



**Infiltration well to reduce the impact of
land use changes on flood peaks**

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Infiltration well to reduce the impact of land use changes on flood peaks: a case study of Way Kuala Garuntang catchment, Bandar Lampung, Indonesia

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Abstract

Significant land use changes due to rapid development, a central issue in Bandar Lampung and high rainfall intensity are the main triggers for frequent flooding. This study was carried out to define design rainfall intensity based on analysis of hourly temporal rainfall pattern for calculating design discharge, predict the impact of land use changes on flood peaks, and predict the impact of infiltration well on flood peak reduction. The results showed that rainfall distribution pattern for storm duration of 4 h are 40, 35, 20 and 5 % for the first, second, third and fourth hour, respectively. Analysis on land use changes underlined that if 30 % of the catchment area is maintained for green land then flood peaks can be decreased. However, with city development, land conversions are intended for settlements, industries and trading areas which will increase flood peaks significantly. Application of infiltration well in the catchment can reduce surface runoff depends on the density and dimension of the well. The results suggest that using infiltration well with diameters between 0.8 to 1.4 m which are applied each in every 4000 m² of land area will reduce flood peaks from 6.9 to 12.6 %. While the application of infiltration well with density of 500 m² will reduce flood peaks from 55.21 to 99.8 %. Commitment and relevant government policies and community participation will encourage to undertake flood reduction measures.

1 Introduction

Flooding often occurs in several locations in Indonesia during wet season, that include Bandar Lampung, the capital city and economic hub of Lampung Province and the entry point to Sumatra. The province is located on Southeast Sumatera island. The city's area is about 169.21 km², with a population of approximately 881 801 people (Bappeda, 2010). Over the last two decades the city has shown significant increase in population and economic growth. Recently several flooding events in the city had caused inundation. In one instance several patients were evacuated from a public hospital located

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Kusumastuti, 2009). Van Breen method was developed in the early twentieth century using rainfall data of Java Island, Indonesia. The temporal distribution of rainfall within the design storm is an important factor that affects the runoff volume and magnitude and timing of the peak discharge.

Increase in settlements, industries, trading and roads due to rapid development of Bandar Lampung city has a dire consequence on flooding. To reduce the impact of flooding, use of infiltration wells was investigated. Infiltration well is one of water conservation techniques which help reducing surface runoff and peak flow. Study about infiltration well in Indonesia was initiated by Sunjoto (1988). It was based on the concept of natural water balance as a part of hydrological cycle (Sunjoto, 1993, 1994). When the ground surface is permeable, some precipitation will infiltrate naturally and the rest as runoff along the ground surfaces. However, massive land use changes in recent times promote more runoff because much of ground area is covered by impervious layers such as roofs and pavements which prohibit water infiltration process. Infiltration well will be ideal in this situation because collected water from roof and channels can be directed to the wells for infiltration (Fig. 1). Some researches on infiltration well found that with appropriate well dimensions and numbers, it can reduce surface runoff significantly. Furthermore, the application of infiltration well support the policy of zero delta discharge which limits water discharge to drainage system (Arafat, 2008; Iriani et al., 2013; Indriatmoko, 2013).

Study area for this research is Way Kuala Garuntang catchment, one of the two major catchments in Bandar Lampung, has an area of 63.9 km² and lies across the centre of the city (Fig. 2). The catchment experienced frequent flooding due to significant land use changes. This research will focus on how land use changes in the Way Kuala Garuntang catchment influence the hydrologic responses. The hydrologic analysis using rational method was applied on some land use change scenarios to see the changes of peak flow of Way Kuala Garuntang catchment. Design rainfall intensity analysis was also carried out to support the rational method calculation in the beginning of hydrologic analysis. Finally, analysis of the impact of infiltration well in the Way

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Kuala Garuntang catchment to reduce flood discharge was carried out. The aim of this study is (1) to define design rainfall intensity based on analysis of hourly temporal rainfall pattern, (2) to predict the impact of land use changes on flood peaks, and (3) to predict the impact of infiltration well on flood peak reduction.

5 **2 Methodology**

Four scenarios of land use changes were applied on a simple rainfall–runoff model, using rational method to see the pattern of flood peak generated by the catchment. The study also concerns to the effort for flood reduction in Way Kuala Garuntang catchment by using infiltration well. Land slope, the depth of shallow ground water and infiltration rate are important considerations for determining the area for infiltration well. Quantum GIS was used in the study Way Kuala Garuntang catchment in order it fulfill the requirement for the application of infiltration well.

2.1 **Rainfall–runoff model of land use change scenarios**

15 This study focuses on peak discharge rather than the hydrograph, hence a simple rainfall–runoff method was utilized. The rational method is widely used to calculate runoff producing potential of a catchment and form as the basis for many small structure designs. The application of rational method is valid for predicting runoff in a small catchment (ASCE, 1992). The area of the catchment is larger than specified in ASCE (1992), but rational method was adopted in this study not to predict runoff precisely, rather the method occupied to get insight of the effect of land use changes on runoff.

20 Application of the rational method is based on a simple formula that relates runoff producing potential of the watershed, rainfall intensity for a particular length of time (the time of concentration), and the watershed drainage area. The formula is

$$Q = C_u \cdot C \cdot I \cdot A \tag{1}$$

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where, Q = design discharge ($\text{m}^3 \text{s}^{-1}$), C_u = unit conversion coefficient, C = runoff coefficient (dimensionless), I = design rainfall intensity (mm h^{-1}), A = catchment area (km^2).

2.1.1 Design rainfall intensity

Design rainfall intensity, I , varies according to geographic location and design exceedence frequency or return period. The longer the return interval (hence, the lower the exceedence frequency), the greater the precipitation intensity for a given storm duration. Series of hourly rainfall data 2001–2012 were obtained from the Meteorological and Climatology Berueu Panjang, Lampung Province. The data was not complete, no data for year 2002 and some missing months for others. The average annual rainfall was between 1500–2500 mm, with wet season occurs between October to April and dry season occurs between April to October.

The series of hourly rainfall data was grouped according to storm durations, with the shortest and longest durations are 3 and 8 h, respectively. Minimum storm depth considered in the analysis is 20 mm. Analysis of rainfall intensity pattern for each storm duration initially was done by calculating the percentage of storm depth occurs in a particular storm event compared to total storm depth occurs during that day (within 24 h). Next, the percentage of hourly rainfall pattern is calculated by comparing hourly rainfall to storm depth. Design rainfall intensity was selected as the highest percentage of hourly rainfall pattern. Flood frequency analyses were used to estimate the probability of the occurrence of a given hydrologic event. Once design rainfall intensity for particular return period was estimated, flood peaks can be calculated using rational method.

2.1.2 Runoff coefficient

Runoff coefficient plays an important role in catchment management as it can be used as surface runoff indicator in a catchment and flood estimation. To estimate runoff

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the conversion of all vacant land into industry area and a quarter of the settlement area into trading and services. Scenarios 3 and 4 assume that despite the projection of very high population by 2030, it will not require more land for settlement as more people will live in apartment and *ruko*. Significant growth of industries requires more space and hence the policy to preserve 30 % of the area for green land cannot be maintained. Description about land use and corresponding percentage of the catchment area for each scenario above is presented in Table 1.

2.2 Infiltration well application

In its application, the natural condition of the area should be checked whether the area fulfill the requirement of National Standard such as (1) land permeability should be more than 2 cm h^{-1} , (2) shallow ground water level is greater than 3 m below ground level in wet season and (3) land slope is less than 30° . In this study the conditions of Way Kuala Garuntang catchment were established by using maps of land permeability, shallow groundwater level and land slope of Way Kuala Garuntang catchment with existing land use map to determine suitable areas for the application. The number of infiltration well can be calculated as follows (Suripin, 2004).

$$\begin{aligned} \text{The number of wells} &= \frac{Q_{\text{runoff}}}{Q_{\text{well}}} \\ &= \frac{Q_{\text{runoff}}}{(A_{\text{well base}} \times V) + (A_{\text{well wall}} \times V)} \\ &= \frac{Q_{\text{runoff}}}{\left(\pi \cdot \Phi_{\text{design}}^2 \times K \right) + (2 \cdot \pi \cdot \Phi_{\text{design}} \cdot h_{\text{design}} \times K)} \end{aligned} \quad (2)$$

where Q_{runoff} is daily peak flow in 1 day ($\text{m}^3 \text{ day}^{-1}$), Q_{well} is total runoff collected in a well within 1 day ($\text{m}^3 \text{ day}^{-1}$), Q_{well} is calculated based on infiltration well discharge ($\text{m}^3 \text{ day}^{-1}$) and wall infiltration well discharge ($\text{m}^3 \text{ day}^{-1}$), K is land permeability

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coefficient (m day^{-1}), Φ_{design} is well radius (m), $A_{\text{well base}}$ is well base area (m^2), $A_{\text{well wall}}$ is well wall area (m^2) and h_{design} is well depth (m).

3 Result and discussion

The characteristics of the storm data analysed in this study are presented in Fig. 4. For storm duration 3 to 8 h the average of total storm depths is between of 38.98 to 62.47 mm (Fig. 4a), standard deviation is between 12.19–36.02 mm (Fig. 4b), minimum storm depth is between 20 to 40.5 mm and maximum storm depth is between 57.8 to 145 mm (Fig. 4c). From the available hourly rainfall data, there were 34.15, 26.83, 13.41, 14.63, 4.88 and 6.10 % storm events for storm duration of 3, 4, 5, 6, 7 and 8 h, respectively (Fig. 4d). Figure 4 shows that the trend of average storm depth, standard deviation and minimum storm depth increase as storm duration increases. However, maximum storm depth tends to decrease as storm duration increases.

The result shows that the most common storm event has 3 h duration, followed by 4 h duration. Storm event with duration of 5 and 6 h have similar percentage of occurrences, but have less significant percentage of event compared to the event with 3 and 4 h duration. The percentage of occurrences for storm duration of 7 and 8 h are similar but they have the least percentage of occurrences compared to others. Storm depth of the event compared to storm depth within 24 h shows that the percentage is between 87 to 97 %.

Storm event with the duration of one and two hours are not taken into account as the duration is too short and in most events, they are followed by another storm event which have longer storm duration and higher storm depth.

Further investigation on hourly storm distribution is done to understand the distribution pattern. Figure 5 shows the percentage of hourly storm distribution for storm duration 3 to 8 h. There are similar trend for all storm duration that the percentage of storm distribution at early hours are relatively high compared to that toward the end

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use as presented in Table 1, the composite runoff coefficient for existing condition, Scenario 1 (Fig. 7a), Scenario 2 (Fig. 7b), Scenario 3 (Fig. 7c) and Scenario 4 (Fig. 4d) are 0.56, 0.45, 0.47, 0.74 and 0.72, respectively. For the existing condition, using calculated design rainfall intensity and composite runoff coefficient for the catchment, design flood for 5 year return period is $260.95 \text{ m}^3 \text{ s}^{-1}$.

Simulation on land use impact as presented in Fig. 8 shows that the results can be categorised into two groups. Simulation results using Scenarios 1 and 2 has an impact in reducing peak discharge, while Scenarios 3 and 4 effect in increasing peak discharge. Those ratios are the comparison between the peak discharge resulted from the scenario and that resulted from existing condition. The reduction of peak discharge ratios are 19.48 and 16.01 % for simulation results using Scenarios 1 and 2, respectively. The decrease in peak discharge is an impact of applying around 30 % of the catchment area into green land. A preliminary work on the impact of land use changes in this catchment has been done by Yuniarti et al. (2013) and found the importance of green land. Green land is required to give sufficient space for infiltration, which is good for groundwater recharge as well as flood control. Scenario 2 includes green land in the simulation, but also converts some of agriculture area into settlement. This means increasing semi impervious area, increasing runoff coefficient and consequently increasing flood peak. As a result, the reduction of peak discharge is less significant as land use conversion tend to change recharge potential area into semi impervious area.

In contrast to simulation results on the first two scenarios, Scenarios 3 and 4 cause significant increase of peak discharges. Scenario 3 aggravates land use change situation which converts all vacant land into industry area and half of settlement area into trading and service. Therefore, the impervious area increases significantly which reflects in the increase of peak discharge to 32.28 %. Compare to that Scenario, Scenario 4 alleviates land use change situation as in addition to the conversion of vacant land into industry area, only a quarter of the existing settlement is converted into trading and service area. Scenario 4 causes the increase of peak runoff to 28.66 %.

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3.2 Analysis of the number of infiltration well applied in the catchment

Understanding possible impact of land use changes presented above, there should be an effort to reduce the impact of land use changes on flooding. From the analysis using Geographic Information System which overlays the existing land use map with land permeability, shallow ground water level and land slope maps, it is found that 52.06 km² or 81.47 % of the catchment area is the effective area for applying infiltration well. Figure 9 presents the map of suitable area for infiltration well application. Based on the effective area, an analysis of the density of infiltration well application was done with the assumption that the depth of the well is 3 m, and diameters of the wells are 0.8, 1.0, 1.2 and 1.4 m. Equation (2) was used in this analysis.

The average land area for applying infiltration well related to flood peak reduction is presented in Fig. 8. It can be seen that the average land area and well diameter have significant impact on reducing flood peaks. Applying an infiltration well per 4000 m² in the whole catchment will reduce flood peaks to 6.9, 8.76, 10.68 and 12.6 % using diameters of 0.8, 1.0, 1.2 and 1.4 m, respectively. Applying an infiltration well per 4000 m² means that there will be about 13015 wells applied in the catchment. Using average density area of 2000 m², infiltration wells may reduce flood peaks to 13.8, 17.52, 21.3 and 25.29 % for diameters of 0.8, 1.0, 1.2 and 1.4 m, respectively. Considerable increase of flood peak reduction showed when applying infiltration well each in the average of 1000 and 500 m² land area. The ranges of flood peak reductions are between 27.61 to 50.57 % and 55.21 to 99.8 % using designated diameters for average applying land area of 1000 and 500 m², respectively.

The results suggest that using infiltration well with diameter 1.4 m which are applied each in every 500 m² of land area nearly reduce all runoff. However, applying an infiltration well per 4000 m² means implementing 104 116 wells in the catchment. It requires a lot of financial commitments not only from the government but also from the people. It also requires willingness of people to allow their land for the implementation. Relating the result with possible land use changes, implementation of infiltration well

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in every 2000 m² is considered sufficient as flood peak reductions correspond to flood peak increases with land use changes. In addition, the number of wells required will only be 26 029 wells, far less compared to applying an infiltration well each in every 500 m².

4 Implication and conclusion

Rainfall data, especially in fine time scale, is less available in developing country such as Indonesia. In practice design rainfall intensity is significant for determining design flood. With limitation of fine scale rainfall data, design rainfall intensity is usually estimated from a 24 h design rainfall. Design of hourly rainfall distribution consequently control design rainfall intensity. There is different hourly rainfall pattern between the method that commonly used and result from this study. For selected runoff method used in this study, as its simplicity, flood peak is only influenced by the highest hourly rainfall intensity distribution and not by the whole rainfall intensity pattern. Therefore, design rainfall intensity resulted in this study is similar with that of the common used method. However, for runoff calculation method which also considers rainfall intensity pattern, flood peak will be different if using rainfall intensity distribution resulted from this study compared to that of commonly used method.

Rainfall intensity pattern resulted from this study was carried out by simple analysis of hourly rainfall data. To have a more accurate result, a thorough statistical analysis needs to be done. Result indicated by this study is expected to provoke further analysis as in a case of limited fine time scale of rainfall data, rainfall intensity pattern will influence not only flood peaks, but also flood hydrographs and intensity duration curves which are vital in hydrology design.

Urbanisation, population growth and economic increase require more settlements, trading area and industries which may cause some land use changes. Bandar Lampung as a developing city will experience some land use changes due to the factors mentioned above. Analysis on some possible land use changes emphasize the

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importance of open green space related to flooding, and particularly flood peaks. In fact there is spatial plan act for this city to maintain 30 % of the area for open green space. However, in practice people may not comply with the rule. This study indicates if conversion of land use to get space for more settlement, industries and trading areas will increase flood peaks significantly.

Application of infiltration wells have been considered in several places in Indonesia including Bandar Lampung, but not widely applied. Most possible places to apply infiltration wells are at the area of government offices or public facilities. Although there are few individuals who had applied infiltration wells at their house. This study does not examine the locations to implement infiltration wells. Instead, it considers the density of infiltration well within the catchment and how it may reduce flood peaks.

Infiltration well is one of the techniques to reduce flood, beside some other techniques such as rain barrel, porous trench, retention pond and bio-retention. Implementation of these techniques simultaneously will have significant impact on flood reduction. Require commitment and relevant policies from the government to encourage use of these applications as well as community participation to tackle flood reduction.

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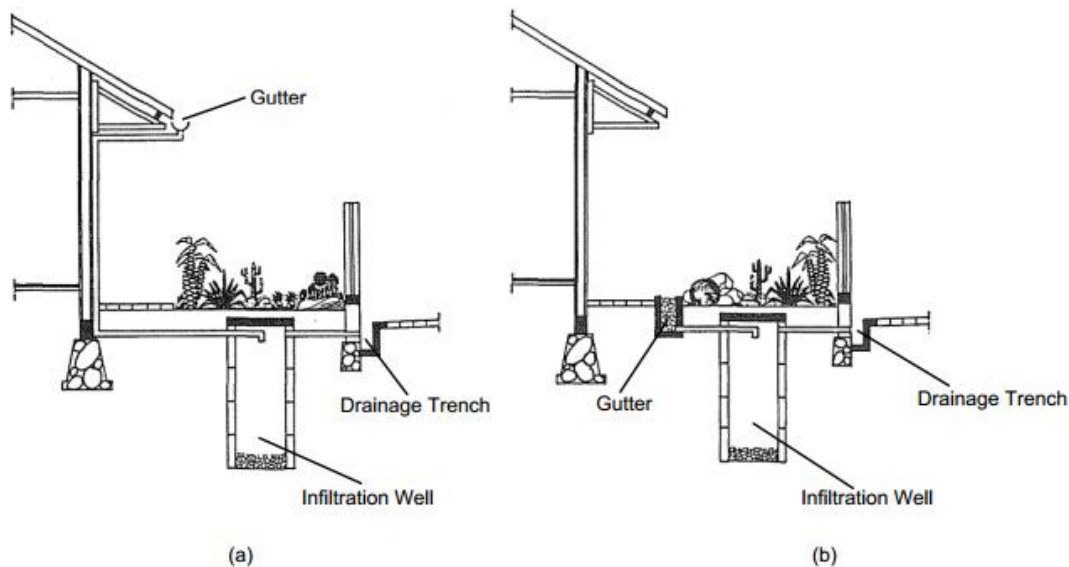


Figure 1. Illustration of infiltration well applications **(a)** house with gutter at the eaves and **(b)** house with gutter at the ground (source: Sunjoto, 1994).

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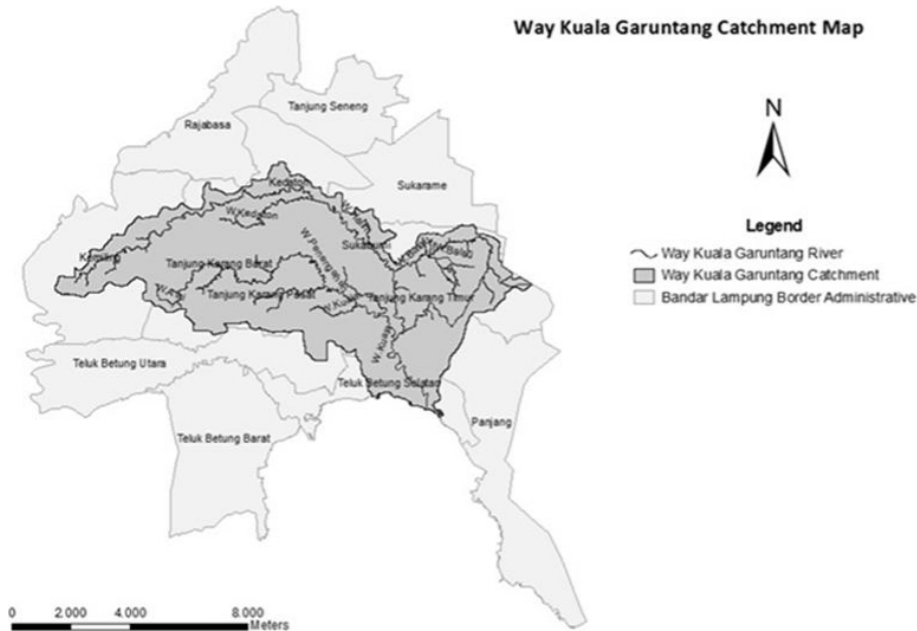


Figure 2. Way Kuala Garuntang catchment within the city of Bandar Lampung.

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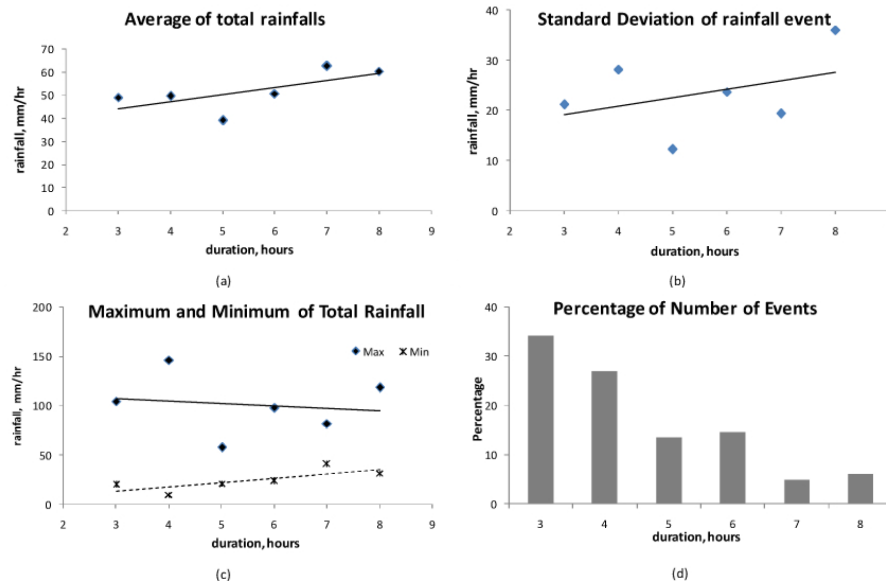


Figure 4. Characteristics of rainfall data.

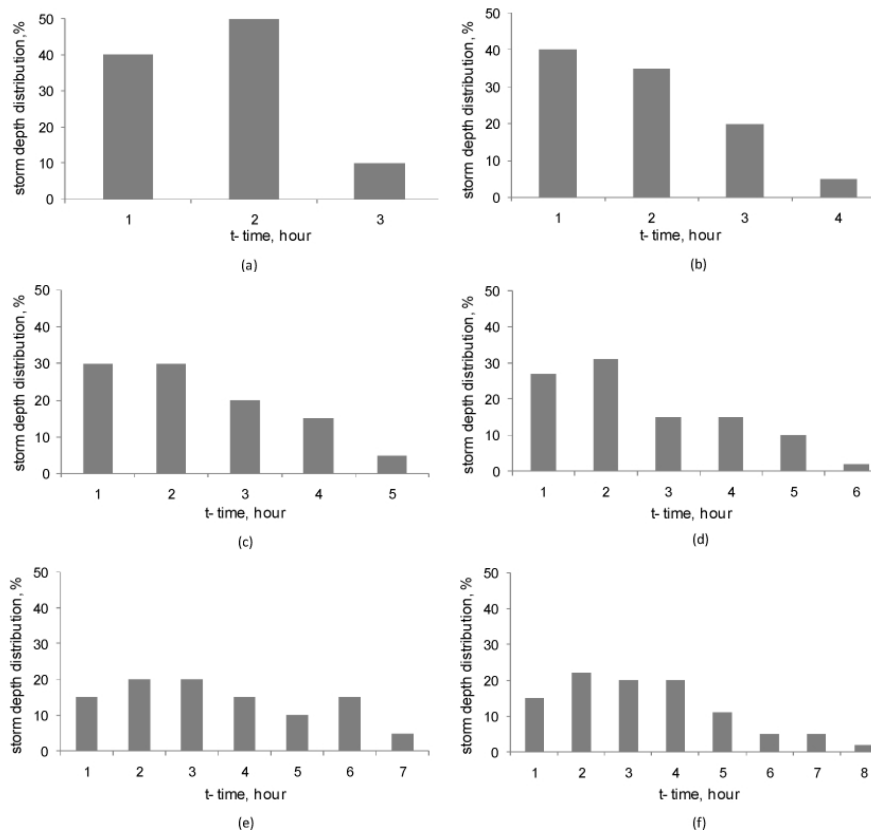


Figure 5. Storm depth distribution for storm durations 3 to 8 h.

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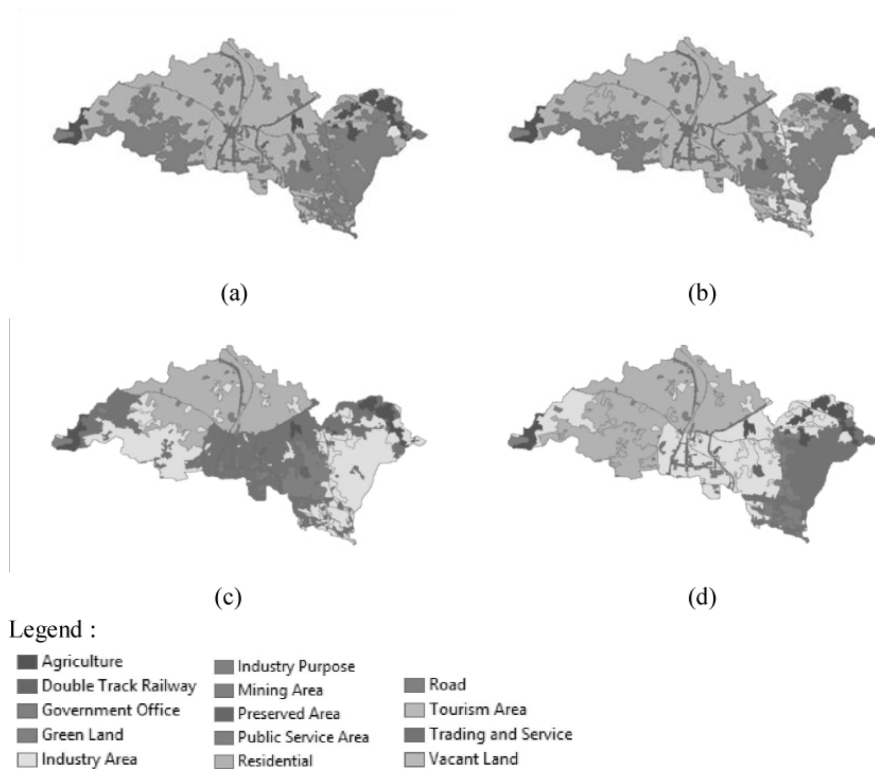


Figure 7. Land use on the four land use change scenarios.

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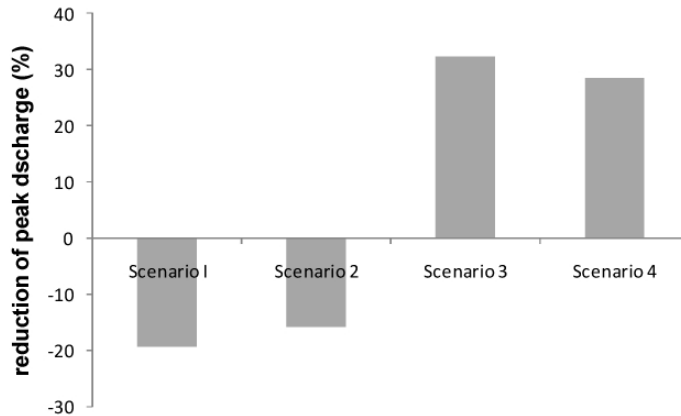


Figure 8. Impact of land use changes on peak discharge compared to existing condition.

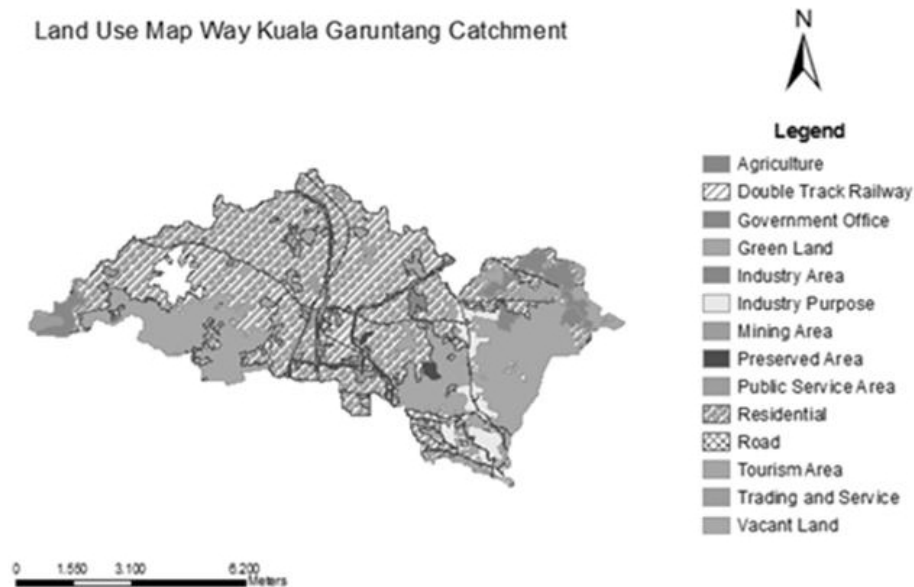


Figure 9. Suitable area for infiltration well application.

HESSD

11, 5487–5513, 2014

Infiltration well to reduce the impact of land use changes on flood peaks

D. I. Kusumastuti et al.

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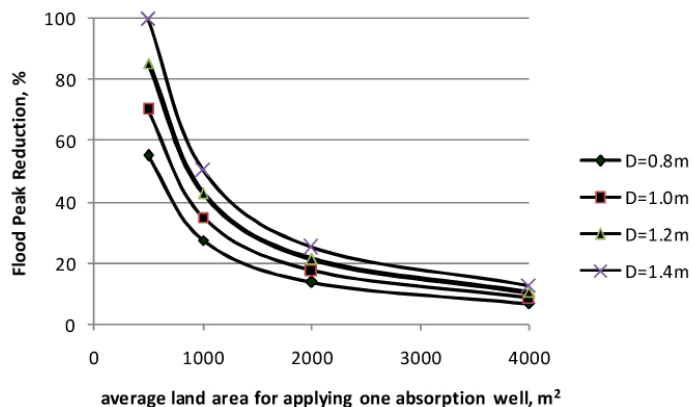


Figure 10. Average land area for applying an infiltration well vs. flood peak reduction.

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