1 2	Prof. Jim Freer
3	Editor
4	Hydrology and Earth System Sciences
5	February 20th, 2015
6	Dear Prof. Freer,
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8 9 10	We would like to acknowledge the revision of our work originally entitled "Uncertainty propagation in a cascade modelling approach to flood mapping". We thank you for your constructive comments, which in addition to the previous comments from the reviewers very much helped to improve our manuscript.
11 12 13 14	We have digested the comments you pointed out concerning the discussion of our work, and in consequence made considerable changes to our manuscript. I would like to draw your attention to the improvement of the rainfall-runoff model description, which now incorporates details with regards its calibration and use within the framework.
15 16 17 18	We made an effort to incorporate precise changes in the manuscript that acknowledged the raised points. Please note that as in the previous revision, following the suggestion of reviewer two, the title was changed to: <b>Propagation of hydro-meteorological uncertainty in a model cascade framework to inundation prediction</b> .
19 20 21 22	In the following lines we explain how (i.e. by writing our reply in red) and where (i.e. by giving line numbers) each point of your comments have been incorporated in the revised manuscript. We hope that this new version proves to be of interest to you and the reviewers and that it is worth to be considered for publication in HESS.
23	
24	Best wishes,
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26	
27	Dr. Adrián Pedrozo-Acuña on behalf of all authors
28	
29	PS. We are very sorry for your loss.
30	

#### 1 Editor Initial Decision: Reconsider after major revisions (12 Jan 2015) by Dr. Jim Freer

#### 2 **Comments to the Author:**

- 3 I thank both the reviewers and the authors for their responses. There are a few of matters to consider to
- 4 allow this paper to be published in HESS and to be completed in a major revision. Some points on
- 5 which the reviews have covered and one or two I feel must be better addressed in the manuscript.
- 6 R: We thank Prof. Freer for leaving the door open for the possible publication of this work in HESS.
- 7 This study represents the first of its kind in Mexico, and we are very thankful not only for the given
- 8 opportunity but also for the raised points, which we believe have served us well in the improvement of
- 9 the manuscript.
- 10 My comments on this discussion phase are as follows:
- 11 1) Novelty – Reviewer 1 brings up the issue of a lack of perceived novelty about the paper. As I stated
- 12 in my initial assessment I feel the paper is 'novel' in the sense there are very few papers that have
- 13 cascaded results through from the meteorology. However the reporting is quite basic and in lack lacks
- 14 scientific rigour in a number of respects. Not all have been fully teased out by the review process
- 15 including:
- R: Once more, thank you to the editor for noting that the novelty of the approach lies in the sense there 16
- 17 are very few papers that have cascaded results through from the meteorology. Our objective is not to
- 18 accurately reproduce what was observed during this event, but to study how errors in the initial stage
- 19 of the model cascade propagate towards the prediction of flood extension. As it will be explained in
- 20 the following response we have made major improvements aiming at a better reporting of what was
- 21 done.
- 22 a) There really is a complete lack of detail how the models have each been run/calibrated. This must
- 23 be improved significantly including the rational behind using certain parameter/model structure/etc.
- 24 limits and ranges and techniques to evaluate them
- 25 R: With this regard, modifications in the sections corresponding to the three modelling stages have been
- 26 incorporated as follows:
- 27 Meteorological model: In order to analyze the propagation of simulation errors originated at the initial
- 28 stage of the model chain, the use of a multi-physics ensemble was thought to generate results with a
- 29 highly varying skill to observations. No weighting or rejection functions were employed as the purpose
- 30 of the study is precisely to analyze how errors propagate, in contrast to the development of an accurate
- 31 model chain. An explicit acknowledgement to this fact appears now at Page 8 Lines 1-13.
- 32 Hydrological Model: With regards to this step of the model cascade, several changes have been
- 33 incorporated. Firstly, a better description of the model appears at Page 10 Lines 11- Page 11 Line 3.
- 34 Following the criticism of the selection of fixed free parameters in the hydrological model (i.e.
- 35 deterministic), we have incorporated the definition of six sets of parameters that adequately reproduce
- 36 flood hydrographs observed in past events (2001, 2005, 2007, 2008 and 2011). The definition of these
- 37 parameters has been made through the implementation of a traditional calibration process for each of
- 38 these events. This enabled the determination of equally valid parameters to reproduce the 2009 flood 39 hydrograph (see new Table 3). These parameters in combination with the 12 members of the multi-
- 40 physics ensemble gave way to the generation of 72 possible hydrographs for the 2009 flood. However,

- 1 as the purpose of this study is to investigate how errors in the meteorological prediction stage propagate
- down to a predicted inundation, the skill of hydrological predictions was limited by selecting only those
- 3 with a Cor>0.7 and NSC>0.6. The explanation for this procedure appears now at Page 11 Line 4 Page
- 4 12 Line 19.
- 5 Inundation model: The section associated to the flood inundation model has been enriched with a
- 6 discussion on the justification of selected friction values, which appears in Page 13 Line 22 Page 14
- 7 Line 6. Additionally, a discussion on the apparent good skill of the simulated inundation extents has
- 8 been included and appears at Page 15 Lines 9 -27.
- 9 b) The authors seem to have no clear grasp of how the resultant uncertainties/sensitivity to final
- propagated outputs is reliant on their experimental design. In that they clearly choose a set of WRF
- outputs that have highly varying skill to observations but include no weighting function (or even
- rejection) of these outputs.
- R: As stated before, the use of a multi-physics ensemble was thought to generate results with a highly
- varying skill to observations. No weighting or rejection functions were employed as the purpose of the
- study is precisely to analyze how errors propagate, in contrast to the development of an accurate model
- 16 chain. See Page 8 Lines 1-13.
- 17 Then there is such limited information on the hydrological model it's difficult to determine what was
- done (and similar comments could be made for the inundation extent). The issue here is their core
- conclusion is based on the fact that 'uncertainty does not increase'. Well that might be the case but
- without a sensible and rationale approach to the limits for each modelling component in the cascade
- 21 this is not an outcome that can be justified. The paper is very weak in this regard and must be
- significantly improved. For example it isn't rationale to me to have an ensemble of input uncertainties
- but then seemingly use a completely deterministic hydrological model and then suggest the
- 24 uncertainties do not increase, this is not exploring an uncertain cascade and the conclusions cannot be
- 25 justified from it (and again in the flooding part of the modelling this is deterministic). Finally just to
- 26 drive this point home it seems difficult that the authors in their conclusions then recognise that models
- are an imperfect representation (so how can single structure/parameter combinations be justified?). I
- would also argue the 'epistemic' nature of the WRF outputs, given observed knowledge of the system
- was available, was not evaluated sensibly in a probabilistic/uncertainty approach such as others have
- 30 done....
- R: In order to take these comments into account, several modifications to the manuscript have been
- incorporated. These are mainly summarized as follows:
- i. We have obtained 72 hydrographs from the combination of 12 multi-physics
- parameterizations in the WRF model with 6 sets of parameters in the hydrological model. This
- has been done to avoid the traditional deterministic use of the hydrological model, however, a
- full GLUE methodology was not deemed necessary as the main aim of the study is to
- propagate uncertainties originated in the meteorological model. For this reason, we have
- reduced the number of possible hydrographs from 72 to 31(that comply with Cor>0.7 and
- 39 NSC>0.6).
- 40 ii. A new graph explaining the sensitivity of final propagated outputs is shown in Figure 9, where
- the relationship between the flood peak discharge and their corresponding maximum-flooded
- 42 areas is incorporated. Results suggest that despite the variability in peak discharge the
- majority of hydrographs determine a flood extension of similar magnitude (see histograms in

- 1 Figure 9b and c). A discussion of these results has been included in Page Page 14, lines 22-2
- 3 iii. With regards to the statement "uncertainty does not increase", we have deleted it following the 4 analysis of new computations and results (incorporating inundation depths), that have shed 5 light on a complex aggregation of errors to the output. This is verified in the observed 6 sensitivity of the simulated inundation depths (see Figure 10), where the geomorphology of 7 the floodplain appears to be dominant in the flood extent determination. This is precisely our 8 case, as small changes in lateral flood extent produced large changes in water levels. 9 However, due to the lack of inundation depths data, we could not assess model performance 10 using this information.
  - c) The authors fail to discuss properly the dynamics of the inundated area and how much this is a valley filling event (so perhaps a lack of sensitivity to inundation area changes would be expected). So are other dynamics in the model changing (like simulated depths?). I also feel there needs to be much better processing of the SPOT images to show the 'observed' extent of the flooding and to effectively relate that to the simulated extent. This is not adequately done at the moment
- 16 R: Thank you for noting this, we have carried out an analysis of flood dynamics both in horizontal (flood 17 extension) and vertical (flood depths) dimensions. We concur that the sensitivity of the foremost features 18 of the inundation: area and depth should be examined in this work. For such reasons, in Figure 10, we 19 show the distribution of the maxima inundation depths at different locations across the floodplain. It can 20 be observed that in most of the simulations, an upper limit (blue box, top with no visible bar) constrains 21 the maxima inundation depths while the lower limit (blue box, both with visible bar) implies a lot of 22 variability in the bottom maximum depth range. This is explained in the manuscript (Page 14, lines 26-23 31). Additionally, we have incorporated a discussion on how the case study fits the characteristics of a 24 valley filling event at Page 15 Lines 1 - 8 and Lines 20-26.
- 25 Moreover, a revision of inundated depths is also presented and discussed. This appears at Page 15 Line 26 27 - Page 16 Line 11.
- 27 Due to the coarse resolution of the SPOT image (124m) a better processing of the image to determine 28 flood extent was not possible. No other image was available for its use. Under the light of the new results
- 29 (Figure 9 and 10), pointing towards a similar flooded area for all model runs, the "observed" flood extent
- 30 is not dominant to assess model performance.
- 31 I do agree with most, if not all, of the points raised by the reviewers. I think the response of the authors 32 seems to be welcome (improving links to a wider range of research papers and improving readability
- 33 and an understanding of what techniques have been deployed). If the authors cover these points well in
- 34
- a revised manuscript and include further work to address the points above then the revised paper will be
- 35 considered for publication, but this might result in another round of discussion first as the changes are
- quite considerable. 36
- 37 R: We would like to warmly thank the referees for the observations and time spent during the review
- 38 process. We believe that the incorporated analysis of uncertainty and sensitivity along the model cascade
- 39 are reflected in this new version.
- 40 We have made considerable changes to our manuscript, hoping that this new version proves to be of
- 41 interest to you and the referees.

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# 1 Uncertainty propagation Propagation of hydro-

- 2 meteorological uncertainty in a model cascade modelling
- 3 approachframework to inundation prediction

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## **Abstract**

The purpose of this investigation is to study the propagation of meteorological uncertainty within a cascade modelling approach to flood mapping. The methodology iswas comprised of a Numerical Weather Prediction Model (NWP), a distributed rainfall-runoff model and a standard 2D hydrodynamic model. The cascade of models is used to reproduce an extreme flood event that took place in the Southeast of Mexico, during November 2009. The event iswas selected as high quality field data (e.g. rain gauges; discharge) and satellite imagery are available. Uncertainty in the meteorological model (Weather Research and Forecasting model) iswas evaluated through the use of a multi-physics ensemble technique, which considers twelve parameterization schemes to determine a given precipitation event. The resulting precipitation fields are used as input in a distributed hydrological model, enabling the determination of different hydrographs associated to this event. Lastly, by means of a standard 2D hydrodynamic model, flood hydrographs are used as forcing conditions to study the propagation of the meteorological uncertainty to an estimated **flooded**inundation area. Results show the utility of the selected modelling approach to investigate error propagation within a cascade of models. Moreover, the error associated to the determination of the runoff, is showed to be lower than that obtained in the precipitation estimation suggesting that uncertainty do not necessarily increase within a model cascade evolution of skill within the model cascade shows a complex aggregation of errors between models, suggesting that in valley-filling events hydrometeorological uncertainty affects inundation depths in a higher degree than that observed in estimated flood extents.

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1 Introduction

4 Hydro-meteorological hazards can have cascading effects and far-reaching implications on 5 water security, with political, social, economic and environmental consequences. Millions of people worldwide are forcibly displaced as a result of natural disasters, creating political 6 7 tensions and social needs to support them. These events observed in developed and developing 8 nations alike, highlight the necessity to generate a better understanding on what causes them 9 and how we can better manage and reduce the risk. 10 The assessment of flood risk is an activity that has to be carried out under a framework full of 11 uncertainty. The source of these uncertainties may be ascribed to the involvement of different, 12 and often rather complex models and tools, in the context of environmental conditions that are 13 at best, partially understood (Hall, 2014). In addition to this, flooding systems events are 14 dynamic over a range of timescales, due to for example climate variability and socio-economic 15 changes, among others, which further increases the uncertainty in the projections. Therefore, 16 numerous types of uncertaintyuncertainties can arise when using formal models in the analysis 17 of risks. 18 Uncertainty is often categorised between aleatory and epistemic uncertainty (Hacking, 2006): 19 aleatory is an essential, unavoidable unpredictability, and epistemic uncertainty reflects lack of 20 knowledge or the inadequacy of the models to represent reality. In the context of any modelling 21 framework, epistemic uncertainties may be ascribed to the definition of model parameters and 22 to the model structure itself (limited knowledge). 23 In a technological era characterised by the advent of computers, there is an increased ability of 24 more detailed hydrological and hydraulic models. Their use and development has been 25 motivated as they are based on equations that have (more or less) physical justification; and 26 allow a more detailed spatial representation of the processes, parameters and predicted variables 27 (Beven, 2014). However, there are also disadvantages, these numerical tools take more 28 computer time and require the definition of initial, boundary conditions and parameter values

in space and time. Generally, at a level of detail for which such information is not available

even in research studies. Moreover, these models may be subjected to numerical

- 1 problems such as numerical diffusion and instability. All of these disadvantages can be
- 2 interpreted as sources of uncertainty in the modelling process.
- 3 Due to wide range of uncertainty sources in the flood risk assessment process, it is of great
- 4 interest to investigate the propagation and behaviour of these different uncertainties from the
- 5 start of the modelling framework to the result. The size of registered damages and losses in
- 6 recent events around the world, reveal the urgency of doing so, even under a context of limited
- 7 predictability.
- 8 In September 2013, severe floods were registered in Mexico as a result of the exceptional
- 9 simultaneous incidence of two tropical storms, culminating in serious damage and widespread
- 10 persistent flooding (Pedrozo-Acuña et al., 2014). More recently, in 2014,2014a). This
- unprecedented event is part of a recent set of extreme flood events over the last decade caused
- 12 by record-breaking precipitation amounts across Central Europe (Becker and flooding was
- observed in the Grünewald, 2003), United Kingdom (Slingo et al., 2014).2014), Pakistan
- 14 (Webster et al., 2011), Australia (Ven den Honert and McAneney, 2011), Northeastern US
- 15 (WMO, 2011), Japan (WMO, 2011) and Korea (WMO, 2011). In bothall cases, the immediate
- action of governments through the implementation of emergency and action plans was required.
- 17 The main aim of these interventions was to reduce the duration and impact of floods. In both
- 18 eventsaddition, risk reduction measures were designed to ensure both a better flood
- 19 management and an increase in infrastructure resilience.
- 20 One key piece of information in preventing and reducing losses is given by reliable flood
- 21 inundation maps that enable the dissemination of flood risk to the society and decision makers
- 22 (Pedrozo-Acuña et al., 2013). Traditionally, this task requires the estimation of different return
- periods for discharge (Ward et al., 2011) and their propagation to the floodplain by means of a
- 24 hydrodynamic model. There is currently a large range of models that can be used to develop
- 25 flood hazard maps (Horrit and Bates, 2002; Horrit et al., 2006).
- 26 The aforementioned accelerated progress of computers has given way to the development of
- 27 model cascades to produce hydrological forecasts, which make use of rainfall predictions from
- 28 regional climate models (RCMs) with sufficient resolution to capture meteorological events
- 29 (Bartholomes and Todini, 2005; Demerrit et al., 2010). Within this approach, the coupling of
- 30 different operational numerical models is carried out, using numerical weather prediction
- 31 (NWP) with radar data for hydrologic forecast <u>purposes</u> (Liguori and Rico-Ramirez, 2012;

- 1 Liguori et al., 2012), or NWP with hydrological and hydrodynamic models to determine
- 2 inundation extension (Pappenberger et al., 2012; Cloke et al., 2013; Ushiyama et al., 2014).
- 3 The use of RCMs in climate impact studies on flooding has been reported by Teutschbein and
- 4 Seibert (2010) and Beven (2011), noting that despite their usefulness, the spatial resolution of
- 5 models (~25km) remains coarse to capture the spatial resolution of precipitation. This is
- 6 particularly important, as higher resolution is needed to effectively model the hydrological
- 7 processes essential for determining flood risk. To overcome this limitation, the utilisation of
- 8 dynamic downscaling in these models is also has been significantly growing (Fowler et al.,
- 9 2007; Leung and Qian, 2009; Lo et al., 2008).
- 10 Significant challenges remain in the foreseeable future, among these, the inherent uncertainties
- in the predictive models is are likely to have an important role to play. For example, it is well
- known that the performance skill of NWPs deteriorates very rapidly with time (Lo et al., 2008).
- 13 To overcome this, the long-term continuous integration of the prediction has been subdivided
- into short-simulations, involving the re-initialisation of the model to mitigate the problem of
- systematic error growth in long integrations (Giorgi, 1990; Giorgi, 2006; Qian et al., 2003).
- Moreover, the use of ensemble prediction systems to obtain rainfall predictions for hydrological
- 17 forecasts at the catchment scale is becoming more common among the hydrological community
- as they enable the evaluation and quantification of some uncertainties in the results (Buizza
- 19 2008; Cloke and Pappenberger, 2009; Bartholmes et al. 2009). In these studies, an ensemble is
- a collection of forecasts made from almost, but not quite, identical initial conditions.
- 21 A key question that arises when using a cascade modelling approach to flood prediction or
- 22 mapping is: how uncertainties associated to meteorological predictions of precipitation
- propagate to a given flood inundation map? Previous work has been devoted to the examination
- 24 of uncertainties in the results derived from different ensemble methods, which address
- 25 differences in the initial conditions in the NWP or even differences in using a single model
- ensemble vs. multi-model ensemble (Pappenberger et al. 2008; Cloke et al., 2013; Ye et al.,
- 27 2014). However, less attention has been paid to the behaviour of errors within a model chain
- 28 that aims to represent a flood event occurring at several spatial scales. In order to understand
- 29 how errors propagate in a chain of models, this investigation evaluates the transmission of
- uncertainties from the meteorological model to a given flood map. For this, we utilize a cascade
- 31 modelling approach comprised by a Numerical Weather Prediction Model (NWP), a rainfall-
- runoff model and a standard 2D hydrodynamic model. The This numerical framework is applied

to an observed extreme event registered in Mexico in 2009 for which satellite imagery is 1 2 available. The investigated uncertainty is limited to the model parameter definition in the NWP 3 model, by means of a multi-physics ensemble technique considering several multi-physics 4 parameterization schemes for the precipitation (Bukosvky and Karoly, 2009). The resulting 5 precipitation fields are used to generate spaghetti plots by means of a distributed hydrological model, enabling the propagation of meteorological uncertainties to the runoff. flood hydrograph. 6 7 Hence, the resulting hydrographs represent the runoff associated to each precipitation field 8 estimated with the NWP. In order to complete the propagation of the uncertainty thoughthrough 9 the cascade of models to the flood map, the hydrographs are used as forcing in a standard 2D 10 hydrodynamic model. On the other hand, it is acknowledged that each of the other models (hydrological and 11 hydrodynamic) within the model cascade, will introduce other epistemic and random 12 13 uncertainties to the result. In order to reduce their influence, the numerical setup of both these models is constructed with the best available data (e.g. LiDAR for the topography) and 14 15 following recent guidelines for the assessment of uncertainty in flood risk mapping (Beven et 16 al. 2011). In this way, the uncertainty associated to the meteorological model outputs is 17 propagated through the model cascade from the atmosphere to the flood plain. Thus, the aim of 18 this investigation is to study the uncertainty propagation from the meteorological model (due 19 to model parameters), to the determination of an affected area impacted by a well-documented 20 hydro-meteorological event. This work is organised as follows: Section 2 provides a description of both, the study area and 21 22 the extreme hydro-meteorological event, which are employed to test our cascade modelling 23 approach; Section 3 introduces the methodology, incorporating a brief description of the 24 selected models setup. Additionally, we incorporate a description of the multi-physics ensemble 25

technique used to quantify and limit the epistemic uncertainty in the NWP model. The resulting precipitation fields, hydrographs and flood maps are compared with available field data and satellite imagery for the event. In Section 4, a discussion of errors along the model cascade, is also presented with some conclusions and future work.

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#### 2 **Case Study**

- 31 The selected study area is within the Mexican state of Tabasco, which in recent years has been
- subjected to severe flooding as reported by Pedrozo-Acuña et al. (2011; 2012). This region 32

1 comprises the area of Mexico with the highest precipitation rate (2000-3000 mm/year), which

2 mostly occurs during the wet season of the year between May and December. The rainfall

3 climatology is also influenced by the incidence of hurricanes and tropical storms arriving from

4 the North.

5 In this paper, the extreme hydro-meteorological event selected for the analysis corresponds to 6 that registered in the early days of November 2009 in the Tonalá river. As it is shown in Fig.1, 7 the river is located in the border of Tabasco and Veracruz and during the event, the substantial 8 rainfall intensity provoked its overflowing leaving extensive inundated areas along its 9 floodplain. Top panel of Fig. 1 shows the geographical location of the catchment, with an area of 5,021 km<sup>2</sup>, as well as the location of 18 weather stations installed within the region by the 10 11 National Weather Service. The event was the result of heavy rain induced by the cold front #9, which persisted for four days along Mexico's Gulf Coast, forcing more than 44,000 people to 12 13 evacuate their homes and affecting more than 90 communities. High intensities in rainfall were 14 recorded in rain gauges from the 31st October to 3rd November, with cumulative daily precipitation values reporting more than 270 mm. The river is approximately 300 km long and 15 before discharging into the Gulf of Mexico, the stream receives additional streamflow from 16 17 other smaller streams such as Agua Dulcita in Veracruz, and Chicozapote in Tabasco. The bottom panel of the same Figure illustrates the lower Tonalá River, where severe flooding was 18 19 registered as it is shown in the photographs on the right. The yellow, blue and red dots on the 20 panel represent the location at which the photographs were taken.

The hydrometric data in combination with the satellite imagery for the characterisation of the affected areas, enabled an accurate investigation of the causes and consequences that generated this flood event. The high quality of the available information, allowed the application of a cascade modelling approach comprised by state-of-the-art meteorological, hydrological and hydrodynamic models. This numerical approach is utilised with the intention to carry out an assessment of the modelling framework, with particular emphasis on the propagation of the epistemic uncertainty from the meteorological model to the spatial extent of an affected area. Such investigation paves the road towards a more honest knowledge transfer to decision-makers, whom consider the reliability of the model results.

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## 3 Methodology and Results

The methodology is comprised of a Numerical Weather Prediction Model (NWP), a distributed rainfall-runoff model and a standard 2D hydrodynamic model. It is anticipated that the selected modelling approach will support the advance of the understanding of the connections among scales, intensities, causative factors, and impacts of extremes. This model cascade with state-of-the-art numerical tools representing a hydrological system, enables the development of a framework by which an identification of the reliability of simulations can be undertaken. This framework is utilised to explore the propagation of epistemic uncertainties from the estimation of precipitation in the atmosphere to the identification of a flooded area. Therefore, the aim is not to reproduce an observed extreme event, but to investigate the effects of errors in rainfall prediction by a NWP on inundation areas. The proposed investigation is important as uncertainties are cascaded through the modelling framework, in order to provide better understanding on how errors propagate within models working at different temporal and spatial scales. It is acknowledged that this information would enhance better flood management strategies, which would be based on the honest and transparent communication of the results produced by a modelling system constrained by

# 3.1 Meteorological model

intrinsic errors and uncertainties.

- Simulated precipitation products from numerical weather prediction systems (NWPs) typically show differences in their spatial and temporal distribution. These differences can considerably influence the ability to predict hydrological responses. In this sense, in this study we utilise the advanced research core of the Weather Research and Forecasting (WRF) model Version 3.2. The WRF model is a fully compressible non-hydrostatic, primitive-equation model with multiple nesting capabilities (Skamarock et al., 2008).
- As it is shown in **Fig. 2**, the model setup is defined using an interactive nested domain inside the parent domain. This domain is selected in order to simulate more realistic rainfall, with the inner frame enclosing the Tonalá river catchment within a 4 km resolution. The 4 km horizontal resolution is considered good enough to compute a mesoscale cloud system associated to a cold front. It is shown that this finer grid covers the central region of Mexico, while in the vertical dimension, 28 unevenly spaced sigma levels were selected. The initial and boundary conditions

- were created from the NCEP Global Final Analysis (FNL) with a time interval of 6 hours for
- 2 the initial and boundary conditions. Each of the model simulations was reinitialised every two
- 3 days at 1200 UTC, considering a total simulation time from the 27<sup>th</sup> October 2009 until the 13<sup>th</sup>
- 4 November 2009.
- 5 Epistemic uncertainty is considered in the WRF model by means of the sensitivity of the results
- 6 for precipitation, due to variations in the model setup. For this, we utilise a multi-physics
- 7 ensemble technique proposed by Bukovsky and Karoly (2009), where the sensitivity of
- 8 simulated precipitation in the model results is examined with twelve different parameterisation
- 9 schemes. The comparison of computed precipitation fields against real measurements from
- weather stations within the catchment, enabled the quantification of uncertainty in the
- meteorological model for this event. **Table 1** shows a summary of the different multi-physics
- parameters used in the WRF model to generate the physics ensemble. In this approach, the
- multi-physics ensemble runs of the model represent a plausible and equally likely state of the
- 14 system in the future.
- 15 Fig. 3 illustrates the cumulative precipitation fields computed for each of the 12 selected
- members of the multi-physics ensemble, where differences in the spatial distribution and
- 17 intensity of precipitation were evident. These results suggested that for this event, the
- precipitation field estimated with the WRF was highly sensitive to the selection of multi-physics
- 19 parameters. To revise in more detail the performance of the WRF in reproducing this hydro-
- 20 meteorological event, the estimated cumulative precipitation by each member of the multi-
- 21 physics ensemble was compared against measurements at the eighteen weather stations located
- within and close to the Tonalá catchment.
- 23 Table 2 presents a summary of the most well-known error metrics calculated at each weather
- station and for each member of the ensemble. Among these are the: Normalised Root-Mean
- 25 Square Error (NRMSE), BIAS, Nash-Sutcliffe Coefficient (NSC), and the Correlation
- 26 coefficient (Cor). The columns show the local value of each coefficient for a given member of
- the ensemble (M1, ..., M12). As shown in all columns (i.e. member runs), the error metrics
- have a great spatial variability, hence, indicating the regions of the study area where the model
- 29 performs better. To illustrate the performance of this ensemble technique at each weather
- station, the ensemble average of these error metrics are is introduced in the last column and
- 31 indicated by < >. Again, the spatial variability of the metrics is evident. The two bottom rows
- 32 in each sub-table correspond to the average of the ensemble averages for the whole catchment

- and for the all the stations. It is shown, that when the average of all stations is taken into account,
- 2 the skill decreases. However, in this investigation the error that is of interest is the one
- 3 corresponding to the average of those weather stations located within the catchment, as these
- 4 will be used as input in the hydrological model. This will enable the propagation of errors in
- 5 the meteorological model within the model cascade. For clarity, in the same table the stations
- 6 within the catchment are highlighted in blue.
- Additionally, results per station are also illustrated for four different cases and are presented in
- 8 Fig. 4, and they confirmed that the range of spatial uncertainty in the WRF predictions is high
- 9 and variable. To give an example, at Station No. 27075, the spread of the estimated cumulative
- precipitation curves is limited and quantified by a NSC=0.917 and a NRMSE = 10.7%,
- 11 indicating a good skill of the WRF precipitation estimates at this point. In contrast, at Station
- No. 27007 the spread of the cumulative precipitation is large and characterised by a NSC=0.766
- and a NRMSE=19.4%, showing less skill in the model performance than that observed in the
- previous case. The observed differences of estimated precipitation for this event, highlight the
- importance of incorporating ensemble techniques in the reproduction of precipitation with this
- type of models.
- A question that has been seldom explored in the literature, is how the uncertainty in the
- prediction of the precipitation (i.e. errors described in this section), cascade into an estimated
- 19 flood hydrograph determined by a distributed hydrological model. In this sense, the next step
- 20 in this work, considers the non-linear transfer of rainfall to runoff using a distributed rainfall-
- 21 runoff model. For this, we employ each one of the 12 precipitation fields derived from the WRF
- as input to determine the associated river discharge with the hydrological model.

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## 3.2 Hydrological model

- 25 The hydrological model used in this study was applied to the Tonalá River catchment in an
- early work presented by Rodríguez-Rincón et al. (2012). This numerical tool was developed by
- 27 the Institute of Engineering UNAM (Domínguez-Mora et al., 2008), and comprises a
- simplified grid-based distributed rainfall—runoff model. The model has been previously applied
- with success in other catchments in Mexico (e.g. Pedrozo-Acuña et al., 20142014b).
- 30 This paper only describes an overview and key components of the hydrologic model. Interested
- 31 readers can find detailed descriptions in Domínguez Mora et al. (2008). The model is based on

- the method of the Soil Conservation Service (SCS) with a modification that allows the 1
- 2 consideration of soil moisture accounting before and after rainfall events. The parameters that
- 3 are needed for the definition of a runoff curve number within the catchment are the hydrological
- 4 soil group, land use, pedology and the river drainage network. Fig. 5 shows for the Tonalá
- 5 River catchment, the spatial definition of the river network (center panels) and the runoff curve
- (right panels). The model is forced with the precipitation calculated from For the WRF 6
- 7 considering the 12 members numerical setup of the multi-physics ensemble. hydrological model,
- 8 we employ topographic information from a LiDAR data set, from which a 10m resolution
- 9 <u>Digital Elevation Model (DEM) is constructed.</u>
- 10 There are two main hypothesis that underpin the SCS curve number method. Firstly, it is
- assumed that for a single storm and after the start of the runoff, the ratio between actual soil 11
- 12 retention and its maximum retention potential is equal to the ratio between direct runoff and
- available rainfall. Secondly, the initial infiltration is hypothesised to be a fraction of the 13
- 14 retention potential.
- 15 Thus, the water balance equation and corresponding assumptions are expressed as follows:

$$16 P = P_e + I_a + F_a (1)$$

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$$P = P_e + I_a + F_a$$
 (1)

17  $\frac{P_e}{P_a - I_a} = \frac{F_a}{S}$  (2)

- 19
- 20 Where P is rainfall,  $P_e$  effective rainfall,  $I_a$  is the initial abstraction,  $F_a$  is the cumulative
- 21 abstraction, S is the potential maximum soil moisture retention after the start of the runoff and
- 22  $\lambda$  is the scale factor of initial loss. The value of  $\lambda$  is related to the maximum potential infiltration
- 23 in the basin.
- Through the combination of equations (1) (3) and expressing the initial abstraction ( $I_a$ ) by 24
- 25 0.2\*S we have:

28 where, the value of S [cm] is determined by: 1  $S = \frac{2450 - (25.4CN)}{CN}$  (5)

2 CN is the runoff curve number, as defined by the Agriculture Department of the USA (USDA, 3 1985). Values for this parameter vary from 30 to 100, where small numbers indicate low runoff 4 potential while larger numbers indicate an increase in runoff potential. Thus, the permeability 5 of the soil is inversely proportional to the selected curve number. Another parameter that allows 6 the modification of the curve number is the soil water potential given by Fs, following S=S\*Fs. 7 The model includes a parameter to reproduce the effects of evaporation on the ground saturation 8  $(F_o)$ . This parameter is useful when the event to be reproduced lasts for several days; however, 9 due to the duration of this event it is assumed equal to 0.9 in all cases. The computation of the runoff in the basin is carried out through the addition of the runoff estimated in each cell to then 10 11 construct a general hydrograph (See Rodríguez-Rincón et al. 2012). With regards to the definition of values for the other two free parameters in the hydrological model ( $\lambda$  and Fs), a 12 traditional calibration process is implemented. For this, we utilise flood hydrographs from past 13 extreme events (2001, 2005, 2007, 2008, 2009 and 2011) observed in this river. Therefore, we 14 15 determine six sets of free parameters that are good enough to represent the rainfall-runoff relationship in this catchment. The selected sets of values are illustrated in **Table 3**, where the 16 17 correlation coefficient and NSC are also reported for each of the years. It is shown that in all 18 the events, the selected set of parameters ensures a good correlation against the observed 19 discharge which is given by Cor>0.7, as well as a positive NSC (accuracy). 20 It is well known that both the amount and distribution of rainfall can significantly affect the final estimated river discharge (Ferraris et al. 2002; De Roo et al., 2003; Cluckie et al., 2004). 21 22 In consequence, the propagation of meteorological uncertainty to the rainfall-runoff model is carried out using the 12 WRF rainfall precipitation ensembles as an input in the hydrological 23

In consequence, the propagation of meteorological uncertainty to the rainfall-runoff model is carried out using the 12 WRF rainfall precipitation ensembles as an input in the hydrological model. For this purpose, considering the six sets of free-parameters reported in the hydrologic model are fixed, assuming Table 3. This procedure enabled the generation of 72 hydrographs that the selected parameters are the best at representing could represent the physical conditions of the catchment for this 2009 event. The selection of these parameters is carried out following the results presented by Rodríguez Rincón et al. (2012) for the same catchment. For the numerical setup of the hydrological model, we employ topographic information from a LiDAR data set, from which a 10m resolution Digital Elevation Model (DEM) is constructed. with different skill. Error metrics of all the computed hydrographs are reported in Table 4.

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For completeness, Fig. 66a illustrates the 72 computed hydrographs for the Tonalá River 1 2 catchment the spaghetti plot of hydrographs computed in relation to the measured river 3 discharge for this the 2009 event, these are (blue dashed line). It is shown that if all 72 4 hydrographs are taken into account, uncertainty bounds are significant. Indeed, this illustrates 5 the interaction of the meteorological uncertainty with that coming from the setup of the hydrological model (definition of free parameters). However, the purpose of this study is to 6 7 investigate in a model cascade framework, how errors in the meteorological prediction stage 8 propagate down to a predicted inundation. In this sense, we narrow down the number of 9 hydrographs shown in Fig. 6a, by selecting only those with a Cor>0.7 and NSC>0.6., as 10 reported in **Table 4** only 31 out of 72 (shown in bold) follow this condition. **Fig. 6b** displays 11 the 31 selected hydrographs along with the measured discharge by for the 2009 event. Although 12 there is a streamflow gauge. The reduction in the uncertainty bounds illustrated by the grey 13 shaded area indicate, it tis shown that errors in the predicted rainfall are indeed propagated to the hydrological model, which usesemploys a finer spatial resolution (1 km). It has been 14 15 established that, in some cases, an error in the meteorological model can be compensated by an 16 error in the hydrological model and vice-versa. To illustrate this in more detail, Table 3 presents 17 a summary average values of the calculated error metrics for the 31 selected hydrographs shown 18 in Fig. 6. It is shown that on average (last columnare estimated and reported in Table 4, with 19 NSC=0.79, Cor=0.96 and BIAS=1.11. Values of the NSC for selected hydrographs in the 20 Table) 4 illustrate the resulting differences in skill resulting from the combination of different setups in the hydrological model has a NSC=0.84, Cor=0.96, BIAS=1.01 and 21 NRMSE=38.12%. Differences between members of the with the multi-physics ensemble are 22 23 also illustrated at this stage, especially by the NSC. For instance, in the rows corresponding to 24 the parametes determined for the 2011 event, member M11M12 indicates a NSC=0.68738 25 showing poora poorer skill at reproducing the river discharge with the precipitation derived 26 from this member, in contrast comparison to that registered for member M3 has a greater 27 skillM2 with NSC=0.93938. The change in the values of the NSC indicates that results from 28 the regional weather model can be enhanced or weakened by the performance of the 29 hydrological model.

The <u>useutilisation</u> of <u>thesethe 31 selected</u> hydrographs in a 2D hydrodynamic model, enables the study of the propagation of errors within the cascade of models. In particular, for estimating the flood extent during this extreme event.

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### 3.3 Flood inundation model

- 3 Several 2D hydrodynamic models have been developed for simulating extreme flood events.
- 4 However, any model is only as good as the data used to parameterise, calibrate and validate the
- 5 model. 2D models have been regarded as suitable for simulating problems where inundation
- 6 extent changes dynamically through time as they can easily represent moving boundary effects
- 7 (e.g. Bates and Horritt, 2005). The use of these numerical tools has become common place
- 8 when flows produce a large areal extent, compared to their depth and where there are large
- 9 lateral variations in the velocity field (Hunter et al., 2008).
- 10 In this study, given the size of the study area the modelling system utilised is comprised by the
- 11 flow model of MIKE 21 flexible mesh (FM). This numerical model solves the two dimensional
- 12 Reynolds-averaged Navier–Stokes equations invoking the approximations of Boussinesq and
- 13 hydrostatic pressure. This involves continuity, momentum, temperature, salinity and density
- 14 equations (for details see DHI, 2014). The equations are solved at the centre of each element in
- 15 the model domain.
- 16 The numerical setup is based on a previous work on the study area (Pedrozo-Acuña et al. 2012),
- with selected resolutions for the elements of the mesh with a size that guarantees the proper
- assimilation of a 10 m DEM to characterise the elevation in the floodplain. The topographic
- data has been regarded as the most important factor in determining water surface elevations,
- base flood elevation, and the extent of flooding and, thus, the accuracy of flood maps in riverine
- areas (NRC, 2009). Therefore, the elevation data used in this study corresponds to LiDAR data
- 22 provided by INEGI (2008). The hydraulic roughness in the floodplain is assumed to be uniform
- 23 and different from the main river channel, in this sense two values for the Manning number are
- 24 used, one for the main river channel ( $M=32 \text{ m}^{1/2}\text{s}^{-1}$ ) and another for the floodplain ( $M=28 \text{ m}^{1/2}\text{s}^{-1}$ )
- 25 <sup>4</sup>). set provided by INEGI (2008). The choice of a 10-m DEM is based on recommendations
- put forward by the Committee on Floodplain Mapping Technologies, NRC (2007) and Prinos
- et al. (2008), as such a DEM ensures both accuracy and detail of the ground surface. The model
- domain is illustrated in Fig. 7, along with the numerical mesh and elevation data, it comprises
- 29 the lower basin of the Tonalá River and additional main water bodies. The colours represent
- 30 the magnitude of the elevation and bathymetric data assimilated in the numerical mesh, where
- 31 warm colours identify high ground areas and light blues represent bathymetric data. The

numerical mesh considers three boundary conditions represented in the Figure by dots as 1 2 follows: where the input hydrograph from the rainfall-runoff model is set (red dot); the Tonalá's river mouth, where the astronomical tide for the period of the event (27th October 12th 3 4 November 2009) (yellow dot) and the Agua Dulcita river set where a constant discharge of 5 100 m<sup>3</sup>/s is introduced (blue dot). 6 It is acknowledged that topographic information is key for the reduction of uncertainty in flood 7 hazard mapping. Therefore, in order to minimise this source of error, the The integration of high 8 quality topographic information in a 2D model with enough spatial resolution, will 9 enableenables the investigation of the propagation of the meteorological uncertainty to the 10 determination of the flood extent. Moreover, as it is illustrated in Fig. 7 the numerical mesh considers three boundary conditions. These are input flow boundary where the hydrograph from 11 12 the rainfall-runoff model is set (red dot); the Tonalá's river mouth, where the astronomical tide occurs for the period of the event (27th October – 12th November 2009) (yellow dot) and the 13 Agua Dulcita river set where a constant discharge of 100 m<sup>3</sup>/s is introduced (blue dot). 14 15 On the other hand, hydraulic roughness is a lumped term known as Manning's coefficient that 16 represents the sum of a number of effects, among which are skin friction, form drag and the 17 impact of acceleration and deceleration of the flow. The precise effects represented by the 18 friction coefficient for a particular model depend on the model's dimensionality, as the 19 parameterisation compensates for energy losses due to unrepresented processes, and the grid 20 resolution (Bates et al., 2014). The lack of a comprehensive theory of "effective roughness" have determined the need for calibration of friction parameters in hydraulic models. 21 22 Furthermore, the determination of realistic spatial distributions of friction across a floodplain 23 in other studies, have showed that only 1 or 2 floodplain roughness classes are required to match 24 current data sources (Werner et al., 2005). Indeed, this suggests that application of complex formulae to establish roughness values for changed floodplain land use are inappropriate until 25 model validation data are improved significantly. Therefore, in this study hydraulic roughness 26 27 in the floodplain is assumed to be uniform and different from the main river channel, in this sense two values for the Manning number are used, one for the main river channel (M=32 m<sup>1/2</sup>s<sup>-1</sup> 28 <sup>1</sup>) and another for the floodplain ( $M=28 \text{ m}^{1/2}\text{s}^{-1}$ ). 29 30 In order to assess whether the 2D model is able to reproduce the flood extent observed in 2009, numerical results of flood extent are compared against the affected area determined from a 31 32 SPOT image (resolution of 124m). This practice is widely used in the literature to evaluate the

results from inundation models and to compare its performance (Di Baldassare et al, 2010b; 1

2 Wright et al., 2008).

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Fig. 8a introduces the result of the hydrodynamic simulation for each of the 1231 selected hydrographs, which resulted from the utilisation of the rainfall-runoff model using as input the WRF multi-physics ensemble output. The illustrated flood map summarises the 1231 different 6 possibilities of the inundation area that could result from the characterisation of precipitation 7 with the WRF model. Differences in the size of these areas, illustrate the propagation of epistemic errors from the meteorological model to the flood map. In this sense, the analysis of uncertainty has been restricted to its propagation along the model chain (atmospherecatchment-river floodplain). Each of these flood maps can also be associated to a probability enabling the representation of a probabilistic flood map, shown in the figure. This allows the identification of the areas highly vulnerable to flooding from this event. Additionally, Fig. 8b introduces the infrared SPOT satellite image of the 12th of November 2009, which is used for 13 comparison against the produced flood maps using all the ensemble members derived from 14 running the 31 hydrographs as inputs in the 2D model. Notably, in the numerical results, the blue area identifies the region of the domain that is most likely to be flooded (90%), the 16 comparison of this area with the observed inundation in the satellite image, show a good skill 18 of the model chain at reproducing the registered flood in the study area.

To better quantify the performance of each of the model runs in reproducing the observed flood extent, the estimation of several error metrics in these results was also performed, among these areDespite the variability in the estimated peak discharge utilised as input in the different hydrodynamic runs, inundation results show similar affected areas in all realisations (only with small differences in its size). This is verified in the results shown in Fig. 9a, where the relationship between peak discharge of the 31 hydrographs, is plotted against the size of the maximum-flooded area. The distribution of points in this graph clearly indicates that although there are differences in the estimated peak flow (see histogram in Fig. 9b), in most cases the resulting size of the inundated area is similar. Histogram plot shown in Fig. 9c indicates a clear concentration numerically derived flooded areas with a size larger than 130 km<sup>2</sup>. Indeed, the mean value of the maximum-flooded estimated area is 138.94 km<sup>2</sup>, while the standard deviation is  $16.09 \text{ km}^2$ .

These results support that the hydraulic behaviour in all hydrodynamic simulations was indeed 31 32

very similar, regardless of the peak discharge of the hydrograph. It is reflected that this may be

the result of induced hydrodynamics by a valley-filling flood event, which is identified with the 1 2 relatively high floodplain area-to-channel-depth ratios in all simulations. Hence, all possible 3 hydrographs generated with the hydrological model show similar levels of lateral momentum 4 exchange between main channel and floodplain. For this reason, the predictive performance of 5 all hydrodynamic simulations used to reproduce the inundation extent appears to be good (see 6 **Table 5**). 7 The estimation of several error metrics in these results was performed using binary flood extent 8 maps, where the comparison is based on the generation of a contingency table, which reports 9 the number of pixels correctly predicted as wet or dry. From this, measures of fit such as: BIAS, 10 False Alarm Ratio (FAR), Probability of Detection (POD), Probability of False Detection 11 (POFD), Critical Success Index (CSI) and the True Skill Statistics (TSS), are estimated. Table 12 45 introduces the results by memberfor all 31 members and metric.error metrics. Clearly, there is somelittle variability in the performance of the model for each of the ensemble membersruns, 13 14 showing that there has been some a small propagation of the error to the flood map. The 15 ensemble average of these quantities is also illustrated in the last column of the table, where BIAS=0.9641.013, FAR=0.453189, POD=0.799819, POFD=0.<del>154</del>180; 16 avalues of 17 CSI=0.831686 and TSS=0.645639 are reported. These As noted before, these results indicate 18 ean apparent good skill of the model chain at reproducing the flood extension, due to the 19 incidence of this extreme event. It should be borne in mind, however, that some misclassification errors may also be included in the observed flooded area due to specular 20 21 reflections that may classify some wet vegetation as water or open water as dry land. In 22 consequence, flood extent maps should be used with caution in assessing model performance 23 (Di Baldassare, 2012). This is particularly true during high-magnitude events where the valley 24 is entirely inundated, such as the case study of this investigation where small changes in lateral 25 flood extent may produce large changes in water levels. In this sense, it has been argued that flood extent maps are not useful for model assessment 26 27 (Hunter et al., 2005) and high water marks are more useful to evaluate model performance. 28 Unfortunately, for the case study information of inundation depths was not available. Despite 29 this fact, a further revision of simulated inundation depths is also carried out. For this, 10 points 30 distributed within the numerical domain are selected. These are illustrated by the coloured dots 31 in Fig. 10, along with the values of mean water depth in all the 31 simulations (red solid line). 32 In all cases, a high variability in the estimated inundation depth on the floodplain is depicted

- 1 (with values varying between 1.5 and 3m). This result supports that in the case of valley-filling
- 2 flood events, there is a higher sensitivity to errors in the vertical dimension of the flood.
- 3 In one hand, this demonstrates that the geomorphological characteristics of the site (e.g. low-
- 4 lying area, smooth slopes in the river channel and floodplain) are dominant in the accurate
- 5 determination of the magnitude of an inundated area, regardless of the peak discharge. This
- 6 implies that for this type of rivers and when predicting inundation extent, it may be more
- 7 important to have a good characterisation of the river and floodplain (e.g. high quality field data
- 8 and a LiDAR derived DEM), than a good characterisation of the rainfall-runoff relationship.
- 9 Current approaches to flood mapping, have pointed out that in order to produce a scientifically
- 10 justifiable flood map, the most physically-realistic model should be utilised (Di Baldassarre et
- al., 2010). Nevertheless, even with these models the amount of uncertainty involved in the
- determination of an affected area is important and should be quantified.

# 14 **4 Discussion and Conclusions**

- 15 It has been largely acknowledged in the literature, that floodFlood risk mapping and assessment
- are highly difficult tasks due to the inherent complexity of the relevant processes, which occur
- in several spatial and temporal scales. As pointed out by Aronica et al. (2013), the process
- 18 isprocesses are subject to substantial uncertainties (epistemic and random), which emerge from
- 19 different sources and idealisation of processes assumptions, from the statistical analysis of
- 20 extreme events and from the resolution and accuracy of the DEM used in a flood inundation
- 21 model.

- 22 By acknowledging that all models are an imperfect representation of the reality, it is important
- 23 to quantify the impact of epistemic uncertainties on a given result. The numerical approach
- 24 utilised in this investigation enabled an assessment of a state-of-the art modelling framework,
- comprised by meteorological, hydrological and hydrodynamic models. Emphasis was given to
- 26 the effects of epistemic uncertainty propagation from the meteorological model to the definition
- of an affected area in a 2D domain. Ensemble climate simulations have become a common
- practice in order to provide a metric of the uncertainty associated with climate predictions. In
- 29 this study, a multi-physics ensemble technique is utilised to evaluate the propagation of
- 30 epistemic uncertainties within a model chain. Therefore, the assessment of hydro-

1 meteorological model performance at the three stages is carried out through the estimation of

2 skill scores.

3 Fig. 911 presents a summary of the propagation of two well-known error metrics, BIAS (top 4 panel) and NSC/TSS (bottom panel). These metrics arewere selected, as they enable a direct 5 comparison of their values at each of the stages within the model cascade. In both metrics, the 6 evolution of the confidence limits is illustrated by the size of the bars. Their evolution from the 7 meteorological model to the hydrological model results, show a clear decrease in both cases. 8 This result may point towards, show an enhancementaggregation of meteorological 9 uncertainties in with those originated from the rainfall-runoff model. However, the skill of the 10 hydrological model is considerably improved from a mean value of 0.65 in the meteorological model, to 0.834.793 in the hydrological model. In the last stage of the model chain, 11 12 (hydrodynamic model), the confidence limits of the results-at the hydrodynamic model results, show a smallan apparent improvement. Nevertheless, in model skill. However, it should be 13 14 noted that this may be ascribed to the complex aggregation of errors in valley-filling events, 15 which is verified in the observed sensitivity of the simulated inundation depths. The mean value 16 of the skill is reduced to TSS=0.645639. The results provide an useful way to evaluate the hydro-meteorological uncertainty propagation within the whole hydro-meteorological 17 18 modelling cascade system. 19 BIAS and NSC/TSS error metrics (Fig. 11) revealed discrepancies between observations and 20 simulations throughout the model cascade. For instance, an increase in the NSC from the rainfall to the flood hydrograph it implies that the hydrological model is more sensitive (wider 21 22 uncertainty bars) to its main input (precipitation) than the WRF model is to the set of micro-23 physics parameterisations. On the other side, the uncertainty bounds in the hydrological model 24 imply a high sensitivity of hydrographs to both, errors from the meteorological model and its numerical setup with free parameters (amplifying the uncertainty). This is observed in the 25 spaghetti plot shown in Fig. 6a, where large uncertainty bounds were identified. In order to 26 reduce errors from the interaction of uncertainties coming from both models, these bounds were 27 reduced with the selection of 31 hydrographs that comply with Cor>0.7 and NSC>0.6 (see 28 29 **Fig.6b**). It is reflected that the estimated error in the meteorological model may reflect a spatial scaling issue (comparing observations from rain gauges to simulations at the meso-scale). 30 Results concerning predictions of inundation extent indicate an apparent good skill of the model 31 32 chain at reproducing the flood extension. The propagation of uncertainty and error from the

1 hydrological model to the inundation area revealed that is necessary to assess model

2 performance not only for flood extension purposes, but also to estimate inundation depths,

3 where results indicate a higher variability (e.g. increase in the error). This last modelling step

- 4 <u>is quite important given the consequences for issuing warning alerts to the population at risk.</u>
- 5 The similar magnitude in inundation extents of all numerical results indicated the predominance
- 6 of a valley-filling flood event, which was characterised by a flooded area strongly insensitive
- 7 to the input flood hydrograph. While this can be explained by the limited effect that the volume
- 8 overflowing the riverbanks and reaching the floodplain will have on the maximum inundation
- 9 area, the difference between the observed and the simulated flooded area remains important
- 10 <u>(TSS=0.639).</u>
- It should be pointed out, that this methodology contains more uncertainties that were not
- 12 considered or quantified in the generation of flood extent maps for this event. To quantify the
- epistemic uncertainty in the larger scale (i.e. atmosphere), a mesoscale numerical weather
- prediction system was used along with a multi-physics ensemble. The ensemble was designed
- 15 to represent our limited knowledge of the processes generating precipitation in the
- 16 meteorological system. The propagation of this uncertainty to a rainfall-runoff model revealed
- 17 large spatial variations of the model skill across scales and models.lower troposphere. It was
- shown that a large amount of uncertainty exists in the NWP model, and this such uncertainty is
- indeed propagated over the catchment and floodplain scales. Members of the ensemble were
- shown to differ significantly in terms of their cumulative precipitation, its spatial distribution,
- 21 river discharge and the size of the affected area by the event., inundation depths and areas.
- 22 Therefore, epistemic uncertainties from each step in the hazard analysis chain accumulate in
- 23 this model cascade can be aggregated up to the final outputsoutput.
- 24 The evaluation of the skill in the model cascade shows further potential for improvements of
- 25 the modeling system. Consequently, future work is planned to include the remaining
- uncertainties as adopted by, e.g. Pedrozo-Acuña et al. (2013). Special attention should be paid
- 27 to the interaction of between hydro-meteorological and hydrological uncertainty, as well as
- 28 flood extent estimation in catchments with that of hydrological origin different morphological
- 29 <u>setting</u>. The assessment of the error propagation within the model cascade is seen as a good step
- forward, in the communication of uncertain results to the society. The However, as shown in
- 31 this work, an improvement in model prediction during the first cascade step (rainfall to runoff)
- 32 can be reverted during the second cascade step (runoff to inundation area) with important

- 1 consequences for early warning systems and operational forecasting purposes. Finally, the
- 2 proposed numerical framework could be utilised as a robust alternative for the characterisation
- 3 of extreme events in ungauged basins.
- 4 The acknowledgment of these uncertainties, by showing their impact on model results, favour
- 5 preventive action in the production of methodologies that evaluate flood extension with some
- 6 level of confidence. Therefore, the investigation paves the road towards a more honest
- 7 knowledge transfer to decision-makers, whom consider the reliability of the model results.

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Table 1. Ensemble members defined for the multi-physics WRF ensemble

Ensemble member	Micro-Physics	surface layer physics	Cumulus physics	Feedback/sst_update
1	WSM5	5-Layer TDM	Kain-Fritsch Eta	off/on
2	WSM5	5-Layer TDM	Kain-Fritsch Eta	on/off
3	WSM5	5-Layer TDM	Kain-Fritsch Eta	on/on
4	WSM5	Noah	Kain-Fritsch Eta	off/off
5	WSM5	Noah	Kain-Fritsch Eta	off/on
6	WSM5	Noah	Kain-Fritsch Eta	on/on
7	Thompson	5-Layer TDM	Kain-Fritsch Eta	off/off
8	Thompson	5-Layer TDM	Kain-Fritsch Eta	off/on
9	Thompson	5-Layer TDM	Kain-Fritsch Eta	on/off
10	Thompson	5-Layer TDM	Kain-Fritsch Eta	on/on
11	Thompson	Noah	Kain-Fritsch Eta	off/off
12	Thompson	Noah	Kain-Fritsch Eta	off/on

Table 2. Error Metrics in the estimation of precipitation by members of the multi-physics ensemble (blue rows indicate the stations located within the Tonalá catchment)

		Root-N	1ean Squa	re Error (	RMSE) an	d Normali	sed RMSE	per Stati	on consid	ering Ens	emble av	erage	
Station					Multi-p	hysics en	semble m	ember					<nor_rmse></nor_rmse>
No.	M1	M2	М3	M4	M5	М6	M7	M8	М9	M10	M11	M12	%
30167	210.26	96.56	144.62	104.42	106.84	76.31	160.48	129.88	101.03	210.95	164.85	86.80	13.96
27003	544.34	578.19	564.46	474.81	427.30	516.95	458.25	484.05	568.20	572.30	385.17	479.47	35.13
27007	234.90	246.00	198.01	135.27	129.43	207.93	126.51	197.32	246.90	328.28	132.09	191.81	19.44
27015	96.68	129.89	151.02	194.33	235.76	179.69	152.06	152.60	118.97	116.87	260.49	188.20	24.01
27074	173.37	211.87	191.22	197.46	78.94	148.88	174.92	247.65	187.98	207.39	123.09	157.21	17.19
27073	227.47	201.91	228.62	256.39	281.38	245.68	186.21	219.36	159.34	147.79	247.69	223.88	46.46
27075	87.04	119.26	104.10	100.82	151.17	64.92	76.45	147.30	85.75	105.68	52.14	68.67	10.72
27076	140.53	160.28	141.95	124.03	108.33	130.53	191.75	162.59	226.04	236.09	129.78	150.84	17.14
27077	89.10	113.42	83.60	225.48	252.24	207.73	254.20	282.40	110.77	83.93	203.01	192.86	30.57
27039	333.50	204.36	197.48	295.84	302.19	261.39	264.08	321.66	172.86	152.14	257.59	430.63	73.28
27054	123.18	30.77	45.28	113.16	119.18	77.41	106.84	112.68	118.83	127.43	110.06	106.67	34.75
27060	70.69	56.23	59.51	33.42	40.13	30.04	78.07	93.80	88.46	80.36	56.73	66.31	19.88
27024	160.33	137.81	140.76	120.58	127.54	73.57	148.27	136.47	145.12	167.79	153.26	151.87	85.04
27084	68.72	71.32	54.58	53.56	106.93	65.65	61.06	72.31	61.46	62.96	50.14	50.92	19.02
7365	172.91	117.44	103.02	252.03	139.79	163.49	301.52	216.38	179.67	129.71	271.88	210.11	24.52
27011	143.70	162.77	143.61	107.82	77.55	86.15	128.03	143.69	106.59	116.49	86.81	81.27	106.83
27036	81.46	60.69	27.36	61.69	19.14	35.64	23.58	45.89	22.13	40.23	39.22	55.55	12.04
27008	158.85	72.82	74.96	131.34	134.94	100.16	102.82	149.97	66.67	79.36	97.87	254.33	19.68
										Avera	ige {Rel_F	RMSE}	
											catch.		23.14
										Averag	e {Rel_RI	VISE} all	33.87

Station					Multi-r	ohysics en	semble m	emher					
No.	M1	M2	М3	M4	M5	M6	M7	M8	М9	M10	M11	M12	<bias></bias>
30167	0.71	0.90	0.81	1.07	1.12	0.99	0.80	0.85	0.91	0.71	1.23	1.06	0.93
27003	0.51	0.48	0.50	0.58	0.62	0.54	0.59	0.57	0.49	0.49	0.66	0.58	0.55
27007	0.72	0.71	0.79	0.91	0.91	0.78	1.13	1.26	0.73	0.61	0.90	0.80	0.85
27015	1.21	1.32	1.40	1.50	1.61	1.46	1.37	1.37	1.24	1.21	1.68	1.48	1.40
27074	0.82	0.76	0.79	0.78	1.08	0.86	0.81	0.71	0.80	0.77	0.88	0.83	0.82
27073	1.74	1.65	1.74	1.83	1.91	1.80	1.58	1.70	1.47	1.44	1.80	1.72	1.70
27075	0.92	0.85	0.88	0.88	1.20	0.96	0.90	0.80	0.89	0.86	0.98	0.93	0.92
27076	0.86	0.82	0.86	0.91	0.95	0.89	0.79	0.84	0.73	0.71	0.89	0.85	0.84
27077	1.12	1.17	1.10	1.48	1.54	1.44	1.54	1.60	1.20	1.14	1.42	1.40	1.35
27039	2.41	1.87	1.84	2.26	2.29	2.11	2.13	2.36	1.73	1.64	2.09	2.84	2.13
27054	1.89	1.08	1.24	1.82	1.87	1.54	1.76	1.81	1.84	1.91	1.79	1.77	1.69
27060	1.42	1.33	0.72	1.08	1.20	1.05	1.47	1.57	1.54	1.49	1.32	1.39	1.30
27024	3.34	2.96	3.03	2.76	2.88	2.07	3.16	2.98	3.11	3.45	3.17	3.17	3.01
27084	1.32	1.35	1.17	1.23	1.61	0.78	1.27	1.36	1.27	1.29	1.07	1.01	1.23
7365	1.43	1.20	1.09	1.63	1.32	0.72	1.78	1.55	1.43	1.26	1.68	1.51	1.38
27011	3.57	3.91	3.55	2.93	2.33	2.49	3.33	3.58	2.91	3.09	2.56	2.45	3.06
27036	1.36	1.25	1.09	1.28	0.97	1.15	0.95	1.20	1.06	1.16	1.15	1.24	1.15
27008	1.37	1.07	1.05	1.29	1.31	1.20	1.21	1.35	0.99	0.93	1.19	1.62	1.22

0.94

1.42

catch.

Average {Rel\_RMSE} all

Nash-Sutcliff Coefficient per Station and Ensemble average													
Station No.					Multi-p	hysics en	semble me	mber					<nsc></nsc>
Station No.	M1	M2	М3	M4	M5	М6	M7	M8	М9	M10	M11	M12	\N3C>
30167	0.72	0.94	0.87	0.93	0.93	0.96	0.84	0.89	0.94	0.72	0.83	0.95	0.88
27003	0.16	0.05	0.09	0.36	0.48	0.24	0.40	0.33	0.08	0.07	0.58	0.34	0.26
27007	0.70	0.67	0.78	0.90	0.91	0.76	0.91	0.79	0.66	0.41	0.90	0.80	0.77
27015	0.88	0.78	0.70	0.50	0.27	0.57	0.70	0.69	0.81	0.82	0.11	0.53	0.61
27074	0.84	0.76	0.80	0.79	0.97	0.88	0.84	0.67	0.81	0.77	0.92	0.87	0.83
27073	-0.27	0.00	-0.28	-0.61	-0.94	-0.48	0.15	-0.18	0.38	0.46	-0.50	-0.23	-0.21
27075	0.94	0.89	0.91	0.92	0.82	0.97	0.95	0.83	0.94	0.91	0.98	0.96	0.92
27076	0.87	0.83	0.86	0.90	0.92	0.88	0.75	0.82	0.65	0.62	0.89	0.85	0.82
27077	0.82	0.70	0.84	-0.17	-0.46	0.01	-0.48	-0.83	0.72	0.84	0.05	0.15	0.18
27039	-4.41	-1.03	-0.90	-3.26	-3.44	-2.32	-2.39	-4.03	-0.45	-0.13	-2.23	-8.02	-2.72
27054	-0.46	0.91	0.80	-0.23	-0.36	0.42	-0.10	-0.22	-0.36	-0.56	-0.16	-0.09	-0.03
27060	0.60	0.75	0.72	0.91	0.87	0.93	0.51	0.29	0.37	0.48	0.74	0.65	0.65
27024	-7.99	-5.64	-5.93	-4.08	-4.69	-0.89	-6.68	-5.51	-6.36	-8.84	-7.21	-7.06	-5.91
27084	0.67	0.64	0.79	0.80	0.20	0.70	0.74	0.63	0.73	0.72	0.82	0.82	0.69
7365	0.50	0.77	0.82	-0.07	0.67	0.55	-0.54	0.21	0.45	0.72	-0.25	0.25	0.34
27011	-16.74	-21.76	-16.72	-8.99	-4.17	-5.38	-13.08	-16.74	-8.76	-10.66	-5.47	-4.67	-11.09
27036	0.61	0.78	0.96	0.78	0.98	0.93	0.97	0.88	0.97	0.91	0.91	0.82	0.87
27008	0.60	0.92	0.91	0.72	0.71	0.84	0.83	0.64	0.93	0.90	0.85	-0.03	0.73
									-	Averag	ge {Rel_R	MSE}	
											catch.		0.63
										Average	{Rel_RN	1SE} all	-0.63

			Coi	relation (	Coefficie	nt per Sta	tion and E	nsemble av	erage/				
Station No.					Multi-p	hysics en	semble me	ember					<cor></cor>
Station No.	M1	M2	М3	M4	M5	М6	M7	M8	М9	M10	M11	M12	(01)
30167	0.99	0.99	0.99	0.97	0.98	0.99	0.99	0.99	0.99	0.99	0.97	0.98	0.99
27003	0.95	0.96	0.97	0.97	0.98	0.98	0.99	0.99	0.99	0.99	0.99	0.99	0.98
27007	0.98	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.95	0.98	0.97	0.97
27015	0.97	0.96	0.97	0.94	0.93	0.95	0.95	0.95	0.94	0.94	0.93	0.94	0.95
27074	0.98	0.98	0.98	0.98	0.99	0.98	0.99	0.98	0.98	0.98	0.99	0.99	0.98
27073	0.95	0.96	0.95	0.94	0.94	0.94	0.92	0.92	0.91	0.92	0.94	0.94	0.94
27075	0.98	0.98	0.98	0.98	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
27076	0.98	0.98	0.97	0.97	0.97	0.97	0.97	0.97	0.96	0.96	0.97	0.97	0.97
27077	0.96	0.95	0.96	0.96	0.95	0.96	0.95	0.95	0.97	0.97	0.95	0.96	0.96
27039	0.95	0.95	0.94	0.93	0.94	0.94	0.94	0.94	0.95	0.95	0.94	0.93	0.94
27054	0.91	0.96	0.94	0.93	0.93	0.94	0.91	0.92	0.91	0.90	0.93	0.93	0.93
27060	0.96	0.97	0.97	0.96	0.97	0.97	0.95	0.95	0.96	0.96	0.97	0.96	0.96
27024	0.91	0.93	0.92	0.90	0.91	0.95	0.89	0.90	0.89	0.89	0.94	0.94	0.91
27084	0.91	0.91	0.92	0.94	0.92	0.95	0.92	0.91	0.92	0.92	0.93	0.93	0.92
7365	0.93	0.93	0.94	0.92	0.94	0.97	0.91	0.92	0.91	0.92	0.91	0.92	0.93
27011	0.94	0.94	0.95	0.93	0.95	0.96	0.89	0.93	0.91	0.92	0.91	0.91	0.93
27036	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
27008	0.97	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.97	0.96	0.96	0.96	0.96
										Avera	ge {Rel_R catch.	MSE}	0.97
										Average	{Rel RN	1SE} all	0.95

Table 3. Flood events in the Tonala River used in the calibration process of free parameters in the hydrological model, along with computed error metrics.

Event	Max Q (m3/s) Obs.	K	Fs	Fo	Max Q (m3/s) Calc.	NSC	Cor	Bias
2001	577.98	0.2	0.1	0.9	584.79	0.529	0.764	1.112
2005	589.25	0.4	0.6	0.9	609.87	0.812	0.907	1.043
2007	538.50	0.2	1.8	0.9	543.87	0.483	0.780	0.902
2008	597.35	0.4	1.8	0.9	823.04	0.155	0.861	0.983
2009	1262.57	0.8	1.8	0.9	1424.56	0.910	0.962	0.942
2011	545.40	0.9	1.6	0.9	597.08	0.413	0.721	1.051

<u>Table 4.</u> Error metrics in the estimation of river discharge by the rainfall-runoff model using the <u>6 parameter sets</u>
and 12 members of the multi-physics ensemble.

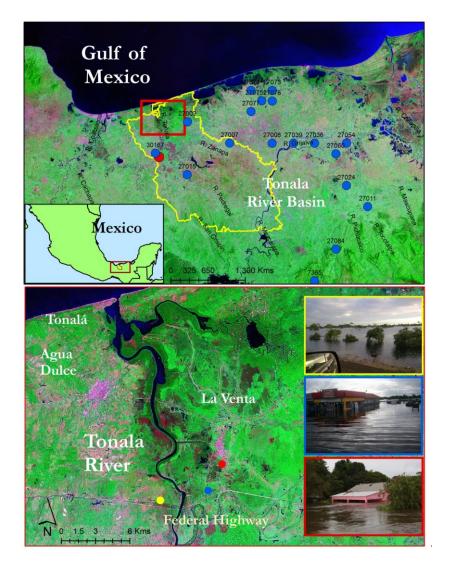
			Error m	etrics per	ensemble	member a	and ensem	ble avera	ge for the	hydrologic	cal model
-	<del>M1</del>	M2	M3	M4	M5	<del>M6</del>	M7	<del>M8</del>	M9	<del>M10</del>	<del>M11</del>
RMSE (m^3/s)	<del>163.05</del>	122.49	110.53	230.13	201.49	<del>153.81</del>	147.25	<del>131.42</del>	123.96	127.51	232.76
NSC	0.84	0.91	0.93	0.69	0.76	0.86	0.87	0.90	0.91	0.90	0.68
Cor	0.97	0.98	0.98	0.97	0.95	0.96	0.95	0.96	0.96	0.96	0.94
BIAS	0.83	0.89	0.92	<del>1.29</del>	<del>1.20</del>	0.86	<del>1.08</del>	0.90	0.94	0.94	1.24
NRMSE (%)	39.75	<del>29.86</del>	<del>26.95</del>	<del>56.11</del>	49.12	<del>37.50</del>	35.90	<del>32.04</del>	<del>30.22</del>	31.09	<del>56.75</del>

(those selected are shown in bold with NSC>0.6 and Cor>0.7).

	Member No.	WRF Member	Hydrological	NSC	Cor	Bias
2 M2 2001 0.074 0.973 1.52 3 M3 2001 -0.035 0.974 1.56 4 M4 2001 -0.035 0.974 1.56 5 M5 2001 -0.638 0.441 1.48 6 M6 2001 -0.023 0.961 1.59 7 M7 2001 -0.192 0.961 1.57 8 M8 2001 -0.043 0.959 1.53 9 M9 2001 0.064 0.958 1.50 10 M10 2001 -0.245 0.971 0.52 11 M11 2001 -1.503 0.944 1.83 12 M12 2001 -0.752 0.954 1.71 13 M1 2005 0.639 0.901 0.74 14 M2 2005 0.639 0.901 0.74 15 M3 2005 0.404 0.977 1.41 15 M3 2005 0.318 0.978 1.44 16 M4 2005 -0.077 0.977 1.56 17 M5 2005 -0.545 0.366 1.36 18 M6 2005 0.181 0.968 1.47 19 M7 2005 0.200 0.968 1.46 20 M8 2005 0.321 0.966 1.38 21 M9 2005 0.408 0.966 1.38 22 M10 2005 0.0408 0.966 1.38 22 M10 2005 0.0408 0.966 1.38 22 M10 2005 0.080 0.991 0.951 1.71 24 M12 2005 0.080 0.991 0.951 1.71 25 M1 2007 0.761 0.979 1.22 27 M3 2007 0.761 0.979 1.22 28 M4 2007 0.761 0.979 1.22 29 M5 2007 0.761 0.979 1.22 29 M5 2007 0.761 0.979 1.23 30 M6 2007 0.711 0.979 1.22 28 M4 2007 0.744 0.976 1.39 30 M6 2007 0.722 0.973 1.25 31 M7 2007 0.647 0.974 1.29 32 M8 2007 0.722 0.973 1.25 33 M9 2007 0.722 0.973 1.25 34 M10 2007 0.630 0.992 0.951 35 M11 2007 0.630 0.992 0.951 36 M12 2007 0.761 0.979 1.23 37 M1 2008 0.240 0.969 1.42 38 M8 2007 0.722 0.973 1.25 39 M3 2008 0.797 0.978 1.24 40 M4 2008 0.570 0.974 1.33 41 M7 2007 0.647 0.974 1.29 35 M11 2007 0.508 0.392 0.32 36 M11 2007 0.702 0.979 1.35 37 M1 2008 0.240 0.992 1.33 38 M9 2007 0.771 0.979 1.25 39 M3 2008 0.797 0.978 1.24 40 M4 2008 0.570 0.978 1.24 41 M8 2008 0.831 0.975 1.19 42 M6 2009 0.881 0.995 1.48 43 M10 2009 0.881 0.995 1.88 44 M8 2009 0.889 0.977 0.998 55 M4 2009 0.889 0.977 0.999 56 M8 2009 0.889 0.979 0.995 57 M9 2009 0.889 0.995 0.992 58 M10 2009 0.889 0.997 0.998 58 M10 2009 0.889 0.997 0.998 59 M11 2009 0.889 0.997 0.998 50 M2 2009 0.889 0.997 0.998 51 M3 2009 0.899 0.997 0.998 52 M4 2009 0.889 0.997 0.998 53 M10 2009 0.889 0.997 0.999 54 M11 2009 0.689 0.997 0.999 55 M11 2009 0.889 0.997 0.999 56 M8 2009 0.899 0.995 0.999 57 M9 2009 0.899 0.999 0.999 58 M3 2011 0.939 0.999 1.13 68 M8 2011 0.931 0.999 1.10 68 M8 2011 0.931 0.999 1.10			Parameters			
3 M3 2001 -0.035 0.974 1.56 4 M4 2001 -0.511 0.975 1.68 5 M5 2001 -0.638 0.441 1.48 6 M6 2001 -0.223 0.961 1.59 7 M7 2001 -0.192 0.961 1.59 7 M7 2001 -0.192 0.961 1.59 8 M8 2001 -0.043 0.959 1.53 9 M9 2001 0.064 0.958 1.50 10 M10 2001 -0.245 0.971 0.52 11 M11 2001 -1.503 0.944 1.83 12 M12 2001 -0.752 0.954 1.71 13 M1 2005 -0.639 0.901 0.74 14 M2 2005 0.404 0.977 1.41 15 M3 2005 0.318 0.978 1.44 16 M4 2005 -0.077 0.977 1.56 17 M5 2005 -0.545 0.366 1.36 18 M6 2005 0.181 0.968 1.47 19 M7 2005 0.200 0.968 1.46 20 M8 2005 0.321 0.966 1.42 21 M9 2005 0.408 0.966 1.83 22 M10 2005 -0.081 0.966 0.42 23 M11 2005 -0.081 0.966 0.42 24 M12 2005 -0.081 0.960 0.42 25 M10 2005 -0.081 0.960 0.42 26 M2 2007 0.761 0.978 1.24 M12 2007 0.761 0.978 1.24 M12 2007 0.761 0.978 1.29 28 M4 2007 0.761 0.978 1.30 30 M6 2007 0.647 0.974 1.30 31 M7 2007 0.647 0.974 1.30 31 M7 2007 0.647 0.974 1.30 31 M7 2007 0.720 0.999 1.42 33 M10 2007 0.761 0.978 1.24 34 M10 2007 0.761 0.978 1.30 35 M6 2007 0.761 0.979 1.27 38 M8 2007 0.710 0.972 1.21 39 M8 2007 0.711 0.979 1.27 30 M6 2007 0.720 0.999 1.42 31 M11 2007 0.720 0.999 1.33 31 M7 2007 0.647 0.974 1.39 32 M8 2007 0.711 0.979 1.27 34 M8 2007 0.721 0.972 1.21 35 M8 2007 0.720 0.999 1.42 36 M8 2007 0.720 0.999 1.42 37 M8 2007 0.721 0.972 1.21 38 M8 2007 0.721 0.972 1.21 39 M8 2007 0.721 0.972 1.21 40 M8 2008 0.837 0.978 1.18 40 M4 2008 0.837 0.978 1.18 41 M5 2008 0.799 0.999 1.32 44 M8 2008 0.831 0.975 1.19 45 M6 2009 0.889 0.975 0.998 55 M1 2009 0.889 0.975 0.998 56 M8 2009 0.899 0.979 0.998 57 M9 2009 0.899 0.979 0.998 58 M10 2009 0.899 0.979 0.998 59 M11 2009 0.889 0.979 0.998 50 M2 2009 0.889 0.995 0.88 50 M2 2009 0.889 0.995 0.88 50 M3 2008 0.999 0.990 0.991 1.10 56 M8 2009 0.899 0.997 0.998 56 M8 2001 0.938 0.997 0.998 57 M9 2009 0.899 0.997 0.998 58 M10 2009 0.899 0.997 0.998 59 M11 2009 0.899 0.997 0.998 50 M3 2008 0.999 0.990 0.99						1.529
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58 M10 2009 -1.233 0.972 0.20 59 M11 2009 0.638 0.938 1.23 60 M12 2009 0.885 0.946 1.04 61 M1 2011 -0.247 0.949 0.39 62 M2 2011 0.938 0.970 1.01 63 M3 2011 0.930 0.971 1.05 64 M4 2011 -0.662 0.055 0.95 66 M6 2011 0.890 0.978 1.13 67 M7 2011 0.899 0.979 1.12 68 M8 2011 0.931 0.979 1.07 69 M9 2011 0.945 0.978 1.04 70 M10 2011 -1.136 0.931 71 M11 2011 0.433 0.967 1.36 72 M12 2011 0.738 0.976 1.24 <ensemble 0.793="" 0.965="" 1.11<="" th=""><th></th><th></th><th></th><th></th><th></th><th>0.929</th></ensemble>						0.929
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60 M12 2009 0.885 0.946 1.04 61 M1 2011 -0.247 0.949 0.39 62 M2 2011 0.938 0.970 1.01 63 M3 2011 0.930 0.971 1.05 64 M4 2011 0.819 0.964 1.16 65 M5 2011 -0.662 0.055 0.95 66 M6 2011 0.890 0.978 1.13 67 M7 2011 0.899 0.979 1.12 68 M8 2011 0.931 0.979 1.07 69 M9 2011 0.945 0.978 1.04 70 M10 2011 -1.136 0.931 0.97 71 M11 2011 0.433 0.967 1.36 72 M12 2011 0.738 0.976 1.24 <ensemble< th=""><th></th><th></th><th></th><th></th><th></th><th>1.236</th></ensemble<>						1.236
61 M1 2011 -0.247 0.949 0.39 62 M2 2011 0.938 0.970 1.01 63 M3 2011 0.930 0.971 1.05 64 M4 2011 0.819 0.964 1.16 65 M5 2011 -0.662 0.055 0.95 66 M6 2011 0.890 0.978 1.13 67 M7 2011 0.899 0.979 1.12 68 M8 2011 0.931 0.979 1.07 69 M9 2011 0.945 0.978 1.04 70 M10 2011 -1.136 0.931 0.99 71 M11 2011 0.433 0.967 1.36 72 M12 2011 0.738 0.976 1.24						1.042
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64 M4 2011 0.819 0.964 1.16 65 M5 2011 -0.662 0.055 0.95 66 M6 2011 0.890 0.978 1.13 67 M7 2011 0.899 0.979 1.12 68 M8 2011 0.931 0.979 1.07 69 M9 2011 0.945 0.978 1.04 70 M10 2011 -1.136 0.931 0.19 71 M11 2011 0.433 0.967 1.36 72 M12 2011 0.738 0.976 1.24 <ensemble 0.793="" 0.965="" 1.11<="" th=""><th></th><th></th><th></th><th></th><th></th><th>1.019</th></ensemble>						1.019
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67 M7 2011 0.899 0.979 1.12 68 M8 2011 0.931 0.979 1.07 69 M9 2011 0.945 0.978 1.04 70 M10 2011 -1.136 0.931 0.19 71 M11 2011 0.433 0.967 1.36 72 M12 2011 0.738 0.976 1.24 <a href="#"><ensemble< a=""> 0 793 0.965 1.11</ensemble<></a>						
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69         M9         2011         0.945         0.978         1.04           70         M10         2011         -1.136         0.931         0.19           71         M11         2011         0.433         0.967         1.36           72         M12         2011         0.738         0.976         1.24           < <a href="#">Ensemble</a> 0.793         0.965         1.11						1.079
71 M11 2011 0.433 0.967 1.36 72 M12 2011 0.738 0.976 1.24 <ensemble 0.793="" 0.965="" 1.11<="" th=""><th></th><th></th><th></th><th></th><th></th><th>1.047</th></ensemble>						1.047
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<ensemble 0.793="" 0.965="" 1.11<="" th=""><th></th><th></th><th></th><th></th><th></th><th>1.364</th></ensemble>						1.364
0.793 0.965 1.11			2011	0.738	0.976	1.246
average or selected merilibers?			0.793	0.965	1.113	
	average 0	, serectea me	ve15>			

Table 45. Error metrics in the estimation of river discharge by the hydrodynamic model using the 1231 members of the multi-physics ensemble.

- Comparison of flooded areas between numerical results from running ensemble members vs. Observed													
Error metrics	Ensemble Member												∠Encemble
<del>en or metrics</del>	M1	<del>M2</del>	<del>M3</del>	<del>M4</del>	M5	<del>M6</del>	<del>M7</del>	M8	<del>M9</del>	<del>M10</del>	<del>M11</del>	M12	average>
BIAS	0.872	0.893	0.904	0.917	0.910	0.872	0.940	<del>1.006</del>	1.022	1.073	1.040	1.119	0.964
FAR: False Alarm Ratio	0.132	0.143	0.148	0.154	0.149	0.132	<del>0.562</del>	0.817	0.824	0.847	0.865	0.877	0.453
POD: Probability of Detection	0.757	0.765	0.770	0.776	0.750	0.757	0.782	0.817	0.824	0.847	0.856	0.877	0.799
POFD:Probability of False Detection	0.107	0.119	0.125	0.132	0.112	0.107	0.145	<del>0.176</del>	0.185	0.210	0.205	0.225	0.154
CSI : Critical Succes Index	0.868	0.857	0.852	0.846	0.851	0.868	0.834	0.812	0.806	0.790	0.791	0.784	0.831
True Skill Statistics	0.650	0.646	0.645	0.644	0.653	0.650	0.645	0.641	0.639	0.637	0.641	0.652	0.645



	Comparison of flooded areas between numerical results from running ensemble members vs. Observed																															
Error metrics		Ensemble Member															<ensemble< th=""></ensemble<>															
	M1	M13	M26	M27	M30	M31	M32	M33	M38	M39	M42	M43	M44	M45	M50	M51	M52	M54	M55	M56	M57	M59	M60	M62	M63	M64	M66	M67	M68	M69	M72	average>
BIAS	0.903	0.838	1.084	1.099	1.119	1.120	1.094	1.078	1.056	1.021	1.092	1.089	1.096	1.051	0.902	0.915	0.891	0.820	1.020	0.982	0.872	1.056	1.004	0.982	0.995	1.047	1.040	1.028	1.016	1.005	1.092	1.013
FAR: False Alarm Ratio	0.148	0.120	0.215	0.217	0.283	0.210	0.216	0.212	0.209	0.217	0.216	0.215	0.152	0.207	0.148	0.154	0.139	0.137	0.193	0.155	0.133	0.206	0.187	0.178	0.182	0.204	0.201	0.225	0.192	0.187	0.216	0.189
POD: Probability of Detection	0.770	0.737	0.851	0.861	0.849	0.849	0.858	0.849	0.836	0.751	0.857	0.854	0.848	0.833	0.769	0.775	0.751	0.810	0.823	0.845	0.756	0.847	0.816	0.807	0.814	0.833	0.831	0.821	0.821	0.818	0.857	0.819
POFD:Probability of False Detection	0.124	0.094	0.217	0.222	0.187	0.187	0.220	0.214	0.205	0.186	0.220	0.219	0.186	0.203	0.124	0.131	0.185	0.185	0.184	0.066	0.108	0.266	0.175	0.163	0.168	0.199	0.195	0.186	0.182	0.175	0.220	0.180
CSI: Critical Succes Index	0.679	0.670	0.690	0.695	0.711	0.711	0.694	0.691	0.685	0.709	0.693	0.692	0.710	0.685	0.679	0.679	0.706	0.654	0.687	0.708	0.677	0.620	0.687	0.687	0.690	0.686	0.687	0.619	0.688	0.688	0.693	0.686
True Skill Statistics	0.645	0.643	0.634	0.639	0.621	0.662	0.638	0.636	0.631	0.660	0.637	0.636	0.661	0.631	0.645	0.643	0.615	0.601	0.639	0.659	0.648	0.660	0.641	0.644	0.640	0.634	0.636	0.610	0.640	0.642	0.637	0.639

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**Figure 1**. Top panel: Location of the Tonala River basin in Mexico, blue line represents the boundary limits of the catchment; blue dots illustrate the location of weather stations; red dot: streamflow gauge. Bottom panel: zoom of the study area and photographs of observed impacts; yellow, blue and red dots represent the location at which photos were taken.

2

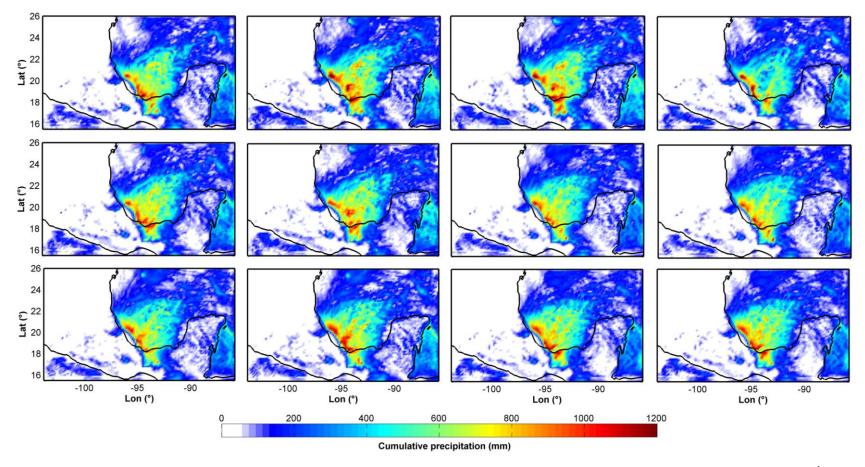
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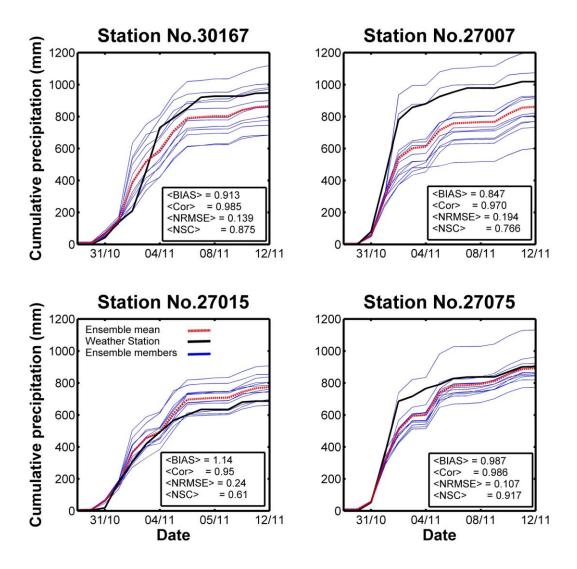
**Figure 2**. Numerical setup of the WRF with a nested domain covering Mexico. Domain 1: 25km resolution; Domain 2: 4km resolution; the orange region illustrates the Tonalá catchment.





**Figure 3**. Cumulative precipitation fields estimated by the WRF model using the 12 members of the multi-physics ensemble ( $27^{th}$  October  $2009 - 12^{th}$  November 2009).





**Figure 4**. Comparison of cumulative precipitation estimated by the 12 members of the WRF model (blue lines) and its mean (red line) vs. measurements (black solid line) at four weather stations from 27<sup>th</sup> October 2009 to 12<sup>th</sup> November 2009.

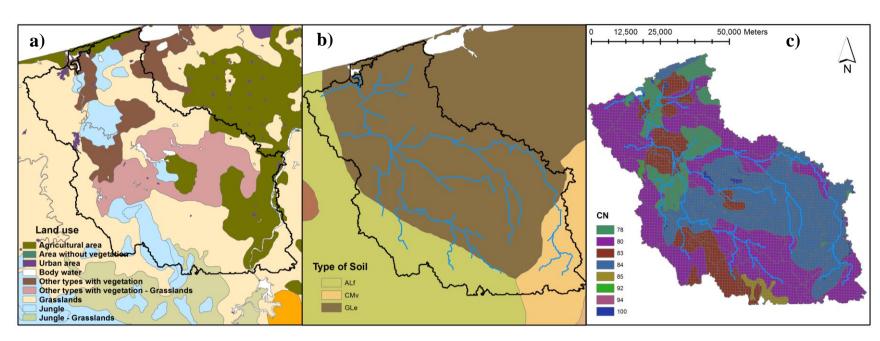
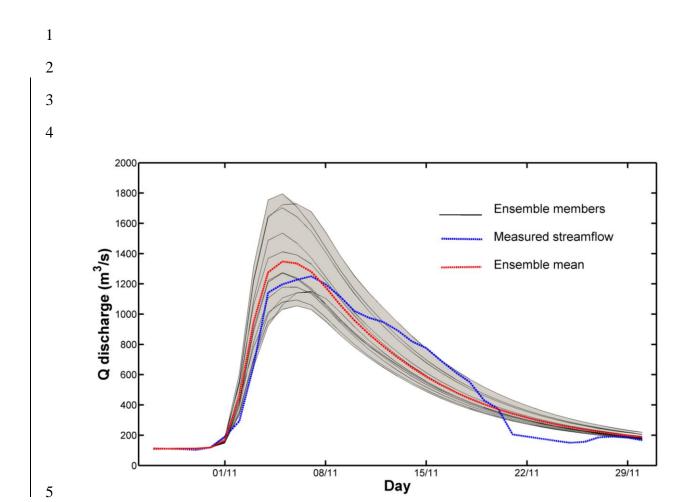
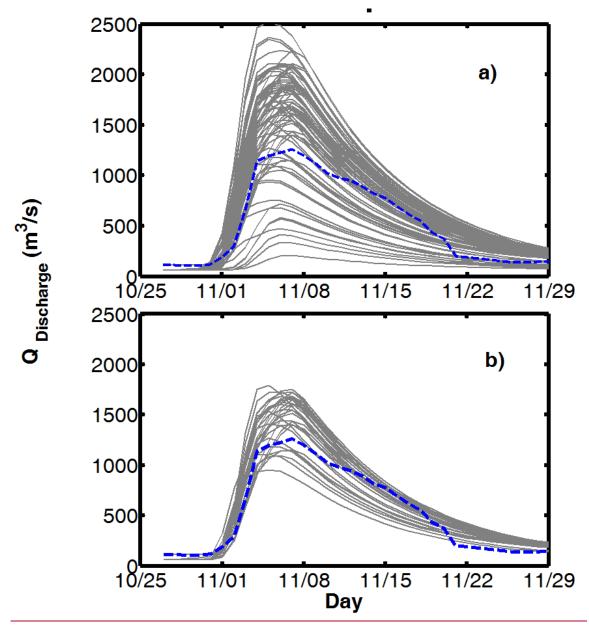
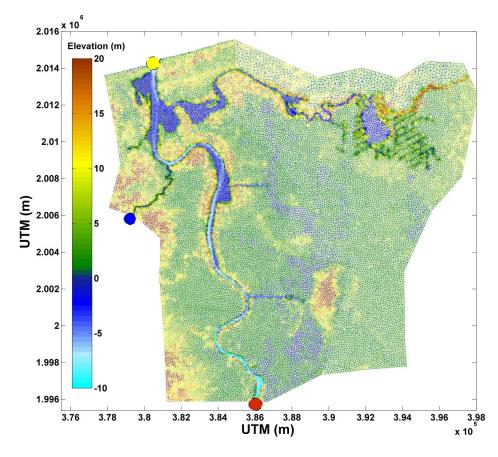


Figure 5. Input data parameters in the hydrological model; a) Land use; b) Pedology; c) River network, curve number and grid.

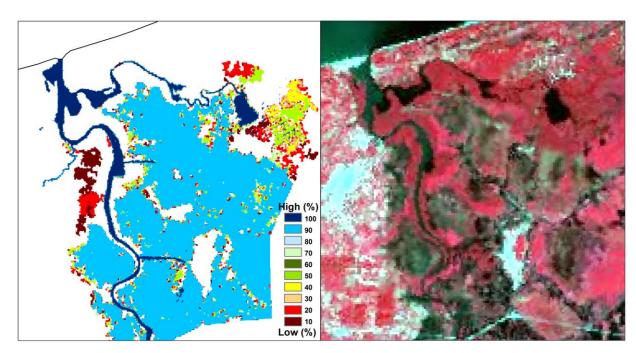




**Figure 6.** Calculateda) 72 hydrographs computed using the rainfall-runoff model with 6 sets of parameters and 12 WRF ensemble precipitation fields as input data; b) 31 selected hydrographs to serve as input in the hydrodynamic model; grey shaded area illustrates the uncertainty bounds (maximum and minimum); lines illustrate the redensemble members and the blue dashed line shows the measured river discharge for thisthe event.

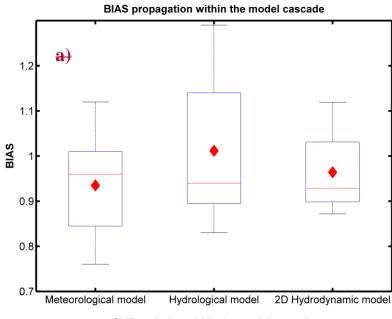


**Figure 7.** Model domain along with the numerical mesh and elevation data in the study area; Boundary conditions are represented by blue dot: Agua Dulcita river; red dot: input hydrograph; yellow dot: river-mouth.



**Figure 8.** Data vs. model comparison of flood extent; a) Probabilistic flood map derived from the ensemble runs with the hydrodynamic model; b) Infrared SPOT image corresponding to the 15<sup>th</sup> November 2009.





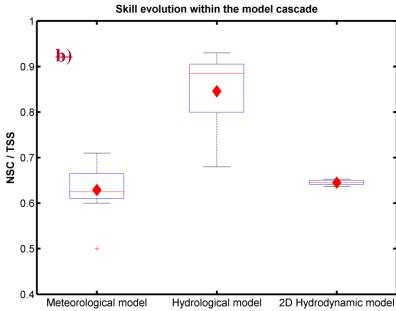
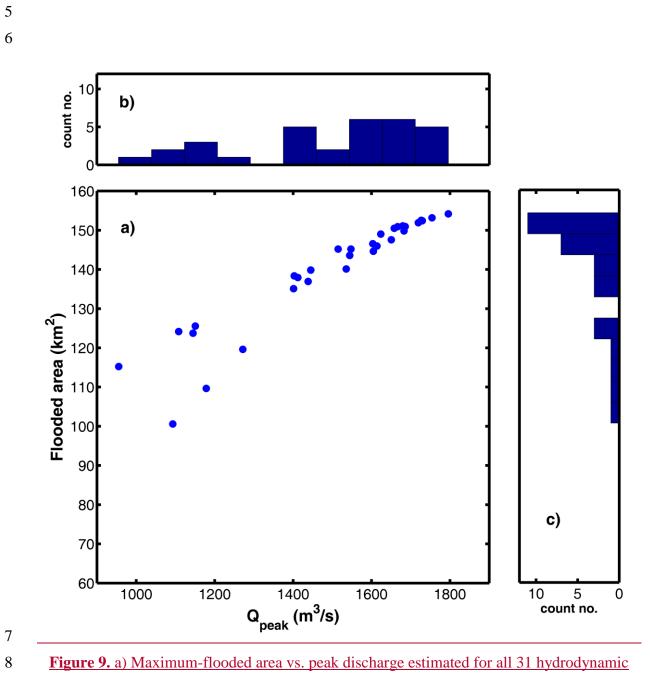


 Figure 9.



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**Figure 9.** a) Maximum-flooded area vs. peak discharge estimated for all 31 hydrodynamic simulations of the 2009 flood event; b)Histogram of peak discharges; c) Histogram of estimated size of maximum-flooded area.

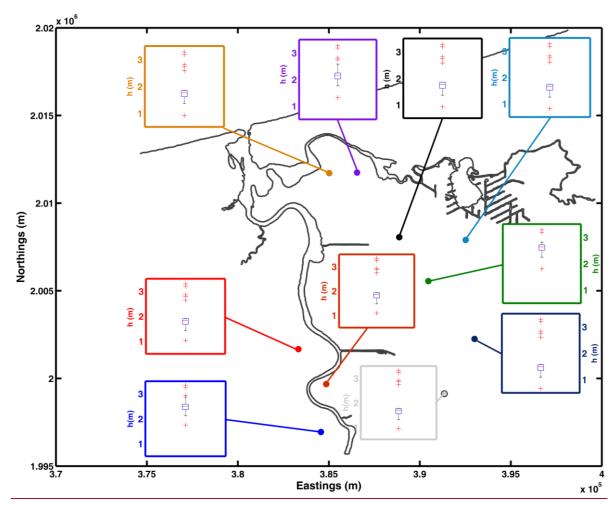
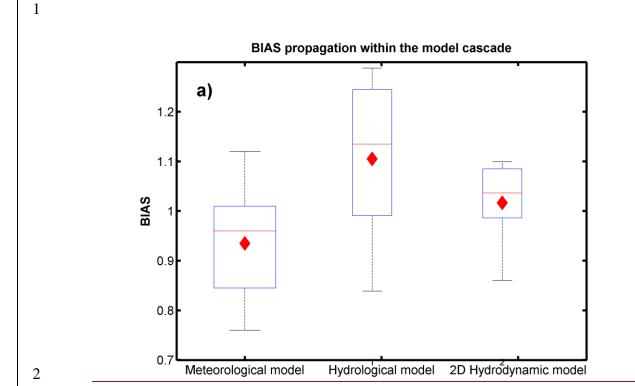
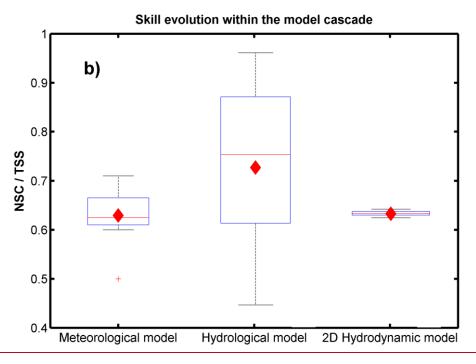


Figure 10. Estimated maxima inundation depths at different locations within the floodplain. Red line represents the median. Bars correspond to the standard deviation. Upper and lower limits of the box are the values of the 25th and 75th, respectively. Crosses depict outliers.





<u>Figure 11.</u> a) BIAS and b) Skill propagation within the model cascade (meteorological-hydrological-hydrodynamic); diamonds: corresponding ensemble mean value.