

Quantitative historical hydrology in Europe

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Abstract. In recent decades, the quantification of flood hydrological characteristics (peak discharge, hydrograph shape, and runoff volume) from documentary evidence has gained scientific recognition as a method to lengthen flood records of rare and extreme events. This paper describes the methodological evolution of quantitative historical hydrology under the influence of developments in hydraulics and statistics. In the 19th century, discharge calculations based on flood marks were the only source of hydrological data for engineering design, but were later left aside in favour of systematic gauge records and conventional hydrological procedures. In the last two decades, there is growing scientific and public interest in understanding long-term patterns of rare floods, in maintaining the flood heritage and memory of extremes, and to develop methods for deterministic and statistical application to different scientific and engineering problems. A compilation of 45 case studies across Europe with reconstructed discharges demonstrates that 1) in most cases present flood magnitudes are not unusual within the context of the last millennium, although recent floods may exceed past floods in some temperate European rivers (e.g. the Vltava and Po rivers); 2) frequency of extreme floods have decreased since the 1950s, although some rivers (e.g. the Gardon and Ouse rivers) show a reactivation of rare events over the last two decades. There is a great potential of gaining understanding of individual extreme events based on a combined multiproxy approach (palaeoflood and documentary records) providing high-resolution time flood series and their environmental and climatic changes; and to develop non-systematic and non-stationary statistical models based on relations of past floods with external and internal covariates under natural low-frequency climate variability.

Key words: Historical hydrology, historical flood, palaeohydrology, extreme flood, flood hazard.

43 **1 Introduction**

44 Historical hydrology is the study of the hydrological cycle before the continuous
45 instrumental recordings on the basis of highly-resolved man-made documentary
46 evidence (Brázdil et al., 2006b, 2012). Most of the documented pre-instrumental records
47 refer to hydrological extremes (floods and droughts) that produced major disruption on
48 past societies. In this context, historical floods have been frequently reported through
49 written, pictorial and epigraphic documentation across Europe (Brázdil et al., 2006b,
50 2012; Herget, 2012). Historical hydrology is on the interface between hydrology and
51 environmental history. Based on data derived from documentary sources (non-
52 instrumental human observations), its analysis involves the use not only historical-
53 archival methods, but of hydrological modelling, and stochastic frequency analysis.
54 Traditionally, the collection of historical flood information has been mainly addressed
55 within the field of historical climatology together with other natural phenomena such as
56 heavy rains, windstorms, snowfalls and droughts (Brázdil et al., 2005b). Over the last
57 20 years, the study of historical floods has gained recognition in Europe as key to
58 understanding the natural hazards dynamics and their response to climate variability.
59 Some major efforts were done in the topic within the European projects SPHERE
60 (Systematic, Palaeoflood and Historical data for improvEment of flood Risk Estimation;
61 Benito et al., 2004) and the FLOODCHANGE Advance Grant (Deciphering River
62 Flood Change; Kiss et al., 2015). The possibility of extending river records towards the
63 past have opened new perspectives in the study of extreme hydrological events whose
64 analysis in terms of return periods, variability and tendency to clustering requires long
65 hydrological data sets (Hall et al., 2014). However, documentary flood data are, in most
66 cases, descriptive information, limited to a location (at human settlements), depending
67 upon human perception (caused damages) and eventually bias by the political,
68 legislative and administrative (local, regional and national) contexts. Recent advances
69 on hydrological and hydraulic modelling and statistical-mathematical methods allow
70 better dealing with the uncertain and categorical data characteristic of historical floods
71 developing new applications in the study of flood hazards and climate change studies.

72

73 In Europe, historical documentary flood sources go back to Roman times (Camuffo and
74 Enzi, 1996) although continuous and homogeneous written archives are known to be
75 available only for the past 500 years (Brázdil et al., 2005a; Glaser et al., 2010).
76 European richness on flood historical documents is only comparable to China with
77 100,000 reports from 8000 localities, although quantitative description of the flood
78 hydrographs in Chinese rivers didn't start till the 18th century (Luo, 1987). China
79 documents on historical floods date back in some cases to 2000 years ago, with detailed
80 descriptions over the recent 600 years and complete and homogenous data over the last
81 200 years (Luo, 1987). The oldest historic nearly continuous flood record is not to be
82 found in Europe or Asia, but in Africa, in ancient Egypt (Popper, 1951) whose
83 economic wealth depended on the flooding of the River Nile and the annual deposition
84 of fertile sediments along the river flood plain. In the Nilometer of Egypt, flood levels

85 were observed and measured since 3000 BC. In the USA, historical floods include
86 information about extreme hydrological events observed in staff gauges (water-level
87 readings), and therefore data derived from non-recording, attendant-read, staff gauges
88 are traditionally considered historical data (Cook, 1987). Early stages of hydrology in
89 Europe (1780–1860) involved human observations of water level readings on staff
90 gauges, which were not always continuous throughout the year. This problem was
91 gradually solved with the modernisation of gauge stations that made possible to record
92 continuous flow stage on counters and data-loggers. This early instrumental period, that
93 started around the 18th century and comprehends the initial flood observations at staff
94 gauges, has been considered part of the historical hydrology (Brázdil et al., 2012).

95

96 Another source of long-term pre-instrumental floods data, though sometimes with a
97 lower time resolution, are sedimentary and botanical records, known as palaeofloods (cf.
98 Baker, 2008; Fig. 1). Palaeostage indicators include various types of geologic evidence
99 (flood deposits and geomorphic features) and woody debris, as well as morphologies
100 related to direct physical damages on riverine vegetation (e.g. scars on trees and tilting;
101 Herget, 2012; Benito and Díez-Herrero, 2015). A number of studies have combined
102 both historical and geological indicators to ascertain the magnitude and frequency of
103 past flood, increasing the robustness of the frequency analysis of rare floods (Benito et
104 al., 2010).

105

106 This paper aims to describe the different techniques and approaches used in order to
107 obtain quantitative information from historical flood data, as well as to draw attention to
108 its different scientific and engineering applications. The specific targets are (1) to
109 describe the historical flood data sources leading to robust estimations of long-term
110 flood discharge records, (2) to review different techniques used for reconstructing the
111 magnitude and frequency of specific past floods using documentary evidence, (3) to
112 describe the approaches used in flood frequency analysis with historical discharges, and
113 (4) to illustrate how historical quantitative hydrology can contribute in the solution of
114 environmental and engineering problems.

115

116 **2 Quantitative historical hydrology**

117 The primary goal of historical hydrology is to collect information on past extreme
118 floods such as date, relative magnitude, damages and socio-economic impacts at the
119 time (Brázdil et al., 2006b, 2012; Glaser et al., 2010). Most recently, there has been a
120 growing interest on quantifying these descriptive data in the reconstruction of flow
121 depths, discharges and hydraulic properties associated to historical flooding (Fig. 1).
122 This quantification may be numerical (peak flow) or categorical (damage classification).
123 Regarding numerical data, the observed flood-water levels associated to a given
124 historical flood can be transformed by hydraulic calculations into velocities and
125 discharges in a procedure analogous to the depth-discharge relationships used to
126 determine the flow rate in gauge stations (Cook, 1987; Benito et al., 2004; Herget et al.,

127 2014). Flood magnitudes can also be classified in terms of resulting damages, or social
128 impacts (e.g. Sturm et al., 2001). The use of secular records for the analysis of possible
129 changes in the magnitude and frequency of individual floods at specific sites or for
130 specific rivers can support not only flood hazard assessment analysis, but enable the
131 identification of interconnections between flood frequency and severity and climate,
132 land-use and river morphology (Macdonald and Black, 2010). Moreover, historical
133 flood data often includes other less commonly used information and data regarding the
134 societal and economic consequences of these natural disasters (Coeur, 2003). Yet, this
135 valuable data on the role of floods, through time, on local and national societal and
136 policy changes are still an unexplored field that can bring new insights on the public
137 perception of risk.

138
139 The use of documentary flood data in hydrological studies usually comprises four
140 phases of analysis: (1) compilation and assessment of flood dates and water levels; (2)
141 classification of events according to flood water-level (exact stage), described
142 inundation zones (minimum or maximum flood level) and from reported damages; (3)
143 estimation of flood magnitude, usually peak discharge, associated with documented and
144 site-observed evidence; and finally (4) use of historical flood data in the flood frequency
145 analysis. The implementation of the first two initial analysis steps entails a previous
146 command in historical archives research, both written and cartographical that will not
147 only produce a record of historical floods, but will also identify flooded sites and
148 morphological changes on river channel and floodplain in the course of historical
149 analysed period. The third task requires the implementation of hydraulic and
150 hydrodynamic analyses, mainly drawn from engineering applications, to assign a flow
151 magnitude derived from documentary evidence. Finally, historical estimated discharge
152 data can be merged with instrumental records in a flood frequency analysis in order to
153 determine discharges associated to probability quantiles.

154 155 **3 History of hydraulics and early flood estimations in Europe**

156 The first hydraulic parameter described in rivers was the water-level reached during
157 extraordinary events. Ancient Romans observed and recorded flood water levels at
158 bridges (e.g. Albenga, 1940), and the Roman engineers used this knowledge for
159 designing their infrastructures (Lorenz and Wolfram, 2011). The study of flood levels
160 was important ensuring efficient and long lasting hydraulic structures, and leave this
161 legacy to future generations as it can be read in the inscription placed at the 30 m high
162 Alcantara Bridge (Spain): “*Pontem perpetui mansurum in saecula mundi*” (Bridge to
163 last forever in the perpetuity of the world; Fernández-Casado, 2008). Nowadays, the
164 preservation of written records about flood data from antiquity is anecdotal since most
165 written records were destroyed. During Medieval times, water marks associated with
166 large floods were made on bridges, houses and even on bedrock outcrops although in a
167 non-systematic way (Brázdil, 1998; Deutsch and Pörtge, 2009; Brázdil et al., 2012).
168 Systematic water-level readings at gauges didn’t start before the later 18th century,

169 namely in Germany, France, Austria and Czech Lands. A review about the history of
170 these first gauge water-level readings in Europe was compiled and published by Brázdil
171 et al. (2012), though at the national level several papers have been published recently
172 regarding the development of discharge measurements (e.g. for northern Germany,
173 Deutsch, 2010). The revolutionary step on flood hydrology records which enabled to
174 register and calculate flow discharge after stage level at gauge stations didn't take place
175 till the 19th century with the measurement of flow velocity. The first rotor current meter
176 was developed by Woltman (1790) and underwent several improvements during the
177 19th and 20th centuries (Lanser, 1953). Current-meter gauging stations permitted the
178 measurement of the flow rating curve (depth-discharge relationship) at the first
179 established gauge stations in European rivers (Fig. 2). Rating curves were established
180 and rated most reliably for low-to-moderate flows. The extreme flow discharges were
181 frequently obtained from extrapolation of the rating curve. As this approach is less
182 reliable and implies numerous uncertainties due to missing calibration, discharges
183 associated to high flood levels were estimated by hydraulic formulae. Hence,
184 reconstructions of historical floods gained robustness simultaneously to the advance in
185 hydraulic research.

186

187 The first equations accounting for resistance law in open channels were established
188 during the late 18th and 19th century. In 1775 the French engineer Antonie Chézy
189 (1718–1789) proposed the first resistance formula based on a study of the water transfer
190 from the River Yvette to Paris by an earthen canal (Herschel, 1897). Chézy's formula
191 can be derived mathematically from two assumptions, as described by Chow (1959).
192 First, Chézy assumed that the force resisting the flow due to friction per unit area is
193 proportional to the square of the velocity V^2 multiplied by a constant of proportionality
194 K , the length of the canal L , and the perimeter P of the section in contact with the water,
195 i.e. KV^2LP . The second assumption in Chézy's formula is equality of the total force of
196 resistance to the effective gravity-force component which is parallel to the channel
197 bottom, namely ωALS , where ω is the unit weight of water, A is the cross-section area,
198 L is the channel length and S the slope. Since $\omega ALS = KV^2 PL$, where A/P is the
199 hydraulic radius R and $\sqrt{\omega/K}$ can be replaced by a factor C , then it is obtained Chézy's
200 equation as:

201

$$V = C\sqrt{RS} \quad (1)$$

202

203 In this formula, the factor C is the main uncertainty for velocity calculations estimated
204 for known river cross-sections by indirect methods or assumptions. For instance, the
205 first discharge estimates of the 1857-flood in the Ardèche (France) were obtained from
206 multiplying the calculated velocity by 0.7 to reflect the unequal distribution of flow rate
207 and channel roughness (De Mardigny, 1860). During the 19th century different
208 experiments were performed to determine the involved variables in Chézy's factor C ,
among which the most relevant were proposed by Ganguillet and Kutter (1869) and

209 Bazin (1897). Henri Emile Bazin (1829–1917) conducted laboratory studies on channels
210 made on cement, brick, wood and rock proposing a formula where C is a factor of R :

$$211 \quad C = \frac{87}{1 + m/\sqrt{R}} \quad (2)$$

212 where m is a roughness coefficient that varies between 0.06 for canals made of concrete
213 to 3.17 for earth channels with rough conditions (Bazin, 1897).

214

215 In 1868, Philippe Gascard Gauckler (1826–1905), the engineer at *Ponts et Chaussées*,
216 proposed two formulae for the estimation of the flow velocity V as:

$$217 \quad V = \lambda_1 R^{4/3} S \quad \text{for } S > 0.0007 \quad (3)$$

$$218 \quad V = \lambda_2 R^{2/3} S^{1/2} \quad \text{for } S < 0.0007 \quad (4)$$

219 where λ_1 and λ_2 are coefficients describing the boundary roughness. The second formula
220 (4) applies for laminar flow regime, that may also be expressed as $\lambda_2 = 1/n$ being n a
221 roughness parameter, as it was proposed later by the Irish engineer Robert Manning
222 (1816–1897), although apparently Manning was unaware of Gauckler's work. The
223 popular Manning equation (Manning, 1891) is expressed in metric units as

$$224 \quad V = KR^{2/3} S^{1/2} \quad (5)$$

225 where K is a factor of flow resistance that later was modified to $1/n$, where n is known
226 as Manning's roughness coefficient. Later, Strickler (1923) proposed a new expression
227 of the Chézy's C coefficient $C = KR^{1/6}$ that applied in the Chézy's formula provides a
228 similar expression of the Gauckler-Manning's formula (5). There is still an open debate
229 on the significance of the different contributions to the still recently frequently applied
230 approach (Williams, 1970; Dooge, 1992; Hager, 2005). Consequently, this equation is
231 also named Gauckler–Manning–Strickler formula. Another fundamental set of hydraulic
232 equations for unsteady open channel flow was formulated by Barré de Saint-Venant in
233 1843 who published the correct derivation of the Navier-Stokes equations identifying
234 the coefficient of viscosity and its role in the local acceleration and fluid turbulence
235 (Anderson, 1997).

236

237 The Gauckler–Manning formula has been used extensively since early estimates of
238 historical floods to calculate the mean flow velocity at sections with observed historical
239 flood levels (Pardé, 1925b). During the first half of the 20th century the research on
240 historical floods was developed with a great influence of physical geographers. Maurice
241 Pardé (1893–1973), Professor of Potomology in Grenoble, was probably the most
242 prolific European author in the study of extreme historical floods, with over three-
243 hundred papers and two-thousand hand-written notes and letters on the subject
244 including a vast compilation of documentary floods worldwide. In Austria, the first
245 known publications calculating discharges using historic flood-marks were carried out
246 by Schwarzl (1956) and Kresser (1950, 1957). The highest flood level marked on public
247 buildings and passage near the river corresponds to the 1501-flood estimated as circa
248 $14,000 \text{ m}^3\text{s}^{-1}$ in Engelhartzell in Upper Austria (Kresser, 1957). In Italy, early

249 discharge estimations from historical flood levels were carried out by engineers at the
250 service of water authorities, and data were reported as internal publications (e.g. HOPR,
251 1935; Giovannelli and Allodi, 1960's, cited in Zanchettin et al., 2008). In the River Po,
252 the 1857, 1868, and 1872 flood levels were studied in the context of large flooding
253 recorded during the early decades of the 20th century (Visentini, 1936, 1938; Visentini
254 and Pardé, 1936), concluding that historical flood peaks were of lower magnitude than
255 contemporaneous extreme events.

256

257 These early 20th century advances on hydraulic models and stream flow measurements
258 supported the re-estimations of discharge associated with certain large historical floods
259 For instance, in the River Isère the 1740-flood discharge originally estimated as 1844
260 m^3s^{-1} by the engineer Christophe Dausse in 1780 (Lang et al., 2003) was later revised in
261 $2000 \text{ m}^3\text{s}^{-1}$ by Pardé (1925b) using Chézy and Gauckler equations. Pardé (1961)
262 provided the first worldwide inventory of flood discharges including a large number of
263 pre-instrumental floods, many obtained from letters and unpublished reports with a
264 reliability difficult to verify. The compilation of discharges from these early historical
265 flood studies resulted in the plotting of regional envelope curves of maximum flood
266 peaks or discharge per unit area versus drainage area (Wundt, 1949; Pardé, 1961;
267 Francou and Rodier, 1967). The envelope curve is a deterministic method to transpose
268 space-for-time under the assumption that there is a limit of precipitation supplied to a
269 basin under a given climatic and geographic domain (Myers, 1967).

270

271 The study of past floods declined since the mid-20th century as databases of stream
272 flow measurements supported by statistical analysis provided standard hydrological
273 methods for flood hazard applications on which historical extremes were considered
274 anecdotal, imprecise and outliers in relation to systematic gauged records (Klemeš,
275 1989).

276

277 **4 Quantitative historical flood records: approaches and methodology**

278 Most of the early historical flood discharge estimates were obtained from flood marks
279 using hydraulic equations under the assumption of uniform flow conditions. However,
280 there is a large amount of documentary evidence providing descriptive evidence of past
281 inundation levels and flood damages (impacts) that offer a quantitative reference of the
282 associated flood discharges after a critical analysis and interpretation (Benito et al.,
283 2004). For the past 20 years, there has been a growing interest on reconstructing flood
284 chronologies and their discharge estimates from documentary descriptions of flood
285 water level (Benito et al., 2003a). The reconstruction of long historical flood records
286 from documentary sources relies upon the availability of historic data for model
287 implementation and calibration, uncertainty on the past river topography, and detailed
288 configuration of the river channel and vegetation for roughness characterisation.
289 Extracting quantitative hydrological data from documentary evidences leads typically to
290 two phases of analysis: (1) documentation and assessment of documentary evidence of

291 flood stage during specific historical floods and (2) relating of identified flood evidence
292 to flood discharge, based on hydraulic calculations. Documentary evidence of flood
293 water level includes flood marks, with indication of single or multiple flood levels (Fig.
294 3), and relative water levels obtained from narrative descriptions from places affected
295 by flooding (e.g. a church, one of several streets, singular buildings). By the
296 computation from water level to discharge, several correlative water surface evidences
297 (marks, inundated sites) of a historical flood are matched to a water surface profile of a
298 known discharge, obtained from hydraulic modelling. A major problem for this
299 hydraulic analysis is the reconstruction of river channel geometry at the time of flooding,
300 which produces a high uncertainty on the discharge calculations mainly in alluvial
301 rivers (i.e. mobile river bed). The existence of historical maps showing the channel
302 morphology at the flood time can be used to reconstruct its former morphology. The
303 historical flood estimated discharges are then structured into different threshold levels
304 that were exceeded by floodwaters over specific periods of time, the input data
305 necessary for flood frequency analysis (Fig. 1b).

306

307 **4.1 Documentary data sources and types**

308 Several review papers have described the main data sources of historical hydrology
309 (Brázdil et al., 2006b, 2012). They are often grouped into three data categories: hand-
310 written documents (e.g. narrative sources, administrative and ecclesiastic reports, and
311 personal correspondence); printed sources (special prints newspapers, reports and
312 technical papers); iconographic sources (stone-marks, historical photographs and
313 paintings, old cartography and cross-sections).

314

315 Narrative descriptions about floods may be biased by perception, both from the writer
316 and from the present day researcher (Brázdil et al., 2006b). Since information from
317 documentary sources is mainly qualitative, several classifications have been suggested
318 taking into account the severity of flood impacts (Sturm et al., 2001). Barriendos et al.
319 (2003) proposed a qualitative classification of flood severity for records prior to the pre-
320 instrumental period, taking as reference the channel overflow: ordinary flood - when
321 water remains within the channel and banks; extraordinary flood - resulting in localised
322 overbank flow, with any damage but without major destruction; and catastrophic flood -
323 with inundation resulting in general damage and destruction of infrastructures.

324

325 A study of historical hydrology involves: (1) collection of documentary sources (e.g.
326 municipal, ecclesiastical and private archives) and consideration of already compiled
327 information (e.g. books, databases and reports); (2) compilation of instrumental data for
328 the area of investigation (rainfall, flow and level of the river, synoptic information); (3)
329 collection of graphic information (e.g. paintings, photographs); (4) cross-reference of
330 historical and palaeoflood (geological records) information, instrumental data and
331 graphic information. It is convenient to produce a standardised data form to be filled for

332 each documentary flood reference (Fernandez de Villalta et al., 2001; Casas et al., 2003;
333 Barriendos et al., 2003; Barriendos et al., 2014).

334

335 As mentioned above, early studies on historical floods were carried out mainly at sites
336 with flood marks, for which peak flow values were calculated. However, most of the
337 documented historical flood information is not recorded as engraved flood marks (plates
338 or inscriptions) but as descriptions of inundations that affected out-of-channel areas.
339 This is the case for instance of the River Ter (NE Spain) for which a record of 170
340 floods was compiled for the period between 1322–1987 (Barriendos and Martin-Vide,
341 1998), from which only 77 floods were found to be registered in plates or wall
342 inscriptions. The majority of the flood events mentioned in documentary sources were
343 nevertheless associated to a comprehensive description of the sites, or streets affected
344 by the flood water-level. The reconstruction of flood discharges from descriptive flood
345 levels beyond rough extrapolations and estimations (e.g. Schiller, 1987) was addressed
346 firstly for the River Tagus in central Spain (Benito et al., 2003a). Discharges associated
347 with documentary-based floods were reconstructed at four places, namely in Aranjuez
348 (since AD 1557), Toledo (AD 1113), Talavera (AD 1203) and Alcántara (AD 1856).
349 The hydraulic analysis and interpretation of the flood level from historical documents
350 was inspired by methods commonly used in palaeoflood hydrology (Baker, 2008).
351 Flood levels associated with documentary data at these locations include: (1) flood
352 marks on houses, mills, monasteries and bridges; (2) descriptions of flooded areas as
353 orchards, roads, streets; (3) descriptions of non-flooded areas (e.g. singular building
354 surrounded by water but non-inundated); (4) relative flood level with respect to
355 previous floods (e.g. the 1840 flood was 2 m higher than the flood occurring in 1820).
356 The interpretation of these flood water level indicators provides four different discharge
357 information records: (1) highest water level or peak discharge (equal to the flood stage),
358 (2) minimum flood discharge, (3) maximum flood discharge, and (4) discharge quoted
359 as a range in the case of two recorded levels. Field work is required to accurately locate
360 (GPS survey) the sites referred in the historical documents (location of buildings, streets,
361 bridges, gates, walls, etc.), as well as to ascertain the altitudes of the referred flood
362 stages or levels. Thus, for all documentary evidences along the study reach, the flood
363 height can be estimated and the associated flood discharge reconstructed (Fig. 4).
364 Similar studies have been conducted later in other European sites (Table 1) with well
365 documented floods and a rich historical archive. New methodological approaches were
366 also developed as in the case of the study conducted by Roggenkamp and Herget (2014)
367 for the River Ahr at Ahrweiler (Germany). The hydrograph of the 1910-flood was
368 reconstructed based on sequenced historic photographs showing the same inundated
369 street with a street clock hanging on a wall of a building, which precisely linked time
370 and flood water level.

371

372 **4.2 Discharge estimation from documentary records**

373 The most critical component of applied historical flood hydrology is the estimation of
374 discharge associated to documented floods. The flow estimates from hydraulic analysis
375 is usually based on the elevation of flooded or non-flooded sites and epigraphic marks
376 relative to local channel geometry. The approaches used to assess discharge estimation
377 from known flood-water levels vary from simple hydraulic formula to the results
378 derived from the running of one or multi-dimensional hydraulic models (Kutija, 2003;
379 Lang et al., 2004). Most historical flood studies assume a one-dimensional flow with
380 calculations based on (1) uniform flow equations (e.g. Gauckler–Manning equation), (2)
381 gradually varied flow models (e.g. standard step method calculation), and (3) one
382 dimensional Saint-Venant equations. In complex reaches, multi-dimensional modelling
383 may reduce uncertainties associated with reconstructing flood discharge (Denlinger et
384 al., 2002).

385

386 The Gauckler–Manning equation is applied for uniform, steady and one-dimensional
387 flow conditions of straight channels of even gradient and regular width (Chow, 1959).
388 In most hydraulic computations, the result of the calculated discharge depends on the
389 uncertainty in the selection of the roughness parameter, changes on cross-section
390 topography and urban configuration on the floodplains since historic times. The typical
391 method for estimating Manning's n is obtained from reference tables, from examination
392 of photographs of typical channels whose roughness coefficients are known, or based on
393 the experience of the researcher in similar river settings. Herget et al. (2014) proposed a
394 method based on the Manning equation in which discharge is calculated separately for
395 individual homogeneous units of the inundated cross-section area. At each sub-section,
396 the intervenient parameters of the Manning equation (R , S , n) are assessed at the time of
397 the historical flooding based on old maps and written descriptions. The uncertainty on
398 the estimation of roughness and of hydraulic geometry is introduced as different
399 scenarios based on varying assumptions considered. Herget and Meurs (2010) applied
400 this method to the 1374 flood of the River Rhine in Cologne, the highest in the local
401 record. The calculated discharge was validated by application of this equation on recent
402 floods and comparison of results with nearby cross-sections. Since flow in natural
403 channels is typically not uniform, large errors can be expected when the Gauckler-
404 Manning equation is applied to a single flood mark and one cross-section. The
405 separation of the cross-section area into more or less homogenous units reduces this
406 problem significantly. This approach cannot be used for floods caused by ice-jam or
407 those with temporal bridge obstruction by woody debris raising the flow level instead of
408 an increased discharge (Herget et al., 2014).

409

410 The most common historical flood discharge calculations are applied to gradually-
411 varied flow conditions (Benito et al., 2003a; Lang et al., 2004; Naullet et al., 2005).
412 River channel geometry is generally irregular in shape and surface roughness resulting
413 in non-uniform flow conditions. Gradually-varied flow analyses usually assume a
414 steady state (constant discharge) for which flow depth varies with distance but not with

415 time (Chow, 1959). The typical approach relating historical flood evidence to discharge
416 uses the step-backwater method for gradually-varied water-surface profile computation
417 (Benito et al., 2003a). In this method, water-surface profiles are calculated from the
418 resolution of the conservation of mass and energy equations in their one-dimensional
419 forms. Available public-domain computer routines, such as the U.S. Army Corps of
420 Engineers HEC-RAS (Hydrologic Engineering Center, 2010), provides computation of
421 water-surface profiles for specified discharges, and energy loss coefficients. Multiple
422 analyses give synthetic rating curves at sites of interest, thus providing a basis for
423 calculating historical flood discharge from the elevation of a water mark, known
424 inundated locations or other high-water evidence (Thorndycraft et al., 2006).
425 Uncertainties in flow modelling variables can be assessed for their resulting influence in
426 historical flood discharges by testing outcomes of plausible ranges of Manning's n
427 values and possible changes in channel geometry. Challenging for this approach is the
428 demand for several quantified cross-sections along a valley which are usually hard to
429 determine from historic descriptions. Consequently, the variation of the geometry and
430 roughness parameters along a valley can only be assumed.

431

432 Recent advances in two-dimensional computing flow hydraulics (Kutija, 2003) have
433 been considered for historical flood studies (Fernandez Bono and Grau-Gimeno, 2003;
434 Calenda et al., 2005). In alluvial rivers, flow over the banks show a three dimensional
435 behaviour and this should be analysed by two-/three-dimensional models. However,
436 already even 2D-modelling requires a large amount of high resolution channel and
437 floodplain topographic information to define the working mesh as well as detail data
438 about changes in historical topography after construction of buildings and roads, as well
439 as spatial variability of roughness.

440

441 Flood hydrographs are essential for different engineering applications including dam
442 operation and safety (Swain et al., 2006). The few essays to obtain hydrographs from
443 palaeoflood studies have used probabilistic hydrographs (England et al., 2003; Benito et
444 al., 2011). Recently, Elleder (2010) reconstructed the February 1784 flood of the River
445 Vltava in Prague based on peak flood marks, daily newspapers and explanatory notes
446 accompanying early instrumental measurements on the Klementinum observatory. The
447 hydrograph showed only 45 hr time to peak in Prague with a 4 m water level rise during
448 a 12 hours, a steep rise exceeded only by the August 2002 flood (Brázdil et al., 2005a,
449 2006a) (Fig. 5).

450

451 **4.3 Assumptions and uncertainty evaluation of the estimated historical discharges**

452 The reconstruction of historical flood records is subject to assumptions, limitations and
453 uncertainties that may affect the interpretation of the number of floods and estimated
454 discharge. A key element in this quantitative analysis is the transformation of known
455 information of flow level to accurate discharge estimates. In this task, it is of critical
456 importance to confirm that the identified flood marks and sites used as flood level

457 indicators are not removed since the time of flood event. Previous experience shows
458 that (1) epigraphic marks could be easily removed from the original site during
459 restoration works (Fig. 6); (2) some original land mark (street, wall, or floor) could be
460 buried or their names changed (Deutsch et al., 2006; Munzar et al., 2006; Macdonald,
461 2007). Non-typical examples of flood marks are signs of flood levels recorded on the
462 River Vltava in Prague with respect to the head of “bearded man” (in Czech *Bradáč*)
463 (Elleder, 2003; Brázdil et al., 2005a) or for the River Elbe at Děčín close to the Czech-
464 German border located on the castle rock (Brázdil et al., 2005a; Kotyza, 2006).

465

466 A second set of uncertainties is related to the hydraulic setting and transformation of
467 water level into discharge. The hydraulic calculations assume a precise characterisation
468 of the channel geometry which remains invariant during the flood event and, in most
469 cases, steady flow in subcritical flow conditions. In a given cross-section, the portions
470 of effective flow (flow in the downstream direction) should be distinguished from
471 regions of the channel that do not convey discharge downstream (e.g. eddy flows).
472 Ideally, the model should be calibrated using known water surface elevation and
473 discharges from contemporary floods, and if necessary carry out changes according to
474 the historical vegetation and past urban configuration. Although the discharge
475 estimation can be made on the basis of a single historical mark or flood evidence,
476 confidence in the discharge determination is enhanced when calculated water surface
477 profiles are matched by several flood marks or other inundation references along the
478 study reach (Machado et al., 2015).

479

480 The effect of bridges, channel constrictions and obstacles in general, if they get blocked
481 by ice jams or woody debris during the flooding, constitute another issue to be
482 considered during the hydraulic modelling implementation (Fig. 5a). This blocking is
483 likely to produce a back-flooding effect raising the flood level upstream. River lining
484 and encroachment of the river bank may vary the floodway area and change the
485 hydraulic conditions through time of referred flood marks. For instance, the Danube
486 inundated ca 1000 km² of floodplains during the September 1899 flood whereas flood
487 storage during the June 2013 flood was only a few hundreds of km² producing
488 significant effects on the flood peak discharges (Blöschl et al., 2013). Note that the
489 largest pan-European flood event of February–March 1784 was also caused by sudden
490 release of water from local ice jams (Brázdil et al., 2010) (Fig. 5).

491

492 Assumptions concerning the hydraulic method and models applied to calculate
493 discharge, type of flow (uniform versus non-uniform), the effective flow area and
494 choice of energy-loss coefficients cause uncertainty in discharge estimates. For instance,
495 in the River Elbe in Dresden, the official peak discharge of the 1845 flood is 5700 m³s⁻¹
496 whereas the water profile calculations by means of one- and two-dimensional hydraulic
497 models provides a value of 4335 m³s⁻¹, a discrepancy attributed to an inaccurate stage-
498 discharge relationships at the gauge (Pohl, 2008).

500 4.4 Flood frequency analysis

501 A fundamental problem in flood hydrology is the analysis of the flood frequency or
502 discharge corresponding to an occurrence interval (return period). This estimation is
503 necessary to the correct design and location of structures (dams, bridges, industrial
504 buildings) and in the flood hazard mapping. The statistical analysis of extreme values
505 has been highly improved since earlier work by Foster (1924), describing the
506 application of frequency curves to engineering problems. Fisher and Tippett (1928)
507 developed frequency distributions of maximum values, subsequently applied by
508 Gumbel (1945) to floods. The flood-frequency analysis (FFA) was presented as a
509 replicable method for quantification of uncertainty based on a large number of flood
510 data. There are several important problems in applied flood statistics to the study of
511 large floods. The first concern is the complexity of natural phenomena and the second is
512 the assumption that data collected on river gauges are representative of the largest and
513 rarest floods (Baker, 1994). The design engineer Vance A. Myers (1967) highlighted the
514 consequences of using FFA methods with short flow measurements for dam design: “In
515 reading the early reports one can sense a confidence by the less cautious that the flood
516 record was stable, that nature had shown what she could do on a particular stream in a
517 relatively few decades. This confidence was later found to be misplaced. The more
518 cautious showed a feeling that major floods were among the imponderables, whose
519 evaluation was impossible by the techniques then available. Some earth dams built
520 during this period have failed due to insufficient spillway capacity”. The design of
521 sensible infrastructures was highly improved when historical flood data was considered.
522 For instance, the spillway capacity of the Saucelle ($13,282 \text{ m}^3\text{s}^{-1}$) and Aldeadávila
523 ($12,500 \text{ m}^3\text{s}^{-1}$) dams in the River Duero (Spain) were designed on the basis of a
524 deterministic application of reconstructed historical discharges from the 1597, 1739 and
525 1909 flood marks (Rodríguez-Marquina, 1949a, 1949b).

526 The use of historical floods for FFA has been more frequent since pioneer publications
527 by Benson (1950) and Leese (1973) incorporating non-systematic (historical) data
528 together with gauge records. Documentary data are particularly valuable where there is
529 an account of all floods exceeding a certain stage (threshold), or censured level, over a
530 long period prior the instrumental gauging (Fig. 1). Generally, this minimum flood level
531 required to assure documentary evidence of flooding is related with a perception
532 threshold to which the contemporary society was susceptible in terms of damage or
533 social disruption (Stedinger and Cohn, 1986; Francés et al., 1994). This perception
534 threshold is frequently related to a flood water-level within urban zones and buildings
535 with distinct characteristics (e.g. market, bridge, church) (Barriendos et al., 2003). The
536 most common approach assumes that each flood exceeding this threshold has been
537 recorded in the documentary record (Fig. 1a,b). For instance, flooding of the Aranjuez
538 Royal gardens (Spain) is produced when the River Tagus overtopped the river banks

539 during discharges exceeding $300 \text{ m}^3\text{s}^{-1}$ (Benito et al., 2003a). A list of k_i observations
540 above an arbitrary specified discharge threshold X_i in n_i years is similar to the analysis
541 of partial duration series (data censored above threshold; Stedinger and Cohn, 1986;
542 Francés et al., 1994; Francés, 2001). Statistically it is important to confirm that years
543 with lack of historical flood record corresponded really to flows smaller than the
544 discharge threshold X_i . The threshold level of flood perception may vary through time
545 with regards to various human activities and occupation of riverside areas. Naulet et al.
546 (2001) classified the documentary flood data on four types (Fig. 1b): (1) exact type
547 when flood discharge is known (e.g. water mark); (2) lower bound type if we know that
548 the flood level was higher than a lower bound (X_i), which is known; (3) upper bound
549 type if it is only known that the flood at time t was smaller than X_u , which is the upper
550 bound; and (4) double bound type if it is known that flood discharge was bracketed by a
551 double bound where X_i and X_u are known. These historical flood data (known as non-
552 systematic) can be combined with systematic annual data from the gauge stations. FFA
553 commonly uses parametric models (defined finite number of parameters) combining a
554 cumulative probability distribution function and a parameter estimation method
555 (Stedinger and Cohn, 1986). Most of the distribution functions (Gumbel, Log-Person,
556 GEV) that are used in conventional FFA has been applied with historical data
557 (Stedinger et al., 1993). Several methods have been used in the estimation of the
558 statistical parameters for the selected distribution functions (Strupczewski et al., 2014).
559 The most efficient methods to incorporate imprecise and categorical data are: (1)
560 maximum likelihood estimators (Leese, 1973; Stedinger and Cohn, 1986; Francés,
561 2001); (2) the method of expected moments (Cohn et al., 1997; England et al., 2003);
562 and (3) Bayesian methods (Kuczera, 1999; O'Connell et al., 2002; O'Connell, 2005;
563 Reis and Stedinger, 2005). Several reviews of these methods have been published by
564 Stedinger et al. (1993) and Francés (2004), and case study applications in Europe can be
565 found, among others in Calenda et al. (2009) and Botero and Francés (2010).

566 A recent review by Kjeldsen et al. (2014) observed a scarce use of historical data for
567 frequency estimates in 16 countries of Europe, identifying three main reasons: (1) the
568 lack of unified database depositories, (2) uncertainty associated with discharge
569 estimates, (3) concerns about violation of stationary assumption when using historical
570 data, i.e. annual probabilities are equated to historical frequencies of occurrence.
571 Concerning the second item, Macdonald et al (2014) showed that frequency analysis
572 using exact discharges or minimum discharges exceeded by the historical event has
573 almost the same uncertainty value. Viglione et al. (2013) demonstrated that the number
574 of floods exceeding the perception threshold is more important than the uncertainty on
575 discharge value A reduction on the error is obtained for a return period of the largest
576 historical flood about twice length of the pre-instrumental record (Strupczewski et al.,
577 2014). Many documentary-based flood studies shown that flood frequency has been
578 influenced by the internal variability of atmospheric circulation, with flood clusters at
579 some time periods (Glaser et al., 2010), or by impacts on the environmental patterns

580 such as land-use (Benito et al., 2010) and engineering works (Machado et al., 2015). A
581 simple test of stationarity for censored samples (systematic and/or non-systematic) was
582 proposed by Lang et al. (1999) assuming that the flood series can be described by a
583 homogenous Poisson process (Naulet et al., 2005). It is recommended to select a sample
584 above a high discharge threshold in order to produce an exhaustive and homogeneous
585 set, avoiding bias in relation with archive availability or flood risk exposure (Barriendos
586 et al., 2003). Macdonald et al. (2014) analysed the sensitivity of the application of
587 different discharge thresholds, showing that the selection of a high discharge threshold
588 decreased the uncertainty for high magnitude flood estimation.

589

590 **5 Discussion and perspectives**

591 **5.1 Discharge of historical floods in the context of instrumental records**

592 Quantitative historical hydrology provides a multi-centennial perspective of extreme
593 flood magnitudes. In Europe, there is a long tradition in the study of historical floods in
594 the context of historical climatology (Brázdil et al., 2005b; Glaser et al., 2010) although
595 reconstruction of peak discharges associated to documentary data is still scarce (Fig. 7).
596 Table 1 shows a compilation of 45 case studies with historical discharge estimates at
597 sites with multiple floods with discharge estimates published in peer review papers or
598 being cross-checked with the original historical sources. Numerous studies suggest that
599 current flood magnitudes are not unusual within the context of last 1000 years, with
600 good examples for the rivers Rhine (Herget and Meurs, 2010; Wetter et al., 2011), Tiber
601 (Calenda et al., 2005), Llobregat (Thorndycraft et al., 2005), Trent (Macdonald, 2013)
602 and Gardon (Sheffer et al., 2008; Neppel et al., 2010). In general, the largest historical
603 floods from the last 500 years show higher peak flows than the largest gauged floods
604 (Fig. 7). The largest difference in discharge between historical and gauged records is
605 mainly characteristic for small catchments in mountain basins and in Mediterranean
606 rivers (e.g. Llobregat, Ter, Ticino, Tiber and Isère rivers). In these regions, the
607 knowledge of historical peak flows may provide important insight in flood hazard
608 prevention. For example, the 2002 flood of River Gardon (France), that claimed the
609 lives of 23 people and cause €1.2 billion worth of damage, was larger than any gauged
610 flood since 1890 (DDE, 2003). However, a documentary and palaeoflood-based study
611 demonstrated that at least five floods larger than the 2002-flood occurred in AD 1400-
612 1800, i.e. during the Little Ice Age (Sheffer et al., 2008). In many mountain catchments
613 historical floods are considerable larger than the instrumental data, that can be explained
614 by different reasons (Peña et al., 2014; Schulte et al., 2015): (i) changes on atmospheric
615 dynamics (e.g. from the 1930s to 1977 in Switzerland); (ii) possible inaccuracy of
616 instrumental data during flood peak conditions (inundation or malfunction of gauge
617 station); (iii) changes on discharge contribution from snow and glacier melt during past
618 cooler climate periods (e.g. Little Ice Age), as well as influence of other flood
619 producing mechanism (e.g. ice jams).

620

621 In some cases, recent flooding in central and northern Europe reached similar
622 magnitude or even higher than those reconstructed from documentary records, such as
623 the River Vltava in Prague (Brázdil et al., 2005a; Elleder et al., 2013), and the records in
624 the lower River Po (HOPR, 1935; Zanchettin et al., 2008) (Fig. 7). In the case of the
625 River Findhorn in UK, the official gauged discharge for the 1970-flood was initially
626 60% higher than the reconstructed peak flow for the “Muckle spate” flood of 1829,
627 although later the 1970 peak flow was recalculated below the 1829-flood (McEwen and
628 Werritty, 2007).

629

630 **5.2 Multi-proxy analysis of past hydrological extremes**

631 Documentary archival data on floods in general are ubiquitous across Europe although
632 the computation of peak discharges depends on the availability of reliable epigraphic
633 flood marks or thoroughly documented water level descriptions. In most cases, these
634 flood marks are located in urban settings with frequent changes of the river channel
635 topography that increase uncertainty of the values obtained when computing flood
636 discharge. The combination of historical and palaeoflood (sedimentary) flood data has
637 been demonstrated to be a very effective tool for improving the catalogue of past
638 flooding and reducing uncertainties on flood discharges (Thorndycraft et al., 2005).
639 Palaeostage indicators from sedimentary records (slackwater flood deposits) are
640 frequently preserved within bedrock-stable cross-sections which are suitable settings for
641 hydraulic estimation of flood discharges (Benito and O'Connor, 2013). Moreover, the
642 age uncertainty of numerical dating (radiocarbon respectively optically stimulated
643 luminescence methods) used in palaeoflood studies may be refined based on known
644 documentary floods (Medialdea et al., 2014). The SPHERE Project has revealed the
645 complementary of palaeoflood and historical flood information (Benito and
646 Thorndycraft, 2004) with major gain on the quality of past flood records in terms of
647 time and discharge, as it is demonstrated in the studies performed for the rivers Gardon
648 (Naulet et al., 2005; Sheffer et al., 2008), Ardèche (Sheffer et al., 2003; Naulet et al.,
649 2005), Llobregat (Thorndycraft et al., 2005), and Guadalentin (Benito et al., 2010;
650 Medialdea et al., 2014). Recent palaeoflood reconstructions from floodplain sediments
651 have analysed geochemical proxies from continuous alluvial records and investigate
652 local documentary flood data to calibrate the palaeohydrological records (e.g. Swiss
653 Alps, Schulte et al., 2015; River Severn in mid-Wales, Jones et al., 2012; River Rhine in
654 The Netherlands, Toonen et al., 2015). Flood sediments accumulated on floodplain
655 sinks (e.g. palaeomeanders and flood-basin environments) can be studied with high
656 resolution techniques (e.g. X-Ray-scanned samples) to obtain continuous records of
657 grain-size and geochemical content (Zr/Ti , Zr/Rb and Sr/Ti) indicative of detrital
658 fraction deposited by floods (Schulte et al., 2015). The reconstructed palaeoflood
659 magnitudes are obtained after calibrating their ages obtained by geochronological
660 techniques (radiocarbon) with known historical events and normalizing grain-size and
661 geochemical content, where the coarse tail of grain-size distribution is used to estimate
662 peak flood discharges or severity indexes (Toonen et al., 2015).

663

664 New emerging palaeoflood archives from lake records show a great potential for
665 synergy with documentary floods to complete regional records of extreme events to
666 understand flood-climate relationships (e.g. Wilhelm et al., 2012; Wirth et al., 2013;
667 Corella et al., 2014). For instance, in Montcortés Lake (NE Spain) a varved sediment
668 core accumulating since the 14th century contains detrital layers associated to intense
669 rainfalls (>80 mm/day) recording higher storm frequency during AD 1347–1400 and
670 AD 1844–1894; both periods coincide with severe floods from the nearby River Segre
671 (Corella et al., 2014). In non-varved lake systems, palaeoflood stratigraphy can be
672 compared to historically documented flood records, as a mean to improve the age-depth
673 model of the stratigraphic log (Schillereff et al., 2014). Another group of palaeoflood
674 techniques suitable to combine with documentary sources are those based on botanical
675 and ecological evidences (Bodoque et al., 2014). The presence of lichens on boulders in
676 river channels can be used to date the flood responsible of their transport, once a
677 lichenometric growth curve for the lichen species for the area of study has been
678 established (Foulds et al., 2014). Dendro-geomorphology uses information from flood
679 damages in trees and bushes, dating floods at annual scale (Bodoque et al., 2014).
680 Commonly, these palaeoflood methods are most suitable for mountain streams
681 environments, where documentary sources provide a mean to establish the age biases to
682 minimize errors during the calibration process

683

684 **5.3 Flood magnitude sensitivity to climate change**

685 Climate variability may affect both flood frequency and magnitude with greater
686 sensitivity on largest “rare” floods (50-year flood and higher) than on smaller frequent
687 floods (2-year floods; Knox, 1993; 2000). The study of historical floods in the context
688 of climate variability has been focussed on high-quality complete datasets classified
689 according to severity of damage (Sturm et al., 2001) to infer changes in flood frequency,
690 meteorological causes and seasonality (Glaser et al., 2010). The classification of
691 historical floods according to peak discharge or discharges over some threshold allows
692 further analysis on the sensitivity of flood frequency in relation to their magnitude.
693 Furthermore, this classification based on discharge classes allows consideration of the
694 most recent instrumental records in an integrated analysis avoiding the bias of
695 classifications based only on flood damages which varied over time with regards to
696 exposition and vulnerability. Based on literature sources (cf. for details below), eight
697 records compiled from different European rivers where numerical or categorical flood
698 magnitude during the historical period was completed with comparable data from
699 gauged records (Fig. 8). Two flood categories were differentiated: (1) catastrophic
700 floods (CAT) associated with high flood discharge or severe damages, and (2)
701 extraordinary floods (EXT) causing inundation of the floodplain with moderate-to-
702 minor damages. The detected flood changes are highly dependant on the observational
703 window (Hall et al., 2014) with identification of flood-rich and flood-poor periods over
704 the historical record and flood trend detection over the instrumental period.

705

706 In Central Spain, increased flood frequency of large floods was identified in AD 1000–
707 1200, 1525–1625 and in the late 19th–early 20th centuries (Benito et al., 2003a; Fig. 8a).
708 During the second half of the 20th century, the frequency of floods decreased, in
709 connection with a dominant positive mode of the North Atlantic Oscillation during
710 winter months; however, flow regulation by dams also played an important role in this
711 flood frequency decline since the mid-1950s. The decreasing trend in annual maximum
712 floods was also detected on the flood analysis from gauge records of a set of rivers
713 within the Tagus River basin under quasi-natural flow conditions (Mediero et al., 2014).
714 In the River Segura (SE Spain) the frequency of catastrophic (autumn) floods decreased
715 since the late 19th century together with the frequency of intense rainfall events except
716 some decades (e.g. the 1970s and 1980s), in which intense rainfall and flooding co-
717 existed with severe drought conditions (Fig. 8b; Machado et al., 2011). In the River
718 Gardon (southern France), the frequency pattern of large floods (>50-yr floods) has
719 decreased since the late 19th century, whereas the extraordinary and ordinary floods
720 increased during the 20th century (Fig. 8c) (Sheffer et al., 2008; Neppel et al., 2010).
721 Similarly, historical flood series from NE Spain indicate a lack of statistical significant
722 trend for large-catastrophic floods, whereas extraordinary floods show a significant rise,
723 especially since 1850 (Barrera-Escoda and Llasat, 2015).

724

725 In the River Tiber (Central Italy) extreme floods were particularly frequent in 1400–
726 1500 and 1600–1700 (Camuffo et al., 2003). Large-catastrophic floods exceeding the 17
727 m stage ($<2900 \text{ m}^3\text{s}^{-1}$) at the Ripetta Landing ($16,545 \text{ km}^2$) were not constant in time:
728 four floods above 18 m ($<3400 \text{ m}^3\text{s}^{-1}$) took place in only 80 years during the 1530–1606
729 period (Calenda et al., 2005), intriguingly a period of reported low flood frequency by
730 Camuffo and Enzi (1996). Recent flooding is difficult to evaluate in the context of
731 climate change due to river regulation structures, although only three extreme floods
732 ($>2550 \text{ m}^3\text{s}^{-1}$) were recorded since 1900 (Fig. 8d). Extraordinary flood events exceeding
733 $1400 \text{ m}^3\text{s}^{-1}$ prior to 1970 occurred with a mean frequency of seven floods per decade,
734 whereas after 1970 the frequency decreased to about five floods. Frequent events within
735 the historical context (2-year flood), such as the December 2008 flood (12.55 m, ~ 1400
736 m^3s^{-1}), are currently producing large economic impacts. This demonstrates the increased
737 flood vulnerability of the Rome region despite of decreasing flood hazard by flow
738 regulation (Natale and Savi, 2007).

739

740 Several types of meteorological events and different storm types result in mixed flood
741 distributions, each characterized by individual probability distribution parameters
742 (Hirschboeck et al., 2000). Climatic variability can lead to flood magnitude / frequency
743 changes affecting one or various types of flood populations (e.g. early spring snow-melt,
744 convective storms) with relevant implications in the non-stationarity of the statistical
745 parameters supporting flood probability analysis (Milly et al., 2008). Documentary
746 records provide information on prevailing circulation types producing floods based on

747 changes in flood seasonality. Macdonald (2012) studying the River Ouse, a large
748 catchment within a UK perspective, has identified a higher frequency of summer floods
749 within AD 1700–1849 than in the AD 1850–1999 period. Furthermore, the combined
750 documentary and instrumental flood record (Macdonald and Black, 2010) illustrates that
751 the frequency of extraordinary floods within the range of 350 to 500 m³s⁻¹ have
752 increased during the 20th century, in particular comparatively to the most extreme
753 floods (>500 m³s⁻¹; Fig. 8e).

754

755 In Central Europe, long records of the Elbe and the Oder/Odra rivers showed a decrease
756 in winter floods during the last 80 to 150 years (Fig. 8g, h), while summer floods
757 showed no significant trend (Mudelsee et al., 2003). This change in seasonal flood
758 patterns is reflected in the recent trend towards an overall decrease on flood magnitude,
759 although in the case of the River Vltava (Czech Republic) the August 2002 flood
760 reached the highest peak flow on record (Fig. 8f; Brázdil et al., 2005a). In the River
761 Rhine at Basel (Switzerland) severe summer (JJA) floods were particularly frequent
762 between 1651 and 1750, in relation to enhanced precipitation; severe winter (DJF)
763 floods have not occurred since the late 19th century, despite a significant increase in
764 winter precipitation (Wetter et al., 2011).

765

766 In some regions, the potential for ice jams on rivers should be considered in the analysis
767 and interpretation of winter peak flows along centennial records. An ice jam can
768 generate water-levels above rainfall floods due to inundation of the area behind the ice
769 blockage, or as a consequence of rapid release of water after the ice jam failure (Beltaos,
770 2008). During the AD 1550–1850 period, ice cover on large mainland European rivers
771 combined with late winter and spring snowmelt generated very large floods, similar to
772 what is observed today at higher latitudes. In The Netherlands many floods over the
773 1750–1860 period were associated with ice jams, particularly on the River Waal (e.g., in
774 1781, 1784, 1799, 1805 and 1809; Driessen, 1994). Detailed records describing the
775 winter ice jam floods in 1784 are widely recorded across much of western and central
776 mainland Europe (Demarée, 2006; Brázdil et al., 2010, 2012) (Fig. 5). In the River
777 Mosel, the 28 February 1784 flood water-level was significantly higher than any other
778 recorded during the past millennium (Sartor et al., 2010), although any discharge
779 estimation should consider that ice jams can raise water levels to much higher
780 elevations than open-water floods (Beltaos, 2008). Other factors enhancing flood
781 severity through time includes timing of melting of glaciers (Debret et al., 2010). Global
782 warming is introducing changes in the spatial (latitudinal) and temporal (seasonal)
783 distribution of flooding related to ice and snowmelt (Beltaos and Prowse, 2009). For
784 example, changes in the hydrometeorological conditions that generate flooding may
785 enhance flood magnitude in Norway, due to an earlier onset of snowmelt related to
786 flooding in the region (Hisdal et al., 2006), whereas in mainland Europe, flooding
787 related to ice-jams are now unlikely to occur (Kundzewicz et al., 2014).

788

789 **5.4 Historical floods in a non-stationary hydrology**

790 The comparative analysis of historical records at different catchments across Europe
791 points to the fact that the temporal distribution of flood frequency is predominantly
792 modulated by regional meteorological triggers (Glaser et al., 2010). In regions where
793 floods are generated by several types of weather conditions, each flood population is
794 composed by a probability distribution resulting in mixed distributions. Long-term
795 climate variability may alter the seasonal weather patterns producing floods (summer,
796 winter, snowmelt, etc.) and consequently the assumption of stationarity of the flood
797 frequency distribution. Stationarity has been qualitatively described as the idea that
798 natural systems oscillate within an unchanging envelope of variability (Milly et al.,
799 2008). In the case of extreme events, secular records of historical floods show a
800 temporal variability (clusters) fluctuating at multi-decadal time scale. However, the
801 underlying driving factors causing past departures from stationarity are far from being
802 random phenomena. The temporal changes in the trajectory and statistics of a variable
803 may be linked to natural, low-frequency variations of the atmospheric circulation,
804 external forcings (solar cycles) or anthropogenic changes. Therefore, a detail
805 characterisation of natural variability of past floods will facilitate the attribution and
806 modelling of future variability due to nature and human impacts. The statistical
807 parameters may show increasing/decreasing changes that can be modelled (as a trend or
808 smooth function) using time as covariate (Villarini et al., 2009), or they can be related
809 to hydro-climatic covariates such as circulation indices (e.g. Pacific Decadal Oscillation
810 – PDO, North Atlantic Oscillation – NAO, Arctic Oscillation – AO) characterising this
811 low frequency climatic variability (López and Francés, 2013). The application of these
812 non-stationary models to historical and palaeoflood hydrology requires a numerical
813 characterisation of the occurrence rate (covariate) during the recorded period. Several
814 studies have demonstrated the relationships between flood frequency and magnitude
815 with circulation indices, such as NAO index (Salgueiro et al., 2013). The application of
816 a non-stationary flood frequency analysis in a 300 yr record with 32 documented floods
817 ($>350 \text{ m}^3\text{s}^{-1}$) of the River Tagus were successful to model the fluctuations of flood
818 quantiles (e.g. “100-year flood”) using the North Atlantic Oscillation index and a
819 reservoir index as external covariates (Machado et al., 2015). This non-stationary
820 modelling was based on Generalized Additive Models for Location, Scale and Shape
821 parameters (GAMLSS; Rigby and Stasinopoulos, 2005) that described the temporal
822 variation of statistical parameters (mean, variance) in probability distribution functions
823 (Villarini et al., 2010; López and Francés, 2013). In this example, the non-stationary
824 models show that the peak flood associated with a “hundred year” flood (0.01 annual
825 exceedance probability) may range between $4180 \text{ m}^3\text{s}^{-1}$ and $560 \text{ m}^3\text{s}^{-1}$, whereas the
826 same model under stationary conditions provided the best fitting results to a log-normal
827 distribution, with a discharge of $1450 \text{ m}^3\text{s}^{-1}$ (Fig. 9). These results illustrate that under
828 stationary statistics the risk assumed is much higher than the one established in the
829 design of infrastructures. Moreover, concepts such as return period, design quantile

830 (return level), and risk under non-stationary conditions should be changed when the
831 annual probability changes every year (Obeysekera and Salas, 2014).

832

833 **6 Concluding remarks**

834 This paper presents a review of the scientific progress in the quantification of large
835 historical floods since the early stages prior to the automatic hydrological stations. In
836 the last two decades, new approaches have been developed to obtain continuous and
837 reliable flood magnitude data sets from documentary records, their statistical analysis
838 and temporal patterns, illustrating the strength, limitation and future prospects of
839 various methods. Most early discharge computations were obtained at sites with known
840 water elevation from flood marks or by extrapolation of a rating curve at sites with staff
841 gauge. Only in the last decades, flood descriptions from rich documentary evidence
842 have been used to estimate discharges of floods exceeding a threshold of perception,
843 extending the record of flood discharges up to several hundreds of years. The
844 reconstruction of secular historical records of extreme floods is relevant to solve major
845 scientific and engineering problems: (1) flood hazard assessment using FFA (data
846 censored over thresholds of perception), and (2) quantification of the largest discharges
847 in a given catchment as evidence for a deterministic approach in safety risk analysis of
848 critical facilities (dams, bridges, power plants). The historical flood records have gained
849 attention among hydrologists on the background of new statistical methods of FFA
850 using non-systematic data and recently in the analysis of non-stationarity modelling.
851 However, the use of historical flood records for flood hazard studies is still scarce in the
852 practical realm. Documentary flood data can benefit from the combined use with
853 palaeoflood records such as fluvial sediments, botanical- and dendrochronological
854 records, flood-produced detrital layers in lakes and marine records. In particular, fluvial
855 sediments deposited in slackwater environments have been demonstrated as very
856 efficient to be combined with documentary data sets to improve the flood frequency
857 analysis of rare and extreme floods. Europe holds numerous, unexplored archives in
858 relation with historical floods, their causes and the socio-economic impacts. There is
859 great opportunity to generate scientific knowledge about the largest and rarest floods
860 reported through historical times and use them to improve the social conscience and
861 perception of natural risks. The presented paper is a significant contribution to historical
862 hydrology in Europe (Brázdil et al., 2006b, 2012) extending its potential on
863 quantification of past documentary-based floods in Europe.

864

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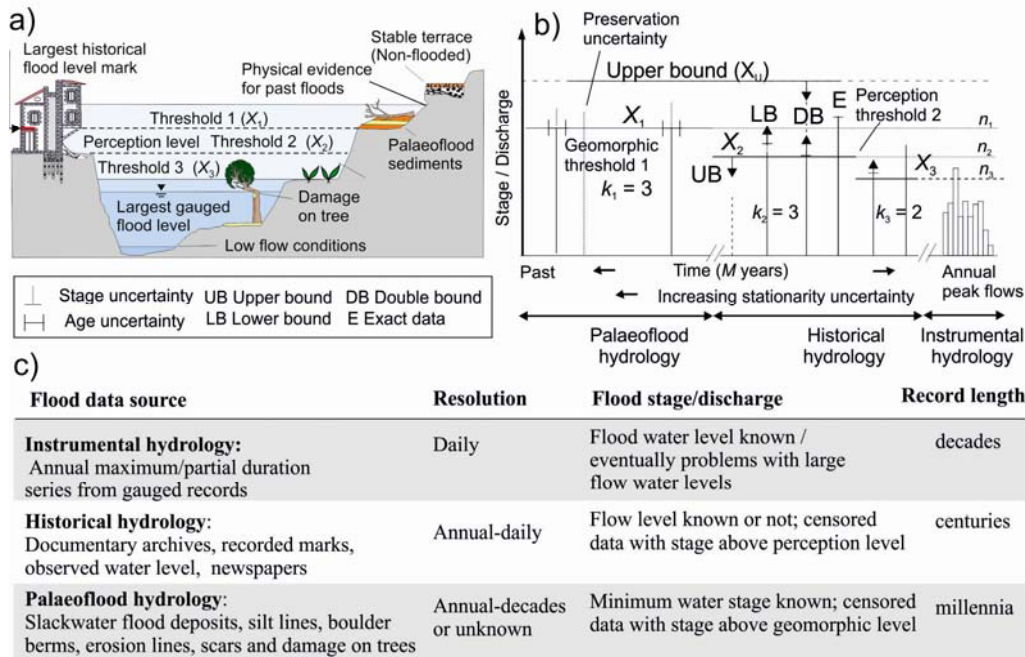
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Figure 1. Sources of quantitative flood information. (a) Sketch of a cross section showing various flood level indicators from palaeofloods (sediments and damage on trees), and documentary-based floods (i.e. those able to cause damage or socio-economic disruption). For historical hydrology, only floods exceeding a flood level related to a perception threshold (X_i) over a period of n_i years ($n_1 > n_2 > n_3$) are recorded. Palaeofloods from stratigraphic records are related to geomorphic thresholds. (b) Organization of historical and palaeoflood data, using the described thresholds (X_i), and multiple types of observations to support flood frequency analysis. K_i corresponds to the number of flood peaks during the last n_i years that exceeded the X_i threshold but not the X_{i-1} threshold. Upper bound level (X_u) may be used to limit the maximum discharge. Data types: E: flood peak is known. LB: flood was bigger than X_i which is known; UB: the upper flood level of known magnitude (X_u) was not exceeded over a certain time period. DB: flow level was within the interval given by X_u and X_i . c) Data source characteristics, timing, stage information, and typical temporal framework of systematic (instrumental) and non-systematic data (palaeoflood and documentary evidence). Modified after Benito and O'Connor (2013).



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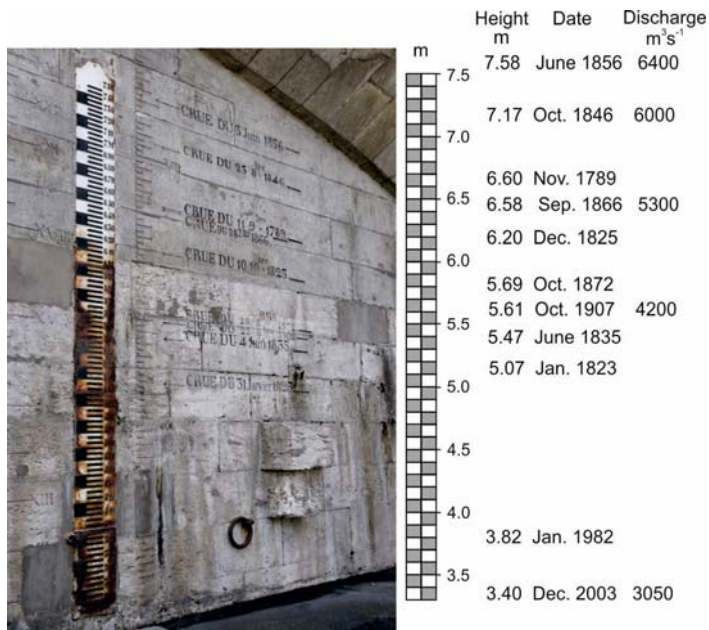
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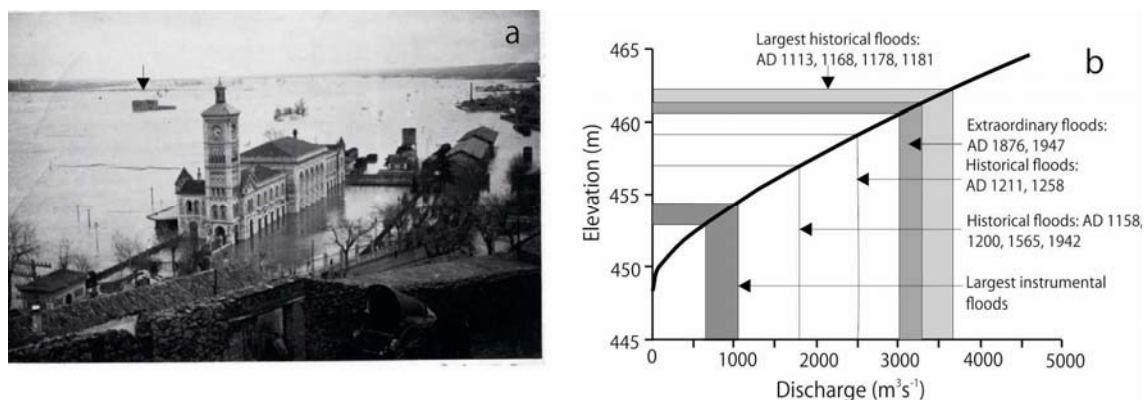
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Figure 2. Stream flow measurements at the gauge station "Borgo a Mozzano" ($43^{\circ} 59'30.73''N$; $10^{\circ} 33'10.04''E$) in the River Serchio (Italy), probably taken in the 1920s or 1930s. The observer is placed in a box suspended on cable that moved along the cross section from which manages an old device to measure the stream flow velocity (Photo courtesy of Regione Toscana - Genio Civile di Bacino Toscana Nord e Servizio Idrologico Regionale).



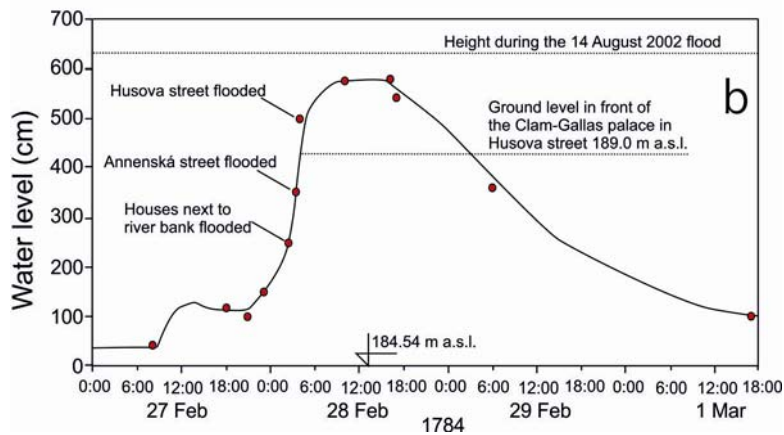
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Figure 3. Historical flood marks of the River Loire in the Wilson Bridge in Tours (France). The elevations of the flood marks after the Direction Regionale de l'Environnement de l'Aménagement et du Logement (DREAL), Centre-Val de Loire (<http://www.centre.developpement-durable.gouv.fr>). Discharge values associated to the flood levels after Duband (1996).



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Figure 4. (a) Upstream view of the River Tagus into the Huerta del Rey (King's Garden) during flooding on 6 March 1947 at Toledo (Spain). The peak discharge was reached at 10 a.m. and the picture was taken at 3.20 p.m. (by courtesy of Mariano García Bargueño). The water level at peak discharge was ca. 1.5 m above the railway station ground level (main building at the centre of the photo). In the background the arrow points the Galiana Palace, on the left bank of the Tagus River, just over 1 km from the old part of Toledo (Galina Palace was built at the site of an earlier summer villa and Arab garden of Al-Mamun, king of the Taifa of Toledo in AD 1043–1075) (Benito et al., 2003a). (b) Rating curve of a cross-section next to the upper picture obtained from step-back water calculations (HEC-RAS model) with the elevation of relevant historical flood evidences (flood marks and description of inundated sites). The largest historical floods occurred during the Medieval Climatic Anomaly and are followed by the 1876 and 1947 floods (Fernandez de Villalta et al., 2001).



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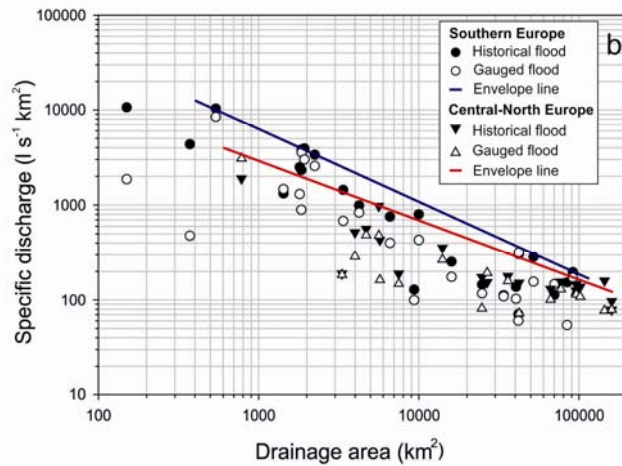
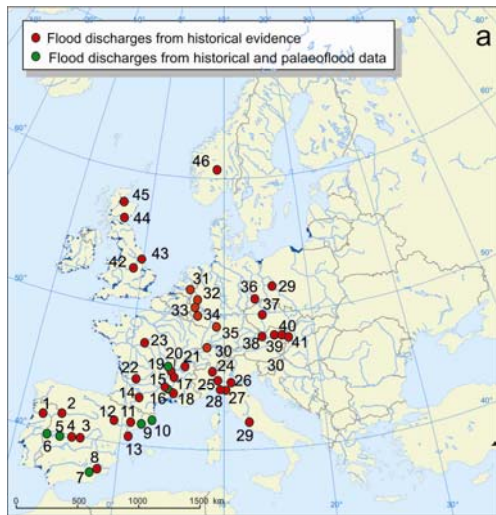
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Figure 5. (a) Pictorial representation of the River Vltava during the February 1784 flood, showing ice floes and woody debris accumulated at the Charles Bridge in Prague (copperplate by F. Erben, Museum of the City of Prague, catalogue no. 125.387). The ice jams at the bridge caused flooding upstream resulting to the highest known water marks until the August 2002 flood (Brázdil et al., 2005a). (b) A flood hydrograph of the Vltava River in Prague at the Monastery of the Knights of Cross reconstructed from documentary data for 27 February – 1 March 1784 with an estimated discharge rate of $4560 \text{ m}^3 \text{ s}^{-1}$ (Brázdil et al., 2005a).



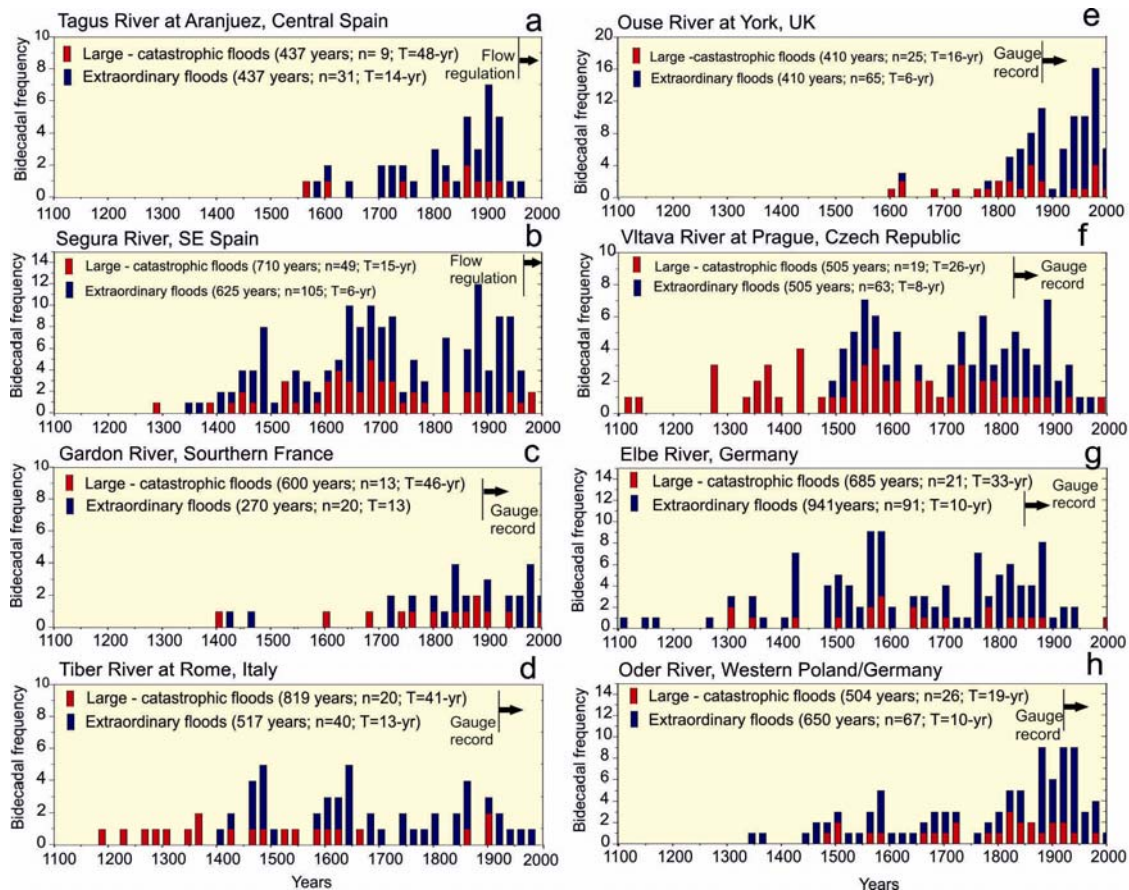
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Figure 6. Stone brick with an inscription of the 1906-flood mark of the River Rhine in Koblenz. The block was originally placed on a railway bridge destroyed during World War II and later used for reconstruction of this building (Herget, 2012). This confirms the necessity to work only with original position of flood marks and use multiple documentary evidences to reconstruct flood levels.



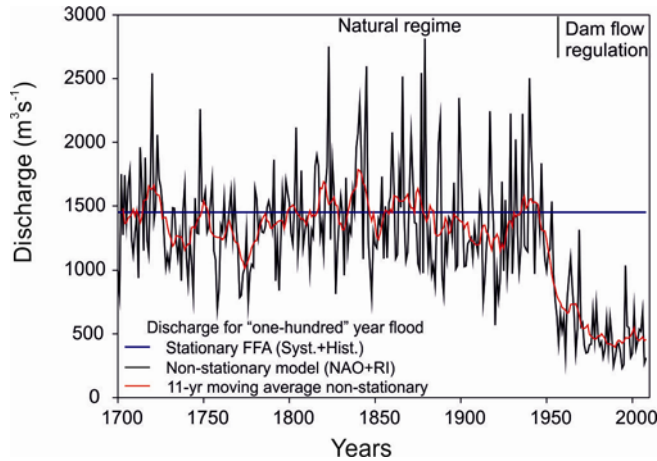
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Figure 7. (a) Major rivers and streams of Europe and studied sites with multiple historical flood discharge estimates. Numbers refers to places cited in Table 1. (b) Maximum specific discharge ($l s^{-1} km^2$) of the largest historical and instrumental floods recorded in the sites referred in Table 1. Southern Europe includes sites from Portugal, Spain, Italy and France (except the Dordogne, Isère, Loire, Rhone and Garonne rivers) and Central-North Europe the rest of rivers. Lines are envelope boundaries of the largest specific discharges for these two data sets. Most of the historical specific discharges are above the instrumental ones, except some rivers in Central-North Europe.

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1563 Figure 8: Bi-decadal (20-yr) flood frequency based on documentary and instrumental
 1564 records in the selected European rivers (floods exceeding a particular discharge
 1565 threshold or height). Two flood categories were differentiated: catastrophic floods
 1566 (CAT) associated with high flood discharge or severe damages, and extraordinary
 1567 floods (EXT) causing inundation of the floodplain with moderate-to-minor damages.
 1568 The length of record in years, number of recorded floods (n), and the mean occurrence
 1569 interval (T in years) for each category and river are indicated. (a) The River Tagus in
 1570 Aranjuez, documentary and instrumental data, CAT: $>400 \text{ m}^3 \text{ s}^{-1}$, EXT: $100\text{--}400 \text{ m}^3 \text{ s}^{-1}$
 1571 (Benito et al., 2003a; Machado et al., 2015). (b) Segura-Guadalentín rivers at Murcia
 1572 (Barriendos and Rodrigo, 2006; Machado et al., 2011). (c) The River Gardon,
 1573 documentary data since the 15th century, historical and daily water-level readings at
 1574 Anduze (1741–2005; Neppel et al., 2010), CAT: $>3000 \text{ m}^3 \text{ s}^{-1}$; EXT: $1000\text{--}3000 \text{ m}^3 \text{ s}^{-1}$;
 1575 complemented with discharges from palaeofloods at La Baume (Sheffer et al., 2008).
 1576 (d) The River Tiber in Rome, observed historical levels since the 12th century,
 1577 continuous water-level readings since 1870 at the Ripetta Landing (Calenda et al., 2005),
 1578 CAT: $>2900 \text{ m}^3 \text{ s}^{-1}$ (flood level $>17 \text{ m}$ at Ripetta), EXT: $2300\text{--}2900 \text{ m}^3 \text{ s}^{-1}$. (e) The
 1579 River Ouse, documentary and instrumental data (Macdonald and Black, 2010), CAT:
 1580 $>500 \text{ m}^3 \text{ s}^{-1}$, EXT: $350\text{--}500 \text{ m}^3 \text{ s}^{-1}$. (f) The River Vltava in Prague, documentary and
 1581 instrumental data (Brázdil et al., 2005a), CAT: $Q >2900 \text{ m}^3 \text{ s}^{-1}$ or a flood index 2 and 3,
 1582 EXT: $2000\text{--}2900 \text{ m}^3 \text{ s}^{-1}$ or flood index 1. (g) The Elbe River, documentary and
 1583 instrumental data (Mudelsee et al., 2003); classes refer to Mudelsee et al. (2003) strong
 1584 (EXT) and exceptionally strong (CAT) flooding. (h) The Oder River, documentary and
 1585 instrumental data (Mudelsee et al., 2003). Data before AD 1500 are incomplete due to
 1586 lack of documentary evidence. Modified from IPCC (2013).

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1590 Figure 9: Non-stationary model of the “one-hundred year” flood over the last 300 years
1591 based on the dependence of the distribution parameters with the associated external
1592 covariates (winter NAO index and Reservoir index). The horizontal line represents the
1593 100-yr flood from a log-normal distribution using documentary and instrumental
1594 records under a stationarity assumption (after Machado et al., 2015).

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