



Supplement of

Two-dimensional differential form of distributed Xinanjiang model

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Section S1: Cross-sectional generalization and hydraulic parameters of river channel

In the two-dimensional differential-form of distributed Xinanjiang model (TDD-XAJ), the cross-section of river channel is generalized into a trapezoid (Fig. S1). Thus, the formulas for cross-sectional area, water surface width, and channel wetted perimeter could be given as:

$$A = \begin{cases} \zeta h_c + h_c^2 / \tan \beta & 0 < \beta < 90^\circ \\ \zeta h_c & \beta = 90^\circ \end{cases}, \quad (S1)$$

$$B = \begin{cases} \zeta + 2h_c / \tan \beta & 0 < \beta < 90^\circ \\ \zeta & \beta = 90^\circ \end{cases} \quad (S2)$$

$$\chi = \begin{cases} \zeta + 2h_c / \sin \beta & 0 < \beta < 90^\circ \\ \zeta + 2h_c & \beta = 90^\circ \end{cases} \quad (S3)$$

where A is cross-sectional area (m^2), B is water surface width (m), χ is channel wetted perimeter (m), ζ is channel bottom width (m), h_c is channel water depth (m), β is river bank slope gradient ($^\circ$).

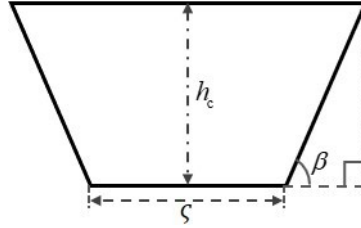


Figure S1. Diagram of trapezoidal cross-sectional generalization of river channel.

Section S2: The determination methods of spatially distributed model parameter

To determine spatially distributed model parameters, the process is generally based on spatially quantified data of watershed physical characteristics. This work is primarily carried out in two ways:

- (1) **Lookup table-based method.** Parameters are determined from tables based on watershed physical attributes. Specifically, the ratio of the impervious area (A_{imp}) and coefficient of deep soil layer evapotranspiration (c) are determined according to land use types (Yao et al., 2012), while the determination of tension water storage capacity curve exponent (b) and free water storage capacity curve exponent (ex) are assigned based on soil types. The value of surface roughness coefficient (n_s) is assigned based on the land use type of each grid cell, with different land uses corresponding to different roughness coefficients, which are derived from existing literature (Miao et al., 2016; Perrini et al., 2024). For channel roughness coefficient (n_c), values are obtained from a roughness coefficient table for river channels (Arcement and Schneider, 1989).
- (2) **Physical meaning-based method.** Parameter values are calculated using quantitative watershed physical characteristics according to the physical meaning of the parameters. Specifically:

a. Tension water storage capacity of the upper, lower, and deep soil layer (W_{um} , W_{lm} , and W_{dm}). The summation of W_{um} , W_{lm} , and W_{dm} represents the tension water capacity of the entire soil layer (W_m), and it can be determined according to soil hydrological parameters and soil layer depth (Yao et al., 2012), which could be expressed as:

$$W_m = (\theta_f - \theta_r)D_s, \quad (S4)$$

where θ_f is field capacity, θ_r is residual water content, D_s is soil layer depth (mm). Subsequently, two watershed-scale uniform coefficients (K_{um} and K_{lm}) and their derived value ($1 - K_{um} - K_{lm}$) are used to divide W_m into W_{um} , W_{lm} , and W_{dm} accordingly, which are given as:

$$W_{um} = W_m K_{um}, \quad (S5)$$

$$W_{lm} = W_m K_{lm}, \quad (S6)$$

$$W_{dm} = W_m (1 - K_{um} - K_{lm}). \quad (S7)$$

b. Free water storage capacity (S_m). S_m usually represents the capacity of free water in the humus layer. Thus, it can be determined according to soil hydrological parameters and the humus layer depth (Yao et al., 2012), which could be expressed as:

$$S_m = (\theta_s - \theta_f)D_h, \quad (S8)$$

where θ_s is saturated water content, θ_f is field capacity, D_h is humus layer depth (mm).

c. Interflow and groundwater outflow coefficient (K_i and K_g). K_i and K_g represent the outflow rate of interflow and groundwater. The method for determining K_i and K_g involves converting the free water storage to corresponding saturated water depth, based on the hillslope storage-discharge theory and steady-state assumptions, which is then multiplied by the slope gradient and saturated hydraulic conductivity using the kinematic wave assumption (Tong, 2022). K_i and K_g are finally expressed as the ratios of corresponding flow distance in the time interval of input forces to the slope length, which could be given as:

$$K_i = \frac{2S_0 K_{su} S_{hill} \Delta T}{1000(\theta_s - \theta_f) L_{hill}^2}, \quad (S9)$$

$$K_g = \frac{2S_0 K_{sl} S_{hill} \Delta T}{1000(\theta_s - \theta_f) L_{hill}^2}, \quad (S10)$$

where S_0 is free water storage (mm), K_{su} and K_{sl} is saturated hydraulic conductivity of the upper (representing interflow) and lower (representing groundwater) soil layer respectively ($m\ s^{-1}$), S_{hill} is the gradient of the slope, ΔT is the time interval of input forces (s), and L_{hill} is the length of the slope (m).

d. Interflow and groundwater storage recession coefficient (C_i and C_g). C_i and C_g represent the time delay for interflow and groundwater runoff as they travel from specific locations on the slope to the river channel. These parameters are determined based on the theory of spatially distributed unit hydrograph (Maidment et al., 1996; Tong, 2022). The grid cells that form the flow path extending from specific locations on the slope to the river channel is first identified using GIS. Then, using the kinematic wave assumption, the flow velocity of interflow and groundwater runoff through each grid cell is computed

55 based on the saturated hydraulic conductivity of the upper and lower layers and the slope gradient. Finally, the time taken for flow through each grid cell is accumulated, which could be expressed as:

$$T_i = \sum_{j=1}^{N_{\text{hill}}} L_{\text{hill}}^j / (K_{\text{su}}^j S_{\text{hill}}^j), \quad (\text{S11})$$

$$T_g = \sum_{j=1}^{N_{\text{hill}}} L_{\text{hill}}^j / (K_{\text{sl}}^j S_{\text{hill}}^j), \quad (\text{S12})$$

60 where T_i and T_g is the accumulated travel time from specific locations on the slope to the river channel through interflow and groundwater respectively (s), N_{hill} is the count of grid cells that form the flow path. C_i and C_g for each grid cell are further derived using theoretical conversion, which could be given as:

$$C_i = \exp(-\Delta T / T_i), \quad (\text{S13})$$

$$C_g = \exp(-\Delta T / T_g). \quad (\text{S14})$$

The primary data used to determine spatially distributed model parameters include soil physical and hydraulic properties, slope gradient, and land use. These can be obtained from open-source datasets, such as Harmonized World Soil Database v2.0 (HWSD v2.0) (FAO and IIASA, 2023), China dataset of soil properties for land surface modelling version 2 (CSDLv2) (Shi et al., 2025), and Global land cover mapping at 30m resolution (GlobeLand30) (Chen et al., 2015).

Table S1: Nomenclature

| Symbol | Signification | Unit |
|--------------------|---|--------------------|
| A | Cross-sectional area | m^2 |
| A_c | Open water surface area of the channel control volume | m^2 |
| A_{imp} | The ratio of the impervious area | - |
| b | The exponent of tension water storage capacity curve (TWSCC) | - |
| B | Water surface width | m |
| c | Coefficient of deep soil layer evapotranspiration | - |
| C_g | Groundwater storage recession coefficient | - |
| C_i | Interflow storage recession coefficient | - |
| ex | The exponent of free water storage capacity curve (FWSCC) | - |
| E_d | Actual evapotranspiration intensity from the deep soil layer | mm s^{-1} |
| E_l | Actual evapotranspiration intensity from the lower soil layer | mm s^{-1} |
| E_n | Net evapotranspiration intensity | mm s^{-1} |
| E_{obs} | Pan evaporation intensity | mm s^{-1} |
| E_u | Actual evapotranspiration intensity from the upper soil layer | mm s^{-1} |
| f_w | Runoff coefficient or the ratio of areas where tension water capacity is satisfied | - |
| F_i | Interflow intensity in external normal vector direction at boundary of slope control volume | mm s^{-1} |
| g | Acceleration due to gravity | m s^{-2} |
| h_c | Channel water depth | m |
| $h_{c,\text{eff}}$ | Effective water depth at the boundary of channel control volume | m |
| h_s | Surface water depth | m |
| $h_{s,\text{eff}}$ | Effective water depth at the boundary of slope control volume | m |

| Symbol | Signification | Unit |
|-------------------|---|----------------------------|
| i | Index of slope control volume in x direction or index of the channel control volume | - |
| I_l | Recharge intensity from the lower to deep soil layer | mm s^{-1} |
| I_u | Recharge intensity from the upper to lower soil layer | mm s^{-1} |
| j | Index of slope control volume in y direction | - |
| K_e | Coefficient of potential evapotranspiration to pan evaporation | - |
| K_g | Groundwater outflow coefficient | - |
| K_i | Interflow outflow coefficient | - |
| n | The length of the simulated or observed discharge sequence | - |
| n_c | Channel roughness coefficient | $\text{s m}^{-1/3}$ |
| n_s | Surface roughness coefficient | $\text{s m}^{-1/3}$ |
| O_g | Groundwater storage | mm |
| O_i | Interflow storage | mm |
| P | Total amount of precipitation | mm |
| P_n | Net precipitation intensity | mm s^{-1} |
| P_{obs} | Observed precipitation intensity | mm s^{-1} |
| Q_c | Channel discharge in external normal vector direction at boundary of channel control volume | $\text{m}^3 \text{s}^{-1}$ |
| $Q_{c,\text{up}}$ | Summation of Q_c from multiple upstream channel segments | $\text{m}^3 \text{s}^{-1}$ |
| Q_g | Outflow intensity of the groundwater storage | mm s^{-1} |
| $Q_{g,\text{up}}$ | Summation of Q_g from multiple upstream grids | mm s^{-1} |
| Q_i | Outflow intensity of the interflow storage | mm s^{-1} |
| $Q_{i,\text{up}}$ | Summation of Q_i from multiple upstream grids | mm s^{-1} |
| $Q_{i,x}$ | The x -directional components of Q_i | mm s^{-1} |
| $Q_{i,y}$ | The y -directional components of Q_i | mm s^{-1} |
| Q_{obs} | Observed discharge | $\text{m}^3 \text{s}^{-1}$ |
| Q_{sc} | Exchange discharge between the slope surface and the channel | $\text{m}^3 \text{s}^{-1}$ |
| Q_{sim} | Simulated discharge | $\text{m}^3 \text{s}^{-1}$ |
| Q_{ZDG} | Outflow discharge at the watershed outlet using zero-depth gradient (ZDG) condition | $\text{m}^3 \text{s}^{-1}$ |
| R | Total runoff intensity | mm s^{-1} |
| R_g | Groundwater intensity | mm s^{-1} |
| R_i | Interflow intensity | mm s^{-1} |
| R_{ps} | Surface runoff intensity from the pervious areas | mm s^{-1} |
| R_s | Total surface runoff intensity | mm s^{-1} |
| S_{fc} | Channel friction term | - |
| S_{fx} | Surface friction term in x direction | - |
| S_{fy} | Surface friction term in y direction | - |
| S_m | Free water storage capacity | mm |
| S_{mm} | Maximum single-point free water storage capacity | mm |
| S_0 | Free water storage | mm |
| S_{oc} | Channel bottom slope term | - |
| S_{ox} | Surface bottom slope term in x direction | - |
| S_{oy} | Surface bottom slope term in y direction | - |
| t | Time | s |
| T | Total simulation time | s |
| u | Surface flow velocity in x direction | m s^{-1} |
| v | Surface flow velocity in y direction | m s^{-1} |

| Symbol | Signification | Unit |
|-------------------------|--|--------------------|
| w | Channel flow velocity | m s^{-1} |
| W_d | Tension water storage of the deep soil layer | mm |
| W_{dm} | Tension water storage capacity of the deep soil layer | mm |
| W_l | Tension water storage of the lower soil layer | mm |
| W_{lm} | Tension water storage capacity of the lower soil layer | mm |
| W_{mm} | Maximum single-point tension water storage capacity | mm |
| W_u | Tension water storage of the upper soil layer | mm |
| W_{um} | Tension water storage capacity of the upper soil layer | mm |
| z_{bank} | Channel bank elevation | m |
| z_c | Channel bottom elevation | m |
| z_s | Surface elevation | m |
| α | Courant-Friedrichs-Lewy (CFL) condition coefficient | - |
| β | River bank slope gradient | $^\circ$ |
| γ | Slope aspect | $^\circ$ |
| Γ | The boundary of control volume | - |
| δl | Distance between two neighboring grid centers | m |
| Δl | Channel segment length | m |
| Δt | Model time step | s |
| Δt_{max} | Maximum model time step without further user-defined or programmatic constrain | s |
| ΔT | Time interval of input forces | s |
| Δx | Grid size | m |
| ε_g | Ratio of groundwater outflow intensity from slope to channel | - |
| ε_i | Ratio of interflow outflow intensity from slope to channel | - |
| η_c | Water surface elevation in channel | m |
| η_s | Water surface elevation on slope surface | m |
| ζ | Channel bottom width | m |
| ϕ_c | Source term of channel | m s^{-1} |
| ϕ_g | Source term of groundwater storage | mm s^{-1} |
| ϕ_i | Source term of interflow storage | mm s^{-1} |
| ϕ_s | Source term of surface | m s^{-1} |
| χ | Channel wetted perimeter | m |
| Ω | Control volume | - |

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