1 HESS-2015-176

2 COMMENTS FROM EDITORS AND REVIEWERS

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6 Editor

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7 I agree with the reviewers comments, in particular reviewer #2 that:

- 9 Ed.1) ... the paper is currently hard to follow as it refers to earlier work which needs to be more clearly10 included.
- **Answer from authors**: The comparison with the previous study gives a great opportunity to answer a
- central question: is it worth the computational effort to include the groundwater as a dynamic reservoir. In
 this updated manuscript we have tried to make clearer links to the previous study and put more effort in
 explaining each comparison.
- 15

16 **Ed.2)** ... Other important issues include a justification for the simplifications.

Answer from authors: We have carefully explained and justified the many simplifications throughout the
 manuscript. The Discussion has now more focus on the different types of simplifications: 1) the

- 19 simplifications needed to keep the chosen SDP method computationally feasible and 2) the simplifications
- 20 needed due to poor data availability.
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results have reference value.

23 Anonymous Referee #1

24 In this paper, a hydroeconomic modelling approach is used to find cost-optimal sustainable surface water 25 and groundwater allocation strategies for a river basin. A simplified management problem with conjunctive 26 use of scarce surface water and groundwater under inflow and recharge uncertainty is presented. Because 27 of head-dependent groundwater pumping costs the optimization problem is non-linear and non-convex, 28 and a genetic algorithm is used to solve the 1-step-ahead sub-problems with the objective of minimizing 29 the sum of immediate and expected future costs. A real-world application in the Ziya River Basin in 30 northern China is used to demonstrate the model capabilities. It's estimated that the annual cost of ending 31 groundwater overdraft in the basin is estimated to be 5.47 billion CNY/year. Both the methods and the

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AR1.1) "Persistent overdraft from the groundwater aquifers on the North China Plain has caused declining groundwater tables, salinization and infiltration of wastewater." Here the expression of "salinization" is confusing. In fact, the lowdown of ground water table has been favorable for the control of salinization and alkalization. Maybe the expression can be changed to "Persistent overdraft from the groundwater aquifers on the North China Plain has caused declining groundwater tables, and infiltration of saline water and wastewater.

40 Answer from authors: Yes. We have also deleted "saline" since this is outside the focus areas of our study:

41 *"Persistent overdraft from the groundwater aquifers on the North China Plain has caused declining*42 *groundwater levels".*

- 43
- 44 AR1.2) For table 3: What the meaning of SP E? Why it's the same for different scenarios (before and after45 SNWTP)?
- 46 **Answer from authors**: "SP E" is the average increase in the total costs as a consequence of introducing a
- 47 minimum in-stream flow constraint. To estimate this shadow price, we need comparable scenario runs with
- 48 and without this additional ecosystem constraint. The presented shadow price is only valid for that
- 49 particular scenario and does not cover different situations, e.g. before/after the SNWTP. The white area
- 50 above and below the listed value in Table 3 has been confusing because it may indicate that the shadow
- 51 price covers all scenarios.
- 52 To avoid confusion, we decided to delete these almost blank columns and instead describe them in the
- 53 *Results section. Thereby, we also avoid the use of SP E.*

- 54 **AR1.3)** For table 3: What's the meaning of SP SNWTP? Why for LGW, the shadow price is the lowest?
- 55 Because the initial condition is more severe, the value of water should be higher. Please give an 56 explanation.
- Answer from authors: "SP SNWTP" is the average reduction in the total costs, associated with the
 introduction of the South-to-North Water Transfer Project (SNWTP). The total costs after the SNWTP is put
 in operation are compared to the scenario without the SNWTP (pre-2008) and divided by the allocated
 SNWTP water. While this is meaningful when comparing identical scenarios with and without the SNWTP, it
 is misleading when the increased costs are caused by other factors. We have deleted the SP SNWTP values
 for all scenarios not identical to the baseline setup. The remaining two SP SNWTP-values (row 2 and 5) have
 been presented in the text. The SP SNWTP and SP E columns have been deleted from Table 3.
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66 Anonymous Referee **#2**

67 The manuscript presents a method consisting in a Stochastic Dynamic Programming (SDP) management 68 model for a system including one reservoir and one aquifer. The aquifer is represented as a box model. The 69 problem is solved with a combination of Genetic Algorithms and Linear Programming (GA-LP) to tackle the 70 non-linearities and non-convexities caused by the head-dependent pumping costs. The framework is 71 applied to the Ziya River system (North China), where groundwater overdraft has led to a significant 72 decrease in the aquifer levels. The results of the SDP are provided in the form of water value tables used as 73 prices in a forward-moving simulation run. The estimated costs given by the model when the aquifer levels 74 reach equilibrium, in comparison with business-as-usual values not considering groundwater 75 overexploitation (previous paper), serve as estimation of the cost associated to a recovery in the aquifer 76 level.

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78 The provided manuscript refers to a critical problem in many arid and semiarid areas: persistent 79 groundwater overexploitation, which has caused considerable damage in both water quantity and quality 80 across the world. The methodology is well-presented and exposed in the case study. Coupling stochastic 81 programming and groundwater simulation is cumbersome, and new approaches to alleviate its complexity 82 and transform those results into management policies could support the application of those tools in water 83 resources management. For that, this paper has a considerable potential interest for publication in HESS. In 84 addition, it is well-written and well-structured. However, there are some important points that the authors 85 should address in order to enhance the manuscript.

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AR2.1) The method strongly simplify the hydrology (just a Budyko model for assessing runoffs, and fixed %
of groundwater recharge no justified), as well as the spatial representation of the system (all surface
reservoirs lumped into a single one) and the groundwater simulation (a lumped box model with unclear if
not missing representation of stream-aquifer interaction). Despite the presentation as a hydroeconomic
model, the economics is also highly simplified (constant water demands, constants curtailment cost). These
simplifications need to be justified, including an analysis of how realistic these assumptions are. This can be
done along the text when the assumptions are presented. Overall, the limitations of the modelling

- 94 approach should be clearly stated either in the Discussion or the Conclusions.
- 95 Answer from authors: There are two main reasons for the high level of simplification: Limited data
- 96 availability and the limitations of the SDP method (curse of dimensionality). All assumptions, simplifications
 97 and their implications are now carefully discussed in the revised manuscript, e.g. as inserted in the
- 98 Discussion: "the simple system representation needed in SDP required assumptions of inflow and storage
- 99 discretization, generalized estimates of pumping cost and a lumped groundwater model which all contribute
- 100 to the uncertainty. Further, poor data availability for the case study area required some rough estimates of
- 101 the natural water availability, single-point demand curves and perfect correlation between rainfall and
- 102 groundwater recharge. The method-driven assumptions generally limit the decision support to basin-scale
- while the simple estimates caused by poor data availability contribute to raising the general uncertainty of
 the model results. Given the computational challenges and the diverse and significant uncertainties, the
- 105 model results should be seen as a demonstration of the model capabilities rather than precise cost
- estimates. Better estimates will require access to a better case dataset and involve a more comprehensive
 sensitivity analysis."

108	AR2.2) The paper constantly refers to the previous analysis done by the authors, published in another
109	paper, whose results represent the business as usual situation, not shown in this one with the exception of
110	the total annual cost (Discussion). Thus, the presented paper looks like a second part of the one previously
111	referred, since which it is quite hard to fully understand it without the other one. Maybe the authors could
112	briefly include more description of the method and results for the business as usual situation, or update
113	those at the light of the findings of this paper, in order to facilitate the comparison between both
114	alternatives in this paper.
115	Answer from authors: The previous study was a traditional implementation of SDP on a single-reservoir
116	system and shows optimal management while disregarding dynamic groundwater storage and head-
117	dependent groundwater pumping costs.
118	We have added a brief and clear summary of the previous study in the case study description with focus on
119	underlining the differences between the previous and the present study. We have also revised the links to
120	the previous study with focus on keeping the explanations short and precise. Together, this facilitates a
121	better comparison between the alternatives within the paper.
122	
123	AR2.3) Introduction. While being successful in presenting the problem, the Introduction seems a little
124	confusing. At first, one would expect some comments about why is important to jointly manage surface
125	and groundwater prior to enumerating the state of-the-art on conjunctive use optimization. While the
126	division between deterministic and stochastic programming is adequate, the state-of-the-art presented
127	consists in describing several references rather than explaining briefly both approaches supporting both
128	explanations with references. It is said in the paper that "has been addressed widely in the literature"
129	(which is true) but then only 4 references for deterministic and 2 for stochastic are shown. I would prefer to
130	not explain what has been done in a little number of papers, but to discuss the different approaches
131	employed and then enumerate the references. Besides, the review seems to not have moved prior to the
132	90's, when the topic appeared in the 60's and 70's.
133	Answer from authors : The introduction has been revised as suggested by the reviewer:
134	• A line has been added to motivate why it is important to jointly manage surface and aroundwater:
135	"Ontimal allocation of the water resources should address coordinated use of the water resources
136	by considering the long term total costs while utilizing the groundwater as a huffer "
127	The literature review has been completely rewritten and now provides a more complete overview of
120	• The interactive review has been completely rewritten and now provides a more complete overview of the major approaches amployed within conjunctive surface water and aroundwater management
120	The last paragraph has been added stronger links to the existing methods.
139	• The fust purugruph hus been duded stronger links to the existing methods.
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141	AD2 () (Case study) = 5025. It is second that the full standard end of the second flavith busit has t
142	AR2.4) (Case study) p. 5935. It is assumed that the full storage capacity can be managed flexibly without
143	consideration of storage reserved for flood protection or existing management rules. Why ?
144	Answer from autnors : Reservoir rule curves and flood control volumes were not available as such
145	information is classified in China. A sentence has been added to clarify this: "While reservoir rule curves and
146	flood control volumes can easily be accommodated, the present policies were not available for the case
14/	area".
148	
149	AR2.5) So how flood protection pools are taken into account? Are you using a realistic useful storage?
150	Answer from authors : Flood protection is not taken into account in this study. It will, however, be easy to
151	implement a volume reserved for flood storage within the proposed framework. This will reduce the
152	available storage and increase water scarcity in the long dry season. In the present model setup, we find the
153	lower limit on water scarcity costs, assuming that the entire storage capacity is available for storing water.
154	Reservoir spills will cause an economic loss, and the model tends to avoid spills by entering the rainy season
155	with a low reservoir storage level.
156	This has been clarified in the second paragraph of the "2.1 Study area".
157	
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- AR2.6) p. 5935.... analysis of dynamic interactions between the groundwater and surface water resources.
- 160 It seems that the box model that you use for groundwater does not account for any dynamic interaction
- 161 between groundwater and surface water. Is this correct? If that is the case, groundwater discharges
- (outflow) and stream-aquifer interaction are not considered . . . Please show that it is correct to neglect thisgroundwater outflow components. Otherwise, we have an incomplete groundwater balance.
- **Answer from authors**: The groundwater model is a simple box model (Infiltration + Storage = Pumping +
- 165 *Overflow).* The groundwater overflow is only used in extreme cases where the total demands + available
- storage < infiltration. The spills will go to the spill variable and leave the system, practically as baseflow to
- 167 the rivers (unavailable for allocation). The aquifer is so heavily over-exploited that no significant baseflow is
- 168 *being created or will be created in any foreseeable future. This has now been clarified in the manuscript.*
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- AR2.7) A rainfall-runoff model previously used in the paper of the business-as-usual run. It is unclear if you
 simply took the resulting inflow values of that study or if you update that model. If it is an update, then the
 calibration results should be presented.
- 173 Answer from authors: The exact same hydrological model results were used in both studies. No new
- calibration was performed, and space was therefore not used to repeat details. Note that the hydrological
 model does not represent the actual modified discharge in the rivers today, but is an estimate of the natural
 water availability. We have clarified this in the manuscript.
- AR2.8) In addition, I do not see the point of developing a daily model and then aggregate the results. It
 would have been easier to directly develop a monthly model.
- Answer from authors: We need an estimate of the natural water availability and chose to reuse the
 estimate from our previous peer-reviewed study. In this study, we had access to daily weather data from the
- 182 *Chinese Meteorological Services and daily runoff from an almost natural river.*
- 183
 184 AR2.9) Besides, it is said that the recharge is estimated upon the precipitation, using the average
 185 precipitation value corresponding to the inflow class as characteristic value. That assumes a perfect
 186 correlation between precipitation and inflows, which is uncommon. Would have then possible to be
- included in the Markov chain? . . . although it would suppose an increase in the curse of dimensionality
 phenomenon . . .
- Answer from authors: It would be possible to include another Markov Chain describing the groundwater
 recharge transition probabilities. With 3 flow classes for both runoff and recharge the number of inflow
 scenarios would increase to 3x3 = 9. However, we do not have any observations of groundwater recharge to
- 192 *develop these statistics. In the absence of such data, we decided to assume perfect correlation.*
- AR2.10) (2.2. optimization model formulation) There is a variable named "groundwater spill". Does it refer
 to "groundwater discharge". Where does physically go this discharge? Please give an explanation about
 what means this spill, and how this is modeled.
- 197 Answer from authors: The groundwater spill is only used in rare extreme cases where the total demands + 198 available storage < infiltration. These spills will go to the spill variable and leave the system, practically as 199 base flow to the rivers (unavailable for allocation). As we are discretizing the entire groundwater storage 200 (empty to full), we experience this situation occasionally in the backward iteration. The resulting lower 201 water values and the large discrete storage intervals will prevent that these spills appear in the forward 202 simulation. We have clarified this in the manuscript (see AR2.6).
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204 AR2.11) (2.4 Solving non-linear and non-convex sub-problems) The non-linearities tackled by your GA-LP 205 algorithm are the decision variables regarding final storages. In an alternative SDP approach, these 206 variables are kept discrete. If you keep them discrete, the problem becomes linear again and there is no 207 need to maintain the timeconsuming GA procedure. In fact, that ability to work out non-linearities is one of 208 the main advantages of Dynamic Programming (DP). Why have you not taken the ending groundwater table 209 Vgw,t+1 discrete? It would have saved you a huge amount of time, although with less quality in the results, 210 as you point out. I would think it would have been worth it, specially regarding at the steady water values 211 found in the aquifer.

Answer from authors: This was also our initial idea. First problem is the discretization. We would need a 212 213 very fine discretization of the groundwater aguifer to allow discrete storage levels and decisions. If not, the 214 discrete volumes of the large aquifer become much larger than the combined monthly demands. Storing all 215 recharge will therefore not be sufficient to recharge to a higher discrete storage level. Similarly, the 216 demands will be smaller than the discrete volumes, and pumping the remaining water to reach a lower 217 discrete level would also be infeasible. For this reason, we decided to allow free end storage. Free end 218 storage requires interpolation between the discrete storage levels. With free surface water and 219 groundwater end storages, the future cost function has three dimensions (surface water storage, 220 groundwater storage and expected future costs). With our head-dependent pumping costs and increasing 221 electricity price, we observed that the future cost function changes from strictly convex (very low electricity 222 price) to strictly concave (very high electricity price). At realistic electricity prices, we observed a mix of 223 concave and convex shape. For the use of Benders' decomposition (require strict convexity), this caused a 224 problem. Instead, we developed the hybrid LP-GA model which was applied successfully. This model can deal 225 with any electricity price (= any groundwater pumping costs) at any storage level. 226 We have focused on communicating this better in the manuscript. 227 228 AR2.12) (2.4 Solving non-linear and non-convex sub-problems) A misunderstanding regarding piecewise 229 linear interpolation is found in this section. You said that, according to Pereira and Pinto, piecewise linear 230 interpolation requires strict convexity. However, Pereira and Pinto used a Benders decomposition, which 231 employs piecewise linear approximations and requires convexity, but it is different from the regular 232 procedure, which does not need the cost-to-go function to be convex. You can fit a linear function between 233 your point and the neighboring ones, as you did when interpolating the future costs with cubic functions.

234 Please correct that.

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- Answer from authors: The previous study used Benders' decomposition and not a piecewise linear
 interpolation. We have updated this paragraph in the manuscript and now explaining why Benders'
 decomposition and a traditional linear interpolation are problematic.
- AR2.13) (3 Results) In the first paragraph of page 5946, it can be read that, at the equilibrium groundwater
 storage level, the willingness to pay is equal to 2.3 CNY m-3. In Figure 6 user's price for groundwater is
 always below that threshold if initial groundwater storage is at equilibrium. If the user's price for
 groundwater is always below the curtailment cost, why is the model curtailing the wheat agriculture? One
 would expect that pumping would fluctuate according to surface water availability, but without any
 curtailment, since it is more profitable to pump. Is there any constraint forcing that curtailment? Please
 elaborate.
- Answer from authors: The 2.3 CNY m-3 is a mistake. The downstream wheat user has a curtailment cost at
 2.12 CNY m-3 (rounded to 2.1 CNY m3 in table 1). The user's price for groundwater reported in Figure 6 is
 ~2.15 CNY m-3 (groundwater value at ~2.06 CNY m-3 and a pumping cost at 0.09 CNY m-3). This exceeds
 the curtailment cost of wheat agriculture (2.12 CNY m-3), and this user is therefore curtailed. These values
 have been updated and the conclusion (curtailment of wheat agriculture) underlined.
- AR2.14) (3 Results) Why a reservoir storage evolution plot does not appear in the manuscript? It would be
 important to see the surface and the groundwater storage in order to identify possible conjunctive use
 patterns. Please include the surface reservoir storage evolution or explain why it is not necessary.
- Answer from authors: The reservoir storage plot was not included in an attempt to reduce the length of the
 manuscript. We have now prepared a figure with a comparison of groundwater and surface water storage
 (see Figure 6).
- AR2.15) (4 Discussion) In the first paragraph of page 5948, you say that SDDP only samples around the
 optimal decisions and, consequently, you will not be able to get the complete set of shadow prices for all
 state combinations. However, the SDDP sampling procedure actually employs samples that are not
 subjected to a pre-defined grid and, therefore, the samples are not evenly distributed across space,
 concentrating in the region located near the optimal decisions. The extrapolation process applied in SDDP
 covers the whole space but with different levels of accuracy depending in which region you look at. The
 difference between SDP and SDDP regards to the fact that the SDP results have the same accuracy for the

- whole space, while the SDDP results' accuracy varies across the space, focusing near the optimal decisions
- while usually decreasing when moving far from them. With SDDP you will get a complete set of shadow
- 268 prices as well, but with different accuracy levels: some of them better than SDP and some of them worse.
- 269 Choosing between them does not regard to having or not shadow prices, but to the degree of accuracy that
- 270 you can accept on them. Please re-elaborate the comparison between SDP and SDDP.
- **271 Answer from authors**: Thanks for clarifying this. We have revised the SDDP-SDP comparison.
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- AR2.16) (3 Results and 4 Discussion) Although a sensitivity analysis was made with regard to the water
- demands, the curtailment costs and the transmissivity; there are other sources of uncertainty that must be
- taken into account. Factors like inflow and storage discretization, assumption of perfect correlation
- between rainfall and in- flow, pumping costs estimation, usage of a lumped model for the aquifer and soon, add a considerable amount of uncertainty to the problem. An explanation about the implications of
- those sources of uncertainty in the results should be added to the manuscript.
- Answer from authors: We have expanded the section on uncertainty and elaborated on the factors that are
 presently not mentioned. We also highlight that "Given the computational challenges and the diverse and
 significant uncertainties, the model results should be seen as a demonstration of the model capabilities
 rather than precise cost estimates. Better estimates will require access to a better case dataset and involve
 a more comprehensive sensitivity analysis."
- AR2.17) (5 Conclusion) As presented, the conclusions would not attract the reader. They seem to appear as part of the discussion rather than a separate section. It should be re-organized in order to clearly highlight what are the novelties of the study and what conclusions can be extracted from the methodology applied and the results obtained in the case study.
- Answer from authors: We have reorganized/rewritten the conclusions and put focus on a brief presentation
 of the clear conclusions related to the method and the results.

292 **Detailed comments**

- AR2.18) (page 5934, line 11) One would expect here references about the water value method, not about
 the SDP one. In addition, Pereira and Pinto (1991) did not used SDP, but SDDP.
- Answer from authors: Yes, this is indeed confusing. We have removed Pereira and Pinto (1991) and left the
 reader with Stage and Larsson (1961) (water value method) and Stedinger et al. (1984) (SDP in reservoir
 operation).
- AR2.19) (page 5935) Line 11: upper storage capacity ?. This is storage capacity, what it is represented
 through a upper bound constraint, but the combination of terms here is unclear. I suggest to remove
 "upper". Please correct it in all the times this appears in the text.
- 302 **Answer from authors**: Yes, the "upper" has been removed as suggested.
- 303 **AR2.20)** (page 5935) Line 24: Why only the upstream users have a pumping limit?
- **Answer from authors**: The river basin has two aquifers (upstream and downstream), which are only
- 305 connected by the river. Ideally, each aquifer should be modelled as a box model, but this extra state variable
- 306 would be computationally challenging within the SDP framework. We therefore set up the box model for the
- 307 downstream and most important aquifer. The upstream aquifer is only bound by an upper pumping limit
- 308 corresponding to the average monthly recharge. This has been clarified.309
- AR2.21) (page 5940, line 21) Replace "the thickness of the aquifer" by "groundwater pumping"
 Answer from authors: Yes, this has been replaced
- AR2.22) (page 5941, line 1) Is it realistic to assume an even distribution of total pumping across all the wells?
- **Answer from authors**: We have added the following sentence to justify this assumption: "The even pumping
- 316 distribution is a fair assumption, as field investigations showed that 1) the majority of the groundwater
- wells are for irrigation, 2) the timing of irrigation, crop types and climate is homogeneous and 3) the
- 318 groundwater wells have comparable capacities."
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- 320 AR2.23) (page 5943, line 18) Replace "program" by "programming".
- 321 Answer from authors: Yes.
- 322

AR2.24) (page 5944, line 24) I think that, besides the larger storage, one important reason beyond the stability shown by the groundwater values is the fact that the interaction between surface water and groundwater is not represented. If some sort of stream-aquifer interaction had been found, the groundwater values would have been affected by surface waters and vice versa.

- 326 groundwater values would have been affected by surface waters and vice versa.
- Answer from authors: Yes, for large permanent rivers this would probably be an important factor. This has
 now been clarified in the following sentence: "Addition of stream-aquifer interactions to the model is
- 329 generally expected to affect this stability, but since the flow in rivers/canals in the case study area is small
- 330 most of the year, and since most areas are far from a river, it is a reasonable assumption to ignore these 331 dynamics."
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- AR2.25) (page 5945, line 1) Rather than decision rules, the water values tables act as pricing policies. In
 fact, you do that in the Discussion and the Conclusions sections.
- Answer from authors: The water value tables are the main drivers behind the release decisions and, if fully
 implemented in the decision process, should be referred to as decision rules. For consistency, we have now
 used "pricing policy" throughout the manuscript.
- AR2.26) (page 5947, line 17) You should add "with SDP" after "feasible today". Other alternatives are able
 to handle large water resources systems.
- 341 *Answer from authors*: Yes, indeed. This has been added.
- AR2.27) (page 5947, line 24) Has a simulation model with higher spatial resolution been used? If not, please
 clearly indicate in the results section (page 5945, line 1) that the forward-moving simulation uses the same
 system scheme.
- Answer from authors: No, we have only used a simulation model with the same system scheme. This has
 now been clarified.
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AR2.28) (page 5949, line 24) I think that the reason beyond the small differences between SDP and DP regard to the inclusion of the aquifer rather than a very good performance of the SDP algorithm (although it is good). If you consider groundwaters in the analysis, their buffer value gives a high robustness to the surface system. This is reflected in the fact that the SDP empties the reservoir almost every year while not doing that if groundwater was not considered: it can always pump so it hedges the reservoir in an aggressive way.

- 355 *Answer from authors*: Yes, we have added this point more clearly in the discussion.
- AR2.29) (page 5950, line 15) The groundwater results are independent in the recharge as well. It should be added to the list.
- **Answer from authors**: Yes, this has now been added here and in the conclusion.
- 360 361 **AR2.30** (page 5951, line 4) I do not understand how the opportunity costs are reduced if electricity prices grow. This would apply exclusively if all the demands could freely pump and all of them had the same 362 363 pumping head, which is not the case (you have demands that are subjected to pumping quotas while other 364 cannot pump). However, the fact that electricity prices can be used to internalize the groundwater prices is 365 valuable regardless of that. 366 **Answer from authors**: This is true. The electricity price statement has been deleted and focus put on the 367 internalization of the groundwater price. 368
- AR2.31) (page 5951, line 7) Rather than opportunity cost pricing (OCP), the name should be marginal cost
 pricing (MCP). Please replace this definition hear and in the rest of the document.
- **371** *Answer from authors*: Yes, this has been updated throughout the manuscript.
- 372
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- AR2.32) (page 5951, line 10) The title of the section should be "Conclusions".
- 375 Answer from authors: Yes, this has been corrected.376
- AR2.33) (page 5951, line 20) The non-convexity is caused by the headdependent pumping costs rather than
 the inclusion of the groundwater reservoir.
- 379 Answer from authors: Yes, this has been clarified in the conclusion: "Non-convexity caused by head and rate
- 380 <u>dependent groundwater pumping costs</u> was accommodated with the use of a GA and was further extended
 381 to include stationary Thiem local drawdown cones"
- AR2.34) (page 5958, Table 2) This table has not been cited in the text. Remove it or cite it.
- Answer from authors: An error has happened in the layout version. The reference is wrongly listed as "Table
 1" on page 5945 in line 15 and 26. We will make sure that the table references are corrected in the final
 version.
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- AR2.35) (page 5963, Figure 4) In the surface water values part of the Figure, Vgw must be 50% rather than
 80%.
- Answer from authors: We have plotted for 80% (SW) and 50% (GW) to better represent the changes. The
 surface water values are changing mostly at higher storage levels, while the groundwater values are not
 depending on the SW values. The figure caption wrongly states 50% this has been corrected.
- AR2.36) (page 5965, Figure 6) Do you mean Davidsen et al (2015) rather than Davidsen et al (2014)? If not,
 please add Davidsen et al (2014) to the reference list.
- Answer from authors: Yes, Davidsen et al (2015) is the correct citation. The paper was only published online
 (2014) when this manuscript was submitted. The reference has been corrected throughout the manuscript.
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400 Other minor changes

- The abstract have been updated to match the revised conclusion.
- Eq. (3) has been updated to match the style of the other equations (location of *t*).
- The units of P in Eq. (11)-(13) have been updated to avoid confusion (before P used both J/m³ and kWh/m³ as unit).
- The explanation and unit of the hydraulic conductivity has been updated.
- The result and discussion sections have been reorganized so that no results are presented in the discussion.
- Reference added in the caption to Table 1.
- In Figure 5, *V*_{max,gw} have been updated to match the used nomenclature.
- Grammar changes throughout the manuscript.

1 The cost of ending groundwater overdraft on the North 2 China Plain

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13 Abstract

Over-exploitation of groundwater reserves is a major environmental problem around the 14 world. In many river basins, groundwater and surface water are used conjunctively and joint 15 optimization strategies are required. A hydroeconomic modelling approach is used to find 16 17 cost-optimal sustainable surface water and groundwater allocation strategies for a river basin, given an arbitrary initial groundwater level in the aquifer. A simplified management problem 18 19 with conjunctive use of scarce surface water and groundwater under inflow and recharge 20 uncertainty is presented. Because of head-dependent groundwater pumping costs the 21 optimization problem is non-linear and non-convex, and a genetic algorithm is used to solve 22 the 1-step-ahead sub-problems with the objective of minimizing the sum of immediate and 23 expected future costs. A real-world application in the water-scarce Ziya River Basin in northern China is used to demonstrate the model capabilities. Persistent overdraft from the 24 25 groundwater aquifers on the North China Plain has caused declining groundwater tableslevels, 26 salinization and infiltration of wastewater. The model maps the opportunity marginal cost of 27 water in different scenarios, and the minimum cost of ending groundwater overdraft in the 28 basin is estimated to be 5.5847 billion CNY/year. The study shows that it is cost-effective to 29 slowly recover the groundwater aquifer to a level close to the surface while gradually 30 lowering the groundwater value to the equilibrium at 2.15 CNY/m³. The model can -be used

to guide decision makers to <u>economic efficient ensure long term</u> sustainabilityle of
 groundwater and surface water resources management in the basin in an economically optimal
 way.

34 **1** Introduction

35 Groundwater aquifers are of high economic importance around the world and often act as buffers in the water supply system during droughts (Tsur and Graham-Tomasi, 1991; Tsur, 36 1990). On the North China Plain, persistent groundwater overexploitation over the past 37 decades has caused decline of the shallow and deep groundwater tables (Liu et al., 2001). The 38 39 immediate benefits of satisfying the water demands greatly exceed the costs of pumping, 40 which highlights the problem of the present self-regulating management. As the groundwater resource is overexploited, the immediate benefits of the increased unsustainable supply have 41 42 to be traded off against the long term increase in pumping costs and reduced buffering 43 capacity. Optimal allocation of the water resources should address coordinated use of the water resources by considering the long term total costs while utilizing the groundwater as a 44 buffer. This is in line with the 2011 Chinese No. 1 Policy Document, which targets 45 improvement of the water use efficiency and reduction of water scarcity (CPC Central 46 Committee and State Council, 2010). 47

48 Optimal management of conjunctive use of surface water and groundwater has been addressed widely in the literature. Harou and Lund (2008) used a deterministic 49 50 hydroeconomic optimization approach to examine the economic effects of ending long-term 51 groundwater overdraft in California. The linear model was run under different scenarios and used to estimate the water users' willingness-to-pay, water scarcity costs and the benefits of 52 53 conjunctive use facilities. Andreu et al. (1996) developed the deterministic AQUATOOL simulation software based on the Out-of-Kilter Algorithm to minimize deficits in demand and 54 55 minimum flows in a coupled surface water- groundwater environment. This model was later applied in a hydroeconomic context by Pulido-Velázquez et al. (2006) to minimize the sum of 56 57 scarcity costs and variable operating costs for a coupled setup with a distributed-parameter groundwater simulation. The integrated aquifer model allowed variable pumping costs in a 58 59 forward moving, scenario-based framework but lacked the ability to give predictions in an 60 uncertain real-time management environment. An alternative optimization approach was demonstrated by Riegels et al. (2013), who maximized welfare subject to ecosystem 61 constraints by adjusting time-constant water prices. 62

While a high level of complexity can be accommodated in deterministic simulation models. 63 the objective functions of stochastic optimization models are kept simpler to remain 64 computationally feasible. Philbrick and Kitanidis (1998) applied Stochastic Dynamic 65 66 Programming (SDP) to a multi-reservoir system to optimize conjunctive use of surface water and groundwater given stochastic inflow. The second order gradient dynamic programming 67 method, a modification of the classical recursive SDP, was used to mitigate the well-known 68 69 curse of dimensionality. Pumping costs were linked linearly to pumping rates but changes in pumping costs due to long term depletion of the aquifer were not considered. Head-dependent 70 71 pumping costs were included in the SDP model by Knapp and Olson (1995), who analyzed 72 conjunctive use of groundwater with randomly generated runoff. Non-linearity arising from the head dependent pumping costs was overcome with lattice programming techniques in this 73 qualitative model setup. Optimal management of conjunctive use of surface water and 74 75 groundwater has been addressed widely in the literature (e.g Booker et al., 2012; Burt, 1964; 76 Knapp and Olson, 1995; Labadie, 2004; Noel and Howitt, 1982). While control-based 77 methods, such as Model Predictive Control (MPC, e.g. Morari and Lee, 1999; Mayne et al., 78 2000) and Reinforcement Learning (RL, Lee and Labadie 2007), focus on deriving real-time 79 optimal control policies, this study will focus on planning oriented optimization techniques. 80 Deterministic optimization problems for a given time horizon allow a detailed representation 81 of the groundwater system using spatially distributed groundwater models (Andreu et al., 1996; Harou and Lund, 2008; Marques et al., 2006; Pulido-Velázquez et al., 2006). 82 Stochasticity is commonly represented in scenarios where a regression analysis is used to 83 formulate operation rules, see e.g. the Implicit Stochastic Optimization (ISO) approaches 84 85 reviewed by Labadie (2004). Singh (2014) reviewed the use of simulation-optimization (SO) modeling for conjunctive groundwater and surface water use. In SO-based studies, efficient 86 groundwater simulation models are used to answer "what if"-questions while an optimization 87 model is wrapped around the simulation model to find "what is best". Groundwater aquifers 88 89 have been represented as simple deterministic box or "bathtub" models (e.g. Cai et al., 2001; Riegels et al., 2013) and as spatially distributed models (e.g. Maddock, 1972; Siegfried et al., 90 91 2009) with stochasticity (Reichard, 1995; Siegfried and Kinzelbach, 2006). While the results obtained from these methods are rich in detail, they yield only a single solution to the 92 93 optimization problem. 94 Dynamic Programming (DP, Bellman 1957) based methods have been used extensively been

95 to demonstrate the dynamics of conjunctive groundwater – surface water use for both

96 deterministic (e.g. Buras, 1963; Provencher and Burt, 1994; Yang et al., 2008) and stochastic 97 (SDP, e.g. Burt, 1964; Philbrick and Kitanidis, 1998; Provencher and Burt, 1994; Tsur and 98 Graham-Tomasi, 1991) optimization problems. In DP-based methods, the original 99 optimization problem is decomposed into subproblems which are solved sequentially over 100 time. The entire decision space is thereby mapped, enabling use of the results as dynamic 101 decision rules. However, the number of subproblems grows exponentially with the number of 102 state variables and this curse of dimensionality has frequently limited the use of DP and SDP 103 (Labadie, 2004; Provencher and Burt, 1994; Saad and Turgeon, 1988). Although it causes loss 104 of detail and inability to disaggregate the results, reservoir aggregation has been suggested as 105 one solution strategy (Saad and Turgeon, 1988).

This study aims to answer the following two macro-scale decision support questions for conjunctive groundwater and surface water management for the Ziya River Basin in North China: 1) what are the minimum costs of ending groundwater overdraft? and 2) what is the cost-efficient recovery strategy of the over-pumped aquifer? Ademonstrates how a hydroeconomic modeling approach can be used to identify the least-cost strategy to achieve 112 sustainable groundwater abstraction, defined as the. In this context, "sustainable" means that the long term average abstraction does not exceeding the long term average recharge. To 113 114 overcome Athe water management problem with conjunctive use of surface water and 115 groundwater similar to Harou and Lund (2008) with is addressed. I increased complexity is 116 caused by uncertain surface water runoff and groundwater recharge, and non-linearity arising 117 from head and rate dependent groundwater pumping costs. the surface water reservoirs are 118 aggregated. This is adequate at macro-scale (Davidsen et al., 2015) and allow use of dynamic 119 programming based approaches. The cost minimization problem is solved with the water 120 value method, a variant of SDP (Stage and Larsson, 1961; Stedinger et al., 1984) which 121 produces dynamic tables of marginal costs linked to states, stages and water source. Head and 122 rate dependent pumping costs introduce non-linearity in the <u>The non-linear</u> discrete sub 123 problems. This nonlinearity is are solvedhandled with a combined hybrid Ggenetic Aalgorithm (GA) - and Linear Pprogramming- (LP) method similar to that used by Cai et al. 124 125 (2001), but here applied to in a coupled groundwater-surface water management problem 126 within an SDP framework.

127 2 Methods

128 **2.1 Study area**

Northern China and particularly the North China Plain (NCP) have experienced increasing water scarcity problems over the past 50 years due to population growth, economic development and reduced precipitation (Liu and Xia, 2004). The deficit in the water balance has historically been covered by overexploitation of the groundwater aquifer, causing a regional lowering of the groundwater table by up to 1 m/year (Zheng et al., 2010).

134 The case study area is the Ziya River Basin, a part of the Hai River Basin, , which is located in the Hebei Province on the NCP and with the upper catchment stretching through the 135 Taihang Mountains into the Shanxi Provincewas selected as case study area (see Figure 1). 136 137 The upper basin is located in the Shanxi Province, while the lower basin is located in the Hebei Province on the NCP. The 52,300 km² basin has approximately 25 million inhabitants 138 139 (data from 2007, Bright et al., 2008), and is subject to severe water scarcity is causing, which causes multiple conflicts. The 52,300 km² basin, shown in , is home to approximately 25 140 million people . A hydroeconomic study of the Ziva River by focused primarily on optimal 141 management of the surface water resources. Five major reservoirs with a combined storage 142 capacity of 3.5 km³ are located in the basin. While reservoir rule curves and flood control 143 volumes can easily be accommodated, policies applied in practical management today were 144 145 not accessible for the case area. Instead it is assumed that the full storage capacity can be managed flexibly without consideration of storage reserved for flood protection or existing 146 147 management rules. Incorporating flood storage volumes will reduce the available storage and increase water scarcity in the long dry season. In the present model setup, we therefore find 148 the lower limit on water scarcity costs, assuming that the entire storage capacity is available 149 150 for storing water. Reservoir spills will cause an economic loss, and the model tends to avoid 151 spills by entering the rainy season with a low reservoir storage level.

In this study it is assumed that the full storage capacity can be managed flexibly without consideration of storage reserved for flood protection or existing management rules.

A previous hydroeconomic study of the Ziya River Basin was a traditional implementation of SDP on a single-reservoir system (surface water reservoir) and showed optimal water management, while disregarding dynamic groundwater storage and head-dependent groundwater pumping costs (Davidsen et al., 2015). Instead, tIn Davidsen et al, 2014, the groundwater resource was included as a simple monthly upper allocation constraint, which
 prevents analysis of dynamic interactions between the groundwater and surface water
 resources and limits the decision space.

161 In the present model setupstudy, the groundwater resource is included as a simple dynamic aquifer box model with a n-upper-storage capacity of 275 km³. The river basin has two 162 aquifers (upstream and downstream) which are only connected by the river. Ideally, each 163 164 aquifer should be modelled as a box model, but this extra state variable would be 165 computationally challenging within the SDP framework. We therefore set up a box model for 166 the downstream and most important aguifer only and abstraction from the upstream aguifer is only bounded by an upper pumping limit corresponding to the average monthly recharge. The 167 box model for the downstream aquifer is formulated as *Infiltration* + *Storage* = *Pumping* + 168 169 Overflow. The groundwater overflow is only used in extreme cases, where the total pumping 170 and available storage is less than the infiltration. The spills will go to the spill variable and 171 leave the system, as baseflow to the rivers (unavailable for allocation). The aquifer is so 172 heavily over-exploited that no significant baseflow is being created or will be created in any 173 foreseeable future. Thise box model allows for more flexible management with larger 174 abstractions in dry years and increased recharge in wet years. The groundwater aquifer can 175 thereby be used to bridge longer drought periods. Except from the groundwater box model, 176 the conceptual model is identical to the one used by (Davidsen et al., (2015)).

177 A conceptual sketch of the management problem is shown in Figure 2Figure 2. The water users are divided into groups of economic activities; irrigation agriculture, industrial and 178 179 domestic water users. Ideally, each water user group should be characterized by flexible 180 demand curves, but due to poor data availability Each water user group is characterized by a constant water demands (m³) and a constant curtailment costs of not meeting the demand 181 182 were used for each group (see <u>Table 1</u>), as also applied by. The water demands are 183 assumed to be deterministic and decoupled from the stochastic runoff. This is a reasonable 184 assumption because the rainfall on the NCP normally occurs in the summer months, while 185 irrigation water demands are concentrated in the dry spring. The irrigation schedule is centrally planned and typically unchanged from year to the same every year. The upstream (u) 186 users have access to runoff and are restricted to an upper pumping limit X_{gw} corresponding to 187 188 the average monthly upstream recharge, while The water users upstream the surface water 189 reservoir (u) have access to the runoff and a monthly limited volume of groundwater, . T the water users located downstream <u>users the reservoir</u> (d) have access to reservoir releases, water
delivered through the South-to-North Water Transfer Project (SNWTP) and groundwater
from the <u>dynamic downstream</u> aquifer.

193 **2.2 Optimization model formulation**

An SDP formulation is used to find the expected value of storing an incremental amount of 194 195 surface water or groundwater, given the month of the year, the available storage in surface and groundwater reservoirs and the inflow scenarios. The backward recursive equation 196 197 calculates the sum of immediate and expected future costs for all combinations of discrete 198 reservoir storage levels (states) and monthly time steps (stages). The immediate management 199 costs (IC) arise from water supply and water curtailment, whereas the expected future costs 200 (EFC) are the optimal value function in t+1 weighed by the corresponding transition 201 probabilities. In the present setup, we decided to weigh the IC and EFC equally, but inclusion 202 of discount rates other than zero is possible. Because of the head and rate dependent 203 groundwater pumping costs, which will be described in detail later, the immediate cost 204 depends non-linearly on the decision variables. The objective is to minimize the total costs over the planning period, given by the optimal value function $F_t^*(V_{gw,t}, V_{sw,t}, Q_{sw,t}^k)$ based on 205 206 the classical Bellman formulation:

207
$$F_{t}^{*}(V_{gw,t}, V_{sw,t}, Q_{sw,t}^{k}) = \min\left(IC(V_{gw,t}, V_{sw,t}, Q_{sw,t}^{k}) + \sum_{l=1}^{L} \left(p_{kl}F_{t+1}^{*}(V_{gw,t+1}, V_{sw,t+1}, Q_{sw,t+1}^{l})\right)\right)$$
(1)

208 with *IC* being the immediate costs:

209
$$IC(V_{gw,t}, V_{sw,t}, Q_{sw,t}^{k}) = \sum_{m=1}^{M} (c_{sw} x_{sw} + c_{gw} x_{gw} + c_{SNWTP} x_{SNWTP} + c_{ct} x_{ct})_{m,t} - r_{sw,t} b_{hp}$$
(2)

210 subject to:

211
$$x_{sw,m,t} + x_{gw,m,t} + x_{SNWTP,m,t} + x_{ct,m,t} = dm_{m,t}$$
 (3)

212
$$V_{sw,t} + Q_{sw,t} - \sum_{u=1}^{U} x_{sw,u,t} - r_{sw,t} - s_{gw,t} = V_{gw,t+1}$$
(4)

213
$$r_{sw,t} + s_{sw,t} = \sum_{d=1}^{D} x_{sw,d,t} + q_{E,t}$$
(5)

214
$$V_{gw,t} + Q_{gw,t} - \sum_{d=1}^{D} x_{gw,d,t} - s_{gw,t} = V_{gw,t+1}$$
(6)

215
$$\sum_{u=1}^{U} x_{sw,u,t} \le Q_{sw,t}$$
 (7)

216
$$\sum_{u=1}^{U} x_{gw,u,t} \le X_{gw,t}$$
 (8)

217
$$r_t \leq R, \quad x_{sw,Bei} + x_{SNWTP,Bei} \leq Q_{SNWTP}, \quad q_{E,t} \geq Q_E, \quad V_{sw,t} \leq V_{\max,sw}, \quad V_{gw,t} \leq V_{\max,gw}$$
(9)

218
$$c_{gw} = f\left(V_{gw}, \sum_{d=1}^{D} x_{sw,d}\right)$$
 (10)

219 <u>See Table 2 where for nomenclature.</u>

229

Eq. (3) is the water demand fulfillment constraint, i.e. the sum of water allocation and water 220 221 curtailments equals the water demand of each user. Eq. (4) is the water balance of the 222 combined surface water reservoir, while Eq. (5) is the water balance of the reservoir releases. 223 A similar water balance for the dynamic groundwater aquifer follows in Eq. (6). The upstream 224 surface water allocations are constrained to by the upstream runoff as shown in (Eq. (7), while 225 the upstream groundwater allocations are constrained to a fixed sustainable monthly average 226 as shown in (Eq. (8). In Eq. (9), the upper and lower hard constraints on the decision variables are shown. Last, Eq. (10) is the marginal groundwater pumping cost, which depends on the 227 228 combined downstream groundwater allocations as described later.

230 A rainfall-runoff model based on the Budyko Framework (Budyko, 1958; Zhang et al., 2008) 231 and has in a previous studyly been applied by (Davidsen et al., 2015) is used to estimate the 232 near-natural daily surface water runoff into reservoirs (Davidsen et al., 2015). The resulting 233 51 years (1958-2008) of simulated daily runoff are-was aggregated to monthly runoff and 234 normalized. A Markov chain, which describes the runoff serial correlation between three flow classes defined as dry $(0 - 20^{th} \text{ percentile})$, normal $(20^{th} - 80^{th} \text{ percentile})$, and wet $(80^{th} - 10^{th} \text{ percentile})$ 235 100th percentile), is was established and validated to ensure second order stationarity 236 237 (Davidsen et al., 2015; Loucks and van Beek, 2005). The groundwater recharge is estimated 238 from the precipitation data also used in the rainfall-runoff model. The average monthly 239 precipitation (mm/month) for each runoff class is calculated, and a simple groundwater 240 recharge coefficient of 17.5% of the precipitation (Wang et al., 2008) is used.

The SDP loop is initiated with EFC set to zero and will propagate backward in time through all the discrete system states as described in the objective function. For each discrete

combination of states, a cost minimization sub-problem will be solved. A sub-problem will 243 have the discrete reservoir storage levels ($V_{gw,t}$ and $V_{sw,t}$) as initial conditions and reservoir 244 245 inflow is given by the present inflow class in the Markov chain The optimization algorithm 246 will search for the optimal solution, given the costs of the immediate management (water 247 allocations and water curtailments, including reservoir releases and groundwater pumping), which have to be balanced against the expected future costs. As the SDP algorithm is 248 249 propagating backward in time, the future costs will be equal to the minimum total costs from 250 t+1, weighted by the Markov chain transition probabilities. The algorithm will continue 251 backward in time until equilibrium is reached, i.e. until the shadow prices (marginal value of 252 storing water for future use) in two successive years remain constant. The SDP model is developed in MATLAB (MathWorks Inc., 2013) and uses the fast cplexlp (IBM, 2013) to 253 254 solve the linear sub-problems.

The sets of equilibrium shadow prices, referred to as the water value tables, can subsequently be used to guide optimal water resources management forward in time with unknown future runoff. In this study, the available historic runoff time series <u>are-is</u> used to demonstrate how the derived water value tables should be used in real time operation. The simulation will be initiated from different initial groundwater aquifer storage levels, thereby demonstrating which pricing policy should be used to bring the NCP back into a sustainable state.

261 **2.3 Dynamic groundwater aquifer**

The groundwater aquifer is represented as a simple box model (see Figure 2Figure 2) with recharge and groundwater pumping determining the change in the stored volume of the aquifer (-(Eq. (6)). The pumping is associated with a pumping cost determined by the energy needed to lift the water from the groundwater table to the land surface (Eq. (10)):

$$266 \qquad P = \left(\rho g \Delta h\right) / \varepsilon \tag{11}$$

where *P* is the specific pump energy (J/m³), ρ is the density of water (kg/m³), *g* is the gravitational acceleration (m/s²), Δh is the head difference between groundwater table and land surface (m) and ε is the pump efficiency (-). The marginal pumping cost c_{gw} (CNY/m³) is found from the average electricity price c_{el} (CNY/Whs) in Northern China:

$$271 c_{gw} = c_{el}P (12)$$

Hence this cost will vary with the stored volume in the groundwater aquifer. The present electricity price structure in China is quite complex, with the users typically paying between 0.4 and 1 CNY/kWh depending on power source, province and consumer type (Li, 2012; Yu, 2011). In this study a fixed electricity price of 1 CNY/kWh is used. The immediate costs of supplying groundwater to a single user follow:

277
$$c_{gw,t} x_{gw,t} = \rho g \Delta h \varepsilon^{-1} c_{el} x_{gw,t}$$
(13)

where Δh is found as the mean depth from the land surface to the groundwater table (see Figure 2Figure 2) between *t* and *t*+1:

280
$$\Delta h = \Delta h_{top} + \left(V_{\max,gw} - \frac{V_{gw,t} + V_{gw,t+1}}{2} \right) S_y^{-1} A^{-1}$$
(14)

where Δh_{top} is the distance from the land surface to the top of the aquifer at full storage (m), S_Y is the specific yield (-) of the aquifer, and A is the area of the aquifer (m²). Here $V_{gw,t+1}$ is a decision variable, and once substituted into Eq. (13); it is clear that the problem becomes non-linear.

In Eq. (14) the drawdown is assumed uniform over the entire aquifer. This simplification might be problematic as the local cone of depression around each well could contribute significantly to the pumping cost and thereby the optimal policy. Therefore, the steady state Thiem drawdown (Thiem, 1906) solution is used to estimate local drawdown at the pumping wells. Local drawdown is then added to Eq. (15) to estimate total required lift:

290
$$\Delta h_{Thiem} = \frac{Q_w}{2\pi T} \ln\left(\frac{r_{in}}{r_w}\right)$$
(15)

where Q_w is the pumping rate of each well (m³/month), T is the transmissivity (m²/month), 291 r_{in} is the radius of influence (m)_a and r_w is the distance from origin to the point of interest 292 (m), here the radius of the well. The transmissivity is based on a hydraulic conductivity of 293 1.3.10⁻⁶ m²/month-s for silty loam (Qin et al., 2013). The hydraulic conductivity which is 294 295 lower than the expected average for the NCP to provide a conservative estimate of the effect of drawdown. was tested to be realistic in a MIKE SHE model of the ZRB (Marker, 2013). 296 Field interviews revealed that the wells typically reach no deeper than 200 m below surface, 297 which results in a specific yield of 5%. The thickness of the aquifergroundwater pumping Q_w 298

is defined as the total allocated groundwater within the stage (m³/month) <u>and</u>, <u>assumed evenly</u>
distributed evenly to the number of wells in the catchment:

301
$$Q_{w,t} = \frac{\sum_{d=1}^{D} x_{gw,d,t}}{n_w} = \frac{V_{gw,t} - V_{gw,t+1} + Q_{gw,t} - s_{gw,t}}{n_w}$$
(16)

where n_w is the number of wells in the downstream basin. The even pumping distribution is a 302 303 fair assumption, as field investigations showed that 1) the majority of the groundwater wells 304 are for irrigation, 2) the timing of irrigation, crop types and climate is homogeneous and 3) 305 the groundwater wells have comparable capacities. -Erlendsson (2014) estimatesd the well density in the Ziya River Basin from Google Earth to be 16 wells/km². Assuming that the 306 wells are distributed evenly on a regular grid and that the radius of influence r_{in} is 500 m, 307 308 overlapping cones of depression from 8 surrounding wells are included in the calculation of 309 the local drawdown. This additional drawdown is included using the principle of 310 superposition as also applied by Erlendsson (2014).

311

312 **2.4** Solving non-linear and non-convex sub-problems

With two reservoir state variables and a climate state variable, the number of discrete states is 313 quickly limited by the curse of dimensionality. A very fine discretization of the groundwater 314 aquifer to allow discrete storage levels and decisions is computationally infeasible. A low 315 number of discrete states increases the discretization error, particularly if both the initial and 316 the end storages $V_{gw,t+1}$ and $V_{sw,t+1}$ are kept discrete. The discrete volumes of the large aquifer 317 318 become much larger than the combined monthly demands, and storing all recharge will therefore not be sufficient to recharge to a higher discrete storage level. Similarly, the 319 demands will be smaller than the discrete volumes, and pumping the remaining water to reach 320 a lower discrete level would also be infeasible. Allowing free end storage in each subproblem 321 322 will allow the model to pick e.g. the optimal groundwater recharge and pumping without a 323 requirement of meeting an exact discrete end state. With free surface water and groundwater 324 end storages, the future cost function has three dimensions (surface water storage, groundwater storage and expected future costs). Pereira and Pinto (1991) used Benders' 325 decomposition approach, which employs piecewise linear approximations and requires 326 327 convexity. With head and rate dependent pumping costs and increasing electricity price, we 328 observed that the future cost function changes from strictly convex (very low electricity price)
329 to strictly concave (very high electricity price). At realistic electricity prices, we observed a
330 mix of concave and convex shapes. An alternative is to use linear interpolation with defined
331 upper and lower bounds. However, with two state variables, interpolation between the future
332 cost points will yield a hyperplane in three dimensions, which complicates establishment of
333 boundary conditions for each plane.

In the previous study by (Davidsen et al., 2015), the optimization problem was strictly linear and strictly convex. The individual sub-problems of the SDP scheme could therefore be solved with a fast linear programming algorithm. In this study however, with non-linearity from the head-dependent groundwater pumping costs, the expected future cost function is no longer strictly convex.

339 Non-linear optimization problems can be solved with evolutionary search methods, a sub 340 division of global optimizers. A widely used group of evolutionary search methods is are 341 genetic algorithms (genetic algorithms (GAs)), which have arebeen found to be efficient tools to for getting the approximate solutions to complex non-linear optimization problems (see. 342 343 e.g., Goldberg, 1989; Reeves, 1997). GAs use a random search approach inspired by natural 344 evolution and have been applied to the field of water resources management by, e.g., Cai et al. 345 (2001), McKinney and Lin (1994) and Nicklow et al. (2010). Cai et al. (2001) used a 346 combined genetic algorithmGA and linear programming (LP) approach to solve a highly non-347 linear surface water management problem. By fixing some of the complicating decision variables, the remaining objective function became linear and thereby solvable with LP. The 348 349 GA was used to test combinations of the fixed parameters while looking for the optimal 350 solution. The combination yielded faster computation time than if the GA was used to 351 estimate all the parameters.

352

A This study uses a genetic algorithm GA implemented in MATLAB is used to solve the cost minimization sub-problems. This GA function will initially generate a set of candidate solutions known as the *population*. Each of the candidate solutions contains a set of decision variables (sampled within the decision space), which will yield a feasible solution to the optimization problem. In MATLAB, a set of options specifies: the *population size*, the stopping criteria (*fitness limit, stall limit, function tolerance* and others), the *crossover fraction*, the *elite count* (number of top parents to be guaranteed survival) and the *generation* *function* (how the initial population is generated). The options were adjusted to achievemaximum efficiency of the GA for the present optimization problem.

362

The computation time for one single sub-problem is orders of magnitude larger than solving a 363 364 simple LP. As the optimization problem becameomes computationally heavier with increasing number of decision variables, a hybrid version of GA and LP, similar to the 365 366 method used by Cai et al. (2001), is was developed (see Figure 3 Figure 3). Decision variables 367 that cause non-linearity are identified and chosen by the GA. Once these complicating 368 decision variables are chosen, the remaining objective function becomes linear and thereby 369 solvable with LP. In the present optimization problem presented (in Eq. (1)), the non-linearity 370 is caused by the head-dependent pumping costs as explained in Eq. (13)-(14). Both the 371 regional lowering of the groundwater table and the Thiem local drawdown cones depend on the decision variable for the stored volume in $t+1, V_{gw,t+1}$. If $V_{gw,t+1}$ is pre-selected, the 372 373 regional drawdown is given, and the resulting groundwater pumping rate Q_w can be calculated from the water balance. The groundwater pumping price is thereby also given, and 374 375 the remaining optimization problem becomes linear.

376 The SDP framework is subject to the curse of dimensionality. With two state variables and 377 non-linearity, the computation time is significant and is a limiting factor when choosing 378 model discretization. With a low number of discrete states, the discretization error increases, 379 particularly if the end storages $V_{gw,t+1}$ and $V_{sw,t+1}$ are kept discrete. Piecewise linear 380 interpolation of the future cost function (Pereira and Pinto, 1991) allows for free end storages but requires strict convexity. With two state variables, interpolation between the future cost 381 points will yield a hyper-plane in three dimensions. In our problem, the EFC is no longer 382 strictly convex and therefore both V_{gwt+1} and V_{gwt+1} are chosen by the GA. 383

For a given combination of stages, discrete states and flow classes, the objective of the GA is to minimize the total costs, TC, with the free states $V_{gw,t+1}, V_{sw,t+1}$ being the decisions:

386
$$TC(V_{gw,t+1}, V_{sw,t+1}) = \min IC(V_{gw,t+1}, V_{sw,t+1}) + EFC(V_{gw,t+1}, V_{sw,t+1})$$
(17)

with *EFC* being the expected future costs. Given initial states and once the GA has chosen
the end states, the immediate cost minimization problem becomes linear and hence solvable
with LP (see <u>Figure 3</u>Figure 3). The IBM CPLEX linear programming solver is used to solve

390 the linear programs. The expected future costs are found by cubic interpolation of the discrete 391 neighboring future cost grid points in each dimension of the matrix. The GA approaches the 392 global optimum until a fitness limit criteria is met. The total costs are stored, and the 393 algorithm continues to the next state. To reduce the computation time, the outer loop through 394 the groundwater states is parallelized.

The performance of the GA-SDP model is compared to a <u>fully_deterministic Dynamic</u> Program (DP)<u>DP</u>, which finds the optimal solution given perfect knowledge about future inflows and groundwater recharge. The DP model uses the same algorithm as the SDP model and 1-dimensional state transition matrices with p=1 between the deterministic monthly runoff data. For low storage capacity and long time scales, the effect of the end storage volume becomes negligible. Similar to the SDP model, the DP model was looped and run until the end of period condition does not affect the present management.

402 **3 Results**

403 Without any regulation or consideration of the expected future costs arising from over-404 exploitation of the groundwater aquifer, the water users will continue maximizing immediate 405 profits (producers) or utility (consumers). Because there are only electricity costs for groundwater, the users will continue pumping groundwater until the marginal groundwater 406 407 cost exceeds the curtailment costs. At 1 CNY/kWh the marginal cost of lifting groundwater 408 200 m (typical depth of wells observed in the study area) can be found with Eq. (13)-(14) to be 0.8 CNY/m^3 and thereby less than the lowest curtailment cost at 2.3 CNY/m^3 . It requires 409 410 an electricity price higher than 2.8 CNY/kWh before the lowest-value user stops pumping 411 from 200 m below surface.

412 The backward recursive SDP algorithm was run with a looped annual dataset until 413 equilibrium water values, i.e. no inter-annual changes, were obtained. The water values 414 increase fastest during the first years, and after approximately 100 years the annual increases become small. Due to the large storage capacity of the groundwater aquifer, equilibrium is 415 416 however not achieved until after 150-180 years. These marginal water values represent the true values of storing a unit volume of water for later use, and vary with reservoir storage 417 418 levels, runoff flow class and time of the year. A sample of the resulting equilibrium water 419 value tables are presented in Figure 4. This figure shows the temporal variations of water 420 values as a function of one state variable, keeping the other state variable at a fixed value. The 421 state variables are fixed at empty, half full and full storage respectively. During the rainy

422 season from June to August, high precipitation rates reduce water scarcity, resulting in lower 423 the surface water values. Because the groundwater storage capacity is much larger, increased 424 recharge can easily be stored for later use, and groundwater values are therefore not affected. 425 Addition of stream-aquifer interactions to the model is expected to affect this behavior, but 426 since the flow in rivers/canals in the case study area is small most of the year, and since most areas are far from a river, it is a reasonable assumption to ignore these dynamics. -The water 427 428 values after 1980 are clearly higher than in the period before 1980 due to increased water 429 scarcity caused by a reduction in the regional precipitation. In contrast, the groundwater value 430 tables are uniform, with variation only with groundwater storage. The detailed water value 431 tables are included as supplementary information.

432 We simulate management using the equilibrium water value tables as decision rulespricing 433 policy and force the system with 51 years of simulated historical runoff. Time series of the 434 simulated groundwater storage levels can be seen in Figure 5 for different initial storage 435 scenarios for the dynamic groundwater aquifer. The groundwater aquifer approaches an equilibrium storage level around 260 km³ (95% full). If the storage in the aquifer is below this 436 437 level, the average recharge will exceed average pumping until the equilibrium storage is 438 reached. If the storage level is above equilibrium, average pumping will exceed average 439 recharge and over time-until equilibrium storage is reached. In Figure 6, the surface water and groundwater storages are shown for a situation with equilibrium groundwater storage. In most 440 years, the surface water storage falls below 1 km³, leaving space in the reservoir for the rainy 441 season. The potential high scarcity costs of facing a dry scenario with an almost empty 442 reservoir is avoided by pumping more groundwater. These additional pumping costs seem to 443 444 be exceeded by the benefits of minimizing spills in the rainy season.

445 The surface water reservoir storage level varies over time, and in contrast to the findings by 446 (Davidsen et al., 2015) the storage capacity now becomes close to zero almost every year. 447 This can be explained by the increased groundwater availability in the model, which allows increased groundwater allocation in multi-annual dry periods. To To demonstrate the 448 449 business-as-usual solution, the simulation model is run for a 20 year period with the present water demands and curtailment costs and with a discount rate set to infinity (= zero future 450 451 costs). The resulting groundwater table is continuously decreasing as shown in Figure 5Figure 452 5.

453 In the simulated management runs, water will be allocated to the users up to a point where 454 reductions in immediate cost are compensated by increases in expected future costs. The 455 user's price, which can be applied in an opportunity marginal cost pricing (MCP) scheme, is 456 the marginal value of the last unit of water allocated to the users. The user's price is the sum 457 of the actual pumping cost (electricity used) and the additional opportunity marginal cost given by the equilibrium water value tables. In Figure 7Figure 6, the user's prices for 458 459 groundwater and surface water are shown for the 51 year simulation at and below the long 460 term sustainable groundwater storage level. When the groundwater storage level is close to 461 equilibrium, the user's prices of groundwater and surface water are equal during periods with 462 water scarcity. In wet months with reduced water scarcity, the model switches to surface 463 water allocation only, and the groundwater user's price is undefined (gaps in the time series in 464 Figure 7Figure 6). If the groundwater storage level is below equilibrium, the groundwater 465 user's price will be higher causing an increase in water curtailments and increasing storage 466 level as shown in Figure 5Figure 5. Under these circumstances the surface water user's price 467 increases up to a point where the two prices meet. With an initial aquifer storage at one third of the aquifer capacity (100 km³), the groundwater value is 3 CNY/m³ (see Figure 7). As the 468 469 aquifer slowly recovers, the groundwater price decreases gradually.

470 At the equilibrium groundwater storage level, the user's prices for groundwater is stable around 2.15 CNY/m³ as shown in Figure 7Figure 6. This indicates frequent curtailment of 471 wheat agriculture in the downstream Hebei Province, which has a willingness to pay of 2.312472 CNY/m^3 (see Table 1). The allocation pattern to this user is shown in Figure 8Figure 7: the 473 474 model switches between high curtailment and high allocations, depending on water 475 availability and storage in the reservoirs. Groundwater allocations fluctuate between 476 satisfying 0% and 80% of the demand. Inclusion of the steady state Thiem drawdown cones in 477 the optimization model increases the marginal groundwater pumping cost with increased pumping rates. Groundwater allocations are distributed more evenly over the months, which 478 479 results in less local drawdown. The total curtailments remain constant, while 1% of the total 480 water abstraction is shifted from groundwater to surface water, if the stationary Thiem 481 drawdown is included. Inclusion of well drawdown significantly changed the simulated 482 management but resulted in only slightly increased computation time.

<sup>The average total costs of the 51 years simulation for different scenarios can be seen in Table
<u>3. The average reduction in the total costs, associated with the introduction of the SNWTP</u></sup>

485 canal can be used to estimate the expected marginal economic impact of the SNWTP water. 486 The minimum total costs after the SNWTP is put in operation are compared to the scenario without the SNWTP (pre-2008) and divided by the allocated SNWTP water. The resulting 487 488 marginal value of the SNWTP water delivered from Shijiazhuang to Beijing (2008-2014 scenario) is 3.2 CNY/m³, while the SNWTP water from Yangtze River (post-2014 scenario) 489 reduces the total costs with 4.9 CNY/m³. Similarly, a comparison of the total costs for the 490 post-2014 scenarios shows a marginal increase of 0.91 CNY/m³ as a consequence of 491 introducing a minimum in-stream flow constraint. 492

493 A simple local sensitivity analysis is used to assess the uncertainty of the model. used Monte 494 Carlo simulations based on 50 samples to estimate the uncertainty of the model outputs. However, the inclusion of an additional state variable has increased the optimization time 495 496 significantly and made such an approach infeasible. Approximately 4000 CPU hours per 497 climate period are needed to reach equilibrium in the present model, equivalent to two weeks, 498 if the maximum of 12 parallel processors are used in MATLAB R2013a. TheA local 499 sensitivity analysis was focused on the local sensitivity related to the water demands and 500 water curtailment costs used directly in the objective function (-(Eq. (1))) - and the 501 transmissivity used to estimate the local drawdown (Eq. (14)). The uncertain input parameters 502 were increased by 10% and the sensitivity evaluated based on the simulation results. The resulting total costs can be seen in Table 3. A 10% increase in the curtailment costs is 503 504 returned as a 6.0% increase in the total costs, while a similar increase of the demands 505 generates a 2.1% increase in costs. The transmissivity can vary over many orders of 506 magnitude because it is a log-normally distributed variable. The sensitivity of log(T) is high: a 1.3% change of log(T) from the baseline value results in a 1.5% change in the cost. The 507 benchmark DP run was run for the post-2014 scenario with Thiem drawdown and minimum 508 509 ecosystem flow constraint. The minimum total costs of this run is $8.46 \cdot 10^9$ CNY/year. This is 1.3% lower than the equivalent SDP run ($8.56 \cdot 10^9$ CNY/year). 510

511 The minimum total costs were lowered from 10.50 billion CNY/year (Davidsen et al., 2015)
512 to 8.56 billion CNY/year (18% reduction) by allowing the groundwater aquifer to be utilized
513 as a buffer instead of a fixed monthly volume. This difference highlights the problem of
514 defining realistic boundaries to optimization problems and shows that simple hard constraints,
515 here fixed groundwater pumping limits, can highly limit the optimal decision space. With a
516 dynamic groundwater aquifer, the model can mitigate dry periods and stabilize the user's

price of surface water as shown in Figure 7. Finally, policies like minimum in-stream 517 518 ecosystem flow constraints can be satisfied with less impact on the expensive users. The total 519 costs without restrictions on the groundwater pumping have been estimated to 2.98 billion 520 CNY/year (Davidsen et al., 2015). To end the groundwater overdraft in the basin, the present 521 study thus estimates a cost increase of 5.58 billion CNY/year, once the groundwater aquifer is 522 at equilibrium storage. The cost of recharging the aquifer from the present storage level below 523 the equilibrium is significantly higher. In Table 3, the LGW scenario shows that the average cost of sustainable management from an initial storage at 100 km³ (one third full) is 13.32 524 525 billion CNY/year. 526 From any initial groundwater reservoir storage level, the model brings the groundwater table to an equilibrium storage level at approximately 95% of the aquifer storage capacity. Only 527

528 small variations in the aquifer storage level are observed after the storage level reaches 529 equilibrium as shown in Figure 6. While addition of the Thiem stationary drawdown has only 530 a small effect on total costs and total allocated water, it is clear from Figure 8 that the 531 additional Thiem drawdown highly impacts the allocation pattern to some of the water users. 532 High groundwater pumping rates result in larger local drawdown and thus in higher pumping 533 costs. This mechanism leads to a more uniform groundwater pumping strategy, which is 534 clearly seen in Figure 8 and results in much more realistic management policy.

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537 4 Discussion

This study presents a hydroeconomic optimization approach that provides <u>macro-scale</u> economic <u>pricing policydecision rules</u> in terms of water values for <u>joint-conjunctive</u> surface water – groundwater management. The method was used to demonstrate how the water resources in the Ziya River Basin should be priced over time, to reach a sustainable situation at minimum cost. We believe that the presented modelling framework has great potential use as a robust decision support tool in real-time water management. However, a number of limitations and simplifications need to be discussed.

A great advantage of SDP-based water value method is the capability to obtain optimal
decision rules for any combination of system states. A first limitation of the approach is the
high level of simplification needed. There are two main reasons for the high level of

548 simplification: Limited data availability and the limitations of the SDP method. SDP-549 approach is, however, the curse of dimensionality as previously mentioned. The curse of 550 dimensionalitynumber of sub-problems to be solved in the backward moving SDP scheme 551 increases exponentially with the number of state variables. In our case we are therefore 552 limiteds the approach to 2-3 inter-linked storage facilities and higher dimensional management problems will not be computationally feasible with SDP today. This limit on the 553 554 number of surface water reservoirs and groundwater aquifers requires a strongly simplified 555 representation of the real world situation in the optimization model. These requirements can 556 be relaxed in tThe simulation phase that followsing the optimization. While the curse of 557 dimensionality applies to the backward moving SDP scheme, the forward moving simulation 558 is not limited to the same extent, as since just only one a single sub-problem is solved at each 559 stage. The water values determined by the SDP scheme can thus be used to simulate management using a much more spatially resolved model with a high number of users; this 560 561 was not demonstrated in this study. The advantage of SDP is that it provides a complete set of 562 decision rules pricing policies that can be applied in adaptive management, provided that the 563 system can be simplified to a computationally feasible level. An alternative approach known 564 as stochastic dual dynamic programming (SDDP, Pereira and Pinto, 1991; Pereira et al., 565 1998) has shown great potential for multi-reservoir river basin water management problems. 566 Instead of sampling the entire decision space with the same accuracy level, SDDP samples 567 with a variable accuracy not pre-defined in a grid, focusing the highest accuracy around the 568 optimal solution. This variable accuracy makes SDDP less suitable However, because SDDP only samples around the optimal decisions, this method will not be able to provide the 569 570 complete set of shadow prices for all state combinations and is therefore less suitable for adaptive management. Despite the highly simplified system representation, we believe that 571 572 the modeling framework provides interesting and non-trivial insights, which are extremely 573 valuable for water resources management on the NCP. Computation time was a a major limitation limitation -in this study. Three factors increased the 574

574 Computation time was <u>a a major limitation</u> limitation in this study. Three factors increased the 575 computational load of the optimization model: 1) inclusion of the groundwater state variable 576 resulted in an exponential growth of the number of subproblems; 2) the non-convexity 577 handled by the slower GA-LP formulation caused an increase in the computation time of 10-578 100 times a single LP; and 3) the SDP algorithm needed to iterate through more than 200 579 years to reach steady-state. A single scenario run required 4,000 CPU hours and was solved in 580 two weeks using 12 cores at the high performance cluster (HPC) at the Technical University 581 of Denmark. This is 50,000 times more CPU hours than a single reservoir SDP model
582 (Davidsen et al., 2015). Since the water value tables can be used offline in the decision
583 making, this long computation time can be accepted.

584 The long computation time made the use of, e.g., Monte Carlo-based uncertainty analysis infeasible. The local sensitivity analysis showed that a 10% increase in the curtailment costs 585 is returned as a 6.0% increase in the total costs, while a similar increase of the demands 586 587 generates a 2.1% increase in costs. The transmissivity can vary over many orders of magnitude because it is a log-normally distributed variable. The sensitivity of log(T) is high: 588 589 <u>a 1.3% change of $\log(T)$ from the baseline value results in a 1.5% change in the cost. At the</u> 590 same time, the simple system representation needed in SDP required assumptions of inflow 591 and storage discretization, aggregation of the surface water reservoirs, generalized estimates 592 of pumping cost and a lumped groundwater model which all contribute to the uncertainty. 593 Further, poor data availability for the case study area required some rough estimates of the 594 natural water availability, single-point demand curves and perfect correlation between rainfall 595 and groundwater recharge. The method-driven assumptions generally limit the decision 596 support to basin-scale, while the simple estimates caused by poor data availability contribute 597 to raising the general uncertainty of the model results. Given the computational challenges 598 and the diverse and significant uncertainties, the model results should be seen as a 599 demonstration of the model capabilities rather than precise cost estimates. Better estimates will require access to a more comprehensive case dataset and involve a complete sensitivity 600 601 analysis.

and the transition from the previous much simpler linear single state SDP model (Davidsen et
al., 2015) to the presented non-linear SDP model with two state variables proved to require
around 50,000 times more CPU hours. We used the high performance cluster (HPC) at the
Technical University of Denmark to solve the SDP, and as the optimization can be run
offline, an optimization time of 2 weeks on 12 cores can be accepted.

Replacement of the hard upper groundwater pumping constraint used by (Davidsen et al.,
2015) with a dynamic groundwater aquifer, lowered the total costs from 11.39 billion
CNY/year to 8.47 billion CNY/year. This difference highlights the problem of defining
realistic boundaries to optimization problems and shows that simple hard constraints, here
fixed groundwater pumping limits, can highly limit the optimal decision space. With inclusion
of a dynamic groundwater aquifer, the model can use the large groundwater storage capacity

as a buffer to the system, which significantly stabilizes the user's price of surface water as
shown in Figure 6. Finally, policies like minimum ecosystem flow constraints can be satisfied
with less impact on the expensive users, which results in reductions in the respective shadow
prices.

617 Another addition to the modelling framework was the Thiem stationary drawdown. The long 618 time steps (monthly) make stationarity a realistic assumption. Inclusion of well drawdown 619 significantly changed the simulated management but resulted in only slightly increased 620 computation time. While addition of the Thiem stationary drawdown has only a small effect 621 on total costs and total allocated water, it is clear from Figure 7 that the additional Thiem 622 drawdown highly impacts the allocation pattern to some of the water users. High groundwater pumping rates result in larger local drawdown and thus in higher pumping costs. This 623 624 mechanism leads to a more uniform groundwater pumping strategy, which is clearly seen in Figure 7 and results in much more realistic management policy. 625

626 From any initial groundwater reservoir storage level, the sustainable management brings the 627 groundwater table to an equilibrium storage level at approximately 95% of the aquifer storage capacity. Only small variations in the aquifer storage level are observed after the storage level 628 629 reaches equilibrium. Intuitively, one would expect the equilibrium groundwater storage level 630 to be as close as possible to full capacity, while still ensuring that any incoming groundwater 631 recharge can be stored. Finding the exact equilibrium groundwater storage level would 632 require a very fine storage discretization, which, given the size of the groundwater storage, is 633 computationally infeasible. Therefore the equilibrium groundwater storage level is subject to 634 significant discretization errors. The long time steps (monthly) make the stationarity required 635 for using the Thiem stationary drawdown method a realistic assumption.

636 In the previous study by (Davidsen et al., 2015), total costs, without restrictions on the 637 groundwater pumping, were estimated to 3.09 billion CNY/year. The present study estimates 638 an increase to 8.56 billion CNY/year for a comparable setup but with sustainable groundwater 639 pumping and groundwater storage at equilibrium. This increase of 5.47 billion CNY/year 640 reflects the expected cost of ending the groundwater overdraft in the basin once the groundwater aquifer is at equilibrium storage. The cost of recharging the aquifer from the 641 present storage level below the equilibrium is significantly higher. In Table , the LGW 642 scenario show that the average cost of sustainable management from an initial storage at 100 643 km³ (one third full) is 13.32 billion CNY/year. 644

645 The difference between total costs with SDP and with DP (perfect foresight) is surprisingly 646 small (1.3%). While this difference indicates a very good performance of SDP, the model 647 setup also simulates small economic consequences of wrong decisions. With the SNWTP in 648 operation (post-2014) the most expensive user Apart from Beijing, which has (Beijing) will 649 always have access to enough the SNWTP water, and the remaining downstream users have 650 unlimited access to groundwater. The large downstream groundwater aquifer serves as a 651 buffer to the system and eliminates the economic consequences of a wrong decision. If too much water is allocated to a user in month 1, the same user will simply receive a bit less water 652 653 in a following time step. The model almost empties the reservoir every year as shown in 654 Figure 6, Wand wrong decisions are therefore not punished with curtailment of expensive 655 users as observed by (Davidsen et al., (2015). The groundwater aquifer reduces the effect of wrong decisions by allowing the model to minimize spills from the reservoir without 656 657 significant economic impact of facing a dry period with an empty reservoir. A dynamic 658 groundwater aquifer thereby makes the decision support more robust, since it is the timing 659 and not the amount of curtailment being affected but will shift allocations in time and between the users with curtailment costs close to the long term equilibrium water price (in 660 661 this study the farmers). The inclusion of a dynamic groundwater aquifer thereby makes the model self-regulate, . as periods with too strict policy will be compensated by periods with a 662 663 more unrestrained policy. The robustness is also supported by the simple local sensitivity analysis. The impact of changes in the input parameters on the total costs is small, as it is 664 665 mainly the timing and not the amount of curtailment being affected. Inclusion of the large groundwater aquifer reduces the effect of wrong timing, which is reflected in small 666 667 differences between the total costs with and without perfect foresight.

668 The derived equilibrium groundwater value tables in Figure 4Figure 4 (and the supplementary 669 detailed water value tables); show that the groundwater values vary with groundwater 670 storage alone and are independent of time of the year, the inflow and recharge scenario and 671 the storage in the surface water reservoir. This finding is important for future work, as a 672 substitution of the groundwater values with a simpler cost function could greatly reduce the 673 number of states and thereby the computation time. The equilibrium groundwater price, i.e. 674 the groundwater values around the long term equilibrium groundwater storage, can possibly 675 be estimated from the total renewable water and the water demands ahead of the optimization, 676 but further work is required to test this. Further work should also address the effect of 677 discounting of the future costs on the equilibrium water value tables and the long term steady

state groundwater table. In the present model setup, the large groundwater aquifer storage
capacity forces the backward moving SDP algorithm to run through 200-250 model years,
until the water values converge to the long term equilibrium. Another great improvement,
given the availability of the required data if data allow, would be to replace the constant water
demands with elastic demand curves in the highly flexible GA-LP setup.

683 A significant impact of including groundwater as a dynamic aquifer is the more stable user's 684 prices shown in Figure 7Figure 6. The user's price of groundwater consists of two parts: the 685 immediate groundwater pumping costs (electricity costs) and the expected future costs 686 represented by the groundwater value for the last allocated unit of water. As the model is run 687 to equilibrium, the user's prices converge towards the long term equilibrium at approximately 2.2 CNY/m³. This long term equilibrium is not affected by the actual electricity price, as 688 689 increasing electricity prices will be offset by a similar reduction in the opportunity costs. 690 TheA constant electricity price can therefore be used as a policy tool to internalize the user's 691 prices of groundwater shown in Figure 7Figure 6. Stable water user's prices will ease the 692 implementation of e.g. an opportunity cost pricing (OCP) MCP scheme, which is one of the 693 available policy options to enforce long-term sustainability of groundwater management.

5 Conclusions

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This study presented describes development and application of how a hydroeconomic 696 697 optimization approach to optimally manage conjunctive use of groundwater and surface 698 water. The model determines the water allocation, reservoir operation and groundwater 699 pumping that minimizes the long-term sum of head and rate dependent groundwater pumping 700 costs and water curtailment costs. The model is used to quantify potential savings of joint 701 water management of the Ziva River Basin in Northern China, but the model can be applied 702 to other basins as well. Estimates of natural runoff, groundwater recharge, water demands and marginal user curtailment costs are cast into a SDP-based optimization framework. Regional 703 704 and Thiem stationary drawdown is used to estimate rate and head dependent marginal 705 groundwater pumping costs. The resulting optimization subproblems become nonlinear and 706 non-convex and are solved with a hybrid GA-LP setup. A central outcome from the SDP 707 framework is tables of shadow prices of surface and groundwater for any combination of 708 time, inflow class and reservoir storage. These tables represent a complete set of pricing 709 policies for any combination of system states and can be used to guide real-time water

management. Despite a significant computational demand to extract the water value tables,
 the method provides a suitable approach for basin-scale decision support for conjunctive
 groundwater and surface water management.

713 The model provides useful insight to basin-scale scarcity-driven tradeoffs. The model outputs time series of optimal reservoir storage, groundwater pumping, water allocation and the 714 715 marginal economic value of the water resources at each time step. The model is used to derive 716 a pricing policy to bring the overexploited groundwater aquifer back to a long-term sustainable state. The economic efficient recovery policy is found by trading off the 717 718 immediate costs of water scarcity with the long term additional costs of a large groundwater 719 head. From an initial storage at one third of the aquifer capacity, the average costs of ending groundwater overdraft are estimated to be 13.32 billion CNY/year. The long-term cost-720 721 effective reservoir policy is to slowly recover the groundwater aquifer to a level close to the 722 surface by gradually lowering the groundwater value from an initial level of 3 CNY/m³. Once 723 at this sustainable state, the groundwater values are almost constant at 2.15 CNY/m³ which suggests that wheat agriculture should generally be curtailed under periods with water 724 725 scarcity. The dynamic groundwater aquifer serves as a buffer to the system and is used to 726 bridge the water resources to multiple years. The average annual total costs are reduced with 727 18% to 8.56 billion CNY compared to a simpler formulation with fixed monthly pumping 728 limits. The stable user's prices are suitable to guide a policy scheme based on water prices 729 and the method has great potential as basin-scale decision support tool in the context of the China No. 1 Policy Document. can be used to derive a pricing policy to bring an 730 overexploited groundwater aquifer back to a long-term sustainable state. The model quantifies 731 732 potential savings of joint water management of a complex river basin in China. Surface water and groundwater management was optimized in a SDP framework based on a coupled GA-LP 733 734 setup. The derived equilibrium water value tables represent the shadow prices of surface and groundwater for any combination of time, inflow class and reservoir storage. The 735 736 groundwater values at equilibrium were found to be almost constant at 2.2 CNY/m³, 737 independent of the time of the year, the surface water storage and the inflow class. Non-738 convexity caused by the groundwater reservoir could be accommodated with the use of a GA 739 and was further extended to include stationary Thiem local drawdown cones. Inclusion of a 740 dynamic groundwater aquifer greatly reduced the total costs of water scarcity, compared to a setup with fixed monthly pumping limits. The sustainable management will recharge the 741 742 aquifer until the equilibrium storage level is reached. From an initial storage at one third of

the aquifer capacity, the average costs of ending groundwater overdraft are estimated to be 743 744 13.32 billion CNY/year. After equilibrium is reached, the average costs are estimated to be 745 5.47 billion CNY/year. The aquifer serves as buffer and allows for overexploitation in dry 746 years and this mechanism stabilizes the user's water prices. These stable user's prices are 747 suitable for use in an OCP scheme. While the representation of the management problem 748 must be kept simple in the optimization model, the OCP prices can be used to drive a much 749 more detailed simulation model, which includes a detailed physical representation of the 750 system.

751 6 Acknowledgements

752 S. Liu and X. Mo were supported by the grant of the Natural Science Foundation of China 753 grants (31171451, 41471026). Key Project for the Strategic Science Plan in Institute of Geographic Sciences and Natural Resources Research, Chinese Academy of Sciences 754 755 (2012ZD003). The authors thank the numerous farmers and water managers in the Ziya River 756 Basin for sharing their experiences; L. S. Andersen from the China-EU Water Platform for 757 sharing his strong willingness to assist with his expert insight from China; and K. N. Marker 758 and L. B. Erlendsson for their extensive work on a related approach early in the development 759 of the presented optimization framework.

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Table 1: Annual water demands and curtailment costs for the users in the Ziya River **<u>Bbasin</u>**. 943 944 Based on the dataset from Davidsen et al. (2015).

	Upstream	Downstream				
Water demands $(10^6 \text{ m}^3/\text{monthyear})$						
Industries	539	543	а			
Domestic	223	864	b			
Maize	569	1,522	c			
Wheat	-	6,089	c			
Beijing	-	1,000	d			
Ecosystems	-	100	e			
Total	1,331	10,119				
Curtailment costs (CNY/m ³)						
Industries	5.3	5.3	f			
Domestic	3.2	3.2	f			
Maize	1.8	2.8	g			
Wheat	-	2.1	g			
Beijing	-	5.5	h			

^aDemands scaled with area, (Berkoff, 2003; Moiwo et al., 2010; World Bank, 2001)

^bBased on daily water demand (National Bureau of Statistics of China, 2011) scaled with the 2007 population from Landscan (Bright et al., 2008)

^cBased on the land cover (USGS, 2013) and irrigation practices collected in the field. The wheat irrigation demand is evenly distributed in March, April, May and June. Maize is irrigated in July.

^dBased on plan by The People's Government of Hebei Province (2012), (Ivanova, 2011)

^eEstimated deficit in the Baiyangdian Lake (Honge, 2006)

^fEstimate by World Bank (2001)

945 946 947 948 949 950 951 952 953 954 955 ^gBased on the water use efficiency (Deng et al., 2006) and producers' prices (USDA Foreign Agricultural Service, 2012)

^hEstimate by Berkoff (2003)

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F_t^*	<u>optimal value function in stage t (2005 Chinese Yuan, CNY)</u>
$V_{gw,t}$	stored volume in the groundwater aquifer, decision variable (m ³)
$V_{sw,t}$	stored volume in the surface water reservoir, decision variable (m ³)
V _{max,sv}	v_{w} upper storage capacity, surface water reservoir (m ³)
$V_{\max,g}$	- w upper storage capacity, groundwater aquifer (m ³)
$Q_{sw,t}$	river runoff upstream reservoirs, stochastic variable (m ³ /month)
$Q_{gw,t}$	groundwater recharge, assumed to be perfectly correlated with $Q_{sw,t}$ (m ³ /month
m	indicates the <u>M</u> water users
gw_	groundwater
SW	surface water
<u>ct</u>	water curtailments
<u>x</u>	allocated volume, decision variable (m ³ /month)
С	marginal costs (CNY/m ³). The costs are all constants, except for c_{gw} whi
	correlated to the specific pump energy. See Eq. (11)-(16)
r_t	reservoir releases through hydropower turbines, decision variable (m ³ /month)
<u>R</u>	upper surface water reservoir turbine capacity (m ³ /month)
S _{sw}	reservoir releases exceeding <u>R</u> , decision variable (m ³ /month)
b_{hp}	marginal hydropower benefits (CNY/m ³)
k	indexes the <u>K</u> inflow classes in stage t
<u>l</u>	indexes the <u>L</u> inflow classes in $t+1$
p_{kl}	transition probability from k to l
dm_m	water demand for user <u>m (m³/month)</u>
u	indexes the U upstream users
d	indexes the D downstream users
	<u>spills from aquifer when</u> $V_{gw,t} + Q_{sw,t} - x_{gw,t} > V_{max,sw}$ (m ³ /month)
X_{gw}	maximum monthly groundwater pumping in the upstream basin (m ³ /month)
$q_{E,t}$	unused surface water available to ecosystems, decision variable (m ³ /month)
$Q_{\scriptscriptstyle E}$	minimum in-stream ecosystem flow constraint (m ³ /month)
Bei	Beijing user

990 Table 3: Average minimum total costs (TC) and hydropower benefits (HP) over the 51 year 991 planning period , hydropower benefits and shadow prices for different scenario runs. SNWTP 992 scenarios: Ppre 2008 = before the SNWTP canal, 2008 - 2014 = SNWTP partly finished 993 (emergency plancanal from Shijiazhuang to Beijing), Ppost 2014 = SNWTP finished (water canal from Yangtze <u>River</u> to Beijing.), <u>Scenarios:</u> -LGW is are results from a run with initial 994 995 groundwater storage at 100 km³ below equilibrium (100 km³, all other scenarios are initiated 996 at equilibrium groundwater storage);- (dm) is are the results with 10 % higher water demands; 997 $\frac{1000}{1000}$ is are with 10% higher curtailment costs; and (T) is with 10% higher transmissivity; 998 **TD** =-is Thiem steady state drawdown-included; -E =-is minimum ecosystem flow constraint; 999 "+" is active and "-" is inactive. (to Baiyangdian Lake), TC = minimum total costs over the planning period (51 years tested), b_{hp} = marginal hydropower benefits, DP = dynamic 1000 1001 programming (perfect foresight), SP - shadow price.

SNWTP scenario	Scenario settings			TC SDP	<mark>b_{hp}HP</mark> SDP
	Special run	TD	Е	10 ⁹ CNY/y	10 ⁶ CNY/y
pre-2008	2	+	+	14.87	103.6
2008-2014	z –	+	+	11.69	103.5
post-2014	z –	±.	Ξ	8.43	103.5
post-2014	=	+	Ξ	8.47	103.6
post-2014	=	+	+	8.56	104.3
post-2014	LGW	+	+	13.32	99.2
post-2014	Т	+	+	8.69	103.5
post-2014	dm	+	+	8.74	103.3
post-2014	ct	+	+	9.08	103.1

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Figure 1: The Ziya River Basin. Watershed and rivers automatically delineated from a digital
elevation map (USGS, 2004) and manually verified and corrected with Google Earth (Google
Inc., 2013). The SNWTP route-s (Central and Eastern) were sketched in Google Earth and
verified with field observations. Provincial boundaries from (NGCC, 2009).



Figure 2: Conceptual sketch of the Ziya River Basin management problem with water users located upstream (u) and downstream (d) the surface water reservoir. Allocation decision variables are indicated for surface water (blue), SNWTP water (green) and groundwater (orange). A conceptual sketch of the downstream dynamic aquifer is included and show how the total lift (Δ h) is composed of the top layer + the regional groundwater lowering + the local Thiem steady state groundwater drawdown.

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Load data for all stages for all runoff flow classes for all surface water states for all groundwater states Upper and lower bounds for GA Generate initial population $(V_{gw,t+1}, V_{sw,t+1})$ Calculate immediate costs (LP) Interpolate future costs New generation Calculate total costs Mutation Calculate fitness Crossover ↓ no Stopping critera met? yes Store total costs next 1021 1022 Figure 3: SDP optimization algorithm design_ 1023 1024 1025



Figure 4: Temporal changes of the water values (CNY/m^3) for the climate period before 1980. The marginal water value is the true value of storing a unit volume of water for later use, and varies with reservoir storage levels, runoff flow class and time of the year. a) Surface water values at fixed [0%, <u>58</u>0%, 100%] groundwater aquifer storage. b) Groundwater values over time at fixed [0%, 50%, 100%] surface reservoir. The reservoir storage is shown from E (empty) to F (full).

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1038 different initial groundwater tables (0, 100, 200, 258 and 275 km³). The perfect foresight DP 1039 and management without consideration of the future (FC = 0) are also shown.

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1060 realistic drawdown model, which includes the stationary Thiem local drawdown cones.