,t} - V_{gw,t+1} + Q_{gw,t} - S_{gw,t}}{n_w} \quad (16)$$

where n_w is the number of wells in the downstream basin. Erlendsson (2014) estimated the well density in the Ziya River Basin from Google Earth to be 16 wells km^{-2} . Assuming that the wells are distributed evenly on a regular grid and that the radius of influence r_{in} is 500 m, overlapping cones of depression from 8 surrounding wells are included in the calculation of the local drawdown. This additional drawdown is included using the principle of superposition as also applied by Erlendsson (2014).

2.4 Solving non-linear and non-convex sub-problems

In the previous study by Davidsen et al. (2015), the optimization problem was strictly linear and strictly convex. The individual sub-problems of the SDP scheme could therefore be solved with a fast linear programming algorithm. In this study however, with non-linearity from the head-dependent groundwater pumping costs, the expected future cost function is no longer strictly convex.

Non-linear optimization problems can be solved with evolutionary search methods, a sub division of global optimizers. A widely used group of evolutionary search methods is genetic algorithms (GA), which have been found to be efficient tools to get the approximate solution to complex non-linear optimization problems (see, e.g., Goldberg, 1989; Reeves, 1997). GAs use a random search approach inspired by natural evolution and have been applied to the field of water resources management by, e.g., Cai et al. (2001), McKinney and Lin (1994) and Nicklow et al. (2010). Cai et al. (2001) used a combined genetic algorithm and linear programming (LP) approach to solve a highly non-linear surface water management problem. By fixing some of the complicating decision variables, the remaining objective function became linear and thereby solvable

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



with LP. The GA was used to test combinations of the fixed parameters while looking for the optimal solution. The combination yielded faster computation time than if the GA was used to estimate all the parameters.

This study uses a genetic algorithm implemented in MATLAB to solve the cost minimization sub problems. This GA function will initially generate a set of candidate solutions known as the *population*. Each of the candidate solutions contains a set of decision variables (sampled within the decision space), which will yield a feasible solution to the optimization problem. In MATLAB, a set of options specifies: the *population size*, the stopping criteria (*fitness limit*, *stall limit*, *function tolerance* and others), the *crossover fraction*, the *elite count* (number of top parents to be guaranteed survival) and the *generation function* (how the initial population is generated). The options were adjusted to achieve maximum efficiency of the GA for the present optimization problem.

The computation time for one single sub-problem is orders of magnitude larger than solving a simple LP. As the optimization problem becomes computationally heavier with increasing number of decision variables, a hybrid version of GA and LP, similar to the method used by Cai et al. (2001), was developed (see Fig. 3). Decision variables that cause non-linearity are identified and chosen by the GA. Once these complicating decision variables are chosen, the remaining objective function becomes linear and thereby solvable with LP. In the present optimization problem (Eq. 1) the non-linearity is caused by the head-dependent pumping costs as explained in Eqs. (13) and (14). Both the regional lowering of the groundwater table and the Thiem local drawdown cones depend on the decision variable for the stored volume in $t + 1$, $V_{gw,t+1}$. If $V_{gw,t+1}$ is pre-selected, the regional drawdown is given and the resulting groundwater pumping rate Q_w can be calculated from the water balance. The groundwater pumping price is thereby also given, and the remaining optimization problem becomes linear.

The SDP framework is subject to the *curse of dimensionality*. With two state variables and non-linearity, the computation time is significant and is a limiting factor when choosing model discretization. With a low number of discrete states, the discretization error increases, particularly if the end storages $V_{gw,t+1}$ and $V_{sw,t+1}$ are kept discrete.

HESSD

12, 5931–5966, 2015

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



Piecewise linear interpolation of the future cost function (Pereira and Pinto, 1991) allows for free end storages but requires strict convexity. With two state variables, interpolation between the future cost points will yield a hyper-plane in three dimensions. In our problem, the EFC is no longer strictly convex and therefore both $V_{gw,t+1}$ and $V_{sw,t+1}$ are chosen by the GA. For a given combination of stages, discrete states and flow classes, the objective of the GA is to minimize the total costs, TC, with the free states $V_{gw,t+1}$, $V_{sw,t+1}$ being the decisions:

$$TC(V_{gw,t+1}, V_{sw,t+1}) = \min (IC(V_{gw,t+1}, V_{sw,t+1}) + EFC)(V_{gw,t+1}, V_{sw,t+1}) \quad (17)$$

with EFC being the expected future costs. Given initial states and once the GA has chosen the end states, the immediate cost minimization problem becomes linear and hence solvable with LP (see Fig. 3). The IBM CPLEX linear programming solver (IBM, 2013) is used to solve the linear programs. The expected future costs are found by cubic interpolation of the discrete neighboring future cost grid points in each dimension of the matrix. The GA approaches the global optimum until a fitness limit criteria is met. The total costs are stored, and the algorithm continues to the next state. To reduce the computation time, the outer loop through the groundwater states is parallelized.

The performance of the GA-SDP model is compared to a deterministic Dynamic Program (DP), which finds the optimal solution given perfect knowledge about future inflows and groundwater recharge. The DP model uses the same algorithm as the SDP model and 1-dimensional state transition matrices with $\rho = 1$ between the deterministic monthly runoff data. For low storage capacity and long time scales, the effect of the end storage volume becomes negligible. Similar to the SDP model, the DP model was looped and run until the end of period condition does not affect the present management.

HESSD

12, 5931–5966, 2015

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



3 Results

Without any regulation or consideration of the expected future costs arising from over-exploitation of the groundwater aquifer, the water users will continue maximizing immediate profits (producers) or utility (consumers). Because there are only electricity costs for groundwater, the users will continue pumping groundwater until the marginal groundwater cost exceeds the curtailment costs. At 1 CNY kWh⁻¹ the marginal cost of lifting groundwater 200 m (typical depth of wells observed in the study area) can be found with Eqs. (13) and (14) to be 0.8 CNY m⁻³ and thereby less than the lowest curtailment cost at 2.3 CNY m⁻³. It requires an electricity price higher than 2.8 CNY kWh⁻¹ before the lowest-value user stops pumping from 200 m below surface.

The backward recursive SDP algorithm was run with a looped annual dataset until equilibrium water values, i.e. no inter-annual changes, were obtained. The water values increase fastest during the first years, and after approximately 100 years the annual increases become small. Due to the large storage capacity of the groundwater aquifer, equilibrium is however not achieved until after 150–180 years. These marginal water values represent the true values of storing a unit volume of water for later use and vary with reservoir storage levels, runoff flow class and time of the year. A sample of the resulting equilibrium water value tables are presented in Fig. 4. This figure shows the temporal variations of water values as a function of one state variable, keeping the other state variable at a fixed value. The state variables are fixed at empty, half full and full storage respectively. During the rainy season from June to August, high precipitation rates reduce water scarcity, resulting in lower surface water values. Because the groundwater storage capacity is much larger, increased recharge can easily be stored for later use, and groundwater values are therefore not affected. The water values after 1980 are clearly higher than in the period before 1980 due increased water scarcity caused by a reduction in the regional precipitation. In contrast, the groundwater value tables are uniform, with variation only with groundwater storage. The detailed water value tables are included as Supplement.

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

[Title Page](#)

[Abstract](#)

[Introduction](#)

[Conclusions](#)

[References](#)

[Tables](#)

[Figures](#)



[Back](#)

[Close](#)

[Full Screen / Esc](#)

[Printer-friendly Version](#)

[Interactive Discussion](#)



increasing storage level as shown in Fig. 5. Under these circumstances the surface water user's price increases up to a point where the two prices meet.

At the equilibrium groundwater storage level, the user's prices shown in Fig. 6 indicate frequent curtailment of wheat agriculture in the downstream Hebei Province, which has a willingness to pay of 2.3 CNY m⁻³. The allocation pattern to this user is shown in Fig. 7: the model switches between high curtailment and high allocations, depending on water availability and storage in the reservoirs. Groundwater allocations fluctuate between satisfying 0 and 80% of the demand. Inclusion of the steady state Thiem drawdown cones in the optimization model increases the marginal groundwater pumping cost with increased pumping rates. Groundwater allocations are distributed more evenly over the months, which results in less local drawdown. The total curtailments remain constant, while 1% of the total water abstraction is shifted from groundwater to surface water, if the stationary Thiem drawdown is included.

The average total costs of the 51 years simulation for different scenarios can be seen in Table 1. A simple local sensitivity analysis is used to assess the uncertainty of the model. Davidsen et al. (2015) used Monte Carlo simulations based on 50 samples to estimate the uncertainty of the model outputs. However, the inclusion of an additional state variable has increased the optimization time significantly and made such an approach infeasible. Approximately 4000 CPU hours per climate period are needed to reach equilibrium in the present model, equivalent to two weeks, if the maximum of 12 parallel processors are used in MATLAB R2013a. The analysis was focused on the local sensitivity related to the water demands and water curtailment costs used directly in the objective function (Eq. 1) and the transmissivity used to estimate the local drawdown (Eq. 14). The uncertain input parameters were increased by 10% and the sensitivity evaluated based on the simulation results. The resulting total costs can be seen in Table 1. A 10% increase in the curtailment costs is returned as a 6.0% increase in the total costs, while a similar increase of the demands generates a 2.1% increase in costs. The transmissivity can vary over many orders of magnitude because

HESSD

12, 5931–5966, 2015

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



HESSD

12, 5931–5966, 2015

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

[Title Page](#)

[Abstract](#)

[Introduction](#)

[Conclusions](#)

[References](#)

[Tables](#)

[Figures](#)



[Back](#)

[Close](#)

[Full Screen / Esc](#)

[Printer-friendly Version](#)

[Interactive Discussion](#)



ming (SDDP, Pereira and Pinto, 1991; Pereira et al., 1998) has shown great potential for multi-reservoir river basin water management problems. However, because SDDP only samples around the optimal decisions, this method will not be able to provide the complete set of shadow prices for all state combinations and is therefore less suitable for adaptive management.

Computation time was a major limitation in this study, and the transition from the previous much simpler linear single state SDP model (Davidsen et al., 2015) to the presented non-linear SDP model with two state variables proved to require around 50 000 times more CPU hours. We used the high performance cluster (HPC) at the Technical University of Denmark to solve the SDP, and as the optimization can be run offline, the resulting optimization time of 2 weeks on 12 cores is acceptable.

Replacement of the hard upper groundwater pumping constraint used by Davidsen et al. (2015) with a dynamic groundwater aquifer, lowered the total costs from 11.39 billion CNY yr⁻¹ to 8.47 billion CNY yr⁻¹. This difference highlights the problem of defining realistic boundaries to optimization problems and shows that simple hard constraints, here fixed groundwater pumping limits, can highly limit the optimal decision space. With inclusion of a dynamic groundwater aquifer, the model can use the large groundwater storage capacity as a buffer to the system, which significantly stabilizes the user's price of surface water as shown in Fig. 6. Finally, policies like minimum ecosystem flow constraints can be satisfied with less impact on the expensive users, which results in reductions in the respective shadow prices.

Another addition to the modelling framework was the Thiem stationary drawdown. The long time steps (monthly) make stationarity a realistic assumption. Inclusion of well drawdown significantly changed the simulated management but resulted in only slightly increased computation time. While addition of the Thiem stationary drawdown has only a small effect on total costs and total allocated water, it is clear from Fig. 7 that the additional Thiem drawdown highly impacts the allocation pattern to some of the water users. High groundwater pumping rates result in larger local drawdown and thus in higher pumping costs. This mechanism leads to a more uniform groundwater

represented by the groundwater value for the last allocated unit of water. As the model is run to equilibrium, the user's prices converge towards the long-term equilibrium at approximately 2.2 CNY m^{-3} . This long-term equilibrium is not affected by the actual electricity price, as increasing electricity prices will be offset by a similar reduction in the opportunity costs. A constant electricity price can therefore be used as a policy tool to internalize the user's prices of groundwater shown in Fig. 6. Stable water user's prices will ease the implementation of e.g. an opportunity cost pricing (OCP) scheme, which is one of the available policy options to enforce long-term sustainability of groundwater management.

5 Conclusion

This study presented how a hydroeconomic optimization approach can be used to derive a pricing policy to bring an overexploited groundwater aquifer back to a long-term sustainable state. The model quantifies potential savings of joint water management of a complex river basin in China. Surface water and groundwater management was optimized in a SDP framework based on a coupled GA-LP setup. The derived equilibrium water value tables represent the shadow prices of surface and groundwater for any combination of time, inflow class and reservoir storage. The groundwater values at equilibrium were found to be almost constant at 2.2 CNY m^{-3} , independent of the time of the year, the surface water storage and the inflow class. Non-convexity caused by the groundwater reservoir could be accommodated with the use of a GA and was further extended to include stationary Thiem local drawdown cones. Inclusion of a dynamic groundwater aquifer greatly reduced the total costs of water scarcity, compared to a setup with fixed monthly pumping limits. The sustainable management will recharge the aquifer until the equilibrium storage level is reached. From an initial storage at one third of the aquifer capacity, the average costs of ending groundwater overdraft are estimated to be 13.32 billion CNY yr^{-1} . After equilibrium is reached, the average costs are estimated to be 5.47 billion CNY yr^{-1} . The aquifer serves as buffer

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



and allows for overexploitation in dry years, and this mechanism stabilizes the user's water prices. These stable user's prices are suitable for use in an OCP scheme. While the representation of the management problem must be kept simple in the optimization model, the OCP prices can be used to drive a much more detailed simulation model, which includes a detailed physical representation of the system.

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HESSD

12, 5931–5966, 2015

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



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C. Davidsen et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



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The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



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The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

[Title Page](#)[Abstract](#)[Introduction](#)[Conclusions](#)[References](#)[Tables](#)[Figures](#)[⏪](#)[⏩](#)[◀](#)[▶](#)[Back](#)[Close](#)[Full Screen / Esc](#)[Printer-friendly Version](#)[Interactive Discussion](#)

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The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

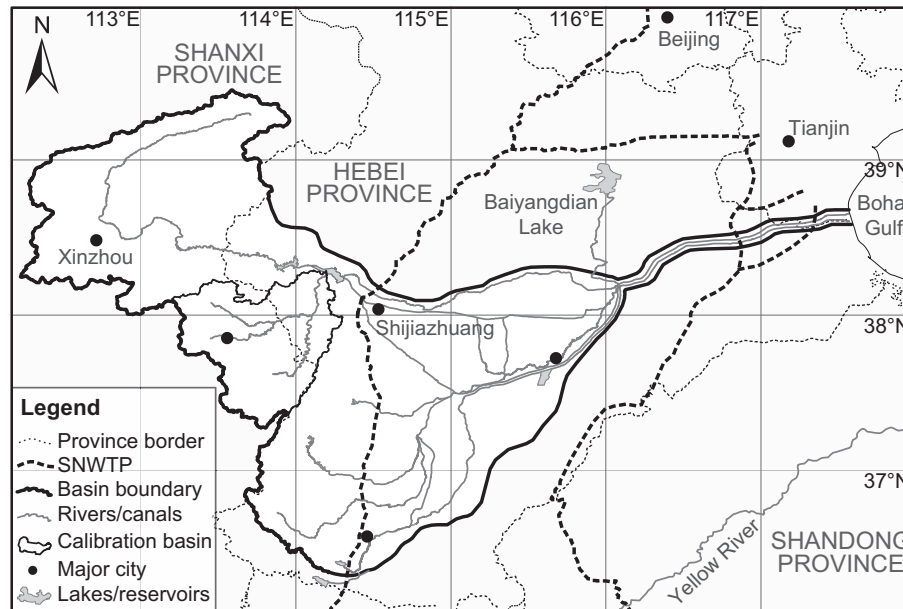


Figure 1. The Ziya River Basin. Watershed and rivers automatically delineated from a digital elevation map (USGS, 2004) and manually verified and corrected with Google Earth (Google Inc., 2013). The SNWTP routes (Central and Eastern) were sketched in Google Earth and verified with field observations. Provincial boundaries from (NGCC, 2009).

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

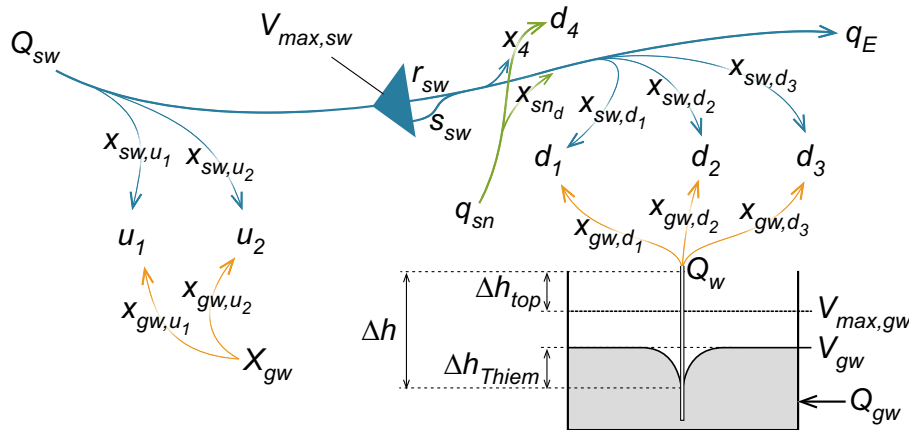


Figure 2. Conceptual sketch of the Ziya River Basin management problem with water users located upstream (u) and downstream (d) the surface water reservoir. Allocation decision variables are indicated for surface water (blue), SNWTP water (green) and groundwater (orange). A conceptual sketch of the downstream dynamic aquifer is included and show how the total lift (Δh) is composed of the top layer + the regional groundwater lowering + the local Thiem steady state groundwater drawdown.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



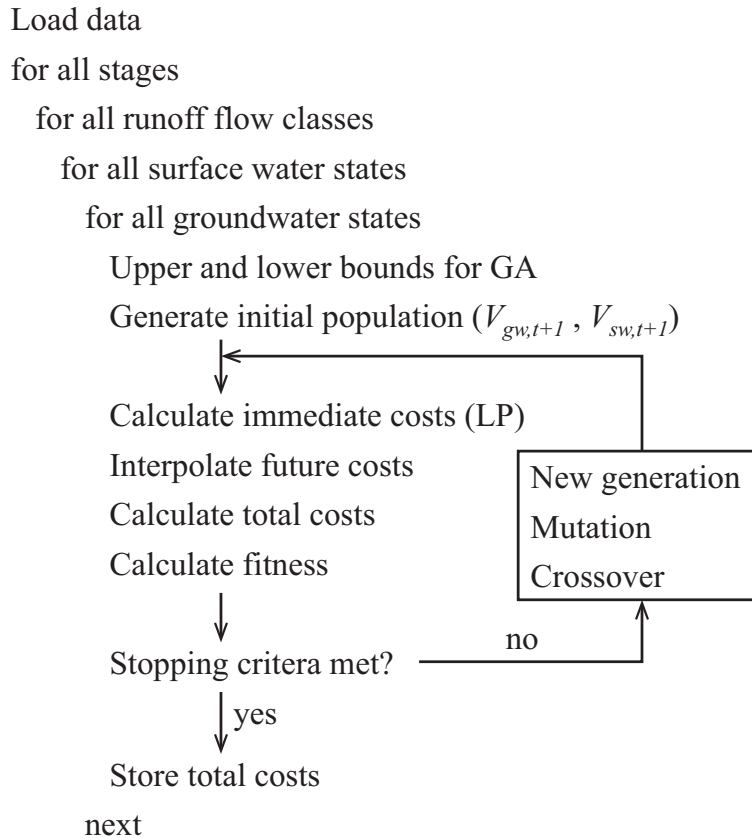


Figure 3. SDP optimization algorithm design.

[Title Page](#)

[Abstract](#) | [Introduction](#)

[Conclusions](#) | [References](#)

[Tables](#) | [Figures](#)

[◀](#) | [▶](#)

[◀](#) | [▶](#)

[Back](#) | [Close](#)

[Full Screen / Esc](#)

[Printer-friendly Version](#)

[Interactive Discussion](#)



The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

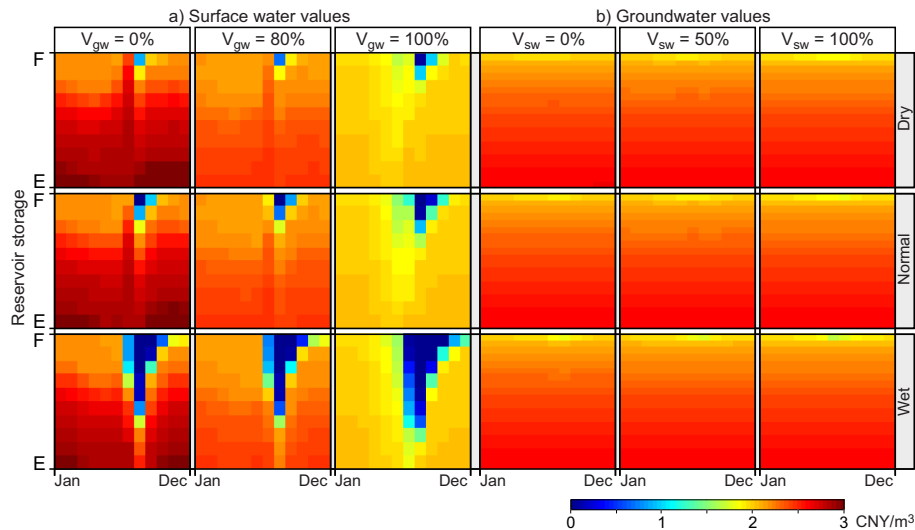


Figure 4. Temporal changes of the water values (CNY m⁻³) for the climate period before 1980. The marginal water value is the true value of storing a unit volume of water for later use, and varies with reservoir storage levels, runoff flow class and time of the year. **(a)** Surface water values at fixed [0, 50, 100 %] groundwater aquifer storage. **(b)** Groundwater values over time at fixed [0, 50, 100 %] surface reservoir. The reservoir storage is shown from E (empty) to F (full).

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

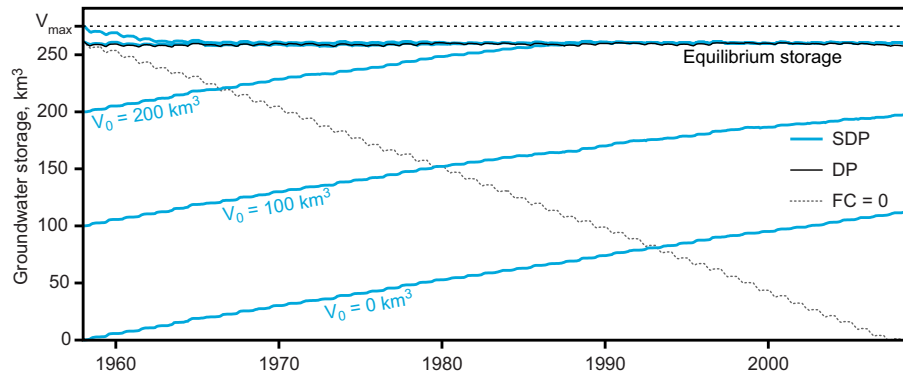


Figure 5. Simulated groundwater aquifer storage levels for 51 years of historical runoff with different initial groundwater storage (0, 100, 200, 258 and 275 km³). The perfect foresight DP and management without consideration of the future (FC = 0) are also shown.

[Title Page](#)[Abstract](#)[Introduction](#)[Conclusions](#)[References](#)[Tables](#)[Figures](#)[◀](#)[▶](#)[◀](#)[▶](#)[Back](#)[Close](#)[Full Screen / Esc](#)[Printer-friendly Version](#)[Interactive Discussion](#)

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

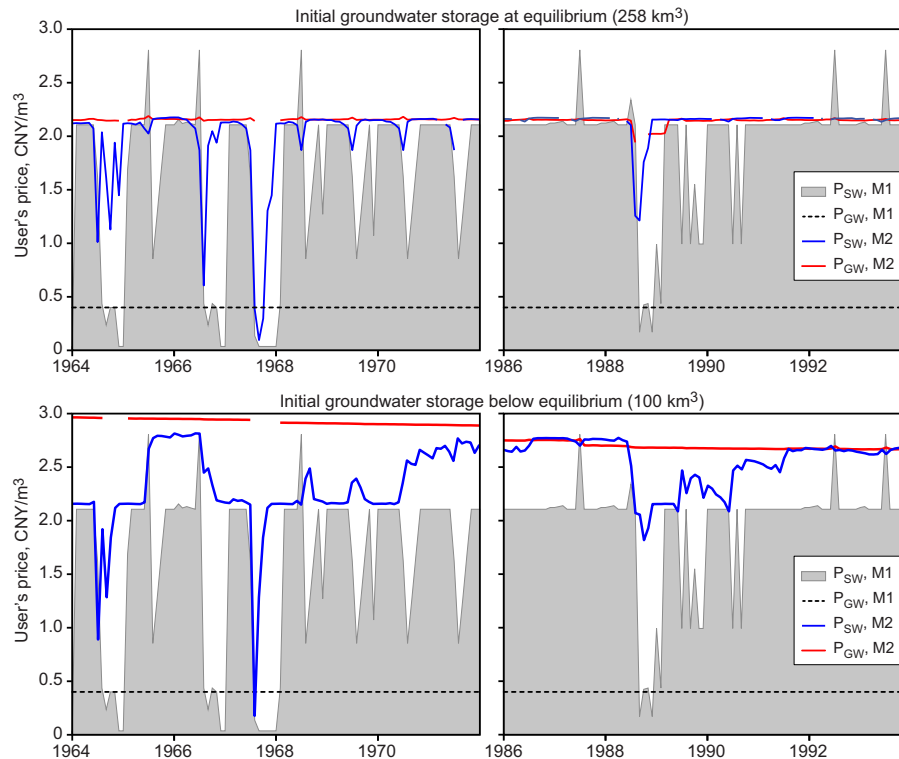


Figure 6. User's price for groundwater and surface water through for a 51 year simulation based on simulated historical runoff for two initial groundwater storages. P = user's price, M1 = results from Davidsen et al. (2014) with a single combined surface water reservoir and constant groundwater costs, M2 = results from the presented model framework with a combined surface water reservoir and a dynamic groundwater aquifer. The user's price for groundwater in M2 is the immediate pumping costs added the opportunity costs from the water value tables.

[Title Page](#)
[Abstract](#)
[Introduction](#)
[Conclusions](#)
[References](#)
[Tables](#)
[Figures](#)
[⏪](#)
[⏩](#)
[◀](#)
[▶](#)
[Back](#)
[Close](#)
[Full Screen / Esc](#)
[Printer-friendly Version](#)
[Interactive Discussion](#)

The cost of ending groundwater overdraft on the North China Plain

C. Davidsen et al.

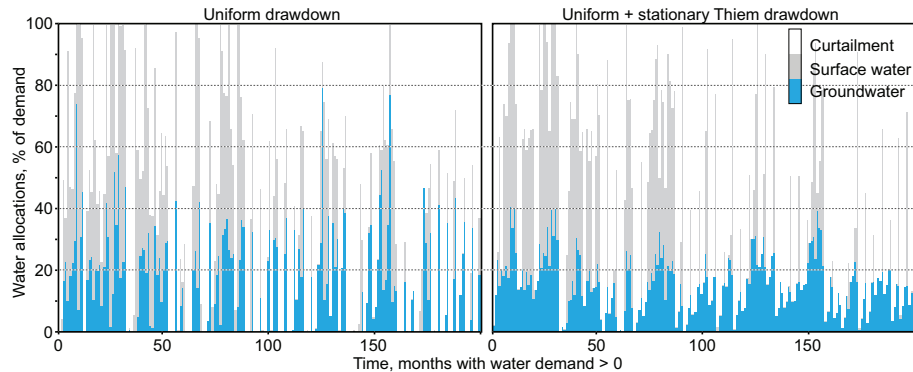


Figure 7. Composition of allocations and curtailments to wheat agriculture in the Hebei Province for the months March, April, May and June through 51 years simulation from an initial groundwater storage at equilibrium (258 km^3). The results are shown for a simple drawdown model with uniform regional lowering of the groundwater table, and a more realistic drawdown model which includes the stationary Thiem local drawdown cones.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

◀

▶

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion

