Alternative configurations of Quantile Regression for estimating predictive uncertainty in water level forecasts for the Upper Severn River: a comparison.

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Abstract.

The present study comprises an inter-comparison of different configurations of a statistical post-processor that is used to estimate predictive hydrological uncertainty. It builds on

- ⁵ earlier work by Weerts, Winsemius and Verkade (2011; herinafter referred to as WWV2011), who used the Quantile Regression technique to estimate predictive hydrological uncertainty using a deterministic water level forecast as a predictor. The various configurations are designed to address two
- ¹⁰ issues with the WWV2011implementation: (i) quantile crossing, which causes non-strictly rising cumulative predictive distributions, and (ii) the use of linear quantile models to describe joint distributions that may not be strictly linear. Thus, four configurations were built: (i) 'classical' Quantile Re-
- ¹⁵ gression, (ii) a configuration that implements a non-crossing quantile technique, (iii) a configuration where quantile models are built in Normal space after application of the Normal Quantile Transform (similar to the implementation used by WWV2011), and (iv) a configuration that builds quantile
- ²⁰ model separately on separate domains of the predictor. Using each, four re-forecasting series of water levels at fourteen stations in the Upper Severn river were established. The quality of these four series was inter-compared using a set of graphical and numerical verification metrics. Intercompari-
- ²⁵ son showed that reliability and sharpness vary across configurations, but in none of the configurations do these two forecast quality aspects improve simultaneously. Further analysis shows that skills in terms of Brier Skill Score, mean Continuous Ranked Probability Skill Score and Relative Operating
- ³⁰ Characteristic Score is very similar across the four configurations.

1 Introduction

Forecasting may reduce but can never fully eliminate uncertainty about the future. Hydrological forecasts will always be subject to many sources of uncertainty, including those originating in the meteorological forecasts used as inputs to hydrological models (e.g. precipitation and temperature), and in the hydrological models themselves (e.g. model structure, model parameters and human influences). Informed decision making may benefit from estimating the remaining uncertainties. A number of research studies suggest that enclosing predictive uncertainty estimates indeed leads to benefits to end users (Krzysztofowicz, 2001; Collier et al., 2005; Verkade and Werner, 2011; Ramos et al., 2013; Dale et al., 2014).

In the literature, various approaches to estimate predictive uncertainty have been presented. One of those is the use of meteorological ensemble forecasts, where initial atmospheric conditions are perturbed to yield an ensemble of atmospheric forecasts. These can be routed through a hydrological model, thus yielding an ensemble of hydrologic model forecasts which provides insight into the sensitivity of hydrological model results to various possible weather scenarios. Increasingly, hydrologic forecasting systems are including these ensemble predictions in the forecasting routines to capture the meteorological uncertainty. An overview of applications and best practices was given by Cloke and Pappenberger (2009). More recent applications include the Environment Agencys National Flood Forecasting System NFFS (Schellekens et al., 2011) and the US National Weather Services Hydrologic Ensemble Forecast Service HEFS (Demargne et al., 2014). Note that HEFS also includes a statistical post-processor developed by (Seo et al., ⁶⁵ 2006).

A second approach is statistical post-processing. Estimating predictive uncertainty through statistical post-processing techniques comprises an analysis of past, observed predictive

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uncertainty to build a model of future predictive uncertain-

- 70 ties. It can be used as either an alternative or additional step 125 to hydrological ensemble forecasting. In many hydrological forecasting applications, postprocessing is used in combination with deterministic forecasts (but it can also be applied to ensemble hydrologic forecasts if available; see, for example,
- ⁷⁵ Reggiani et al. 2009 and Verkade et al. 2013). A historical record of past forecasts and their verifying observations is then used to build a model of forecast error. (Note that other configurations are possible, but this one is the most straightforward and common one.) On the assumption that
- 80 this error will be similar in future cases, the error model is 135 then applied to newly produced deterministic forecasts, thus producing an estimate of predictive hydrological uncertainty. This estimate then includes uncertainties originating in both the atmospheric forecasts as well as those in the numerical
- simulation of streamflow generation and routing processes. 140 Post-processing assumes a stationary relation between the predictors and the predictors. It follows that both the forecasts and the observations used for calibration have to be stationary. Also, ideally the calibration record is sufficiently
- 90 long as to include events that are (relatively) extreme. The 145 reason for this is that the relationship between forecast and observations at extreme events may be different from the relationship in non-extreme hydrological regimes. If the assumption of stationarity cannot be met or if the calibration
- 95 record is short, the quality of the post-processed forecasts 150 is likely to be reduced. Several hydrologic post-processors have been described in the scientific literature, including the Hydrological Uncertainty Processor (HUP - Krzysztofowicz and Kelly 2000), Bayesian Model Averaging (BMA - Raftery
- ¹⁰⁰ et al. 2005), the Model Conditional Processor (MCP Todini 2008), UNcertainty Estimation based on local Errors and Clustering (UNEEC - Solomatine and Shrestha 2009), the Hydrologic Model Output Statistics (HMOS - Regonda et al. 2013) and Quantile Regression (QR - Weerts et al. 2011).
- ¹⁰⁵ The present paper focuses on the latter technique. Quantile Regression (QR, Koenker and Bassett Jr. 1978; Koenker and Hallock 2001; Koenker 2005) aims to describe a full probability distribution of the variable of interest (the predictand), conditional on one or more predictors. Contrary
- 110 to some of the other post-processors (such as HUP or BMA), 165 QR requires few prior assumptions about the characterization of the model error. While it was originally developed for applications in the economic sciences, it has since been introduced into environmental modelling and climate change
- 115 impact assessment (e.g. Bremnes, 2004; Nielsen et al., 2006). The technique has been applied in various research studies as a post-processing technique to estimate predictive hydrological uncertainty, including those described by Solomatine and Shrestha (2009), Weerts et al. (2011), Verkade and Werner
- 120 (2011), and Roscoe et al. (2012). In each of these applica- 175 tions, the quantiles of distribution of the model error are estimated using single valued water level or discharge forecasts as predictors.

Weerts et al. (2011; hereinafter referred to as WWV2011) describe an implementation of QR for the Environment Agency in the United Kingdom. The "Historic Forecast Performance Tool" (HFPT; Sene et al. 2009) makes use of QR to estimate a predictive distribution of future water levels using the deterministic water level forecast as a predictor. The $WWV2011$ configuration of QR includes a transformation into Gaussian space using the Normal Quantile Transform (Krzysztofowicz and Kelly, 2000; Montanari and Brath, 2004; Bogner and Pappenberger, 2011). In QR, the quantiles are estimated one at a time. Potentially, these quantiles cross, thus yielding implausible predictive distributions. The quantile crossing problem was addressed by omitting the domain of the predictor where the crossing occurred from the QR procedure and instead, in that domain, imposing a prior assumed distribution of the predictand.

The results of the WWV2011analysis were verified for reliability and showed to be satisfactory. However, this verification was unconditional in the sense that only the full available sample of paired (probabilistic) forecasts and observation was assessed for reliability. When the HFPT was further tested (Vaughan, 2012), it was noticed that the probabilistic forecasts did not perform equally well in high flow conditions. One of the contributions of the present paper consists of a conditional analysis of forecast skill. Forecast skill is assessed for progressively higher flood levels, in terms of commonly used verification metrics and skill scores. These include Briers probability Score, the Continuous Ranked Probability Score and corresponding skill scores as well as the Relative Operating Characteristic Score.

The configuration of QR in WWV2011included two elements that, in the present paper, are explored in additional detail. These steps are (i) the technique for avoiding crossing quantiles and (ii) the derivation of regression quantiles in Normal space using the Normal Quantile Transform (NQT).

In WWV2011, crossing quantiles were avoided by manu-¹⁶⁰ ally imposing a distribution of the predictand in the domain of the predictor where crossing occurred. Since designing and implementing that particular configuration, an alternative technique for estimating non - crossing quantile regression curves has emerged (Bondell et al., 2010). As the latter technique requires less manual interference by the modelers, the present paper investigates whether implementation thereof yields estimates of predictive uncertainty that are of equal or higher quality.

In WWV2011, QR was applied using first degree polynomials, i.e. describing the distribution of the predictand as a linear function of the predictor. This, of course, assumes that the joint distribution of predictor and predictand can be described in linear fashion. To facilitate this, both marginal distributions (of forecasts and of observations) were transformed into Normal or Gaussian domain using the NQT. The joint distribution was subsequently described in Normal space using linear regression quantiles, and then backtransformed into original space. The resulting regression

quantiles are then no longer linear. While this procedure ¹⁸⁰ yielded satisfactory results, there is no requirement on the part of QR of either the marginal or joint distributions to be marginally or jointly Normal distributed. Also, the transformation and especially the back-transformation impose addi-

tional assumptions on the marginal distributions and can thus ¹⁸⁵ be problematic. Hence a justified question is whether this transformation to and from Normal space actually yields better results. In the present paper, this is tested by comparing multiple configurations of QR: derivation of regression quantiles in original space and in Normal space. As an additional ¹⁹⁰ step, a piecewise linear configuration is tested, where the domain of the predictor is split into several, mutually exclusive and collectively exhaustive domains, on each of which the

The objective of this work is to thoroughly verify uncer-¹⁹⁵ tainty estimates using the implementation of QR that was used by WWV2011, and to inter-compare forecast quality and skill in various, differing configurations of QR. The configurations are (i) classical' QR, (ii) QR constrained by a requirement that quantiles do not cross, (iii) QR derived on

regression quantiles are calibrated.

- ²⁰⁰ time series that have been transformed into the Normal domain (similar to WWV2011QR configurations, with the exception of how the quantile crossing problem is addressed), and (iv) a piecewise linear derivation of QR models. A priori, it is expected that imposing a non-crossing requirement
- ²⁰⁵ yields results that are at least as good as those of the classical' implementation of QR, and that derivation in Normal space and piecewise linear derivation each constitute a further improvement in quality and skill compared to derivation in original space.
- ²¹⁰ The novel aspects and new contributions of this work include the thorough verification of an earlier implementation $_{260}$ of QR, the application of the non-crossing QR to this particular case study and the exploration of techniques for ensuring that joint distributions can be described using linear QR ²¹⁵ models.

This paper first describes the approach, materials and methods, including the study basin, the hindcasting process, the analysed QR configurations and the verification process. Subsequently, results and analysis are presented. The paper

²²⁰ ends with conclusions and discussion.

2 Approach, materials and methods

The present study consists of an experiment in which verification results of four differently configured post-processors (each based on the Quantile Regression technique) are inter– ²²⁵ compared. By the varying configurations, two potential issues are addressed: quantile crossing and possible nonlinearity of the joint distribution of predictor and predictand.

2.1 Study basin: Upper Severn River

The Upper Severn basin (Figure 1) serves as the study basin for the present study. Its predominantly hilly catchment extends from the Welsh Hills at Plynlimon to the gauge at Welshbridge in Shrewsbury and is approximately $2,284 \text{ km}^2$ large. Lake Vyrnwy (Vyrnwy River) and Llyn Clywedog (Clywedog River) are two reservoirs located in the headwaters of the catchment. The Upper Severn includes rock formations classified as non-aquifers as well as loamy soils characterised by their high water retention capacity. Annual precipitation varies with topography from 700 to 2,500 mm (EA, 2009). Flooding occurs relatively frequently, with major floods occurring in autumn 2000, February 2002, February 2004, summer 2007, fall 2012 as well as at the time of writing this manuscript, early 2014. To manage flood risk, the UK Environment Agency developed the River Severn Catchment Flood Management Plan in 2009. Flood risk management is supported by the Midlands Flood Forecasting System (MFFS), which is based on the Delft-FEWS forecast production system (Werner et al., 2013). The Upper Severn configuration in MFFS consists of a sequence of numerical models for modeling of rainfall-runoff (MCRM; Bailey and Dobson 1981), hydrological routing (DODO; Wallingford 1994) and hydrodynamical routing (ISIS; Wallingford 1997) processes as well as an internal MCRM error correction procedure based on the Autoregressive Moving Average (ARMA) technique. The input data for MFFS consists of Real Time Spatial (RTS) data (observed water level data, rain gauge data, air temperature and catchment average rainfall data), Radar Actuals, Radar Forecast, and Numerical Weather Prediction data. This input data is provided by the UK Meteorological Office.

The uncertainty models are used to estimate predictive uncertainty at fourteen hydrological stations on the Upper Severn River, each having different catchments characteristics. Figure 1 shows a map with the forecasting locations and their basins. Table 1 summarizes some key hydrological data.

2.2 Hindcasting process

The uncertainty models (Section 2.3) are derived using a joint historical record of observations and forecasts. The latter is acquired through the process of reforecasting or hindcasting. For this, a standalone version of the forecast pro-²⁷⁰ duction system MFFS is used. Prior to every forecast, the models are run in historical mode over the previous period to produce an estimate of internal states (groundwater level, soil moisture deficit, snow water equivalent, snow density, etc). In this historical mode, models are forced with observed precipitation, evapotranspiration and temperature. The system is subsequently run in forecast mode twice daily, with forecast issue times of 08:00 and 20:00 UTC, with a maximum lead time of 48 hours. The selected reforecasting period is from January 1st, 2006 through March 7th, 2013. Of this pe-

Station name	River	Basin area		Elevation Mean annual rainfall	Mean flow	Highest river level recorded Basin lag time	
		$\lceil \mathrm{km}^2 \rceil$	[m AOD]	\lceil mm \rceil	$\left[\mathrm{m}^3/\mathrm{s}\right]$	[m]	$[h] \centering% \includegraphics[width=1.0\textwidth]{Figures/PN1.png} \caption{The 3D maps of the parameter Ω with the same parameters. The 3D maps are the same as in Figure \ref{fig:13}. The 3D maps are the same as in Figure \ref{fig:13}. The 3D maps are the same as in Figure \ref{fig:13}.} \label{fig:13}$
Caersws	Severn		119			3.69	$8 - 10$
Abermule	Severn	580	83	1291	14.58	5.26	$13 - 17$
Buttington	Severn	-	62	$\qquad \qquad \blacksquare$		5.5	$8 - 10$
Montford	Severn	2025	52	1184	43.3	6.96	$10 - 15$
Welshbridge	Severn	2025	47			5.25	$15 - 20$
Vyrnwy Weir	Vyrnwy	94.3	226.34	1951	4.24	1.8	$2 - 5$
Pont Robert	Vyrnwy	675	100			3.07	$5 - 9$
Meifod	Vyrnwy	675	81			3.67	$7 - 10$
Llanymynech	Vyrnwy	778	62	1358	21.08	5.19	$3 - 6$
Bryntail	Clywedog	49	212.05	2026	2.4	1.61	$2 - 4$
Rhos Y Pentref	Dulas	52.7	178.49	1313	1.45	2.42	$1 - 3$
Llanerfyl	Banwy	$\overline{}$	151			3.5	$3 - 5$
Llanyblodwel	Tanat	229	77.28	1267	6.58	2.68	$7 - 10$
Yeaton	Perry	108.8	61.18	767	1.6	1.13	$15 - 20$

Table 1. Hydrometeorological and topographical information of analysed catchments at Upper Severn River (adapted from EA 2013 and Marsh and Hannaford 2008).

Fig. 1. The Upper Severn basin including the 14 forecasting locations used in the present study. Note that the smallest river streams are not shown in the stream network. (The digital elevation model is made available by the European Environment Agency on a Creative Commons Attribution License; http://www.eea.europa.eu/data-andmaps/data/digital-elevation-model-of-europe)

²⁸⁰ riod, the period up to March 6th, 2007 is used to spin up' the models. The remaining six years are used for the calibration and validation of the uncertainty models.

2.3 Uncertainty models

In the present study, predictive uncertainty is modelled using ²⁸⁵ Quantile Regression (QR). The basic configuration is simple,

and identical across all cases: the predictive distribution of future observed water levels is modeled as a series of quantiles, each estimated as a linear function of a single predictor which is the deterministic water level forecast. Four differ-²⁹⁰ ent configurations are inter-compared. Configuration Zero (QR0) constitutes the most straightforward case, where QR is applied 'as is', i.e. in its most basic form, in which no attempt is made to avoid crossing quantiles and no transformation or piecewise derivation is applied. Configuration One ²⁹⁵ (QR1) addresses the problem of the crossing quantiles using the technique proposed by Bondell et al. (2010). If quantile crossing problem does not occur, this technique provides the same estimates as in the base scenario. Because of this, it is also applied to the remaining configurations. In some cases, ³⁰⁰ the joint distribution of forecasts and observations is not best modelled using linear quantile regression models across the full domain of the predictor. However, by applying a transformation or by modelling sub-domains of the predictor, linear models may be used nonetheless. This is what is done 305 in Configurations Two (QR2) and Three (QR3) respectively. The configurations are each described in detail in the following four subsections; for reference, they are also listed in Table 2. As the non-crossing quantiles are applied to Configurations Two through Four, the comparison in the present paper is effectively between these three latter configurations (QR1, QR2 and QR3).

The joint distribution of forecasts and their verifying observations is based on the UK Environment Agency archives of water level observations and on the forecasts from the ³¹⁵ hindcasting procedure. The available record is crossvalidated through a leave-one-year-out cross-validation analysis. From the six years' worth of forecasts that are available for calibration and validation, five are used for model calibration and the single remaining year is used for model validation. Subsequently, another year is chosen for validation

and the calibration period then comprises the remaining five years. This is repeated until all six years have been used for validation.

Uncertainty models are developed for each combination

- ³²⁵ of lead time and location separately. While the forecasts have a maximum lead time of 48 hours with one-hour intervals, the QR models are derived on a limited number of lead times, namely for 1 hour lead time and then 3 through 48 355 hours lead time with 3-hour increments. The leave-one-year-
-
- out cross validation procedure yields approximately 3,760 observation-forecast pairs for every combination of lead time and location.

Identifier Description

2.3.1 QR0: Quantile Regression

rameters need to be estimated.

Quantile Regression (Koenker and Bassett Jr., 1978; Koenker³⁷⁰ ³³⁵ and Hallock, 2001; Koenker, 2005) is a regression technique for estimating the quantiles of a conditional distribution. As the sought relations are conditional quantiles rather than conditional means, quantile regression is robust with regards to outliers. Quantile Regression does not make any prior as-³⁴⁰ sumptions regarding the shape of the distribution; in that sense, it is a non-parametric technique. It is, however, highly parametric in the sense that for every quantile of interest, pa-

In the present work, Quantile Regression is used to estimate lead time *n* specific conditional distributions of water level,

$$
\phi_n = \{H_{n,\tau_1}, H_{n,\tau_2}, \dots, H_{n,\tau_T}\}\tag{1}
$$

where T is the number of quantiles τ ($\tau \in [0,1]$) con-345 sidered. If T is sufficiently large and the quantiles τ jointly cover the domain $[0,1]$ sufficiently well, we con- 385 sider ϕ_n to be a continuous distribution. Here, $T = 25$ and $\tau \in \{0.02, 0.06, \ldots, 0.98\},\$

$$
\phi_n = \{H_{n,\tau=0.2}, H_{n,\tau=0.6}, \dots, H_{n,\tau=0.98}\}\tag{2}
$$

We assume that (cf. $WWV2011$), separately for every lead 350 time n considered, for every quantile τ there is a linear relationship between the water level forecast S and its verifying observation H,

$$
H_{n,\tau} = a_{n,\tau} S_n + b_{n,\tau} \tag{3}
$$

where $a_{n,\tau}$ and $b_{n,\tau}$ are the slope and intercept from the linear regression. Quantile Regression allows for finding the parameters $a_{n,\tau}$ and $b_{n,\tau}$ of this linear regression by, through a process of linear programming, minimising the sum of residuals,

$$
\min \sum_{j=1}^{J} \rho_{n,\tau} (h_{n,j} - (a_{n,\tau} s_{n,j} + b_{n,\tau})) \tag{4}
$$

where h_j and s_j are the jth paired samples from a total of J samples, $a_{n,\tau}$ and $b_{n,\tau}$ the regression parameters from the linear regression between water level forecast and observation and ρ is the quantile regression function for the τ^{th} quantile,

$$
\rho_{n,\tau}(\varepsilon_{n,j}) = \begin{cases}\n(\tau - 1)\varepsilon_{n,j} & \text{if } \varepsilon_{n,j} \le 0 \\
\tau \varepsilon_{n,j} & \text{if } \varepsilon_{n,j} > 0\n\end{cases}
$$
\n(5)

The quantile regression function (Equation 5) is applied for the residual $(\varepsilon_{n,j})$, which is defined as the difference 365 between the observation $(h_{n,j})$ and the linear QR estimate $(a_{n,\tau} s_{n,j} + b_{n,\tau})$ for the selected quantile, τ and the specific lead time, *n*. By varying the value of τ , the technique allows for describing the entire conditional distribution of the dependent variable H.

In the present work, solving Equation 4 was done using the quantreg package (Koenker, 2013) in the R software environment (R Core Team, 2013). Figures 2, 3 and 4 give a graphical overview of the resulting quantiles. These plots are discussed in the Results and Analysis section.

2.3.2 QR1: Non-Crossing Quantile Regression

A potential problem with using QR for derivation of multiple conditional quantiles is that quantiles may cross, yielding predictive distributions that are not, as a function of increasing quantiles, monotonously increasing. WWV2011have addressed this issue by assuming a fixed error model in the domain of the predictor where there is the danger of quantiles crossing. In the present research study, the technique proposed by (Bondell et al., 2010) is used. This technique imposes a non-crossing restriction on the solution of Equation 4. Without this restriction, the solution to the proposed optimization problem is identical to that of classical quantile regression, i.e. to the models derived using QR0. For a more detailed description of the non-crossing quantiles technique, the reader is referred to (Bondell et al., 2010). The technique is freely available online (NCSU Statistics, 2010) and is coded in the statistical computing language R (R Core Team, 2013).

Fig. 2. Quantile Regression models for Llanyblodwel. Rows show the four different configurations; columns show different lead times.

2.3.3 **QR2: Quantile Regression in Normal space**

In this configuration, timeseries of water level observations and water level forecasts are first transformed into the Nor-395 mal domain. This results in timeseries that are marginally 405 Normal distributed. Subsequently, Quantile Regression models are calibrated using the non-crossing quantiles technique. Posterior to the derivation of QR models, the variables are

back-transformed into original space. The rationale for using 400

the transformation is that the joint distribution of transformed timeseries appears to be more linear, and can thus be better described by linear conditional quantiles.

The Normal Quantile Transformation (NQT) is a quantile mapping or cdf-matching technique that matches the (empirical or modeled) cdf of the marginal distributions with a standard normal cdf. Here, the empirical cdf of the marginal distributions is used. Thus, the variables are mapped to a standard normal distribution,

Fig. 3. Quantile Regression models for Pont Robert. Rows show the four different configurations; columns show different lead times.

which is solved by minimising the sum of residuals,

$$
H_{\text{nqt}} = \mathbf{Q}^{-1}(\mathbf{F}(H))
$$
\n
$$
S_{\text{nat},n} = \mathbf{Q}^{-1}(\mathbf{F}(S_n))
$$
\n(6)

where $F(\cdot)$ is the Weibull plotting position of the data point 410 considered. The equivalent of Equation 3 then becomes

$$
H_{\text{nqt},n,\tau} = a_{n,\tau} S_{\text{nqt},n} + b_{n,\tau} \tag{7}
$$

$$
\min \sum_{j=1}^{J} \rho_{n,\tau} (h_{\text{nqt},n,j} - (a_{n,\tau} s_{\text{nqt},n,j} + b_{n,\tau})) \tag{8}
$$

Posterior to the analysis in normal space, the variables are back-transformed to original space using a reversed proce-

Fig. 4. Quantile Regression models for Welshbridge. Rows show the four different configurations; columns show different lead times.

dure,

415

$$
H = Q(F(H_{\text{nqt}}))
$$
\n
$$
S_n = Q(F(S_{\text{nqt},n}))
$$
\n(9)

Back-transformation is problematic if the quantiles of interest lie outside of the range of the empirical distribution of the untransformed variable in original space. In those cases, assumptions will have to be made on the shape of the tails of the distribution (see Bogner K and Pappenberger for a more

elaborate discussion). Some authors have chosen to parameterize the distribution of the untransformed variable and use those statistical models for the back-transformation (see, for example, Krzysztofowicz and Kelly 2000). In the present study, this matter is treated through a linear extrapolation on a number of points in the tails of the distribution which was the solution chosen by Montanari (2005) and by WWV2011.

Fig. 5. Quantile Regression models for Llanyblodwel, Pont Robert and Welshbridge in Normal space (QR2). Rows show the three different locations; columns show different lead times.

QR3: Piecewise Linear Quantile Regression 2.3.4 425

430

In an effort to try and use linear quantile models to describe 435 a joint distribution that may be slightly nonlinear in nature, Van Steenbergen et al. (2012) applied linear models to partial domains of the predictor. They found the resulting distributions to be both more reliable and sharper. Multiple, mutually exclusive and collectively exhaustive domains were

identified based on a visual inspection of the data and taking into account the requirement that each sub group will have to contain a sufficiently sized data sample. As this selection more or less coincided with two splits at the 20th and 80th percentile, thus three sub-domains were defined, comprising 20%, 60% and 20% of the data respectively.

2.4 Verification strategy

To understand and inter-compare the performance of differ-⁴⁴⁰ ent QR configurations, an extensive verification of forecast quality was carried out. The post-processing procedure separated calibration from validation hence the verification can be considered to be independent. The old(-ish) adage has it that probabilistic forecasts should strive for sharpness subject to ⁴⁴⁵ reliability (Gneiting et al., 2005): an improvement in sharpness at the expense of reliability is not desirable. In addition,

decision makers may be interested in event discrimination skill for specific flood thresholds, for example. Forecasts were therefore assessed for reliability, sharpness and event ⁴⁵⁰ discrimination, and a number of metrics were calculated.

These verification metrics include the Brier Score (BS), the mean Continuous Ranked Probability Score (CRPS) and area beneath the Relative Operating Characteristic (ROCA). Reliability was assessed using reliability diagrams, that plot ⁴⁵⁵ the relative frequency of occurrence of an event versus the

- predicted probability of event occurrence. Proximity to the 1:1 diagonal, where observed frequency equals predicted probability, indicates higher reliability. Sharpness was explored by determining the width of the centred 80% inter-
- ⁴⁶⁰ val of the predictive distributions; the full sample of these widths is shown by means of an empirical cumulative distribution function (ecdf). The Brier Score (Brier, 1950) is defined as the mean squared error of a probabilistic forecast of a binary event. The mean CRPS (Brown, 1974; Matheson
- ⁴⁶⁵ and Winkler, 1976) is a measure of the squared probabilistic error in the forecasts across all possible discrete events. The area beneath the Relative Operating Characteristic is a measure of the forecasts' ability to discriminate between the exceedence and non-exceedence of a threshold, for exam-
- ⁴⁷⁰ ple a flood threshold. A detailed description of these measures with their mathematical formulation can be found in Appendix A.

To allow for comparison across different locations, BS, CRPS and ROCA are expressed as skill, thus becoming ⁴⁷⁵ Brier Skill Score (BSS), Continuous Ranked Probability Skill Score (CRPSS) and the Relative Operating Characteristic Score (ROCS),

$$
skill = \frac{score - score_{ref}}{score_{perfect} - score_{ref}} \tag{10}
$$

where score is the score of the system considered, score $_{ref}$ is the score of a reference system and score perfect is the high- $_{515}$ 480 est possible score. Skill scores range from $-\infty$ to 1. The highest possible value is 1. If $\text{skill} = 0$, the system's score is as good as that of the reference system. If $\text{skill} < 0$ then the system's score is less than that of the reference system. In the case of BSS and CRPSS, the reference score com-

⁴⁸⁵ prises that of the sample unconditional climatology; in case of the ROCS, the reference score is the ROCA associated with an unskilled forecast which states that the probability 520 of event occurrence is equal to the probability of non-event occurrence.

As the post-processor is intended to be used in flood forecasting, forecast skill is not only assessed for the full available sample of forecast, observation pairs, but also for subsets of high and extreme events. These subsets are defined by the climatological probability of non-exceedence P of the observation. For example, $P = .95$ denotes the sub–sample of forecast, observation pairs where the observation falls in the top 5% of observations. Increasing the value of P from 0 (i.e. the full available sample) to a value close to 1 thus gives an indication of forecast performance for high events.

Fig. 6. Sample size as a function of the climatological probability of non-exceedence P.

By construction, sample size for the computation of every verification metric varies with the climatological probability of non-exceedance P considered (Figure 6). Increasing the value of P means lower sample size. Sampling uncertainties of the verification metrics were explored by bootstrapping. The stationary block bootstrapping techniques was applied. This method constructs resample blocks of observations to form a pseudo time series, so that the statistic of interest may be recalculated based on the resampled data set (Politis and Romano , 1994). The minimum sample size was set to 50 ⁵¹⁰ and the number of bootstrap samples to use in computing the confidence intervals was set to 1000. The applied resampling method estimates the sampling distribution of each verification score. Here, the $5th$ and $95th$ percentiles of those distributions are shown. These thus constitute the centred 90% confidence intervals.

Verification metrics were calculated using the Ensemble Verification System (Brown et al., 2010).

3 Results and analysis

Results were produced for each of the fourteen locations listed in Table 1 and all of the lead times were considered.

For practical reasons, the present section includes results for a limited number of lead times and locations only: 24 hour, 36-hour and 48-hour lead times at Llanyblodwel, Pont Robert and Welshbridge. This combination thus comprises ⁵²⁵ forecasting locations with varying sizes of contributing area.

Pont Robert is located upstream, Llanyblodwel somewhere in the middle, and Welshbridge at the very outlet of the Upper Severn basin.

3.1 Uncertainty models

- 530 Uncertainty models for the three locations are shown in Fig-585 ures 2, 3 and 4. All scatter plots show observed water levels on the vertical axis versus water level forecasts on the horizontal axis. Each of the figures consists of a matrix of multiple panels, with rows showing the four configurations con-
- 535 sidered and columns showing various lead times. Note that $_{590}$ across configurations, the scattered pairs are identical. On the scatter plots, a summary of the estimated uncertainty models is superimposed, consisting of a selection of quantiles only: $\tau \in \{0.01, 0.05, 0.1, 0.25, 0.5, 0.75, 0.9, 0.95, 0.99\}$. Note
- ⁵⁴⁰ that these quantiles were derived for plotting purposes only, and do not necessarily coincide with the quantiles derived for verification. In the analysis, a more elaborate set of quantile is used. The latter quantiles are derived using a leaveone-year-out procedure (see Section 2.3 for details), whereas
- ⁵⁴⁵ this was not the case for the example quantiles in Figures 2 through 5. However, the derived models do not differ markedly. In the plots, the QR-estimated quantiles are shown in grey with the exception of the median quantile which is shown in blue.
- ⁵⁵⁰ From the figures (Figures 2, 3 and 4), a few general observations can be made. All scatter plots show that there is an obvious correlation of forecasted and observed water levels, although in none of the combinations of location and lead time, all forecasts are equal to the observations. Spread of
- ⁵⁵⁵ the forecast, observation pairs increases with increasing lead time. At zero lead time, the error correction technique ensures that modeled (i.e. simulated or forecasted) water levels are equal to the water level observation, hence at issue time there is no forecasting uncertainty. With increasing lead
- ⁵⁶⁰ time, this uncertainty increases. The location with largest lag time (Welshbridge) shows spread that is more concentrated around the 1:1 diagonal than the other locations that have smaller contributing areas and shorter lag times. The 615 location and slope of the quantiles show that in most cases,
- ⁵⁶⁵ spread is modeled to be very small at low predicted values of the forecast, and increases with increasing value of the forecast.

The figures show how the uncertainty models, each based 620 on a different configuration of Quantile Regression, differ

⁵⁷⁰ from one another. Configurations 0 and 1 appear to be very similar. They differ only in those instances where the former configuration would lead to quantile crossing but are identical otherwise, which was indeed anticipated. Configurations 625 2 (derived using NQT transform) and 3 (piecewise linear approach) are quite different from the first two configuration, but not dissimilar to one another. In these configurations, the quantiles are not a linear function of the water level forecast, that is, not along the full domain. Note that this non-linearity constituted the very reason why these configurations were included in the analysis. Both models often - but not always - show a very small spread at lowest water level forecasts, followed by an increasing spread. At high water level forecasts however, spread no longer increases and sometimes decreases. This means that sharpness of the resulting probability forecasts then no longer reduces with increasing values of the water level prediction; sometimes it even increases.

Figure 5 gives some additional background to the QR2 scenario and shows the estimated quantiles in Normal space, i.e. prior to back-transformation to original space. Similar to the other configurations, the estimated quantiles are linear. The strong non-linearity that is shown in Figures 2 through 4 is a result of the back-transformation from Normal to original space.

From the pairs and the models, we can see that at both Llanyblodwel and Pont Robert, the deterministic forecast has a tendency towards underforecasting, i.e. to underestimate future water levels. This underforecasting is corrected for by the uncertainty models, that thus include a bias correction by resulting in a median forecast that is higher than the deterministic forecast. The joint forecast, observation distribution for Welshbridge shows that there is much less obvious underforecasting, or overforecasting for that matter.

3.2 Hydrographs

Hydrographs are shown in Figure 7 at Welshbridge for a flood event that took place late 2011 and early 2012. The multiplot panel is composed by three columns representing three different lead times; 24-hour, 36-hour and 48-hour, and four rows for each of the four QR configurations. Each of these plots shows time in the horizontal axis, approximately 3 months and water level in the vertical axis. Deterministic forecast water level (green line), observations (blue dots), median quantile (light blue) and centered 50 %, 80 % and 90 % confidence intervals are included (in shades of grey). Across the configurations for a particular lead time, water level observations and deterministic forecasts are identical.

From the plots, a number of observations can be made, each consistent with what was to be expected based on the QR models. Uncertainty increases with lead time, as is shown by the widest intervals at highest lead times, and vice versa. The deterministic forecast tends to underestimate water level observations. With increasing lead time, underforecasting increases. At 48-hour lead time for high water levels the deterministic forecast provides a higher underestimation than for low and medium water levels, which is consistent with QR models shown in Figure 4.

The probabilistic forecasts resulting from configurations 0 and 1 are quite similar to one another. They both show highest uncertainty at higher deterministic water level forecasts. Configuration 2 does not show this behaviour; at higher ⁶³⁰ deterministic forecasts, probabilistic forecasts are sharper.

Again, this is consistent with the QR model plots in Figure 4. Configuration 3 results in forecasts whose width in the top 20 $%$ of forecasts varies only slightly (at 24-hour lead time) or $_{685}$ almost not at all (at 36-hour and 48-hour lead times) with the ⁶³⁵ value of the predictor.

From a visual inspection, it appears that the median quantile obtained with the four QR Configurations improves the deterministic forecast. QR Configurations 0 and 1 provide a median quantile with a minor improvement. Differences be-⁶⁴⁰ tween the median quantile of QR Configuration 2 and the deterministic forecast are the lowest ones. QR Configuration 3 median quantile reproduces with the highest accuracy water level observations, including high, medium and low values.

3.3 Verification

645 3.3.1 Reliability and sharpness

Figures 8 and 9 show reliability diagrams for the full data sample and for the forecasts whose verifying observation falls in the top 10% of observations ($P = 0.90$), respectively.⁷⁰⁰ When looking at the full available sample, the diagrams show

⁶⁵⁰ reasonably high reliability: most plotting points are very near, or on the 1:1 diagonal. At 24-hour leadtime, there is some underforecasting but this is no longer the case at the longer leadtimes shown.

At $P = 0.90$, forecasts are considerably less reliable. At ⁶⁵⁵ all locations and at all leadtimes, there is considerable underforecasting at all but the tails of the predictive distributions. This overforecasting is more pronounced for the smaller basins, and vice versa. Forecasts from QR0 and QR1⁷¹⁰ are equally (un-)reliable. When comparing these to forecasts

- ⁶⁶⁰ from QR2 and QR3, there is no configuration that yields more, or less, reliable forecasts across all cases. QR3 forecasts are nearly always among the least unreliable forecasts, although in many cases this is a shared position with varying other configurations.
- ⁶⁶⁵ Figures 10 and 11 show the distribution of width of the centred 90% predictive intervals for the full available sample $(P = 0)$ and the top 10% of observations only $(P = 0.90)$, respectively. The figures show that sharpness reduces with increasing lead-time as well as with increasing basin lag time.
- ⁶⁷⁰ Inter-comparison of sharpness between the different cases shows that for the full sample (Figure 10) there is little if any difference between the four configurations, and virtually none between QR0 and QR1. Forecasts for events that are 725 more extreme ($P = 0.90$) show larger differences. Again,
- ⁶⁷⁵ QR0 and QR1 yield forecasts of more or less equal width, but there are some differences between these configurations and QR2 and QR3. These differences increase with increas-

ing lead time and increasing basin lag time. At Welshbridge, QR2 yields sharpest forecasts, followed by QR3.

Unconditionally, both sharpness and reliability are more or less similar across four configurations. At $P = 0.90$ however, some forecasts are sharper than others but at the expense of reliability. On balance, usefulness of these forecasts may be equal. The trade-off between probability of detection and probability of false detection can be seen as a measure of this; the derived ROCS is analysed in the next section.

3.3.2 Skill scores

Figures 12, 13 and 14 present the skill scores computed for probabilistic forecast verification. These plots show BSS, CRPSS and ROCS (vertical axes; each score on a new row) versus the magnitude of the verifying observation, as a function of the observation which is expressed by its climatological probability of non-exceedance P (horizontal axes) for various lead times (columns). In each of the plots, results 695 are shown for four QR configurations considered. To give an indication of the uncertainty in the estimation of metrics, median as well as 10% and 90% estimates are shown.

From the figures, some general observations can be made. First of all, skills are mostly positive, with the exception of BSS and ROCS at the tails of P . Furthermore, skills deteriorate with increasing lead time, increase with increasing basin size and vary with the observation. Many of the plotted results are very similar in that the distribution of verification metrics is very similar - both in terms of the median as ⁷⁰⁵ well as the confidence bounds shown - across all leadtimes (columns) and values of P (horizontal axes). As the distributions are approximations – the verification pairs used are not strictly independent – a formal statistical hypothesis testing procedure cannot be used. Hence the interpretation is necessarily largely subjective.

The Brier Skill Score (BSS) as a function of P has a concave, inverse U shape curve. BSS is lowest — sometimes even negative — at the tails of P and highest near median P . This is because BSS is calculated using event probabilities; and extreme events, whether low or high, are more difficult to correctly predict than non-extreme events. In terms of difference across the configurations: these are very limited. Only at the low tail do these become apparent, but often the differences are not significant.

Contrary to BSS and ROCS, CRPSS is a smooth, continuous measure that factors skill across all possible thresholds for each paired sample. This different formulation is reflected in its behaviour with increasing value of the observation. For short lead times, CRPSS is approximately constant. With increasing lead time, a small dip in CRPSS values is detected close to the median P . At nearly all lead times, the four QR configurations show very similar skill. The only exception is the highest lead times (48 hours), in which QR Configuration 3 outperforms the remaining cases.

Fig. 7. Hydrographs of late 2011 and early 2012 events at Welshbridge.

⁷³⁰ ROCS is a binary event skill with a similar formulation to BSS. However, ROCS values do not show the same pattern than BSS. ROCS is largely constant for the whole cli_{750} matological distribution of the observations, as it can be seen at Welshbridge in Figure 11. Pont Robert (Figure 12) and ⁷³⁵ Llanyblodwel (Figure 13) present lower skill for the top half of the observations. Forecast quality decreases with increasing lead time, as it happens with BSS and CRPSS. No significant differences can be pointed out among the analysed QR Configurations.

⁷⁴⁰ 4 Summary, conclusions and discussion

The research described in this paper had two objectives: (i) to extensively verify the estimates of predictive uncertainty for Upper Severn basins that were produced using the Quantile Regression post-processing technique as described by ⁷⁴⁵ WWV2011; (ii) to address two issues with the 'as is' im-

plementation of linear models of QR: (a) invalid predictive distributions due to the crossing quantile problem; (b) the description of slightly non-linear joint distributions by a linear QR model.

The verification of forecast quality builds on the verification that was carried out in an earlier paper (Weerts et al., 2011). In the present paper, multiple metrics and skill scores are presented. Also, a 'conditional verification' was carried out, i.e. verification was done for a large number of sub-sets of available data, each representative for increasingly higher events. Verification showed that, unconditinally, in terms of all skills and metrics, forecast quality is positive. However, the analysis also shows that forecast quality and skill decreases with increasing value of the event.

⁷⁶⁰ The two issues described above were addressed by implementing several techniques, thus arriving at four configurations of Quantile Regression. The problem of crossing quantiles was addressed by adopting the non-crossing quantiles technique that was proposed by Bondell et al. (2010). This resulted in near-identical sharpness, reliability and skill. From a forecaster's point of view, the technique constitutes a methodological improvement as the post-processor will no longer produce invalid predictive distributions as a result of

Fig. 8. Full sample reliability plots.

crossing quantiles, at no noticeable extra computational ex-⁷⁷⁰ pense. The problem of linearly describing joint distributions of forecasts and observations that may not be linear in nature was addressed by two different approaches. The transformation to the Normal space attempted to produce a joint distribution that is 'more linear'. The piecewise linear derivation ₇₉₅ ⁷⁷⁵ approach constitutes dividing the data into sub-samples on which the joint distribution is linear.

The intercomparison shows that none of the four Quantile Regression configurations consistently outperforms the others. Sharpness and reliability may vary across configura-⁷⁸⁰ tions, but there none results in a more favourable combination of the two. In terms of BSS, CRPSS and ROCS, the four configurations yield comparable forecast quality.

Addressing the problem of the non-linearity of the joint distributions by the solutions proposed in the present paper 805 ⁷⁸⁵ has not resulted in higher skill. Either the data was sufficiently linear for the techniques not to be required, or the techniques have not performed to expectation. In any case, a skill improvement does not provide a rationale for derivation of Quantile Regression models in Normal space as was done by wwv2011.

While none of the configurations has a proven higher skill, there may be alternative reasons for choosing one over the other. If the post-processors are used in operational forecasting systems, the forecasters will have to be able to explain to an end user how predictive uncertainty was estimated. Hence more complicated configurations will be less likely to be used. Also, forecasts have to be consistent with forecasters' beliefs (Murphy, 1993), hence the post-processor will have to fit with the forecasters' perceptual model of forecasting error.

Like all post-processing techniques, QR requires a long calibration and validation data set containing several extreme events. If the magnitude of the forecasted water level is outside of the calibration sample range, then any estimate of hydrological predictive uncertainty is not supported by data in that range. In an operational setting, it is important for the forecaster to be aware that this issue may surface. A suggestion to overcome this issue may be to "flag" the uncertainty estimate if it is based on extrapolation outside of the calibration range. Possibly, in those cases the uncertainty estimate

Fig. 9. Reliability plots for the forecasts associated with the top 10% observations ($P = 0.90$).

can be replaced by an assumed estimate that the forecasters are comfortable with.

What would be a promising route to try and improve the $_{835}$ skill of the estimates of predictive uncertainty that are pro-815 duced by Quantile Regression? There are multiple possible answers here. First of all, there may be merit in adding predictors, i.e. by further conditioning forecast error on additional available variables. These could, for example, include $_{840}$ the internal state variables of a model (dry or wet) and/or ⁸²⁰ available observations at upstream locations. This route was taken by Solomatine and Shrestha (2009) in their UNEEC

- approach, and by Dogulu et al. (2014). Both compare a more complex UNEEC approach to QR and found improvement in skill. Stratification of the post-processing depending on dif-825 ferent seasons or water level ranges could represent another
- alternative configuration. Both the addition of predictors as well as stratification, however, introduce additional data requirements that may not be met, and in the absence of which the quality of post-processed forecasts may be reduced. Al-
- ⁸³⁰ ternative techniques may be considered, a recent article by van Andel et al. (2013) discusses various techniques in the

context of the HEPEX intercomparison experiment. Another option would be to fully investigate additional configurations of the piecewise linear approach. For example, c-means or K-means clustering would allow for partitioning data to be used to build several regression models. All the configurations inter-compared in the present work are parametric Quantile Regression estimations. Non-parametric or semiparametric Quantile Regression approaches, based on local smoothing could also be considered in future studies. For example, a comparison between here presented parametric QR configurations and the non-parametric estimation of the water level or discharge conditional distribution with copulas proposed by Smith et al. (2014), would be of interest.

Appendix A Verification metrics

For ease of reference, the probabilistic verification metrics used in this study are briefly explained. Further details can be found in the documentation of the Ensemble Verification System (Brown et al., 2010) as well as in reference works on

Fig. 10. Empirical cumulative distribution function of the centred 80% confidence interval of the predictive distributions.

⁸⁵⁰ forecast verification (Wilks, 2006; Jolliffe and Stephenson, 2012).

A1 Reliability diagrams

One desired property of probabilistic forecasts is that the predicted probabilities coincide with observed relative frequen-⁸⁵⁵ cies. Here, reliability diagrams are shown that, for each leadtime separately (indicator n is omitted from below equatons), plot the relative frequency of non-exceedence of the estimated quantiles f_{τ} of the predictive distribution versus the 870 probability of non-exceedence τ ,

$$
f_{\tau} = \frac{\sum_{j=1}^{J} I_{\tau,j}}{J} \tag{A1}
$$

860 where $I_{\tau,j}$ is an indicator variable

 $I_{\tau,j} = \begin{cases} 1 \text{ if } S_{\tau,j} < H_j; \\ 0 \text{ if } S > H. \end{cases}$ $\begin{array}{c} 0 \text{ if } S_{\tau,j} \leq H_j \end{array}$ that is determined for all j of J pairs of forecasts S and observations H.

A2 Sharpness

Sharpness is indicated by the width of the centred 80% inter-⁸⁶⁵ val of the predictive distribution,

$$
w_j = S_{\tau=0.90,j} - S_{\tau=0.10,j},\tag{A2}
$$

for all J forecasts. Again, sharpness is determined for each lead time n separately and the lead time indicators have been omitted from above equation. The combined record $w_{j=1,2,...,J}$ is shown as an empirical cumulative distribution function.

A3 Brier score and Brier skill score

For a given binary event, such as the exceedence of a flood threshold, the (half) Brier score (BS, Brier 1950) measures the mean square error of J predicted probabilities that Q ex-875 ceeds q ,

$$
BS = \frac{1}{J} \sum_{j=1}^{J} \{ F_{S_j}(q) - F_{H_j}(q) \}^2,
$$
 (A3)

Fig. 11. Empirical cumulative distribution function of the centred 80% confidence interval of the predictive distributions associated with the top 10% observations.

where
$$
F_{S_j}(q) = \Pr[S_j > q]
$$
 and $F_{H_j}(q) = \begin{cases} 1 & \text{if } H_j > q; \\ 0 & \text{otherwise} \end{cases}$.

The Brier Skill Score (BSS) is a scaled representation of forecast quality that relates the quality of a particular system BS to that of a perfect system $BS_{perfect}$ (which is equal to 0)⁸⁹⁰ aso and to a reference system BS_{ref} ,

$$
BSS = \frac{BS - BS_{ref}}{BS_{perfect} - BS_{ref}}= \frac{BS - BS_{ref}}{0 - BS_{ref}} = \frac{BS_{ref} - BS}{BS_{ref}}= 1 - \frac{BS}{BS_{ref}}
$$

BSS ranges from $-\infty$ to 1. The highest possible value 885 is 1. If $BSS = 0$, the BS is as good as that of the reference system. If BSS < 0 then the system's Brier score is less than that of the reference system.

A4 Mean Continuous Ranked Probability Score and Skill Score

The Continuous Ranked Probability Score (CRPS) measures the integral square difference between the cumulative distribution function (cdf) of the forecast $F_S(q)$, and the corresponding cdf of the observed variable $F_H(q)$, averaged across J pairs of forecasts and observations,

$$
\overline{\text{CRPS}} = \frac{1}{J} \int_{-\infty}^{\infty} \{ \mathbf{F}_S(q) - \mathbf{F}_H(q) \} \, \mathrm{d}q. \tag{A5}
$$

⁸⁹⁵ The Continuous Ranked Probability Skill Score (CRPSS) is a scaled representation of forecast quality that relates the quality of a particular system \overline{CRPS} to that of a perfect system $\overline{\text{CRPS}}_{\text{perfect}}$ (which is equal to 0) and to a reference system $CRPS_{ref}$,

Fig. 12. Verification results for water level forecasts at Pont Robert station (5 9 h lag time). In the rows, three different metrics are shown; from top to bottom these are the Brier Skill Score (BSS), the mean Continuous Ranked Probability Skill Score (CRPSS) and the Relative Operative Characteristic Score (ROCS). Columns show various leadtimes: 24, 36 and 48 h.

$$
{}_{900} \quad \text{CRPSS} = \frac{\overline{\text{CRPS}} - \overline{\text{CRPS}}_{\text{ref}}}{\overline{\text{CRPS}}_{\text{perfect}} - \overline{\text{CRPS}}_{\text{ref}}} = \frac{\overline{\text{CRPS}}_{\text{ref}} - \overline{\text{CRPS}}_{\text{ref}}}{0 - \overline{\text{CRPS}}_{\text{ref}}} = \frac{\overline{\text{CRPS}}_{\text{ref}} - \overline{\text{CRPS}}_{\text{ref}}}{\overline{\text{CRPS}}_{\text{ref}}} = 1 - \frac{\overline{\text{CRPS}}}{\overline{\text{CRPS}}_{\text{ref}}}.
$$
\n(A6)

A5 Relative Operating Characteristic score

For a given binary event, such as the exceedence of a flood ⁹⁰⁵ threshold, the relative operating characteristic (ROC; Green and Swets 1966) plots the hit rate or probability of detection (PoD) versus the false alarm rate or probability of false detection (PoFD) for several probability thresholds. For each probability threshold, PoD and PoFD are calculated using 910 the elements of a contingency table, which is valid for a sin- 920 gle probabilistic decision rule, (a probability threshold above

which the discrete event is considered to occur) and are defined as follows,

PoD =
$$
\frac{\text{# hits}}{\text{# observed events}} = \frac{h}{o}
$$
 (A7)
PoFD = $\frac{\text{# false alarms}}{\text{# events not observed}} = \frac{f}{o'}$,

The ROC score is a skill score that relates the area un-915 der the curve (AUC) of the forecast considered to the AUC associated with an unskilled forecast where probability of event occurrence and probability of event non-occurrence are equal, i.e. 50 %

$$
ROCS = 2 \times (AUC - 0.5). \tag{A8}
$$

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Fig. 13. Verification results for water level forecasts at Llanyblodwel station (7 10 h lag time). In the rows, three different metrics are shown; from top to bottom these are the Brier Skill Score (BSS), the mean Continuous Ranked Probability Skill Score (CRPSS) and the Relative Operative Characteristic Score (ROCS). Columns show various leadtimes: 24, 36 and 48 h.

Table A1. Contingency table.

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Fig. 14. Verification results for water level forecasts at Welshbridge station (5 9 h lag time). In the rows, three different metrics are shown; from top to bottom these are the Brier Skill Score (BSS), the mean Continuous Ranked Probability Skill Score (CRPSS) and the Relative Operative Characteristic Score (ROCS). Columns show various leadtimes: 24, 36 and 48 h.

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